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**OMITIOMIRE COPPER PROJECT: HEAP LEACH
EARTHWORKS AND FOUNDATION DESIGN, PHASE 2
GROUND WATER AND SURFACE WATER STUDIES
FEASIBILITY LEVEL GEOTECHNICAL
INVESTIGATION - FACTUAL AND
INTERPRETIVE REPORT**

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ABBREVIATIONS

AAR	Alkaline Aggregate Reactivity (AAR)
AM	Additional Materials
BH	Borehole
CBR	California Bearing Ratio
Cu	Copper
CUT	Consolidated Undrained Triaxial
DSS	Direct Shear Strength Test
EOH	End of Hole
ERT	Electrical Resistivity Tomography
GEM	Global Earthquake Model
GSHM	Global Seismic Hazard Map
HLP	Heap Leach Pad
I _{s(50)}	Corrected Point Load Index
KP	Knight Piésold Consulting (Pty) Ltd
MDD	Maximum Dry Density
ML	Mining License
MM/A	Millimeters per annum
Myr	Million years
NGL	Natural Ground Level
OCP	Omitiomire Copper Project
OMC	Optimum Moisture Content
PGA	Peak Ground Acceleration
RD	Road
RQD	Rock Quality Designation
SPT	Standard Penetration Test
SWD	Stormwater Diversion Channel
TP	Test Pit
UCS	Uniaxial Compressive Strength
WGS	World Geodetic System 1984
WRD	Waste Rock Dump

1.0 INTRODUCTION

1.1 BACKGROUND

Knight Piésold Consulting (KP) was appointed by Craton Mining and Exploration Pty (Ltd) to provide feasibility level geotechnical investigation services to support engineering designs for the Bankable Feasibility Study of the Omitiomire Copper Project (OCP) in Namibia.

Omico through its Namibian subsidiary, Craton Mining and Exploration (Pty) Ltd (Craton), holds Mining Licence ML197 and Exclusive Prospecting Licence EPL8550, together a 30,000 Ha licence area which makes up the Omitiomire Copper Project. The mining licence is valid until March 2036. The development base case anticipates the production of 30,000 tonnes per annum of LME Grade A copper cathode for at least 15 years, targeting only open-pit mineralisation [Omico, November 2022 Press Release].

The Project is located 120km northeast from Windhoek in central Namibia and will comprise an open pit, a processing plant and heap leaching facility with associated crushing, agglomeration, stacking system, leaching process and SE/EW copper cathode extraction. The Project operation will be phased, with an estimated 104 million tonnes crushed throughput throughout the estimated 18 years Life of Mine (LOM), which is to be confirmed during the study.

1.2 SCOPE OF WORK

The purpose of this feasibility level geotechnical investigation was to characterize the foundation conditions, determine the nature and distribution of the soils and bedrock across the various sites, assess excavatability and to provide recommendations for the design of the foundations and earthworks required. In addition, areas of seepage and material sources for re-use as construction materials were investigated.

The feasibility level geotechnical and materials investigation provided in this report will aid feasibility level engineering designs for the following structures:

- Heap Leach Pad (HLP),
- Processing Plant (Plant),
- Stormwater Diversion Channel (SWD),
- New Road (RD) and
- Waste Rock Dump (WRD),

In addition, potential borrow areas (AM) and the re-use of the materials encountered across the site (AM) were investigated.

This report details the fieldwork carried out and provides the factual data retrieved from the geotechnical investigation, summary of all the laboratory results and the evaluation of all the data in order to provide feasibility level geotechnical recommendations for the project.

1.3 PREVIOUS INVESTIGATIONS

Previous geotechnical investigations undertaken by SRK focused at the TSF area (now referred to as the heap leach pads and plant area), documented in report 462312/Geotech Final, indicates that the project site is covered by a thin layer of reddish brown micaceous silty sand (assumed to be aeolian Kalahari sands). Areas close to the Black Nossob River, are underlain by a lighter-coloured alluvial soils. In some areas, where conditions are favourable, the Kalahari sands overlie a pedogenic hardpan calcrete horizon. Bedrock was noted to be highly irregular in weathering profile and varies according to fracture intensity and rock type.

2.0 SITE DESCRIPTION

2.1 PROJECT LOCATION

The Omitiomire Copper Project (OCP) is located 120 km northeast of Windhoek in the Khomas region, Namibia at latitude 802842.00 m E and longitude 7582808.00 m S (WGS84 UTM 33S). Smaller settlements border the site to the north (Hochfeld), east (Steinhausen) and south (Omitara).

The area is dominated by commercial wildlife farming with the overall proposed project footprint area measuring approximately 1000 Ha. Access to the site is via the M53 and D1435 gravel road, which is connected to the B6 National Trans Kalahari Highway from the capital city of Windhoek.

Appendix A1 provides a locality plan, whilst Appendix A2 provides the layout of the site investigated points at the stormwater diversion channel, waste rock dump, processing plant, heap leach pad, new road and additional materials investigation area.

2.2 TOPOGRAPHY

The terrain of the OCP project areas is generally flat, with flood plains and sporadic occurrences of low-lying pan / depressions containing clay soils within the central and western portions of the site.

The ephemeral Black Nossob intersects the OCP project site flowing in a general easterly direction through the southern part of farm Omitiomire.

Vegetation cover generally comprises a predominant mix of grassland plants, trees and shrubs. Soil conditions vary from transported material underlain by pedogenic soil with shallow bedrock.

The project site is generally flat lying with elevation ranging from 1650 meters above mean sea level (mamsl) to 1657 mamsl.

2.3 CLIMATE

The project area is located in a semi-arid climate region. It is characterized by tropical warm summer rainfalls and dry winter seasons. Rainfall distribution follows an irregular pattern, caused by rainfall falling in thunderstorms, generally during the months of January, February and March. Rainfall is of short duration, but intense and is characterized by extreme spatial and temporal variability. Typically, the Hochfeld area has an average precipitation rate of 400 mm/a (Mendelsohn, 2010).

2.4 SEISMICITY

A Global Earthquake Model (GEM) Global Seismic Hazard Map (GSHM) has been developed by (Pagani et al., 2018) which shows the global distribution of the Peak Ground Acceleration (PGA) with a 10% probability of exceedance (POE) in 50 years. The GSHM is based on the combination and computation of data from 30 hazard models which are distributed across the globe. Figure 2-1 shows the distribution of the PGA (g) values for the central region of Namibia. The PGA values are for a site condition with an average shear wave velocity in the upper 30 m (VS30) of 760-800 m/s. Based on the

GSHM, Omitiomire Copper Mine is situated on a low seismic hazard zone with a PGA value of approximately 0.0188227g.

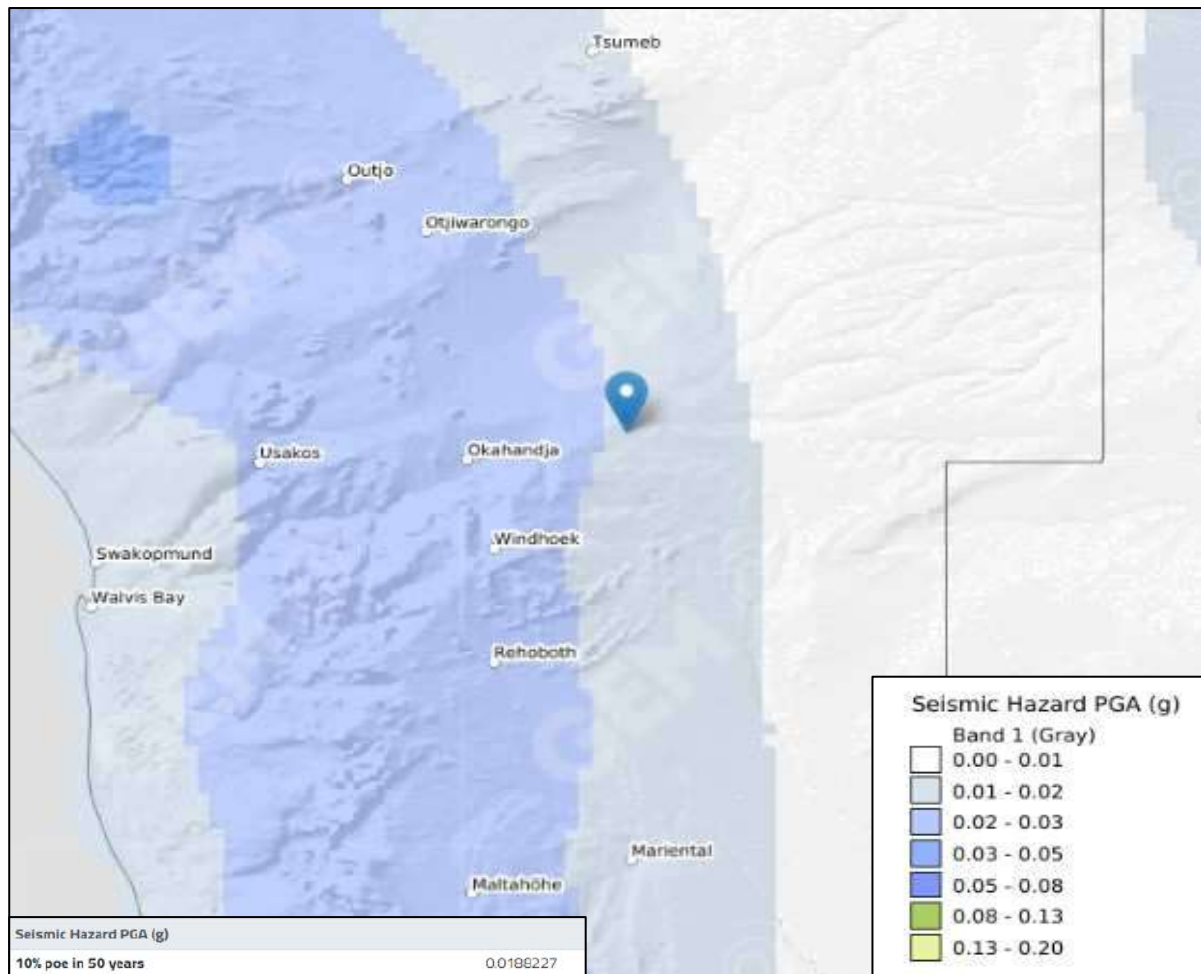


Figure 2-1: GEM Seismic Hazard Map for a Return Period of 475-years (Pagani et al., 2018)

3.0 GEOTECHNICAL METHODS OF INVESTIGATION

Details of the feasibility level geotechnical methods of investigation undertaken from March 2023 to May 2023 to investigate the scope as detailed in Section 1 are provided in the subsections below.

3.1 GEOPHYSICAL TESTING

A total of thirteen (13) Electrical Resistivity Tomography (ERT) Surveys were conducted by Gregory Symons Geophysics across the site from 3 March 2023 to 14 March 2023.

The ERT survey was performed by injecting a current into the ground and measuring a voltage. From this, the resistivity could be derived at certain points below the surface. The soil resistivity surveys were used to determine the depth of bedrock and measure the resistivity of various subsurface strata. The ERT test positions are presented in Appendix A2, the detailed report data is provided in Appendix F.

3.2 TEST PIT INVESTIGATION

A total of one-hundred and twenty seven (127) tests pits were excavated across the site from 10 May 2023 to 31 May 2023 to a maximum depth of 5m or until refusal was encountered, using a hired 20-ton Lovol (FR220D) tracked excavator from Sandworx cc. Test pitting in the SWD, WRD, Processing Plant, HLP, new road and borrow areas focused on ease of excavation for cost estimation, foundation conditions and seepage considerations, while also locating potential borrow areas for construction of platforms, roads and low permeability soils for liner materials. A layout of the test pit positions is provided in Appendix A2.

The nomenclature used for the naming of the test pits was adopted using the investigation area prefix defined in Section 1.1, followed by test pit prefix (TP) and the unique position number, e.g., SWD-TP1 refers to the stormwater diversion test pit 1.

The test pit positions planned were set out using Google Earth prior to establishment, based on the proposed footprint of the infrastructure at that time. The final positions of the test pits were adjusted on site, considering accessibility and safety factors. The test pits were logged in accordance with current standards (Guidelines for Soil and Rock Logging in South Africa, Brink and Bruin, 2002), using standard descriptors (moisture content, colour, consistency, soil structure, soil type and origin). The test pits were backfilled directly after completion of the logging, sampling and photographing.

The following test pits information is summarised in the respective appendices:

- The positions of all test pits are indicated on the site investigation plan in Appendix A2.
- A summary of the test pit logs is included in Appendix B1.
- The detailed test pit logs are provided in Appendix C1.
- Photographs of the test pit investigation are provided in Appendix D1.
- The terminology, conventions and symbols used in the logging are displayed in Appendix E.

3.3 BOREHOLE INVESTIGATION

In addition to the excavation of test pits, nineteen (19) rotary cored boreholes (BH) were drilled by RA Longstaff between 13 April 2023 and 22 May 2023. The boreholes were drilled to depths ranging from 10.4 m to 25.1 m. End of hole (EOH) was determined by drilling into 6 m of bedrock with a consistency of soft rock or better or deeper to investigate the potential of geological structures which may affect the structure. A similar system was used for naming of the boreholes, by inserting a prefix of the area and borehole (BH) followed by a numeral for the unique position number.

Where conditions permitted, Standard Penetration Tests (SPT) were conducted to determine the SPT N-value and provide an indication of the material consistency of the subsurface strata.

The core samples obtained from the rotary core borehole drilling were logged according to current standards (Guidelines for Soil and Rock Logging in South Africa, Brink and Bruin, 2002).

The following borehole information is summarised in the respective appendices:

- The positions of the boreholes are shown on the site investigation plan in Appendix A2.
- A summary of the rotary core boreholes is included in Appendix B2.
- A summary of the SPT testing is included in Appendix B3.
- The detailed borehole logs are provided in Appendix C2.
- Photographs of borehole core are provided in Appendix D2.
- The terminology, conventions and symbols used in the logging of the boreholes are displayed in Appendix E.

3.4 LABORATORY TESTING

Representative soil and rock samples were taken from the test pits and boreholes and submitted to Namibia Technical Services (NTS) for the following laboratory testing:

- Soils:
 - 52x Foundation Indicator (particle size distribution, Atterberg limits, hydrometer including specific gravity).
 - 28x California Bearing Ratio (CBR) tests.
 - 28x Modified AASHTO Maximum Dry Density (MDD) Including Optimum Moisture Content (OMC).
 - 3x Standard Proctor and Optimum Moisture Content.
 - 4x Basson's Index Tests.
 - 5x Direct Shear Strength Test.
 - 13x Falling/ Constant Head Permeability.
 - 2x Consolidated Undrained (CU) Triaxial Tests.
- Rock
 - 11x Uniaxial Compressive Strength (UCS).
 - 25x Point Load Test (I-s, 50).
 - 1x ACV (Wet and Dry).
 - 1x Petrographic Analysis for Concrete Use.
 - 1x Alkaline Aggregate Reactivity (AAR).

In addition to the samples collected from the boreholes for durability testing, samples will be sourced from the Open Pit pre-stripping works and submitted for laboratory testing. The results of those additional tests will be included in the interpretive report only.

A summary of the geotechnical laboratory testing schedule is provided in Appendix B4.

4.0 GEOPHYSICAL TESTING

Electrical Resistivity Tomography (ERT) Survey interpretation and results undertaken across the site in March 2023 are described below.

Thirteen (13) ERT lines were surveyed with a total traverse length of 2800 m near to and along the banks of the Nossob River and specifically at the inlet and outlet structures of the SWD. The locations of the survey lines are shown in Appendix A2. Detailed descriptions and results of the geophysical investigation are included in Appendix F.

The ERT surveys were used to provide possible depth to bedrock and identify water bearing structures using the Schlumberger array.

A summary of the geophysical report and the inverted resistivity depth sections show the following:

- A maximum of 3 overburden layers were identified. The subsurface strata are defined as follows:
 - A top sandy resistive layer,
 - A conductive clay bearing layer, and
 - A deeper resistive sandy layer.
- Below the overburden materials a weathered conductive basement is encountered, which transitions into resistive fresh rock.
- In places vertical to sub-vertical conductive features are observed in the weathered and fresh basement indicative of vertical faulting. The vertical faulting appears not to penetrate the overburden.
- Appendix F Figure 4 shows a structural interpretation of the area based on an aeromagnetic interpretation. Thrust faults are purported to be bringing in the copper (Cu) mineralization and are typically “old” (late Damaran 480-500 Myr). These faults are not expected to be identified in the resistivity data and in general this is the case with the current resistivity data set.
- Vertical faulting in a N/S and E/W direction is related to Cretaceous and late Karoo tectonics which appears to be detected by the resistivity as conductive water bearing faults and fractures.
- Overburden is Kalahari aged and the resistive layers may be related to windblown Kalahari sands.
- The conductive overburden layer appears to be associated with the Nossob River and could be related to flood events where the Nossob River has overflowed its banks and deposited more clay like lithologies on the flood plain.

5.0 TYPICAL PROFILE

This section provides an overview of the material conditions encountered at the OCP site for the feasibility level geotechnical investigation undertaken. The areas investigated during the March to May 2023 program include the following investigation areas for the proposed structures detailed in Section 1.1. of this report.

A summary of the test pit and borehole logs is described for each investigation area in the subsections to follow.

5.1 HEAP LEACH PAD (HLP)

The heap leach pad area is located directly north of the plant and west of the deposit. Twenty-nine (29) test pits and four (4) boreholes were undertaken during the field investigation. The locations of the test pits are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed heap leach pad.

5.1.1 ALLUVIUM

Transported soils (mostly fine and coarse alluvium) were seen to form a surficial blanketing layer within the HLP. Fine alluvium is typically found across the HLP from surface in all the test pits and boreholes. This horizon was observed to have a variable thickness ranging from 0.3 m to 2.3 m. These soils are typically described as dry to slightly moist, loose to medium dense, brown to dark brown, silty sand with root voids.

Firm to stiff, sandy clay deposits occurring at shallow depths within the low-lying pans.

Localized occurrence of coarse alluvium is present below the fine alluvium across the HLP, and is described as reddish-brown, medium dense, matrix supported, silty sandy gravel comprising abundant fine to coarse subrounded to sub-angular quartz gravel with minor iron nodules. The alluvium is typically 0.2 m in thickness and but may be up to 1.5 m in places.

5.1.2 PEDOGENIC SOILS

The alluvium has been indurated by pedocretes of varying degrees of cementation.

These pedogenic soils predominantly comprise of calcified alluvium with sporadic occurrences of calcareous, ferruginous and ferruginised alluvium. The pedogenic soils in the HLP area extend from a depth of 0.3 m to 4.0 m, with excavator refusal commonly occurring on calcified and ferruginised alluvium between depths of 1.3 m to 2.35 m. Consistencies within the pedogenic soils are typically medium dense that become dense with depth.

Moderately to strongly cemented hardpan calcrete of medium hard rock consistency is present in the north of the HLP (HLP-BH01) as a 1 m thick horizon encountered at a depth of 3 m.

5.1.3 RESIDUAL GNEISS

Where encountered, the residual gneiss occurs below the alluvium and pedogenic horizons in the HLP at depths ranging between 1.4 m to 6.8 m. The residual gneiss comprises generally less than 0.5 m thick reddish brown to light brown, medium dense to dense, silty sandy gravel. With depth the residual soil becomes very dense towards bedrock.

5.1.4 GNEISS BEDROCK

The transported / pedogenic soils overly gneiss bedrock within the footprint of the HLP at depths ranging from 0.65 m to more than 6.1 m.

The typical bedrock profile is described as grey to brown speckled black banded white, medium to coarse grained, very closely jointed to widely jointed, soft to medium hard rock gneiss.

Shallow excavator refusal typically occurred on soft rock in the central and southern portion of the HLP.

Gneiss bedrock was not encountered in all test pits but was encountered in all boreholes. The HLP encountered gneiss from a minimum depth of 0.65 m to deeper than 6 m.

Joint surfaces generally infilled with silt, sand and iron oxide stained. On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric and quartz veins. Hydrothermal vein was observed within HLP-BH01 at a depth of 6.3 m to 13.3 m.

5.1.5 GENERAL

The bedrock topography is undulating across the HLP but generally expected within the upper 5 m below the dense to very dense ferruginised and calcified alluvium. Slight groundwater seepage was encountered within HLP-BH03 at a depth of 6.3 m.

5.2 PROCESSING PLANT (PLANT)

The proposed processing plant is located west of the deposit and south of the proposed Heap Leach Pad. The geotechnical site investigation aimed to obtain information on the material types and foundation conditions. Four (4) boreholes and four (4) test pits were completed at the plant site. The locations of the test pits and boreholes are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed processing plant.

5.2.1 ALLUVIUM

The typical profile at the processing plant comprises dark brown to brown, medium dense, silty sand, fine alluvium with minor sub-angular to subrounded fine gravel. This fine alluvium overlies medium dense to dense coarse alluvium (pebble marker) gravel with abundant quartz cobbles and boulders. The pebble marker was highly undulating with an average thickness of 0.3m.

Ferruginised alluvium is found southwest of the processing plant and is described as orange-brown mottled and blotched red, dense, silty gravelly sand with minor ferricrete nodules.

5.2.2 PEDOGENIC SOILS

Nodular tending to honeycomb ferricrete sporadically occurs in the southern portion of the of the Plant comprising silty sandy gravel with minor to traces of quartz cobbles. Refusal occurred on nodular tending to honeycomb calcrete at a depth ranging from 1.7m to 1.9m.

5.2.3 RESIDUAL GNEISS

The residual gneiss is very sparse in the processing plan area and was only encountered in Plant-BH02 as a thin light brown speckled white, very dense, silty sand horizon of 0.5 m.

5.2.4 GNEISS BEDROCK

Depth to bedrock at the plant site is encountered at shallow depths as soft to medium hard rock from 1.5m in (H/Plant Access-TP30). The gneiss bedrock encountered in Plant-BH2 is described as completely weathered to unweathered, grey to brown speckled black streaked and banded white, medium to coarse grained, extremely soft rock transitioning to very hard rock gneiss with depth.

The joint surfaces generally infilled with silt, sand, calcite and iron oxide staining. On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric.

5.2.5 GENERAL

Evidence of geological structures was present as breccia in Plant-BH02 from 22.9 m to 23.1 m and 23.8 m to 24.2 m. Significant pyrite mineralization was also noted along foliation in Plant-BH03.

No groundwater seepage was encountered in any of the test pits or boreholes undertaken at the Processing Plant. The presence of pedogenic materials, however, confirms the possibility of a seasonal perched water table.

5.3 STORMWATER DIVERSION CHANNEL (SWD)

The proposed Stormwater Diversion Channel is located south and east of the deposit in a north to south (N/S) direction. To the possibility of extreme flooding of the Nossob River during the rainy season flooding the pit requires the construction of the SWD prior to the commencement of mining.

Twenty-six (26) test pits and ten (10) boreholes were investigated to determine the ease of excavation for cost estimation, founding conditions along the proposed channel and suitability for material reuse.

Six (6) 63 mm diameter slotted PVC standpipes were installed at the inlet (SWD-BH02, SWD-BH03, SWD-BH04 and SWD-BH05) and outlet (SWD-BH08 and SWD-BH09) structures for monitoring of groundwater levels, with the completion zone at 20.0m to 25.1m.

The following sub-sections provide an overview of material horizons which were encountered at the proposed Stormwater Diversion Channel

5.3.1 ALLUVIUM

Transported soils comprise fine alluvium which is present in the majority of the test pits. The fine alluvium encountered at the inlet / outlet structures and along the diversion channel extends to variable

depths varying from surface to 4.9 m, comprising brown to dark brown, clayey silty sand / sandy silty clay with minor to traces of sub-angular to subrounded quartz gravel. The consistency of this horizon is highly variable from collapsing very loose in the upper profile typically becoming dense with depth, and often from medium dense or dense at surface.

Coarse alluvium was generally found below the fine alluvium above the deep bedrock. Where found the coarse alluvium is typically 1 m thick were encountered and largely comprises medium dense to dense, brown silty sandy gravel with traces of cobbles and boulders. The coarse alluvium is sporadic in occurrence across the site and occasionally includes a pebble marker horizon.

SPT tests were exclusively carried out in the fine and coarse alluvium in the upper 4 m where penetrable. The SPT typically either refused in dry, stiff fines alluvium or coarse gravel and cobbles. SPT N-values of less than 13 were also recorded for the fine alluvium indicating soft to firm or loose to medium dense conditions.

The alluvium, as anticipated for a depositional horizon is variable and variance within the profile should be anticipated over small distances.

5.3.2 PEDOGENIC SOILS

Pedogenic horizons are highly variable within the proposed SWD and comprise predominantly ferruginised alluvium with sporadic occurrences of ferruginous, calcareous and calcified alluvium. These pedogenic soils are generally encountered below the fine and coarse alluvium and extend to depths of more than 4.7 m, with refusal commonly occurring on dense to very dense ferruginised alluvium at depths ranging from 1.1 m to 4.3 m.

Medium dense to dense nodular calcrete / ferricrete is sparse along the channel comprising abundant nodules of soft to medium hard rock strength.

It is anticipated that the pedogenic soils, much like the alluvium, will vary considerably in horizontal and vertical distribution.

5.3.3 RESIDUAL GNEISS

The residual soils predominantly located at the inlet and outlet structures in limited test pits (SWD-TP04 and SWD-TP25) and often these horizons in the boreholes are derived from the gneiss rock of the area and consists of brown to grey, firm to very stiff sandy silt with occasional large gravel to cobble fragments present.

5.3.4 GNEISS BEDROCK

Gneiss bedrock was encountered in all the boreholes drilled at the SWD. Bedrock is generally encountered within 5m from surface as soft rock, becoming medium hard rock or better with deeply weathered joints or mafic bands with depth. Medium hard rock gneiss or better is generally encountered at depths greater than 6m.

SWD- BH6 along the south-eastern bend of the channel and SWD- BH8 and SWD- BH9 at the outlet are notably more deeply weathered compared to other profiles along the channel.

The typical bedrock profile is described as follows:

- Highly to moderately weathered, grey to brown speckled black streaked and banded white, medium to coarse grained, closely to moderately jointed infilled with silty sand and iron oxide stained, very soft to medium hard rock gneiss.
- Occasionally deeper and better-quality gneiss is encountered as slightly to unweathered, grey speckled white and black or blotched red, medium to coarse grained, moderately jointed, medium hard rock to very hard rock gneiss.
- The mafic bands are generally more deeply weathered along joints.
- Shallow excavator refusal on soft to medium hard rock occurred in some test pits at the inlet (SWD-TP01, SWD-TP05, SWD-TP07) from a depth of 0.6 m.
- Shallow refusal occurred in generally medium hard rock at the outlet structure (SWD-TP20, SWD-TP23, SWD-TP24, SWD-TP25 and SWD-TP26) from a depth of 2.4 m.
- The joints are described as being infilled with silty sand and iron oxide stained and the majority of joints are smooth to rough and planar, often breaking along fabric.
- Gneissic folding was noted in SWD-BH06.

5.3.5 GENERAL

The geophysics survey lines identified possible fault zones in the vicinity of SWD-BH08 and SWD-BH09. These, however, was not clearly identified during the borehole drilling at the SWD however a possible breccia zone was identified in SWD-BH2. Shear zones were identified in other areas of the site.

Slight to moderate groundwater seepage was encountered within SWD-TP04, SWD-TP05, SWD-BH01, SWD-BH04 and SWD-BH05, at depths ranging between 1.5 m and 14.6 m.

5.4 NEW ROAD (RD)

The new road is located east of the SWD channel oriented in a NE/SW direction. Access constraints in the south-western part of the proposed route restricted the test pit excavations. A total of sixteen (16) test pits were excavated along the route. The locations of the test pits are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered along the new road.

5.4.1 ALLUVIUM

Similar to the SWD, WRD, Plant and HLP a surficial layer of fine alluvium blankets the entire road alignment in all but one the test pit. This material comprises a typically 1.4 m thick to 4.3 m thick in places brown to dark brown, medium dense to dense clayey silty sand with a root voided to pinholed soil structure. Very loose to loose zones occur in the upper profile. Excavator refusal was encountered within the fine alluvium at depths ranging from 1.4 m to 4.3 m.

Where refusal did not occur in the fine alluvium, coarse alluvium was encountered below in three test pits (RD-TP12, RD-TP13 and RD-TP14) along the new road. The coarse alluvium typically comprised yellowish brown to reddish brown mottled black and white, medium dense to dense, silty sandy gravel / clayey gravelly silty sand with minor ferricrete nodules. This horizon has an average thickness of 1.9m.

5.4.2 PEDOGENIC SOILS

Sporadic occurrences of pedocretes with little to no cementation (ferruginous / calcareous) of 0.6 m thick alluvium or indurated by cementation of 2.2 m thick calcified alluvium occur within the lower alluvium profile. These soils were profiled as having a medium dense to dense consistency.

5.4.3 GNEISS BEDROCK

Gneiss bedrock was encountered below the coarse alluvium at depths ranging from 1.4 m to 4.0 m often as a thin horizon of completely weathered gneiss becoming highly to moderately weathered, dark grey streaked white, stained orange, fine to medium grained, closely to moderately jointed, very soft to soft rock gneiss.

5.4.4 GENERAL

All but one test pit (RD-TP13) refused within the upper 5 m in alluvium or occasionally on very soft to soft rock gneiss.

5.5 WASTE ROCK DUMP (WRD)

The proposed WRD borders the proposed open pit to the north, east and south. In total, forty (40) test pits and one (1) borehole were completed during the field investigation to confirm soil/bedrock profile for founding and assess the in-situ rock and soils for use as construction materials. The locations of the test pits and borehole completed during the current feasibility level geotechnical investigation are shown on Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed Waste Rock Dump.

5.5.1 ALLUVIUM

The site of the proposed WRD facility is characterized by an approximately 1.1 m thick layer of fine alluvium, overlying a coarse alluvium. The fine alluvium consists of light brown to dark reddish brown, very loose to dense (non-cohesive soils) and firm to stiff (cohesive soils), silty sand / sandy clay or silt with presence of roots.

The underlying soil is coarse alluvium (pebble marker) comprising, typically a brown mottled black blotched white stained orange silty sandy gravel with traces subangular to subrounded cobbles and boulders extending to an average depth of 0.6m. Ferricrete nodules were observed to be present within the identified pebble marker.

5.5.2 PEDOGENIC SOILS

Below the fine and coarse alluvium horizons, pedocretes comprising ferruginous, ferruginised and calcified alluvium. Calcified alluvium is the most common of the pedocretes within the southern portion of the WRD. The calcified alluvium generally consists of brown speckled white, loose to medium dense (non-cohesive soils) or firm to stiff (cohesive soils), gravelly silty sand.

The central and northern portions of the WRD are characterized by well developed nodular, honeycomb and hardpan calcrete pedogenic horizons.

The nodular and honeycomb calcrete are similar, comprising pale brown blotched, medium dense to dense, white sandy silt with abundant calcrete nodules and cobbles.

Hardpan calcrete generally occurs in the northern parts of the site. The hardpan calcrete is described as moderately to very strongly cemented, light brown blotched white with very soft rock to medium hard rock consistency.

Ferruginous and ferruginised soils were encountered sporadically across the site and are anticipated to indicate localised perched water tables. These horizons are considerably more sporadic than the calcium-rich horizons. Occasional refusal occurred in ferruginised alluvium at shallow depths of approximate 1.5 m.

Refusal was typically encountered in the ferruginised alluvium or nodular, honeycomb or hardpan calcrete where encountered.

5.5.3 RESIDUAL GNEISS

Residual gneiss was encountered sporadically on site above the shallow bedrock. This residual gneiss is generally less than 0.3 m thick and described as white and brown to grey, very stiff/ dense, sandy silt to silty gravelly sand with traces of subrounded to subangular quartz gravel.

WRD-BH01 exposed the variation in weathering where soft rock was encountered then a completely weathered zone described as soil to a depth of 7.7 m.

5.5.4 GNEISS BEDROCK

Bedrock was encountered sporadically across the WRD area at shallow depths. The typical bedrock profile is described as follows:

- Highly weathered to moderately weathered, grey to brown speckled black, streaked and banded white, stained orange in places, medium to coarse grained, very soft rock to medium rock gneiss at the base of test pits.
- The borehole drilled in the northern portion of the WRD encountered very soft rock gneiss from 4.8m becoming medium hard rock with completely weathered zone of very soft rock becoming slightly to unweathered, hard to very hard rock from 14.3 m.
- Joint surfaces generally infilled with silty sand, iron oxide stained and void of any infill with depth.
- On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric.

5.5.5 GENERAL

Groundwater seepage was encountered at the southwestern corner (WRD-TP01) and north (WRD-BH01) of the WRD at a depth ranging between 1.4 m and 7.4 m respectively.

It should be noted that groundwater level in the area is subject to seasonal fluctuations and precipitation events. The presence of pedogenic soils, further, indicates the possibility of a perched water table.

5.6 POTENTIAL BORROW SOURCES (WRD AND AM)

Local borrow sources are required to provide materials for construction of the mine. The borrow materials are required to be geochemically innocuous (non-reactive), free of organics and non-deleterious. A total of twenty-four (24) test pits were excavated to investigate potential borrow sources. Twelve (12) test pits were undertaken on the footprint of the WRD on the southern bank of the Black Nossob river and an additional twelve (12) test pits were excavated south of the farmhouse at the northern end of the proposed new road. The main types of borrow source materials required include:

- Earthworks construction materials, including:
 - Low permeability soil (soil liners),
 - General fill,
 - Structural fill.
- Fine and coarse aggregate for use in concrete.

The interpretive report will include a full evaluation of the re-use of materials encountered across the site in the various areas. This section will focus on the materials targeted along the river as part of the WRD and the additional materials south of the farmhouse (AM). The following observations of the typical profile are made:

- The excavatable soil profiles are shallow across the majority of the investigated area and comprise of upper fine alluvium soils, underlain by coarse alluvium or calcified alluvium horizons at shallow depths.
- The fine alluvium is typically described as less than 1 m thick gravelly silty sand with minor rootlets and root voids or pinhole voided soil structure.
- The fine alluvium generally grades into a coarse alluvium with depth comprising clayey silty sandy gravel. The coarse alluvium contains trace to minor sub-angular cobbles. The thickness of this coarse alluvium is generally 0.4 m thick in the AM area and more than 1m thick at the WRD materials area.
- Pedogenic soils of varying developmental stages occur deeper in the WRD profile. The ferruginous / calcareous to more commonly encountered sandy silty gravel to gravelly silty sand, calcified alluvium is common. Occasionally with depth nodular calcrete (WRD-TP6, WRD-TP16 and WRD-TP17) is encountered as pale brown mottled white, sandy silt with abundant calcrete nodules.
- Gneiss bedrock was rarely encountered in the materials test pits at depths from 1.3 m and bedrock topography is anticipated to be undulating along the river.
- In addition to the typical profile described for the WRD above, the AM profile comprises fine alluvium described as clayey silty sand, marginally finer than that of the WRD with limited coarse alluvium and pedogenic soil development. Refusal of the excavator occurred at a typical depth of 1.1 m in dense to very dense fine alluvium. Shallow gneiss bedrock was encountered in AM-TP02 and AM-TP04 as very soft to soft rock at a depth of approximately 1m.
- The soil profile in the AM area is notably shallower than that of the WRD materials area but is considered more consistent in material variation within the alluvium. The fine alluvium is generally a 1m thick horizon in both the AM and WRD areas. The WRD encounters the deeper profile of coarse alluvium and more frequent pedogenic soil development.
- No groundwater seepage was encountered in any of the test pits.

6.0 LABORATORY TESTING

Laboratory testing was carried out at Specialised Testing Laboratory on samples obtained during the geotechnical investigation. A summary of the laboratory results is presented in Appendix B4 and discussed below. The full results are contained in Appendix G. Triaxial tests are still ongoing due to laboratory backlog and will be issued separately as per client's request to not delay the final report.

The materials have been assessed according to material type and where necessary the variations within specific areas are referred to.

6.1 ALLUVIUM

The nature of alluvium is generally variable by type and distribution during deposition. Despite typical materials being observed and tested, it should be noted that variation within the layers is anticipated as coarse or fine lenses were encountered.

6.1.1 FINE ALLUVIUM

The fine alluvium is present across the site. It is described as a silty sand to sandy silt. The material is generally variable by grain size within the deposited lenses. The silty sand generally comprises more than 40% fines (clay and silty) with varying clay percentage but generally low Plasticity Index (PI) values of less than 8 %. The USCS classification considers the material generally a clayey sand (SC) due to the high clay percentage and AASHTO class A-4. One sample (HLP-TP08) indicated high PI of 17 % grading as high plasticity silt (MH). The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC material typically varies from 1963 kg/m³ at 12 % to 2079 kg/m³ at 9 %. The ML sample indicates MDD and OMC values as low as 1517 kg/m³ at 23 %.

Coarser zones are encountered within the fine alluvium described as silty coarse sand or silty gravelly sand classifying as SM with low PI values and generally less than 30 % fines. The AASHTO classification generally indicates A-2-4 materials for the coarser zones. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the material vary from 2041 kg/m³ at 8 % to 2119 kg/m³ at 7 %.

The fine alluvium is anticipated to be corrosive towards steel and very highly aggressive towards concrete. It is considered generally low potential for expansiveness with specific gravity (SG) of 2.5 g/cm³ to 2.7 g/cm³.

Three samples were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of $\times 10^{-7}$ m/s for the alluvium classified as SC and may become less permeable with as the fines increase to $\times 10^{-9}$ m/s for the MH soils described above.

6.1.2 COARSE ALLUVIUM

The coarse alluvium is present across the site and includes a sporadic basal pebble marker layer. It is described as a silty sandy gravel to coarse sandy gravel with varying amounts of gravel. The material is generally variable by grain size within the deposited lenses. The silty sand/ silty sandy gravel generally comprises less than 45% fines (clay and silty) with varying clay percentage but generally low Plasticity Index (PI) values of less than 5 %. The USCS classification considers the material generally a clayey

sand (SC) to Gravel of low to medium plasticity (GP to GM). The coarse alluvium is classified as AASHTO class A-1, A-2 or A-4. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC to GM material typically varies from 1858 kg/m³ at 13 % to 2204 kg/m³ at 6 %. The material has a high variability according to the COTO classification ranging from G9 to G4 quality materials.

The coarse alluvium was not tested for corrosivity but is anticipated to be corrosive towards steel and very highly aggressive towards concrete as in the fine alluvium. It is considered low potential for expansiveness with specific gravity (SG) of 2.5 g/cm³ to 2.6 g/cm³.

Three samples were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of x10⁻⁷ m/s for the alluvium classified as SC and may more permeable gravels at x10⁻⁶ m/s.

6.2 PEDOGENIC SOILS

6.2.1 CALCAREOUS/ CALCIFIED ALLUVIUM

Present across most of the site but not encountered in the Plant area. The pedogenic soils are at various phases of development from the weaker calcareous to stronger calcified alluvium comprising gravelly silty sand to gravelly sandy silt. It is noted that the fines content of the calcified alluvium is generally higher than that of calcareous alluvium.

The calcareous and calcified alluvium test results are within the limits of the fine alluvium, but the material typically grades as A-1 to A-2 material with one sample RD-TP08 indicating medium expansive silty sand (SM) of A-7 quality similar to that observed for HLP-TP08 discussed in Section 6.1.1 above.

The samples that were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of x10⁻⁶ m/s for the calcareous alluvium and x10⁻⁷ m/s for the calcified alluvium.

6.2.2 FERRUGINOUS/ FERRIGINISED ALLUVIUM

The ferruginous and ferruginised alluvium is present across the site. Testing was carried out predominantly on samples obtained from the plant, SWD and WRD due to the frequency of materials encountered. The ferruginised alluvium is described as gravelly silty sand to silty sandy gravel classified as low expansive clayey sand to silty sand (SC - SM) of AASHTO A-2 in the gravel zones and A-4 in the sandy zones. The specific gravity (SG) was recorded as 2.5 g/cm³ to 2.6 g/cm³. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC sample was recorded at 2053 kg/m³ at 7 %. The material has a low CBR strength likely due to the crushing of the poorly cemented ferricrete nodules and is thus classified as poorer than G9 according to COTO.

The ferruginised alluvium is anticipated to be corrosive towards steel and very highly aggressive towards concrete as in the fine alluvium.

One sample was tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-value in the order of x10⁻⁷ m/s similar to the alluvium described above.

The ferruginous alluvium sample tested indicates generally similar properties to those described for the ferruginised alluvium above but has a slightly higher PI (<10%) and classifies as A-6 material.

6.2.3 NODULAR AND HONEYCOMB CALCRETE ALLUVIUM

Pedocrete is present across most of the site but not encountered in the road and borrow areas. Samples were tested from the HLP and WRD of the nodular and honeycomb calcrete described as sandy silty gravel to silty sandy gravel which has similar properties, although more gravelly, to the calcareous alluvium. The nodular calcrete was tested for permeability values and indicates generally less permeable condition than those encountered in the alluvium generally and shows typical permeability values in the order of $\times 10^{-8}$ m/s but grading as typical G7 and G8 quality materials.

6.2.4 HARDPAN CALCRETE

Hardpan calcrete was only encountered in the HLP and WRD areas and is similar to the other pedogenic horizons. The hardpan calcrete classifies as generally a G6 quality material with MDD and OMC AASHTO values of 1651 kg/m³ at 20%.

6.3 RESIDUAL GNEISS

The residual gneiss is present across most of the site but not encountered in the test pits along the new road, along the SWD and in the borrow area.

Where encountered the residual gneiss is often described as calcified or ferruginised silty gravelly sand with a fines content varying from 5 % to 34 % and is classified as clayey to silty sand (SC-SM) to silty gravel (GM, GP or GC).

The material is classified as A-1 or A-2 according to the AASHTO classification and as G7 Quality material according to COTO with AASHTO MDD and OMC of 1973 kg/m³ and 11 %. One sample was tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-value in the order of $\times 10^{-7}$ m/s similar to the alluvium described above.

6.4 GNEISS BEDROCK

The very soft rock gneiss present across the site which was excavatable was tested as well. The horizons are generally thin but indicate similar results to the residual soils derived from them. The fines content is less than 25% with PI values less than 5 %. The material is classified as clayey to silty sand (SC to SM) to silty gravel (GM) as typically AASHTO A-1 to A-2 and COTO G6 to G9 quality material. The very soft gneiss is anticipated to be corrosive towards steel and very highly aggressive towards concrete.

6.4.1 UNIAXIAL COMPRESSIVE STRENGTH (UCS)

According to (SAICE, Guidelines for Soil and Rock Logging in Southern Africa, 2002), very soft rock is expected to have uniaxial compressive strengths (USC) ranging between 1 MPa and 3 MPa while soft rock is between 3 MPa and 10 MPa.

Two core samples were tested for rock strength in the HLP. The first sample was obtained from borehole HLP-BH02 and the second from HLP-BH04, the results are presented in Table 6-1. Minor strength increase was noticed in very soft rock cores on desiccation and may be the reason for the over

estimation of rock strength of the sample obtained from HLP-BH2. Both HLP samples indicate soft rock conditions are encountered from a depth of 2.6 m.

Table 6-1: UCS test results

Borehole No.	Sample ID	Depth (m)	General Rock Mass Description	Failing Load (kN)	Uniaxial Compressive Strength (UCS) (MPa)	Failure Description	Rock Hardness (SAICE, 2002)
HEAP LEACH PAD							
HLP-BH02	C/10590	5.35-5.65	Highly weathered, very soft rock gneiss	22	8	Normal failure	Soft Rock
HLP-BH04	C/10593	2.66-2.86	Highly weathered, soft rock gneiss	21	7	Normal failure	Soft Rock
SWD INLET							
SWD-BH01	C/10575	2.04-2.26	Very Soft Rock Gneiss	22	8.0	Normal	Soft Rock
SWD-BH01	C/10577	3.14-3.48	Medium Hard Rock Gneiss	66	23.5	Normal	Medium Hard Rock
SWD-BH02	C/10580	7.96-8.16	Soft Rock Gneiss	96	33.5	Normal	Hard Rock
SWD-BH03	C/10582	3.82-4.13	Soft to Medium Hard Rock Gneiss	34	12.0	Normal	Medium Hard Rock
SWD-BH05	C/10586	9.69-9.93	Medium Hard Rock Gneiss	73	25.5	Normal	Medium Hard to Hard Rock

The samples taken in the SWD are logged as generally weaker rock mass than the tested results, specifically at SWD-BH02. This was noticed where logging of the core was carried out directly after drilling in a wet state leading to lower estimated rock parameters than during testing in the dry state. This is likely attributed to the alteration minerals present in the rock core.

6.5 SHEAR STRENGTH

The shear strength parameters of the soils on site were evaluated by shearbox testing. At the time of writing two triaxial tests on clayey samples were still ongoing. In the interim, indicative values are provided based on the material types as presented in Table 6-2 below.

Table 6-2 Indicative shear strength parameters

Soil parameter	Fine alluvium	Coarse alluvium	Pedogenic horizons	Residual gneiss
Typical material classification	SC to MH	SC to GM	SC - SM	SC-SM, G
Maximum dry density (kg/m ³)	1500 - 1970	1650 - 2000	1650 - 1970	1650 - 2000
Cohesion (kPa)	0 - 8	0 - 5	0 - 5	0 - 5
Friction (degrees)	28 - 32	30 - 38	30 - 34	30 - 38

The shearbox test results are generally inline with the upper bound parameters for the tested alluvium and residual gneiss. SWD-TP15 and WRD-TP35 indicated slightly higher values than anticipated, specifically the cohesion, this is attributed due to the fines component causing slight build up of excess pore water pressure.

WRD-TP04 indicated slightly higher values than anticipated, specifically the cohesion, this is attributed due to the “apparent” cohesion due to the sand interlock.

Triaxial tests are still ongoing due to laboratory backlog and will be issued separately as per client’s request to not delay the final report.

7.0 GEOTECHNICAL EVALUATION AND RECOMMENDATIONS

The following section comprises the geotechnical evaluation of the various areas and materials based on the material type, laboratory results and site observations. Recommendations are provided for founding, excavatability and re-use of materials for construction.

Note that a general comment for all areas requiring earth works – an adequate source of water supply will be required during the earth works for the various phases of construction.

7.1 HEAP LEACH PAD

7.1.1 DISCUSSION

The test pit and borehole data indicate that the majority of the HLP site is covered by 0.3 m to 1.0 m of very loose to medium dense, silty sand to sandy silt, fine alluvium. The fine alluvium is generally loose in the upper 0.3 m and occasionally loose conditions are encountered to a depth of 1.45 m. Fine alluvium may occasionally extend to a depth of 3.0 m generally in the western and north-western portion.

Below the fine alluvium, generally medium dense coarse alluvium, medium dense calcareous or medium dense to dense calcified alluvium is encountered. The coarse alluvium and calcareous alluvium typically extend to depths of 2.5 m below ground level. This sandy gravel coarse alluvium has a typical medium dense consistency. The calcified alluvium is generally encountered within the upper 1.8 m. The alluvium is generally ferruginous/calcareous silty sand with variable gravel in the center of the HLP towards the north to north-western portions and becomes medium dense to dense ferruginised/calcified silty sand (increasing consistency) north of the HLP site boundary.

The coarse alluvium includes a sparsely occurring pebble marker in places at the base of the coarse alluvium.

The transported horizon generally transitions to a thin medium dense to dense, silty sandy gravel, residual gneiss from 1.4 m typically along the western border of the HLP. The varying transition depths into residual soils are generally attributed to the weathering of the underlying undulating gneiss bedrock topography. The western boundary of the HLP comprises sporadic very soft rock gneiss encountered from 2.5 m below surface.

The test results indicate that the fine and calcareous alluvium at the HLP is corrosive towards steel and very highly aggressive towards concrete.

7.1.2 FOUNDING RECOMMENDATIONS

The total anticipated load exerted by the approximately 50 m high (1.8 km x 1,1 km area constructed in three phases) HLP will be approximately 1 MPa. Soft rock gneiss which is encountered at an undulating depth deeper than 3.0 m below surface has an allowable bearing capacity of more than 1MPa. The HLP earthworks (bottom of liner and drainage layer) required to create adequate drainage of the pad is anticipated to be a maximum height of up to 8 m above ground level in the north of the facility. Due to

the amount of excavation required to reach suitable in situ founding material, it is rather recommended to in situ densify the soils on site as part of the earthworks design as detailed below:

- Excavate to stockpile 0.5 m below surface extending 2m beyond the HLP footprint.
- Localised in situ rehabilitation may be required in loose pockets.
- Compact the excavation floor at 95% Modified AASHTO compaction at OMC using impact compaction. A trial section should be carried out to determine the optimum number of passes required.
- Backfill the excavation to founding level (top of HLP earthworks platform) in 200 mm layers using minimum G7 quality material compacted to 95% Modified AASHTO compaction at OMC using conventional compaction equipment. Should thicker backfill layers be required, trial compaction tests must be carried out.
- Plate load testing should be carried out on the backfill material to determine the expected settlements and the time for settlement of the fill.
- Place the drainage layer and liner as per design.

The laboratory results indicate that the fine alluvium has low permeability when compacted to 90% Modified AASHTO. This fine alluvium can be used as a low permeability layer, but it is not regarded as a suitable source for the clay liner itself due to the variability within the alluvium. The nodular calcrete and residual gneiss generally do not meet the required permeability for liner use.

The soils at the HLP site are corrosive towards steel and very highly aggressive towards concrete. Treatment of these soils with neutralising agents may be required.

7.1.3 EXCAVATABILITY

The average test pit refusal depth in the outer areas of the HLP is less than 1.4 m while the center portion test pits typically refuse between 1.4 m and 3.1 m on shallow bedrock or the presence of very dense soil.

The HLP generally experiences soft excavation conditions to a depth of at least 1 m below surface satisfying the recommendations above. Should deeper excavation be required, localised intermediate to hard excavation may be anticipated with depth.

7.2 HLP ANCILLARY STRUCTURES AND PONDS

For the purpose of evaluation based on the significant difference in design concept the HLP ancillary structures and ponds have been treated separately from the HLP and plant area. This includes the area along the southern boundary of the HLP (HLP-TP20, HLP-22 to 27 and BH03-HLP, BH04-HLP and BH01-Plant and BH04-Plant). The typical profile of the HLP Ponds at the southern boundary of the HLP indicates that firm to stiff and loose fine alluvium is present from surface to variable depths. Medium dense alluvium is typically encountered from a depth of 0.5 m to 1.2m below surface with a typical allowable bearing capacity of at least 100 kPa and shallow very soft rock transitioning to medium hard rock gneiss underlies the HLP ponds from a depth of 0.6 m but typically deeper than 1.4 m with allowable bearing capacity of more than 500 kPa.

Based on the field observations and laboratory results, the overburden is described as fine alluvium silty to clayey sands (USCS classification "SC" and "SM") to coarse alluvium coarse sand s and gravelly sand ("GP-GM") with low plasticity and have low potential expansiveness as well as a low

compressibility rating (Hazelton & Murphy, 2007). Minor calcareous development was observed in TP-24 and ferruginised alluvium is present in the vicinity.

According to the AASHTO Classification system, the alluvial soils classify as A-1 to A-2 quality soils.

7.2.1 FOUNDING RECOMMENDATIONS

Based on the typical profile it is recommended that any lightly loaded structures (limited to 100 kPa bearing pressure) may be founded on medium dense alluvium encountered varying from depths of 0.6m to 1.1 m below surface with minor ground preparation for the foundations. Moderately loaded structures (up to 500 kPa bearing pressure) may be founded on very soft gneiss bedrock at typical depths ranging from 1.2 m to 1.7 m. Founding of structures on soft to medium hard rock (up to 1 MPa bearing pressure) may be encountered as shallow as 0.6 m as seen in TP25-HLP but typically from a depth of 1.4m to deeper than 2m.

The HLP ponds are anticipated to be excavated to a depth of more than 5 m below ground level encountering soft to medium hard rock gneiss within the excavation. It is recommended to consider raising the floor level to reduce intermediate to hard rock excavation, if possible to 3-4m below ground level.

A liner preparation layer should be used above the rock excavation floor comprising the 2x 150 mm fine alluvium compacted to 95% Standard Proctor at OMC before lining of the pond due to the uneven rock surface, closely jointed nature of the rock, low Rock Quality Designation (RQD) and brecciation indicative of faulting observed in HLP-BH3.

It is recommended that the foundations be inspected by a suitably qualified geo-professional to ensure no significant variation is present and that founding conditions are achieved at the required depths.

7.2.2 EXCAVATABILITY

Based on the test pit and borehole data excavation is classified as soft to a typical depth of 1.8 m, thereafter, intermediate to hard excavation anticipated in the very soft to medium hard rock below.

7.3 PROCESSING PLANT

The typical profile of the processing plant indicates that medium dense alluvium is encountered from surface with a typical allowable bearing capacity of at least 100 kPa and shallow very soft gneiss bedrock underlies the plant area from a depth of 1.5 m below ground as encountered in TP30-HLP and BH1 to BH4-Plant. The excavator generally refused on a dense coarse alluvium or a dense ferruginised alluvium in the test pits that did not reach rock within the upper 1.9 m. It is expected that the dense soils overly the shallow bedrock (allowable bearing capacity of more than 500 kPa) below.

Based on the field observations and laboratory results, the alluvium is described as silty to clayey sands (USCS classification "SC" and "SM") with low plasticity and have low potential expansiveness as well as a low compressibility rating (Hazelton & Murphy, 2007). The alluvium is occasionally ferruginised and minor ferricrete nodules and honeycomb development are encountered.

According to the AASHTO Classification system, the alluvial soils classify as A-2 to A-4 quality soils.

7.3.1 FOUNDING RECOMMENDATIONS

Based on the typical profile it is recommended that lightly loaded structures (limited to 100 kPa bearing pressure) may be founded on medium dense alluvium varying from surface with minor ground preparation required for the foundations. Moderately to heavy loaded structures (up to 500 kPa bearing pressure) may be founded on very soft rock gneiss bedrock at a depth of more than 1.5 m.

Heavy dynamic structures (e.g., crusher) may be founded on soft rock gneiss at a dynamic bearing pressure not exceeding 500 kPa. It is anticipated that suitable founding will be possible from a depth of 2 m below ground surface based on the results of Plant-BH3.

It is recommended that the foundations be inspected by a suitably qualified geo-professional to ensure no significant variation is present and that founding conditions are achieved at the required depths.

7.3.2 EXCAVATABILITY

Based on the test pits excavated at the plant area, refusal was generally encountered within the upper 1.9 m and are classified as soft excavation, thereafter, intermediate to hard excavation is anticipated in the rock below.

7.4 STORMWATER DIVERSION CHANNEL (SWD)

For the purpose of this evaluation, this section is divided into three sections, namely the inlet (west), channel (alignment length) and outlet (east) structures. The figure in Appendix A2 shows the position of the test pits and boreholes along the SWD structure.

The evaluation and recommendations are based on the data from the laboratory results of the tested materials, as well as the data from 26 test pits and 10 rotary core boreholes which are positioned along the proposed stormwater diversion channel area.

The results of the geophysical surveys correspond well with the observed profile in the test pits and boreholes at the SWD inlet (line 3) and outlet (line 7 and line 8). It is noted that the “clay” overburden horizon is likely the pedogenic horizons observed in the profile.

7.4.1 INLET

Based on the test pit (TP01-SWD to TP09-SWD and TP23-SWD) and borehole (BH01-SWD to BH05-SWD and BH10-SWD) data, fine alluvial soils occur generally from the surface to a maximum of 3.95 m. Coarse alluvium is generally below the fine alluvium with a maximum depth of 3.8 m. Pedogenic soils were sporadically encountered from 0.6 m within the alluvium. The residual gneiss was limited in occurrence at the river and was encountered below the transported and pedogenic soils above the bedrock at depths greater than 2 m. The completely weathered gneiss zones were often described as a soil zone.

Gneiss bedrock was found in and along the river at shallow depths. The typical refusal depth at the inlet in the river course is approximately 3.5 m to dense residual gneiss/ hard rock gneiss. The test pits and boreholes on the flanks indicate that soft rock is typically encountered from a depth of 1.3 m to 3 m.

A possible fault breccia zone was encountered in SWD-BH2 from 10.34 m to 12.4 m while SWD-BH5 encountered medium hard rock with soft rock schistose bands.

Figure 7-1 shows the typical anticipated materials at various depths based on the test pit and borehole data. This does not denote the frequency of occurrence of a material type with depth. For more detailed material, depth, frequency of material types refers to Appendix B.

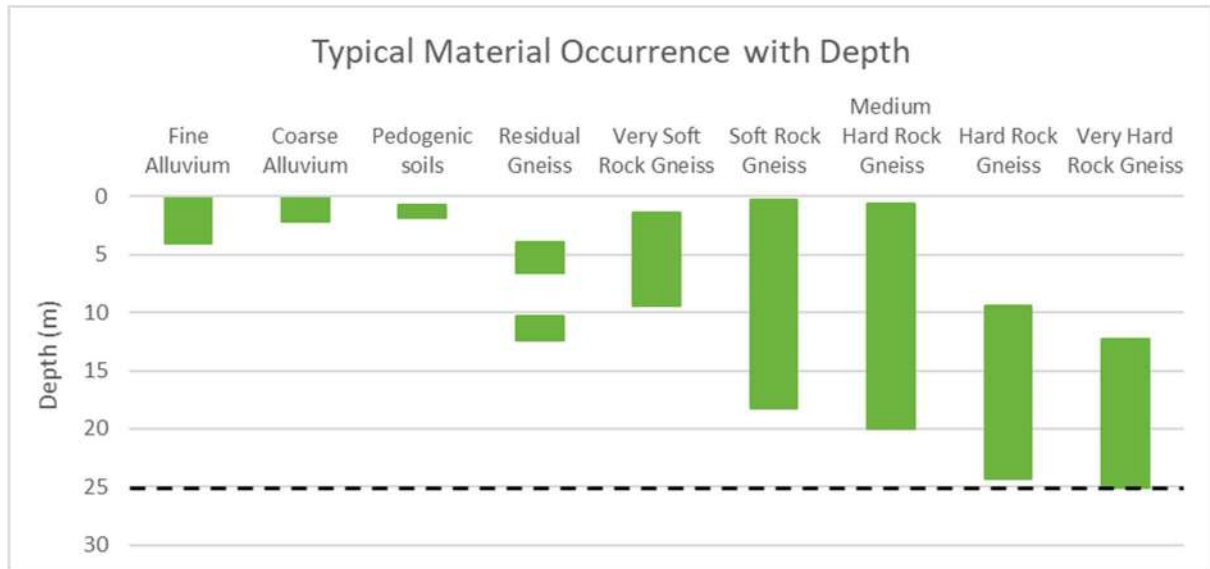


Figure 7-1: Typical Material Occurrence with Depth – SWD – Inlet

Groundwater was encountered in two test pits, TP04-SWD and TP05-SWD, from 1.5 m in boreholes BH01-SWD from 7.4 m, BH04-SWD from 14.6 m and BH05-SWD from 8.0m below ground.

The fine alluvial soils grade mainly as silty to clayey sands and have USCS classification of “SC” and “SM”. The fine alluvium also classifies as “A-2” and “A-4” based on the AASHTO classification system and has low potential expansiveness.

The ferruginised and the very soft rock gneiss both have similar soil characteristics grading clayey sand “SC” and “A-2”.

7.4.1.1 FOUNDATION RECOMMENDATIONS

The following recommendations are made for the SWD inlet structure:

- Excavate to 0.5 m below the SWD embankment footprint.
- Further excavate the 3 m wide cut-off trench to refusal of the excavator and batter-back to safe slope angle of 1:1.5 (V:H).
- In situ compact the excavation floor 90% Modified AASHTO compaction density. Back fill in maximum 200 mm layers using the fine alluvium compacted to 90% Modified AASHTO compaction density at OMC.
- On the downstream side of the cut-off trench, backfill the upper 0.5 m with coarse alluvium to 90% Modified AASHTO compaction density to create a drainage layer as per design.
- Continue the embankment earth works as per the embankment design.

7.4.1.2 EXCAVATABILITY

The river section is anticipated to experience soft excavations varying between 2.5 m and 3.9 m becoming intermediate to hard with depth. Loose soils cover the surface of the river area, and it is

recommended that the excavation walls be battered back at a safe slope of 1:2 (V:H). Seepage was also encountered in SWD-TP04 from 1.5 m indicating that dewatering of excavations should be considered.

The river flanks encounter soft excavation conditions in the upper 1.2 m, thereafter, becoming intermediate and hard excavation.

7.4.2 CHANNEL

The SWD channel was investigated by TP09-SWD to TP17-SWD and TP26-SWD as well as BH06-SWD and BH07-SWD. The test pits excavated through fine alluvium along the channel and generally indicate variable refusal depths in ferruginised or calcified alluvium most commonly between depths of 1.3 m and 4.4 m. The boreholes further indicate (very soft to medium hard) rock is encountered from 3 m to 7.6 m below ground level. There is no clear correlation with depth to refusal along the channel and variable conditions are anticipated along the route.

Figure 7-2 shows the potential materials at various depths based on the test pit and borehole data.

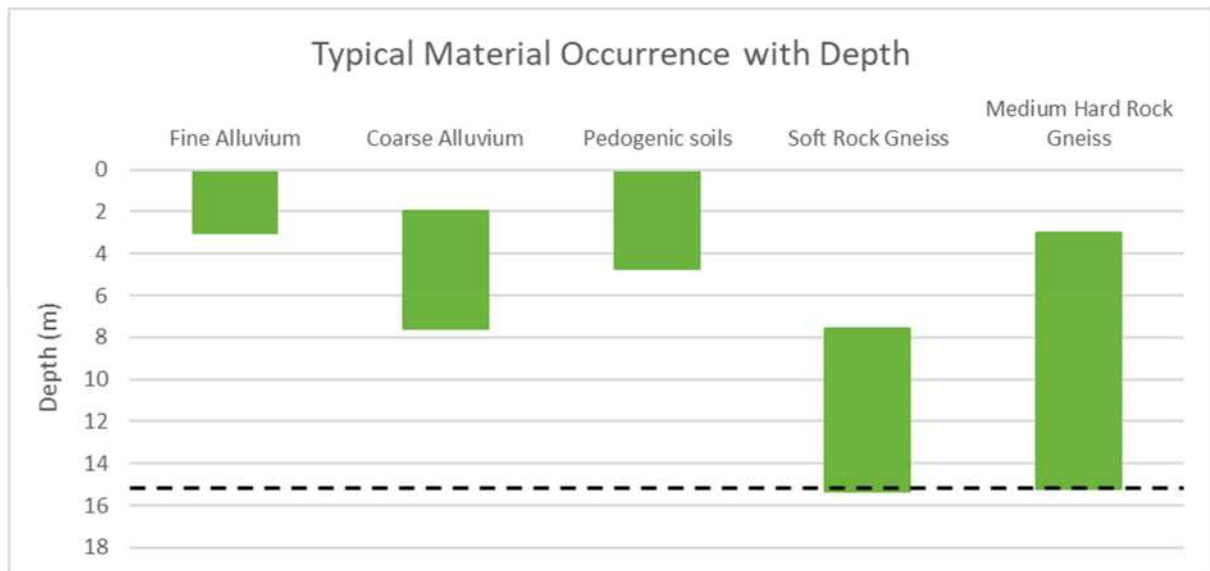


Figure 7-2: Typical Material Occurrence with Depth – SWD – Channel

The fine alluvial soils generally grade as silty sands and classify as “SC” and “A-4” based on the USCS and AASHTO classification systems. The material has low potential expansiveness and a “Good to Fair” compaction rating.

The calcified and ferruginised alluvium both classify as “SC” based on the USCS classification. Calcified alluvium grades as a gravelly sand and classifies as “A-2” while the ferruginised alluvium grades as a silty sand and classifies as “A-4”. Both materials have low potential expansiveness and a “Good to Fair” compaction ratio.

The sparsely encountered nodular ferricrete grades as a silty sandy gravel “GC” and “A-2” and has low potential for expansiveness.

7.4.2.1 FOUNDATION RECOMMENDATIONS

Excavate the channel to invert level (up to 10 m below ground level as per hydraulic design) battered back to a safe slope angle of 1:3 (V:H).

It is anticipated that intermediate and hard excavation will be required for the bulk of the excavation. Where soils are encountered in the channel, the following is recommended:

- in situ rip and recompact the channel
- place rip-rap or suitable erosion protection
- monitor the channel following rainfall and flooding events and remediate the channel as required.

7.4.2.2 EXCAVATABILITY

The excavatability conditions vary significantly along the channel area; however, the majority of the test pits were excavated to deeper than 2.5 m below the surface. It is anticipated that soft excavation conditions will be encountered to a depth of at least 1.1m below ground level and likely to a typical depth of 4 m. It is anticipated that intermediate and hard excavation will be encountered from a variable depth of 4m.

Groundwater was encountered only in test pit SWD-TP05 at 1.7 m below the surface. This test pit was dug in the river, and it is expected that groundwater mitigation measures may be required during construction.

7.4.3 OUTLET

The outlet area was investigated by TP20-SWD to TP24-SWD and BH08-SWD and BH09-SWD.

The boreholes drilled in the river course indicate soft rock encountered at 4.6 m to 6.8 m. The test pits excavated in the river course show refusal on coarse alluvium or soft rock to medium hard rock gneiss approximately 4.9 m.

The test pits on the flank refused at 1.1m on the southern flank and 4.3m on the northern flank in ferruginous alluvium. It is likely that the northern flank may have deeper alluvial soils. Soft conditions are anticipated above the refusal depth becoming intermediate to hard excavation from refusal and deeper.

Figure 7-3 shows the potential materials at various depths based on the test pit and borehole data.

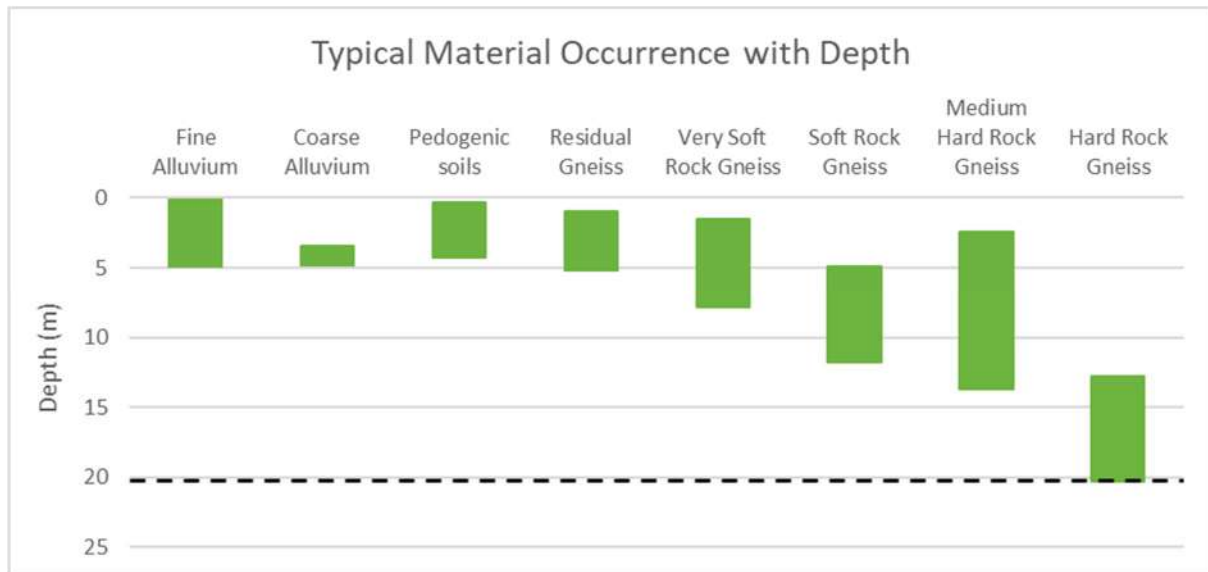


Figure 7-3: Typical Material Occurrence with Depth – SWD – Outlet

No groundwater was encountered in any of the test pits or boreholes drilled at the area of the SWD outlet.

7.4.3.1 RECOMMENDATIONS

The outlet structure requires erosion protection and is anticipated to tie in with the level created in the SWD channel. In situ compaction with rip-rap or adequate erosion protection is recommended.

7.4.3.2 EXCAVATABILITY

Based on the refusal depths of the test pit at the outlet area, it is expected that soft excavation conditions will be encountered from surface to 4 m in the river and from the surface to 2.5 m on the river flanks thereafter intermediate to hard excavation conditions are anticipated due to the presence of medium hard rock gneiss at depth.

7.4.4 VOLUME ASSESSMENT

The volumes assessment was based on the test pit and borehole data of the material at four sections along the SWD which include the inlet (Section 1), south-western portion of the channel until the bend (Section 2), from the bend towards the north-eastern portion of the channel (Section 3), from the north-eastern portion to the outlet of the channel (Section 4). Figure 7-4 presents the localities of the four sections along the SWD.



Figure 7-4: Localities of the Four Sections Along the SWD

Table 7-1: Estimated Material Volumes at Section 1

Material	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected Volume (m ³)
Fine Alluvium	22,300	0.4 – 4.0	9,200 – 92,000
Coarse Alluvium	22,300	0.3 – 1.2	6,500 – 27,500
Pedogenic Soils	22,300	0.7 – 0.8	16,000 – 18,000
Residual Gneiss	22,300	0.3 – 2.1	6,500 – 48,000
Very Soft Rock Gneiss	22,300	0.5 – 2.7	11,500 – 62,000
Soft Rock Gneiss	22,300	0.3 – 8.0	6,500 – 184,000
Medium Hard Rock Gneiss	22,300	2.5 – 13.7	57,500 – 315,000
Hard Rock Gneiss	22,300	1.2 – 9.4	27,500 – 216,000
Very Hard Rock Gneiss	22,300	± 5.8	± 133,000

Table 7-2: Estimated Material Volumes at Section 2

Material	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected Volume (m ³)
Fine Alluvium	171,300	0.3 – 3.0	51,000 – 513,500
Coarse Alluvium	171,300	± 4.6	± 787,500
Pedogenic Soils	171,300	0.9 – 2.8	154,000 – 479,500
Residual Gneiss	171,300	-	-
Very Soft Rock Gneiss	171,300	-	-
Soft Rock Gneiss	171,300	± 7.7	1,319,000
Medium Hard Rock Gneiss	171,300	-	-
Hard Rock Gneiss	171,300	-	-
Very Hard Rock Gneiss	171,300	-	-

Table 7-3: Estimated Material Volumes at Section 3

Material	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected Volume (m ³)
Fine Alluvium	167,000	0.5 – 1.9	83,500 – 317,000
Coarse Alluvium	167,000	0.7 – 1.9	116,500 – 317,000
Pedogenic Soils	167,000	1.0 – 2.1	167,000 – 350,500
Residual Gneiss	167,000	-	-
Very Soft Rock Gneiss	167,000	± 1.3	217,000
Soft Rock Gneiss	167,000	-	-
Medium Hard Rock Gneiss	167,000	12.2	2,037,000
Hard Rock Gneiss	167,000	-	-
Very Hard Rock Gneiss	167,000	-	-

Table 7-4: Estimated Material Volumes at Section 4

Material	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected Volume (m ³)
Fine Alluvium	87,600	0.3 – 4.9	26,000 – 429,000
Coarse Alluvium	87,600	± 1.4	122,500
Pedogenic Soils	87,600	0.3 – 2.9	26,000 – 254,000
Residual Gneiss	87,600	± 0.5	43,500
Very Soft Rock Gneiss	87,600	0.9 – 2.6	78,500 – 227,500
Soft Rock Gneiss	87,600	0.1 – 4.0	8,500 – 350,000
Medium Hard Rock Gneiss	87,600	1.0 – 5.7	87,500 – 499,000
Hard Rock Gneiss	87,600	6.5 – 7.4	569,000 – 648,000
Very Hard Rock Gneiss	87,600	-	-

7.5 NEW ROAD (RD)

The evaluation and recommendations in this sub-section are based on the 16 excavated test pits of the 20 planned test pits for the new road. Four test pits (TP06-RD, TP17-RD, TP18-RD and TP19-RD) were inaccessible and thus not excavated in the western portion of the new road area.

Test pits TP02-RD, TP03-RD and TP16-RD were moved further downstream outside of the original neighbouring farm area to be excavated. This section will be divided into three subsections which will comprise the first two road-river crossings located near the south-western area of the RD site, the third and fourth road-river crossings located on the north-eastern side of the RD site, and the road alignment between the river crossings. The structure coordinates and localities are indicated below in Table 7-5 and Figure 7-5, respectively.

Table 7-5: Road-River Crossing Positions

Structure	Zone	Latitude/Northing	Longitude/Easting
Road-River Crossing 1	33 K	7578647.06 m S	800786.81 m E
Road-River Crossing 2	33 K	7579935.53 m S	801451.66 m E
Road-River Crossing 3	33 K	7583486.00 m S	806364.43 m E
Road-River Crossing 4	33 K	7583366.36 m S	806879.69 m E



Figure 7-5: Road-River Crossing areas of investigation for the new road

7.5.1 CROSSINGS 1 AND 2

Laboratory results, as well as test pit data from test pits TP01-RD, TP02-RD, TP03-RD and TP16-RD were used to determine the expected foundation conditions for the structures at the river crossings at the south-western portion of the new road.

The test pits in this area refused on dense silty sand with varying gravel, alluvium to a depth of 3.9 m or in soft rock gneiss at a depth of 4.3 m in test pit TP02-RD which was dug in the middle of the river. It is expected that the rock hardness increases with depth. Figure 7-6 illustrates the typical materials encountered at various depths for this area is shown below.

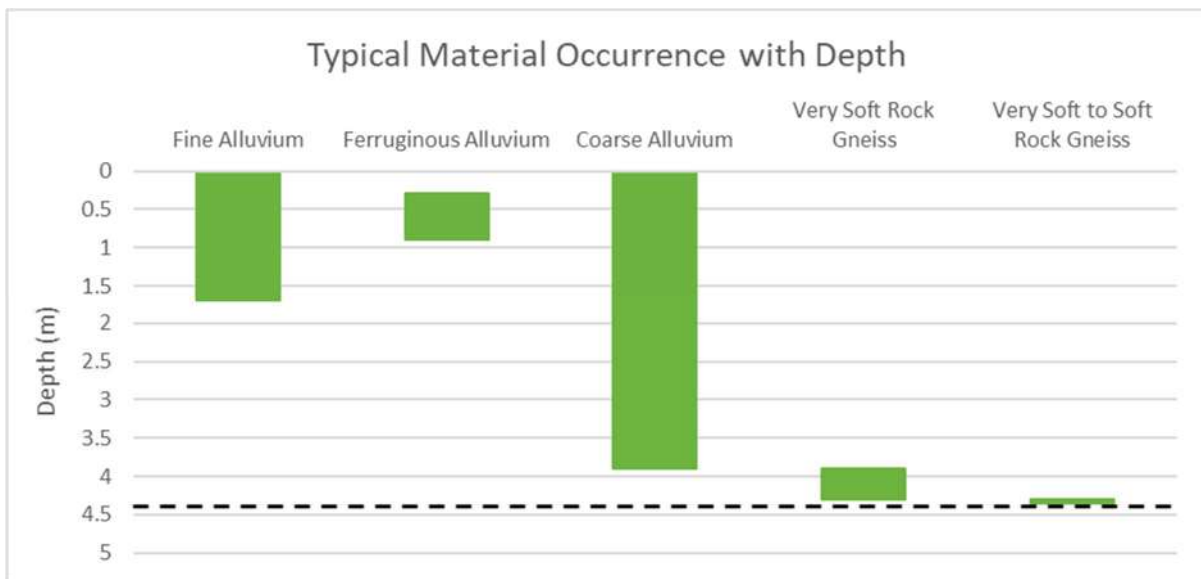


Figure 7-6: Typical Material Occurrence with Depth – Crossings 1 & 2

Based on the test pit data, the fine alluvium and ferruginous soils comprise loose to generally dense silty sand with occasional sub-angular to subrounded gravels with a typical allowable bearing capacity of 200 kPa.

The coarse alluvium as well as the very soft rock gneiss comprise mainly gravelly sands and classify both as “SC” and “A-2” according to the USCS and AASHTO classification systems. The potential expansiveness of both materials is low and has “Good to Fair” compaction ratings.

7.5.1.1 FOUNDING RECOMMENDATIONS

It is recommended that founding of any bridge embankments be placed on at least dense soils not exceeding the allowable bearing capacity of 200 kPa at depths varying from 1.7 m to 4.3 m across the area. Foundation floors should be in situ compacted to at least 90% Modified AASHTO compaction density at OMC.

Seepage was encountered from a depth of 2.3 m below surface and dewatering measures should be considered during construction.

7.5.1.2 EXCAVATABILITY

Based on the refusal depths of the test pits next to the river, it is anticipated that soft excavation conditions will be encountered to a depth of 1.7 m to 4.3 m becoming intermediate to hard with depth.

Test pit or trench which will be dug in the river will not experience difficult excavation conditions due to the loose material. Collapse of excavation sidewalls and trench walls are expected during excavation in the upper horizons. It is recommended that the trench and excavation walls be benched or sloped at a safe slope angle of at least 1:2 (V:H).

7.5.2 CROSSINGS 3 AND 4

The data from test pit TP11-RD and TP13-RD and the laboratory results were used to evaluate the river crossing at points 3 and 4 of the RD site.

The soil profile near the river (TP11-RD) comprises medium dense to dense fine alluvium to a depth of 1.4 m while the soil profile always from the river towards the south comprises dense fine alluvium to 1.8 m, becoming calcareous to 2.6 m. The medium dense to dense pebble marker (coarse alluvium) extends to a depth of 4.0 m below surface whereafter very soft rock gneiss is encountered to 4.5 m before test pit refusal on soft rock. Figure 7-7 shows the typical materials at various depths for this area is shown below.

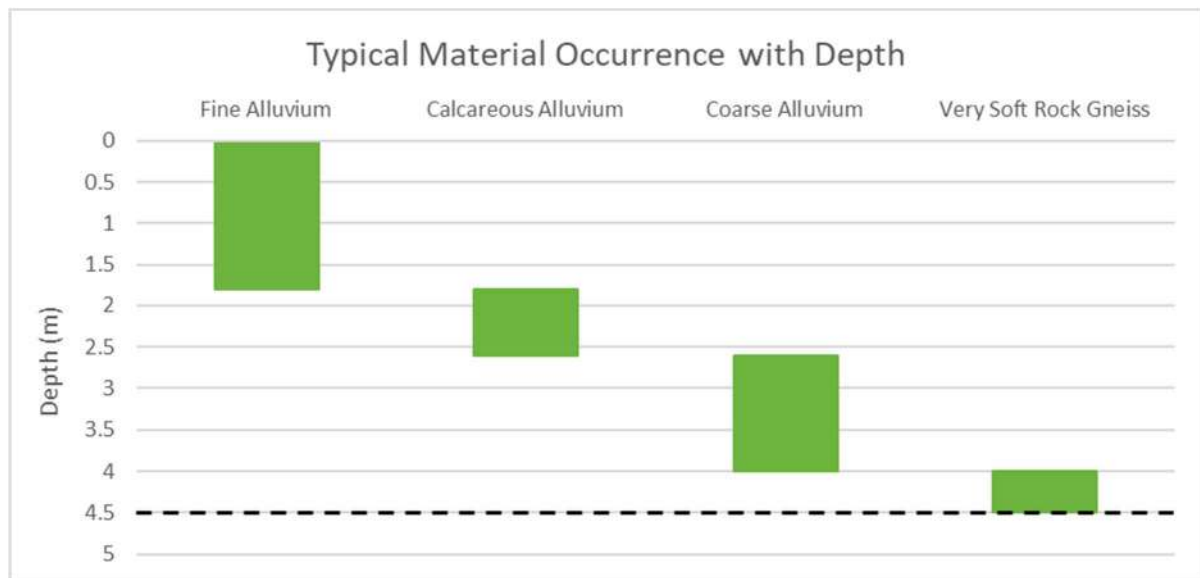


Figure 7-7: Typical Material Occurrence with Depth – Crossings 3 & 4

The silty sand coarse alluvium grades as USCS and AASHTO classification of “SC” and “A-4”. This material has low potential expansiveness and has a “Good to Fair” compaction rating (Hazelton & Murphy, 2007).

7.5.2.1 FOUNDING RECOMMENDATIONS

Similar foundation conditions are present as at Crossing 1 and 2 as here in Crossing 3 and 4 and as such it is recommended that the bridge embankment foundations be founded on the dense alluvium or very soft rock gneiss with an allowable bearing capacity of at least 200 kPa.

Foundation floors should be in situ compacted to at least 90% Modified AASHTO compaction density at OMC.

7.5.2.2 EXCAVATABILITY

Similar excavation conditions as Crossings 1 and 2 are expected at Crossings 3 and 4, with test pits or trenches next to the river being having soft excavation conditions to a depth varying from 1.4 m to 4.5 m.

Excavations walls in the river should also be benched or sloped at a safe slope angle of at least 1:2 (V:H) or battered back to support the excavation walls.

Dewatering of the excavation may be required during construction.

7.5.3 ROAD ALIGNMENT

The typical soil profile between the river crossings comprises mainly medium dense to dense, with occasional loose pockets of fine alluvium from surface down a depth of 4.3 m. Coarse alluvium from 0.3 m to 1.4 m or ferruginised alluvium from 0.3 m to 0.7 m is encountered below shallow fine alluvium. Medium dense becoming dense calcified alluvium was identified in test pit TP08-RD from 1.0 m down to 3.2 m.

Very soft rock gneiss was encountered only in test pit TP14-RD from 1.4 m down to 2.6 m after which the excavator refused on soft rock gneiss. Figure 7-8 shows the materials that are typically encountered with depth across this area.

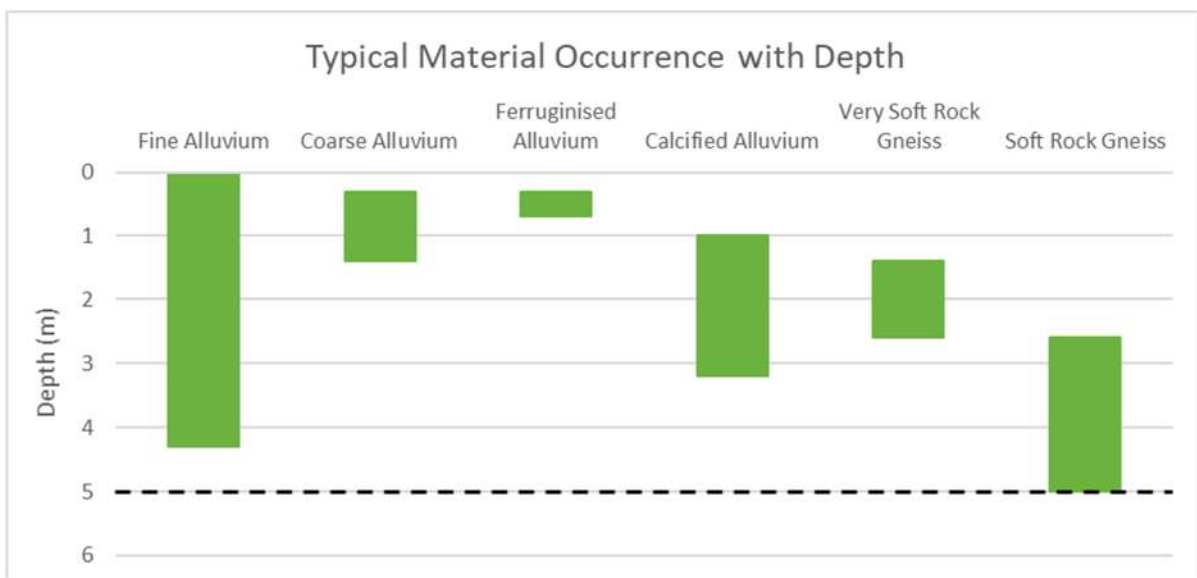


Figure 7-8: Typical Material Occurrence with Depth – Road Route

The fine and calcified alluvium grade mainly as silty or clayey sands with low gravel content in the coarse alluvial material. The fine alluvial soils comprise more than 60% sand and no gravel. The alluvium classifies as “SM” based on the USCS system; however, the fine alluvium classifies as “A-2” while the coarse alluvium classifies as “A-7” based on the AASHTO classification. The materials have a low potential expansiveness and low compressibility rating, as well as a “Good” compaction rating.

Very soft rock gneiss comprises mainly silty sandy gravel and classifies as a well graded gravel (GM) based on the USCS system. An AASHTO group of “A-2” was assigned to this material.

The plasticity index of the soils is generally below 10 %.

The compaction characteristics of the tested materials are summarised in the Table 7-6 below.

Table 7-6: CBR Values of the Tested Samples Between the Road-River Crossings

Test Pit No.	Sample Depth (m)	Material	MDD (kg/m ³)	OMC (%)	CBR at MOD AASHTO Compaction			% Swell	COLTO
					93%	95%	98%		
TP04-RD	0.6-2.9	Fine Alluvium	2041	8.1	32	46	81	0.1	G4*
TP08-RD	1.0-3.2	Calcified Alluvium	1640	20	4	4	5	0.0	NC
TP12-RD	1.1-4.3	Fine Alluvium	2119	7.4	23	30	46	0.0	G6
TP14-RD	1.4-2.6	Very Soft Rock Gneiss	2016	12.9	15	17	20	0.0	G7

Notes:

1. NC – Not Classifiable.
2. The G4 grading of TP04-RD is not considered representative of the material and is anticipated due to the uncharacteristic coarse portion of this sample.

7.5.3.1 UNPAVED WEARING COURSE FOR GRAVEL ROADS

The materials encountered on site have been further evaluated for re-use as unpaved wearing course according to THR20 [8] and the materials are plotted in Appendix B4 by material type. The classes of materials are briefly described below:

- Class A: Materials in this area generally perform satisfactorily but are finely graded and particularly prone to erosion by water: they should be avoided, if possible, especially on steep grades and sections with steep cross-falls and super-elevations. Most roads constructed from these materials perform satisfactorily but may require periodic labour-intensive maintenance over short lengths and have high gravel losses due to water erosion.
- Class B: These materials generally lack cohesion and are highly susceptible to the formation of loose material (ravelling) and corrugations. Regular maintenance is necessary if these materials are used, and the roughness is to be restricted to reasonable levels.

- Class C: Materials in this zone generally comprise fine, gap-graded gravels lacking adequate cohesion, resulting in ravelling and the production of loose material.
- Class D: Materials with a shrinkage product in excess of 365 tend to be slippery when wet.
- Class E: Materials in this zone perform well in general, provided the oversize material is restricted to the recommended limits.

The alluvium generally classifies as material that is erodible (Class A) and which ravel and corrugates (Class B). This has been seen on the unpaved roads on site. The alluvium also classifies to a lesser extent as slippery (Class D) and good for unpaved roads (Class E).

The pedogenic soils generally follow the same trend as the alluvium classifying as material that is erodible (Class A) and which ravel and corrugates (Class B). The pedogenic alluvium (calcareous/calclified/ferruginous/ ferruginised alluvium) further classifies to as slippery (Class D) and good for unpaved roads (Class E).

The residual gneiss soils generally follow the same trend as the alluvium classifying as material that is erodible (Class A) and which ravel and corrugates (Class B).

The very soft rock gneiss that was tested reveals material that is generally good for unpaved roads (Class E), but which ravel and corrugate (Class B).

The CBR values that were tested vary significantly within each material type typically from as low as 3% to 36% when compacted at 93% Modified AASHTO. The COTO classification generally varied from poorer than G9 to G6 quality material. Occasionally, high gravel content has shown G4 quality material which is not considered to be well represented.

7.5.3.2 FOUNDING RECOMMENDATIONS

The road alignment soils are generally low plasticity soils that lack binding materials. The following is recommended:

- Excavate and spoil the upper 0.3 m organic layer.
- In situ rip and recompact the subgrade along the road alignment to 93% Modified AASHTO compaction at OMC to ensure a minimum CBR value of 7 %.
- Localised deeper compaction in loose zones is anticipated along the alignment.
- Founding of the surface wearing course on dense alluvium.
- Import a suitable surface wearing course placed as per road design.
- Ensure that the road is shaped to prevent ponding of water along the alignment and ensure that water is carried away.

7.5.3.3 EXCAVATABILITY

Based on the test pit data soft excavations conditions are expected to a typical depth of 3.5 m thereafter intermediate and hard excavation conditions are expected with localised deeper excavation nearer the river areas.

7.6 WASTE ROCK DUMP (WRD)

The waste rock dump (WRD) area covers approximately 710 ha of the mine and is divided into two areas for the purpose of this section. The first area, namely Area A (579 ha), is located on the eastern portion of the site and the second area, Area B (130 ha), which is located between the HLP and SWD. The localities of Area A and Area B (outlined in yellow) is shown below in Figure 7-9.

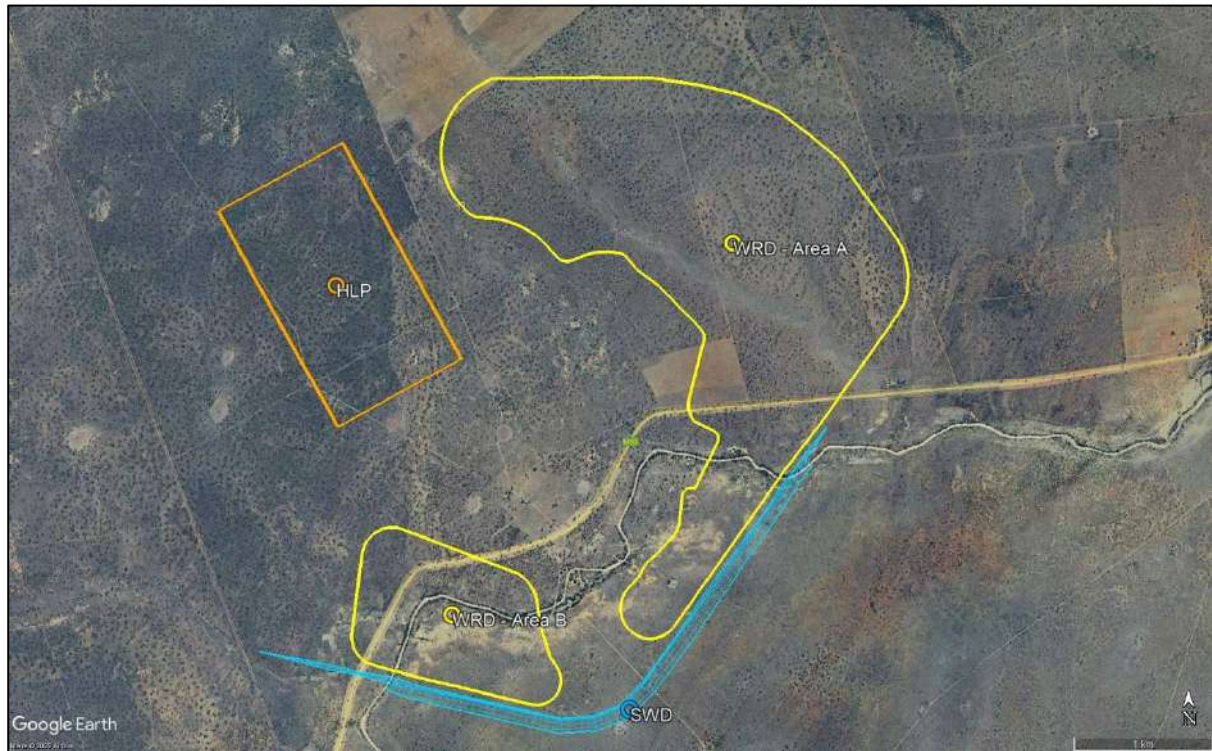


Figure 7-9: WRD - Area A and Area B

7.6.1 AREA A

The majority of Area A is covered with fine alluvium from the surface down to depths varying between 0.2 m and 1.6 m. A coarse alluvium occurs below this and extends generally between 1.1 m and 2.5 m below the surface. Deeper coarse alluvium (encountered to depths of 4.1 m) was recorded near the river area of Area A (WRD-TP17). It is anticipated that a downward coarsening trend is anticipated for the river area.

The alluvium often becomes ferruginous and calcareous with depth, which can be attributed to the existing drainage feature in the north running through Area A from the north-west to the south-east boundary of the WRD boundary joining the Black Nossob river at the new road crossings 3 and 4. The pedogenic soils was identified from as shallow as 0.6 m to as deep as about 5.0 m near the riverbed.

Hardpan calcrete was recorded in 6 test pits in the north to north-western portions of Area A and occurs from roughly 0.2 m down to 2.6 m. The depth and layer thickness of the hardpan calcrete varies significantly across the area of occurrence.

Residual gneiss is generally observed below the transported and pedogenic soils and was encountered from depths as shallow as 1.1 m down to as deep as 2.4 m below the surface. These residual soils mainly occur mainly in the eastern and central portions of Area A.

Very soft rock gneiss occurs either below the alluvial or pedogenic soils from as shallow as 0.5 m down to as deep as 2.9 m below ground level when encountered. Soft rock to medium hard rock follow with depth down to approximately 3.4 m below surface. Refusal often occurs above the rock surface in fine or calcified alluvium, or on hardpan calcrete.

Based on the laboratory results, the alluvium classifies as clayey sand to low plasticity clays (SC to CL) according to the USCS classification and as “A-2” and “A-6” according to the AASHTO classification system. This material has low potential expansiveness and a compaction rating of “Good to Fair” based on the USCS classification. The WRD soils range between silty clayey sands and sandy gravels.

The pedogenic soils comprise mainly of sandy gravel of varying grading (GC, GP and GM) based on the USCS and as “A-2” based on the AASHTO classification. This material has a low potential expansiveness and a “Good” compaction rating.

The residual gneiss is described as silty clayey sand (SC and A-2), with sand making up more than 60 % of the total texture. In addition, the potential expansiveness of the residual gneiss is low.

7.6.2 AREA B

The northern portion outside the WRD site boundary has alluvial soils occurring from the surface down to less than 2.0 m while the southern and eastern portions have deeper alluvial horizons generally to 3.0 m but occasionally as deep as 5.0 m. This alluvial material classifies as clayey sand (SC) based on the USCS system and as “A-4” and “A-6” according to the AASHTO classification system. This material also has a “Good to Fair” compaction rating and a low potential expansiveness.

Pedogenic soils (mainly calcified alluvium) occur along and south of the riverbed from as shallow as 0.3 m to 4.4 m. The thickness of the pedogenic horizon does vary but is generally thicker than 2.5 m in this area. The pedogenic soils comprise mainly of sandy gravel with minor variations such as silty and gravelly sands. The material classifies as GC, SC and SM according to the USCS classification system and “A-2” and “A-6” in the AASHTO system. The potential expansiveness of this material is low, and it has a “Good to Fair” compaction rating based on the USCS classification.

Residual gneiss was identified near the riverbed as well as slightly north of this area of the WRD. The depth of occurrence range generally between 1.2 m and 1.6 m with the exception of test pit TP1-WRD having residual material down to 3.5 m. This residual material is generally a sand with minor amounts of clay and silt, with the exception of test pit TP2-WRD grading as a sandy gravel. It is therefore expected that the residual material closer to the riverbed be slightly finer grained and become coarser further away from the riverbed. The USCS classification of the residual soils range between “SC” and “SM” for the finer graded soils and between “GP” and “GM” for the residual soils further away from the riverbed. The AASHTO group for the finer soils is “A-2” while the coarser soils are assigned an “A-1” classification.

Very soft rock occurs from approximately 1.3 m below surface and extends to 1.8 m after which it transitions to soft rock from 1.8 m. Test pit TP6-WRD has medium hard rock gneiss occurring from 4.3 m below surface and it is therefore expected that the transition between soft rock and medium hard rock may occur from 2.5 m and 4.0 m. The very soft to soft rock gneiss grades as a gravelly sand and has a USCS classification of “SC” and an AASHTO code of “A-2” assigned to the tested material.

7.6.3 RECOMMENDATIONS

The recommendations for the waste rock dump are to scarify the surface soils to remove the organic component. In situ rip and recompact to 90% Modified AASHTO compaction density at OMC. Place the waste rock with safe side slopes of no steeper than 1:2 (V:H).

7.6.4 EXCAVATABILITY

The test pits at and around Area A of the WRD generally refused at depths shallower than 2 m towards the north with slightly deeper test pits present near the center and southern portions. Softer excavation conditions are expected to depths of 2 m becoming intermediate to hard below.

The test pits at Area B and near the riverbed generally refused or terminated at depths between 3 m and 5 m.

It is therefore expected that excavation conditions near the riverbed will be “soft” to depths of 3.0 m thereafter intermediate and hard excavation conditions are expected.

7.7 POTENTIAL BORROW SOURCES

The identified potential borrow pit area is divided into two areas namely, Area A which is located south of the road and north of the river and Area B adjacent to the river on both sides. The approximate boundary of each area is highlighted in light blue in Figure 7-10 below. Note that the typical soil profile is described below for the whole AM site and not separately for Area A and Area B.



Figure 7-10: Additional Material Source - Area A and Area B

The borrow pit area comprises mainly alluvium from the surface down to 1.0 m below surface. The majority of the test pits refuse on this layer due to the dense soil consistency of this alluvium.

Where the test pits did not refuse on this dense fine alluvium, a 0.3 m coarse alluvium layer is seen underneath it. These alluvium layers often become ferruginous or calcareous with depth and may become calcified as seen in test pit AM-TP04.

Very soft rock gneiss is observed below the alluvium and pedogenic soils from approximately 1.0 m below surface which quickly transitions to soft rock where seen in the two test pits AM-TP2 and AM-TP4 in the west of Area A.

A prominent hardpan ferricrete outcrop was identified during the field investigation along the road on the western boundary of Area A. The size of the ferricrete outcrop is estimated at 8,800 m². The occurrence of the ferricrete outcrop is highlighted in orange in Figure 7-11 below.



Figure 7-11: Ferricrete Outcrop at the AM site

Based on the laboratory results, the fine alluvium comprises mainly silty to clayey sands, with fines component (clay and silt) greater than 40 %. The clay portions of these soils typically range between 19 % and 34 % with a plasticity index (PI) of less than 12 %. A USCS classification of “SC” was generally assigned to these alluvial soils and an AASHTO classification of “A-4”.

The coarse alluvium (pebble marker) comprises mainly sandy gravel with more nearly 50 % gravel. A USCS classification of “GC” and an AASHTO classification of “A-2” was assigned to this material.

The pedogenic material comprised of 75 % gravel and 21 % sand and was classified as poorly graded gravel (USCS classification: GP). The AASHTO classification of “A-1” was assigned to this material.

The gneiss bedrock comprising very soft rock comprised mainly clayey silty sand, with sand making up more than 70 % of the soil. The material is classified as non-plastic and has a low potential

expansiveness. A USCS classification of “SC” was assigned to this material, along with an AASHTO classification of “A-1”.

Where it was possible to excavate into the deeper, soft to medium hard rock gneiss, the laboratory results indicate that equal amounts (40%) of gravel and sand are present with a PI of 12 %. This material classifies as “SC” and “A-4” according to the USCS and AASHTO classifications, respectively.

7.7.1 VOLUME ASSESSMENT

The material volumes are estimated based on the area of occurrence and the layer thickness of each assessed material. Table 7-7 and Table 7-8 show the estimated volumes for the materials encountered at Area A and Area B, respectively.

Table 7-7: Estimated Material Volumes at Potential Borrow Pit Area A

Material	Position	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected In Situ Volume (m ³)
Fine Alluvium	Area A	379,500	0.3 – 0.7	113,500 – 265,500
Coarse Alluvium	Area A	379,500	0.2 – 0.3	75,500 – 113,500
Pedogenic Soils	Area A	379,500	0.1 – 0.8	37,500 – 303,500
Very Soft Rock Gneiss	Area A	379,500	0.2 – 0.3	75,500 – 113,500
Soft Rock Gneiss	Area A	379,500	N/M	N/M

Notes:

1. N/M – Not measurable.
2. “-” – Material not encountered.

Table 7-8: Estimated Material Volumes at Potential Borrow Pit Area B

Material	Position	Approximate Area Coverage (m ²)	Approximate Layer Thickness (m)	Expected In Situ Volume (m ³)
Fine Alluvium	Area B	330,900	0.6 - 1.2	198,500 - 397,000
Coarse Alluvium	Area B	330,900	-	N/M
Pedogenic Soils	Area B	330,900	± 0.4	± 132,000
Very Soft Rock Gneiss	Area B	330,900	-	N/M
Soft Rock Gneiss	Area B	330,900	-	N/M

Notes:

1. N/M - Not measurable.
2. "-" - Material not encountered.

7.7.2 EXCAVABILITY

The alluvial soils covering the surface of the potential borrow pit area has a dense soil consistency on which the excavator generally refused during the field investigation. Based on (SANS634, 2012), the excavation down to refusal depths, generally the upper 1.2 m, can generally be classified as “soft” while the conditions to refusal depth can be classified as “Intermediate to hard”.

“Soft” excavation conditions refer to material, which can be excavated efficiently using a back-acting excavator while “Intermediate” refers to material which can be removed using a back-acting excavator or that needs to be removed using pneumatic tools.

7.8 MATERIAL REUSE

The materials encountered across the site have been evaluated by type and discussed in the relevant sections. It should be noted that the variability within the soils on site is significant, owed both to the nature of the alluvium during its deposition causing fine or coarse lenses and due to the erratic nature of the pedogenic soils distribution both laterally and horizontally within the profile.

The materials on site are generally classified as clayey sands to gravels with varying CBR values. COTO classification generally varies between poorer than G9 and G6 quality materials classifying as A-1, A-2 and A-4 materials with occasional A-6 soils.

No distinct suitable borrow source of clay was identified during the investigation, although fine grained soils (classified SC and SM) occur throughout the site at various depths. The plasticity of the fine-grained soils is generally less than 10%.

The permeability of the materials when remoulded vary between 10^{-6} m/s and 10^{-8} m/s due to the generally fine-grained nature of the materials. The laboratory results indicate that the fine alluvium has a low permeability when compacted to 90% Modified AASHTO compaction density. This fine alluvium can be used for the construction of the clay liner preparation layer for the heap leach pad, since the permeability of such a material should typically be 1×10^{-8} m/s.

The reuse of soils for road construction has been discussed in Section 7.5.3.1 and is generally susceptible to erosion, ravelling and corrugation due to the low PI.


The alluvium can be reused as general fill during construction. The coarse alluvium and pebble marker is suitable material for the subgrade of roads, selected earthworks for foundation platforms and untreated road layers. The material can also be used for trench backfilling. The gravelly nature of the pedogenic soils as well as the residual gneiss make these materials suitable to be used as general fill, selected layers for foundation platforms, as well as subgrade and subbase material for road construction. The very soft to soft rock gneiss can also be used as subgrade and subbase for road construction. In addition, the very soft rock gneiss and the soft to medium hard rock has a “Good” compaction rating and can also be used as general fill material or as subgrade in road construction.

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9.0 CERTIFICATION


This report was prepared and reviewed by the undersigned.

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Senior Engineering Geologist

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Technical Consultant

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APPENDIX A

Geotechnical Site investigation Program

Appendix A1

Site Locality Map

Appendix A2

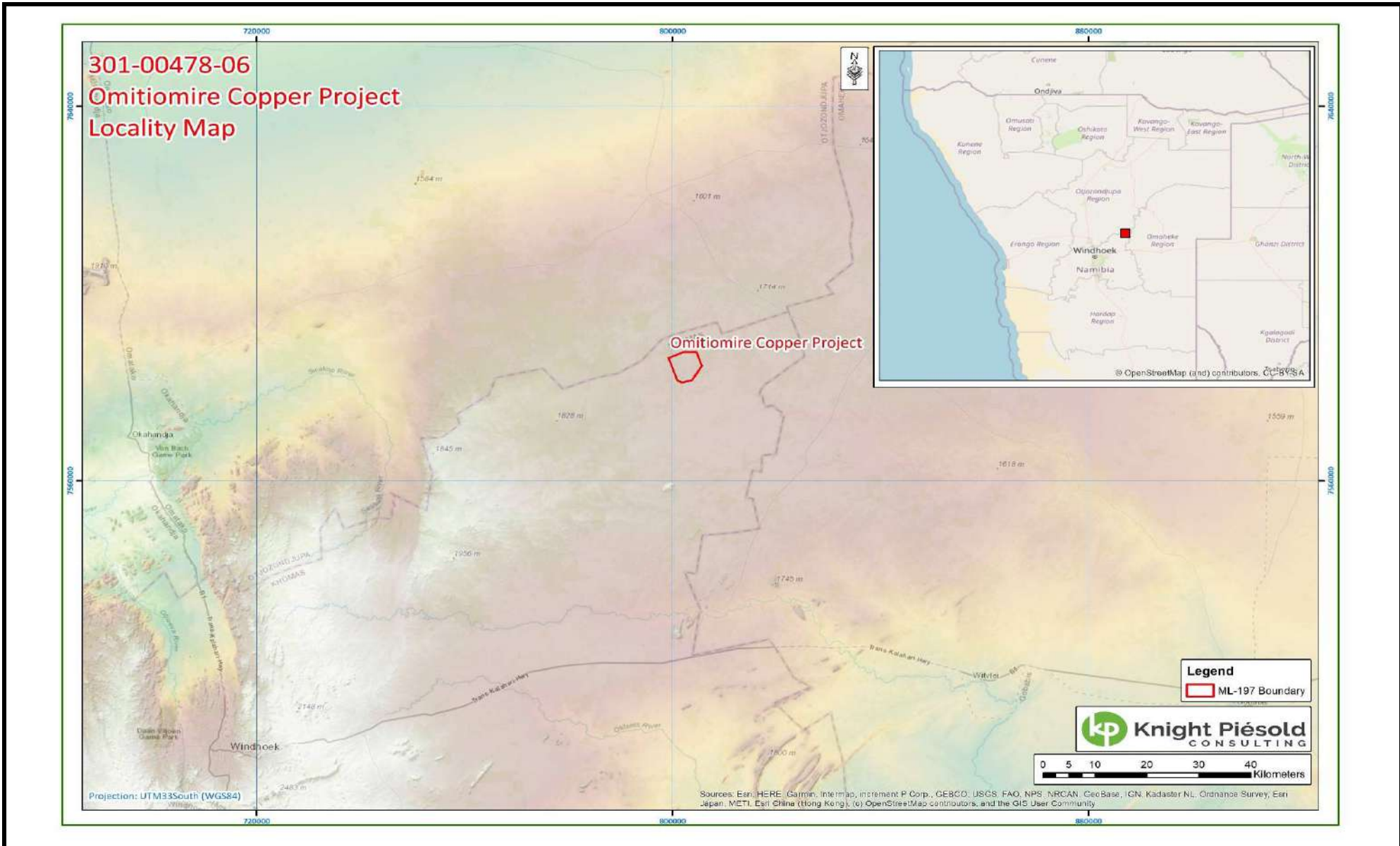
Layout of Investigated Points


Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX A1

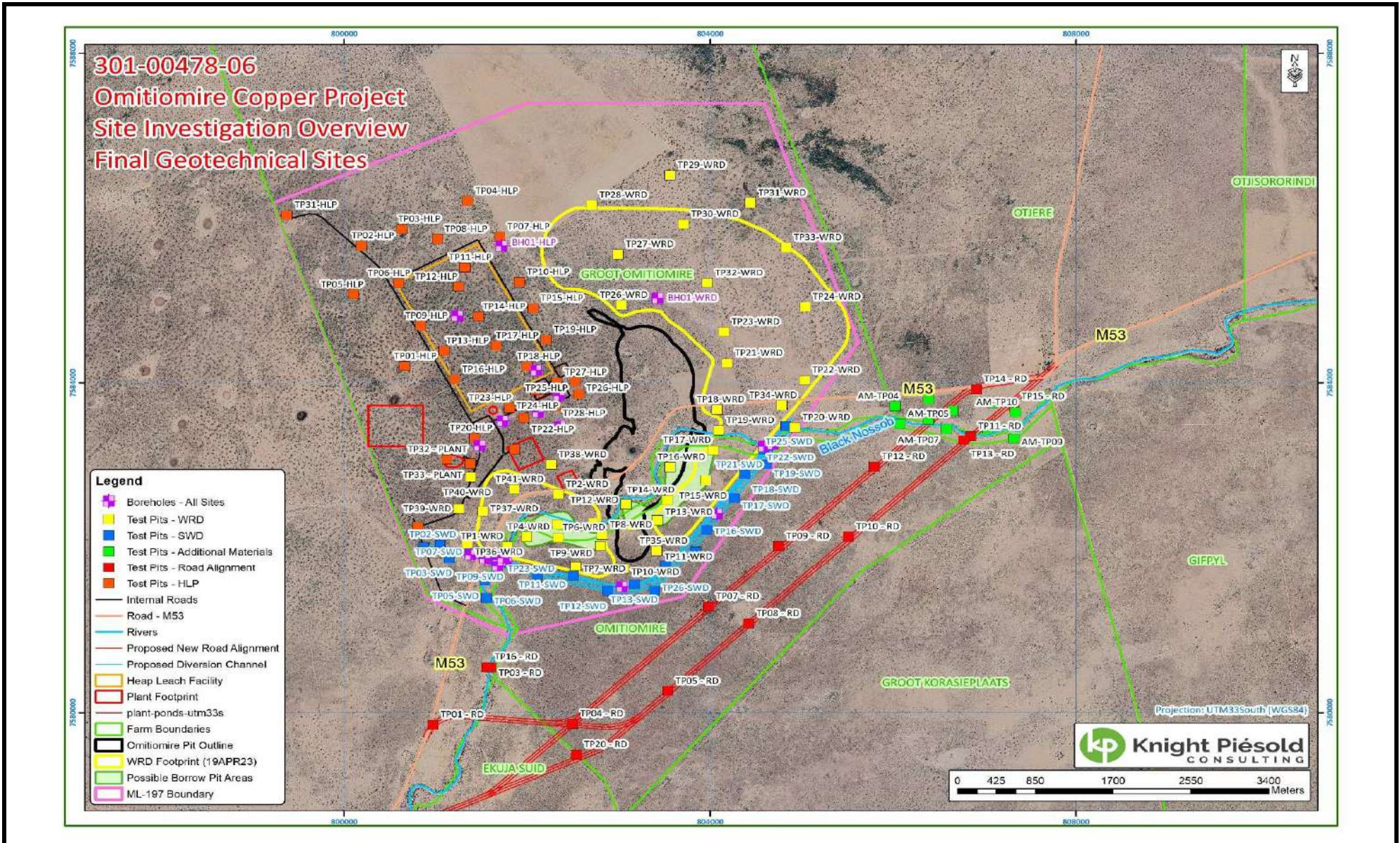
Site Locality Map




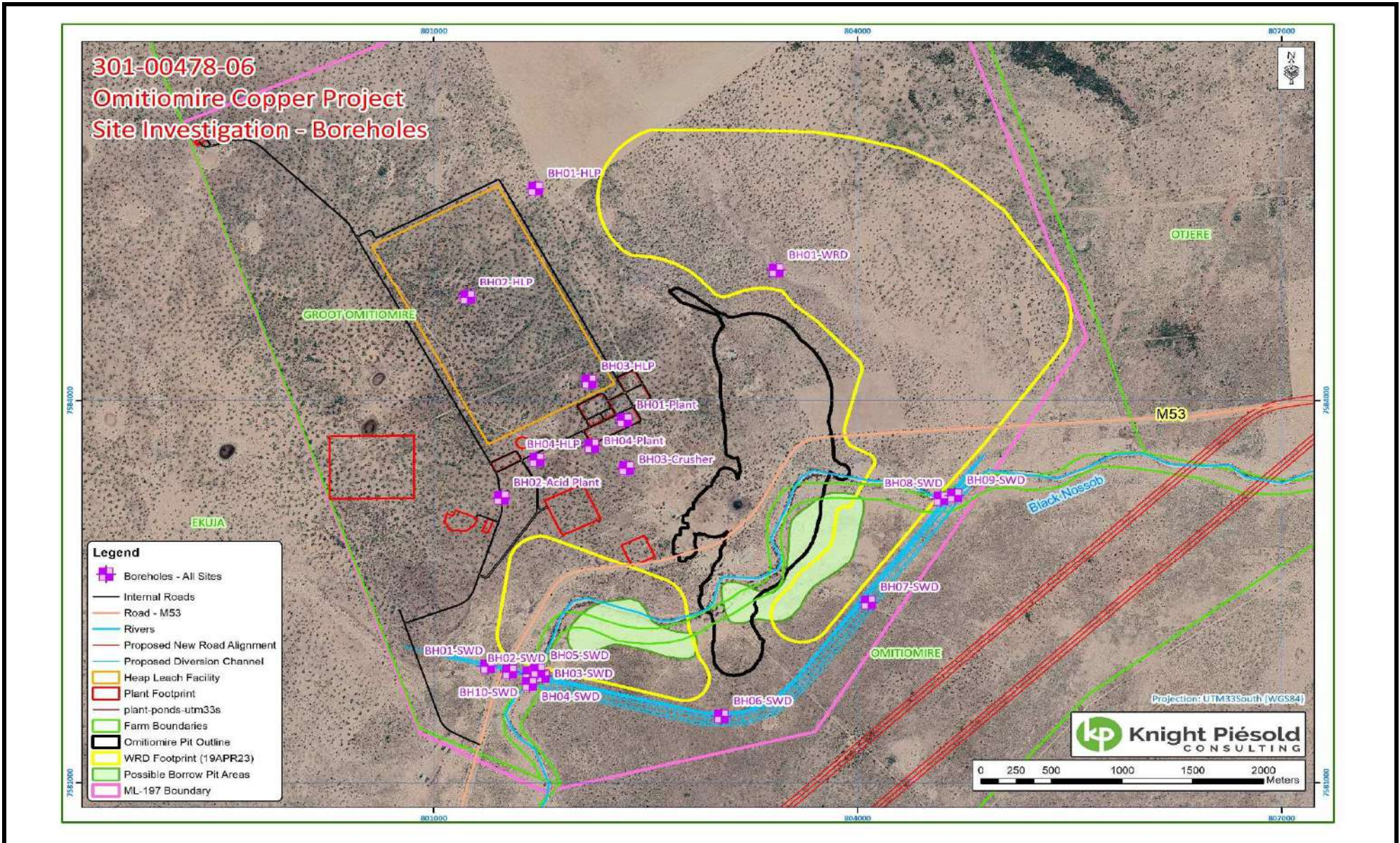
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	BY:	T Oosthuizen		CLIENT: Craton Mining and Exploration (Pty) Ltd	FIGURE NO: A-1	
	CHECKED:	B Klaas				
	DRAWING DATE:	30-Jun-23				
	REVISION:	A				

APPENDIX A2

Layout of Investigated Points



	PROJECT NO.	WI 301-00478/06	Coordinate System: WGS 1984 UTM Zone 33S	Omitiomire Copper Project: Location of geotechnical boreholes & test pits - All sites	
	BY:	T Oosthuizen	Projection: Transverse Mercator		
	CHECKED:	B Klaas	Datum: WGS 1984		
	DRAWING DATE:	30-Jun-23	Units: Meter		
REVISION:	A			CLIENT:	Craton Mining and Exploration (Pty) Ltd
				FIGURE NO.:	A-2



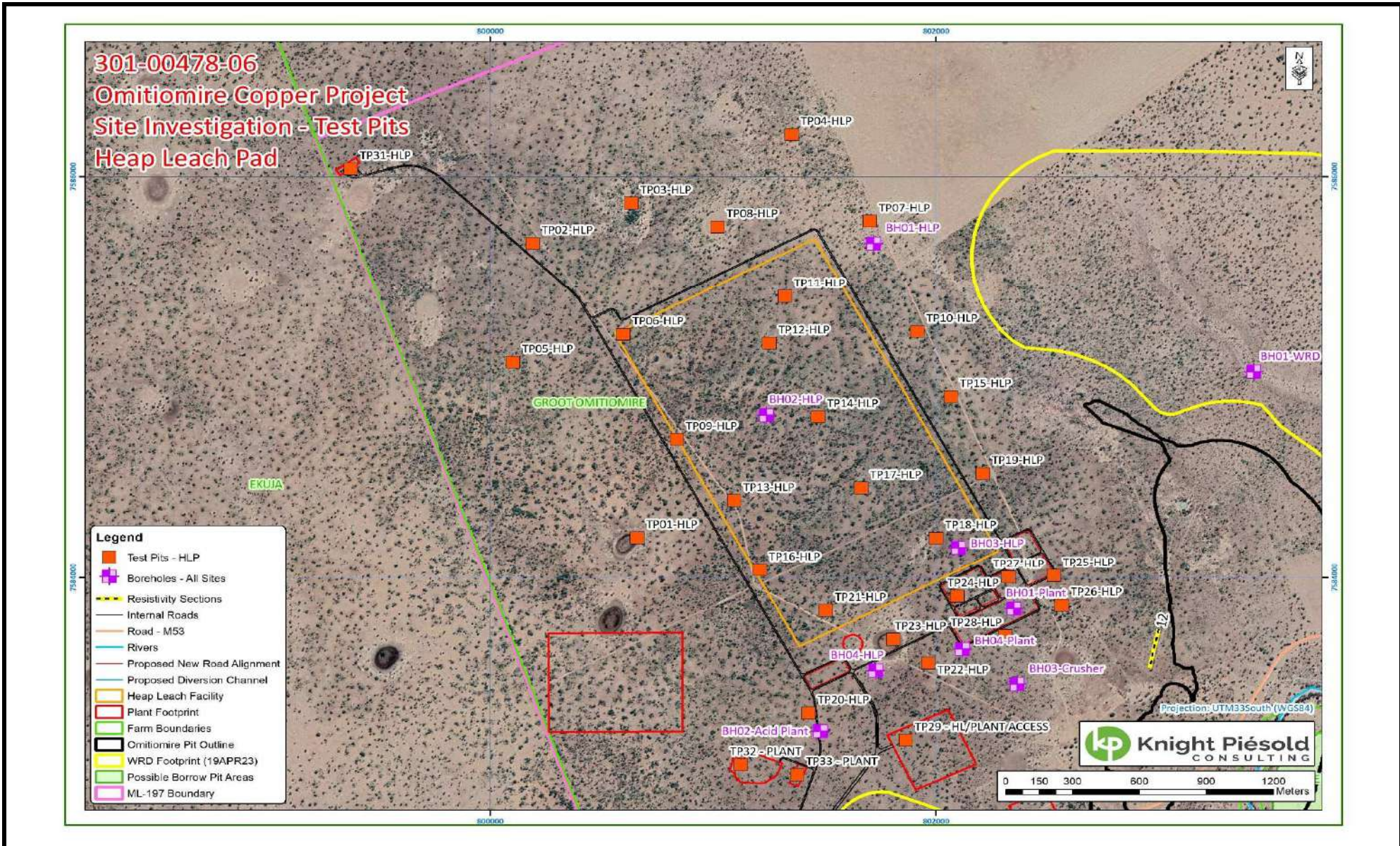
PROJECT NO.	WI 301-00478/06
BY:	T Oosthuizen
CHECKED:	B Klaas
DRAWING DATE:	30-Jun-23
REVISION:	A


Coordinate System: WGS 1984 UTM Zone 33S
 Projection: Transverse Mercator
 Datum: WGS 1984
 Units: Meter

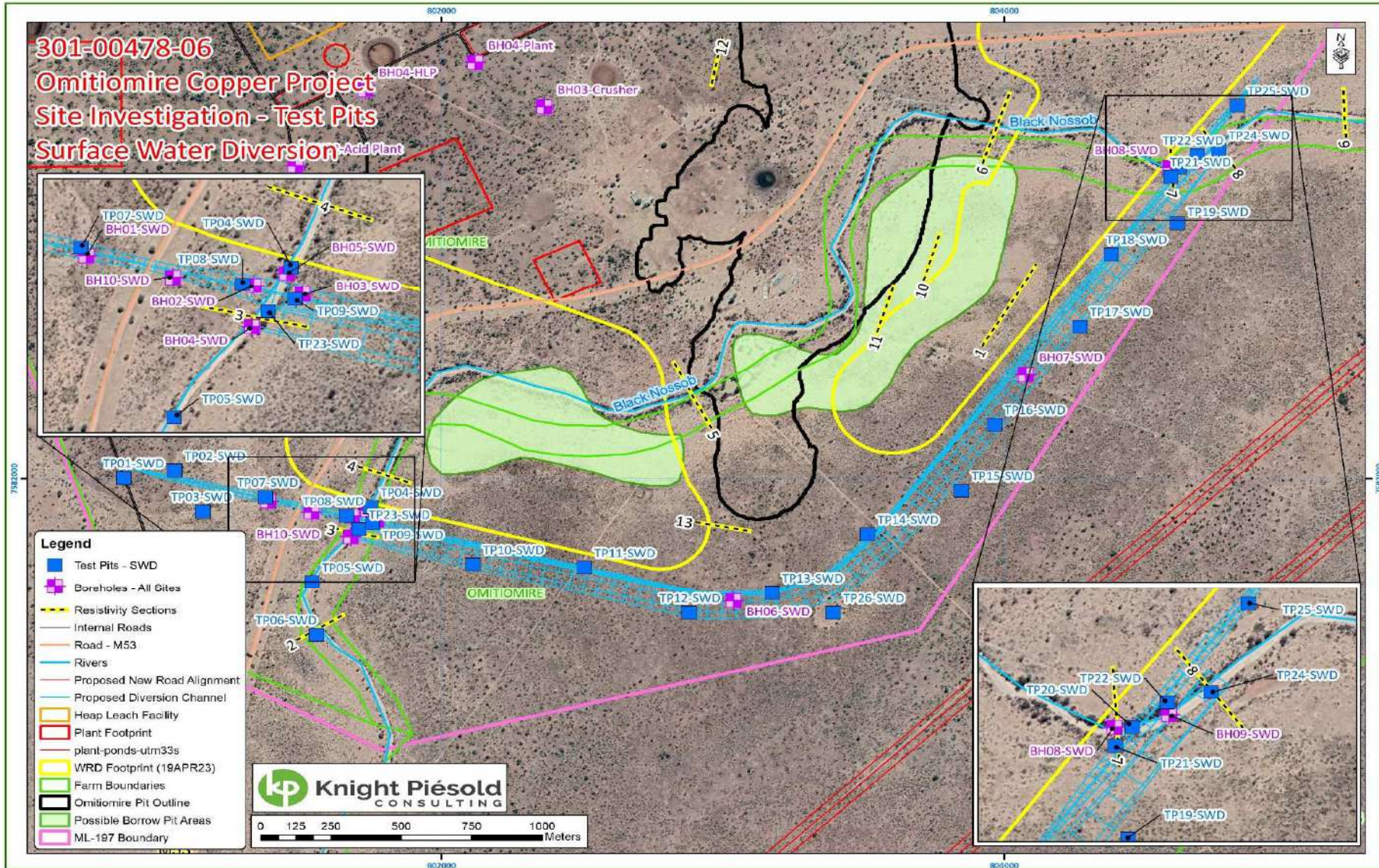
Omitiomire Copper Project:

Location of geotechnical boreholes - All sites

CLIENT: Craton Mining and Exploration (Pty) Ltd	FIGURE NO: A-3
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	PROJECT NO.	WI 301-00478/06	Coordinate System: WGS 1984 UTM Zone 33S Projection: Transverse Mercator Datum: WGS 1984 Units: Meter	Omitiomire Copper Project: Layout of geotechnical boreholes and test pits - Heap Leach Pad (HLP) and Plant Area	
	BY:	T Oosthuizen			
	CHECKED:	B Klaas		Craton Mining and Exploration (Pty) Ltd	A-4
	DRAWING DATE:	30-Jun-23			
REVISION:	A				



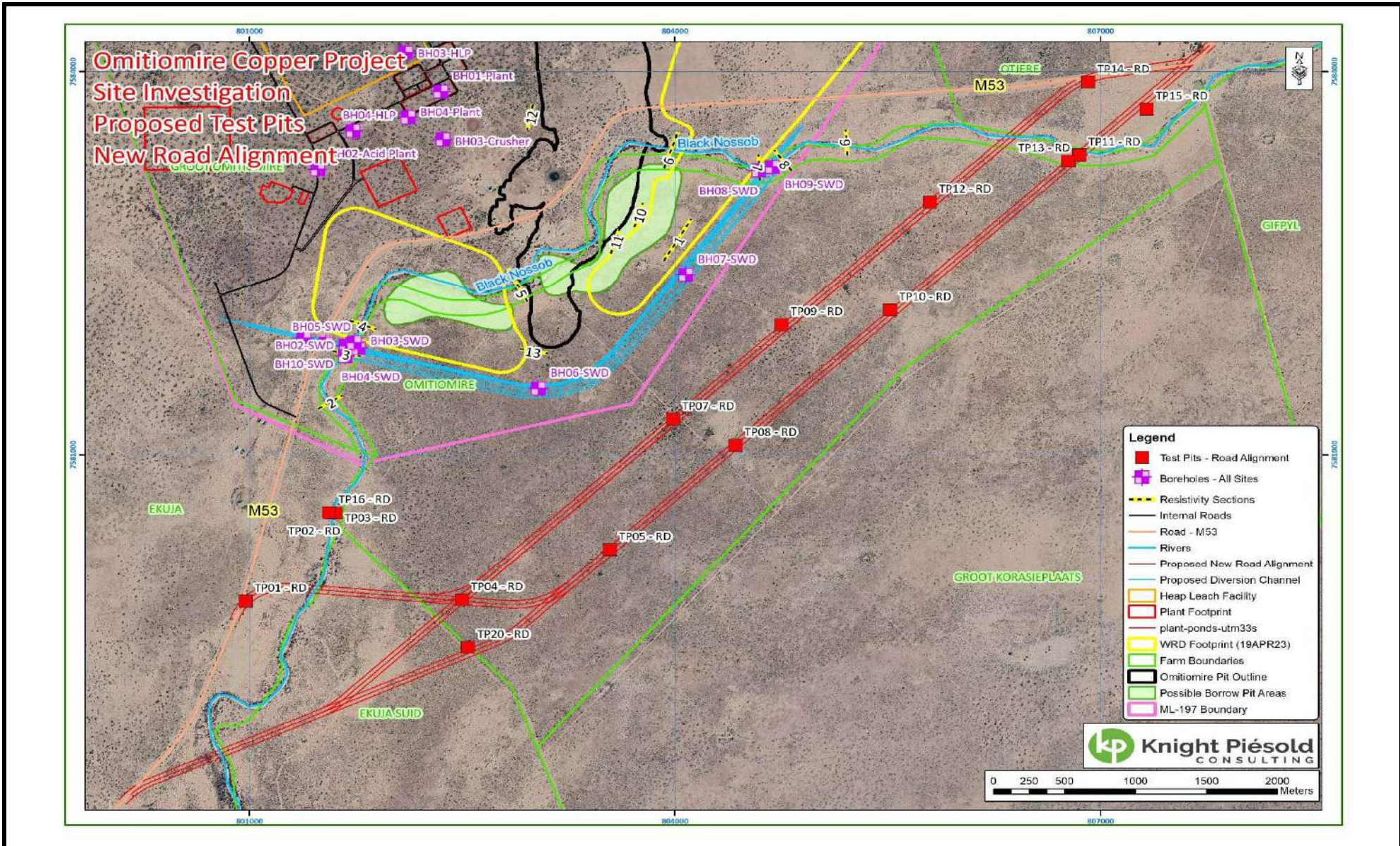
PROJECT NO.	WI 301-00478/06
BY:	T Oosthuizen
CHECKED:	B Klaas
DRAWING DATE:	30-Jun-23
REVISION:	A

Coordinate System: WGS 1984 UTM Zone 33S
 Projection: Transverse Mercator
 Datum: WGS 1984
 Units: Meter

Omitiomire Copper Project:
Layout of geotechnical boreholes and test pits -
Stormwater Diversion Canal (SWD)

CLIENT:
 Craton Mining and Exploration (Pty) Ltd

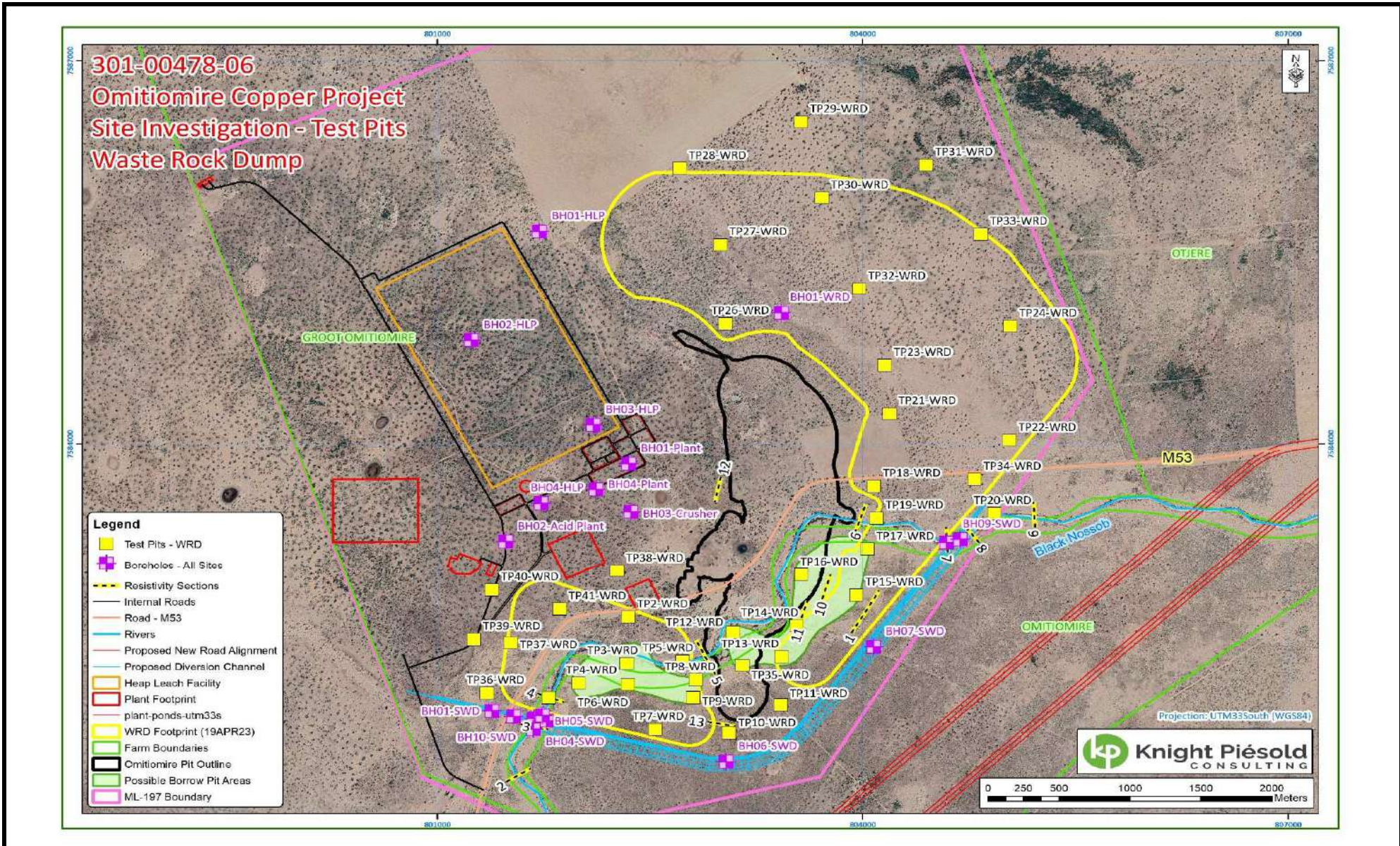
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 A-5




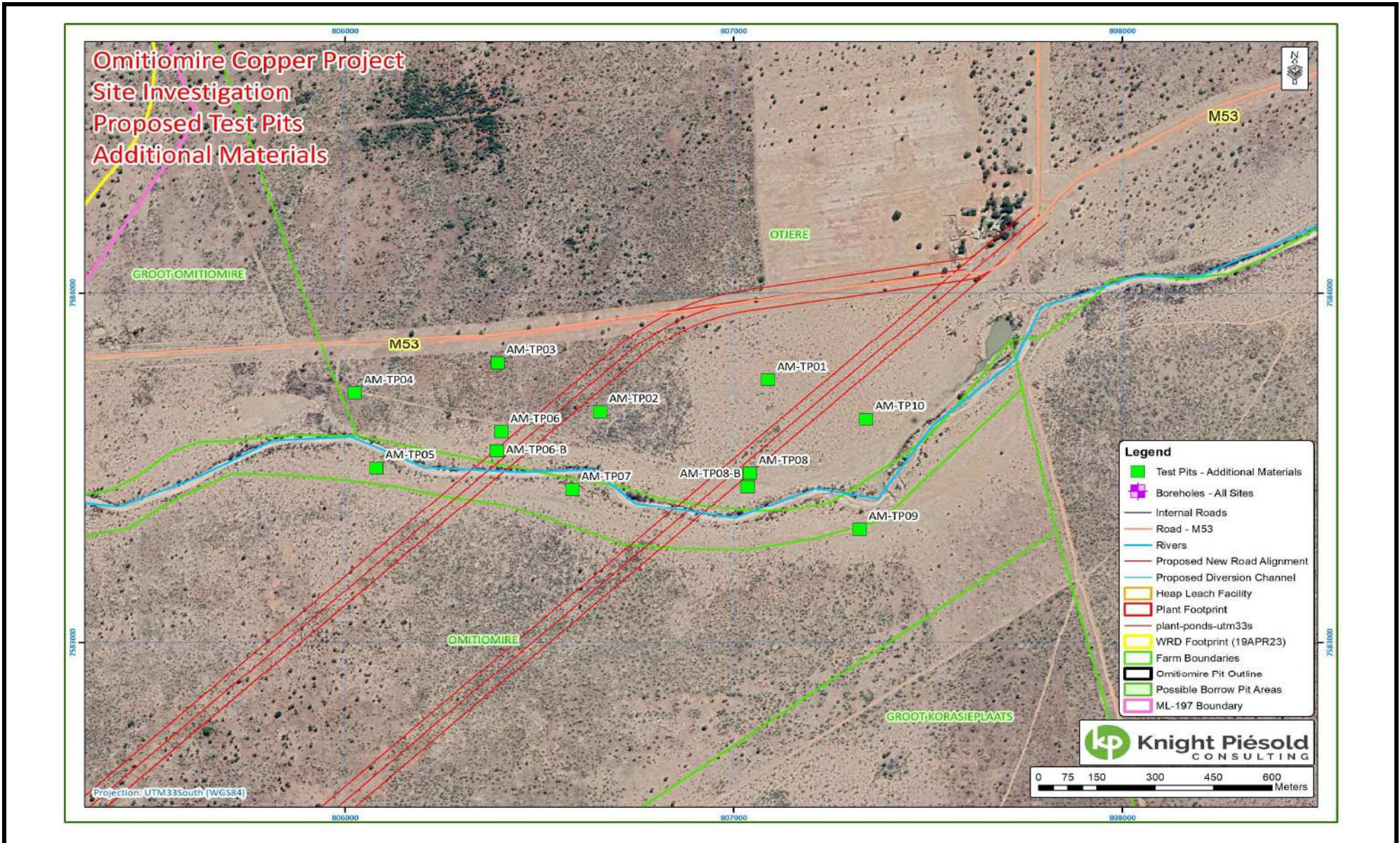
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BY:	T Oosthuizen
CHECKED:	B Klaas
DRAWING DATE:	30-Jun-23
REVISION:	A

Coordinate System: WGS 1984 UTM Zone 33S
 Projection: Transverse Mercator
 Datum: WGS 1984
 Units: Meter

Omitiomire Copper Project:	
Layout of geotechnical test pits - Road diversion	
CLIENT:	FIGURE NO:
Craton Mining and Exploration (Pty) Ltd	A-6



	PROJECT NO.	WI 301-00478/06	Coordinate System: WGS 1984 UTM Zone 33S Projection: Transverse Mercator Datum: WGS 1984 Units: Meter	Omitiomire Copper Project: Layout of geotechnical boreholes and test pits - Waste Rock Dump (WRD)	
	BY:	T Oosthuizen			
	CHECKED:	B Klaas		Craton Mining and Exploration (Pty) Ltd	A-7
	DRAWING DATE:	30-Jun-23			
REVISION:	A				



PROJECT NO.	WI 301-00478/06
BY:	T Oosthuizen
CHECKED:	B Klaas
DRAWING DATE:	30-Jun-23
REVISION:	A

Coordinate System: WGS 1984 UTM Zone 33S
 Projection: Transverse Mercator
 Datum: WGS 1984
 Units: Meter

Omitiomire Copper Project: Layout of geotechnical test pits - Additional Materials (AM)	
CLIENT: Craton Mining and Exploration (Pty) Ltd	FIGURE NO: A-8

APPENDIX B

Summary Tables for Soil Profiles, Borehole Logs, SPT Test Results and Laboratory Test Results

Appendix B1

Summary Table of Test Pit Logs

Appendix B2

Summary Table of Rotary Core Boreholes

Appendix B3

Summary Table of SPT Test Results

Appendix B4

Summary Table of Laboratory Test Results

APPENDIX B1

Summary of Test Pits

APPENDIX B1.2

CRATON MINING AND EXPLORATION (PTY) LTD
OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
SUMMARY OF STORMWATER DIVERSION CHANNEL TEST PIT PROFILES

STRUCTURE ID.	TEST PIT No.	AS BUILT COORDINATES		TOTAL DEPTH (m)	THICKNESS OF LAYER									GROUNDWATER LEVEL	
		UTM 33K			(m)										
		X-COORD	Y-COORD		Transported Soil		Pedogenic Soil			Residual Soil	Gneiss Bedrock				
					Fine Alluvium	Coarse Alluvium	Ferruginous/ *Ferruginised Alluvium	Calcareous/ *Calcified Alluvium	Nodular Ferricrete/ * Nodular Calcrete/ **Honeycomb Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock		
Stormwater Diversion Channel (SWD) Inlet	TP01-SWD	800870.00	7582003.00	0.60 + R	-	0 - 0.30	-	-	-	-	-	-	0.30 - 0.60	0.60 + R	-
	TP02-SWD	801050.00	7582033.00	1.50 + R	0 - 0.70	-	-	*0.70 - 1.50	***1.50 + R	-	-	-	-	-	-
	TP03-SWD	801151.00	7581871.00	1.80 + R	0 - 0.60	-	-	0.60 - 1.80 + R	-	-	-	-	-	-	-
	TP04-SWD	801747.00	7581888.00	3.90 + R	0 - 3.90	-	-	-	-	3.90 + R	-	-	-	-	1.50
	TP05-SWD	801539.00	7581592.00	3.80 + R	0 - 2.00	2.00 - 3.80	-	-	-	-	-	-	-	3.80 + R	1.70
	TP06-SWD	801555.00	7581382.00	1.95 + R	0 - 1.60	-	*1.60 - 1.95 + R	-	-	-	-	-	-	-	-
	TP07-SWD	801373.00	7581928.00	1.90 + R	0 - 0.40	0.40 - 1.40	-	-	-	-	1.40 - 1.90	1.9 + R	-	-	-
	TP08-SWD	801661.00	7581854.00	1.40 + R	0 - 0.70	-	*0.70 - 1.40 + R	-	-	-	-	-	-	-	-
	TP09-SWD	801754.00	7581825.00	1.20 + R	0 - 1.20 + R	-	-	-	-	-	-	-	-	-	-
TP23-SWD	801706.00	7581801.00	2.50 + R	0 - 2.50	-	-	-	-	-	-	-	-	2.50 + R	-	
Stormwater Diversion Channel (SWD) Channel	TP10-SWD	802111.00	7581662.00	3.20 + R	0 - 0.35	-	*0.35 - 1.20 *2.10 - 3.00	-	1.20 - 2.10 *3.00 - 3.20 + R	-	-	-	-	-	-
	TP11-SWD	802507.00	7581650.00	1.40 + R	0 - 0.30	-	*0.30 - 1.40 + R	-	-	-	-	-	-	-	-
	TP12-SWD	802880.00	7581470.00	4.70+	0 - 0.50	-	*0.50 - 3.00	*3.00 - 4.70+	-	-	-	-	-	-	-
	TP13-SWD	803174.00	7581550.00	4.40+	0 - 0.45	-	*0.45 - 1.60	*1.60 - 4.40+	-	-	-	-	-	-	-
	TP14-SWD	803514.00	7581781.00	1.35 + R	0 - 1.35	-	*1.35 + R	-	-	-	-	-	-	-	-
	TP15-SWD	803848.00	7581953.00	2.50 + R	0 - 0.50	-	0.50 - 2.50 + R	-	-	-	-	-	-	-	-
	TP16-SWD	803965.00	7582214.00	2.80 + R	-	2.10 - 2.80 + R	0 - 2.10	-	-	-	-	-	-	-	-
	TP17-SWD	804269.00	7582600.00	2.50 + R	0 - 1.30	-	*1.30 - 2.50 + R	-	-	-	-	-	-	-	-
	TP18-SWD	804380.00	7582887.00	0.90 + R	0 - 0.90	-	-	-	-	-	-	-	-	-	-
TP26-SWD	803392.00	7581470.00	4.20 + R	0 - 1.90	-	-	*1.90 - 2.90	-	-	2.90 - 4.20	4.20 + R	-	-	-	
Stormwater Diversion Channel (SWD) Outlet	TP19-SWD	804615.00	7583010.00	2.40 + R	0 - 1.00	-	1.00 - 1.40 *1.40 - 2.40 + R	-	-	-	-	-	-	-	-
	TP20-SWD	804622.00	7583230.00	4.80 + R	0 - 3.40	3.40 - 4.80	-	-	-	-	-	-	4.80 + R	-	
	TP21-SWD	804592.00	7583193.00	1.10 + R	0 - 0.30	-	*0.30 - 1.10 + R	-	-	-	-	-	-	-	
	TP22-SWD	804685.00	7583279.00	4.30 + R	0 - 1.40	-	*1.40 - 4.30 + R	-	-	-	-	-	-	-	
	TP24-SWD	804762.00	7583299.00	4.95 + R	0 - 4.90	-	-	-	-	-	-	4.90 - 4.95	4.95 + R	-	
	TP25-SWD	804829.00	7583474.00	2.40 + R	0 - 0.65	-	-	-	*0.65 - 0.95	0.95 - 1.50	1.50 - 2.40	-	2.40 + R	-	

NOTES:

1. R = REFUSAL.
2. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.

A	2023/05/31	ISSUED WITH LETTER	SY	BK
REV	DATE	DESCRIPTION	PREPD	REV'D

APPENDIX B1.3

CRATON MINING AND EXPLORATION (PTY) LTD
OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
SUMMARY OF NEW ROAD TEST PIT PROFILES

STRUCTURE ID.	TEST PIT No.	AS BUILT COORDINATES		TOTAL DEPTH (m)	THICKNESS OF LAYER						GROUNDWATER LEVEL (m)
		UTM 33K			(m)						
		X-COORD	Y-COORD		Transported soils		Pedogenic soils		Bedrock		
					Fine Alluvium	Coarse Alluvium	Ferruginous/ *Calcareous Alluvium	Calcified Alluvium	Very Soft Rock	Soft Rock	
New Road	TP01 - RD	800971.00	7579851.00	0.90 + R	0 - 0.30	-	0.30 - 0.90 + R	-	-	-	-
	TP02 - RD	801584.00	7580544.00	4.30 + R	-	0 - 3.90	-	-	3.90 - 4.30 + R	-	2.30
	TP03 - RD	801605.00	7580544.00	1.70 + R	0 - 1.70 + R	-	-	-	-	-	-
	TP04 - RD	802497.00	7579862.00	2.90 + R	0 - 2.90 + R	-	-	-	-	-	-
	TP05 - RD	803541.00	7580258.00	3.70 + R	0 - 3.70 + R	-	-	-	-	-	-
	TP07 - RD	803985.00	7581279.00	2.80 + R	0 - 2.80 + R	-	-	-	-	-	-
	TP08 - RD	804425.00	7581074.00	3.20 + R	0 - 1.00	-	-	1.00 - 3.20 + R	-	-	-
	TP09 - RD	804750.00	7582018.00	3.40 + R	0 - 3.40 + R	-	-	-	-	-	-
	TP10 - RD	805515.00	7582133.00	2.80 + R	0 - 2.80 + R	-	-	-	-	-	-
	TP11 - RD	806850.00	7583349.00	1.40 + R	0 - 1.40 + R	-	-	-	-	-	-
	TP12 - RD	805797.00	7582981.00	4.30 + R	0 - 4.30 + R	-	-	-	-	-	-
	TP13 - RD	806772.00	7583302.00	4.50 +	0 - 1.80	2.60 - 4.00	*1.80 - 2.60	-	4.00 - 4.50 +	-	-
	TP14 - RD	806913.00	7583919.00	2.60 + R	0 - 0.30	0.30 - 1.40	-	-	1.40 - 2.60	2.60 + R	-
	TP15 - RD	807325.00	7583709.00	0.70 + R	0 - 0.30	-	0.30 - 0.70 + R	-	-	-	-
	TP16 - RD	801561.00	7580548.00	1.40 + R	0 - 1.40 + R	-	-	-	-	-	-
	TP20 - RD	802541.00	7579491.00	2.50 + R	0 - 2.50 + R	-	-	-	-	-	-

NOTES:

1. R = REFUSAL.
2. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.
3. TP06-RD, TP17-RD TP18-RD, TP19-RD WERE NOT EXCAVATED DUE TO ACCESS CONSTRAINTS.

A REV	2023/05/31 DATE	ISSUED WITH LETTER	DESCRIPTION	SY PREP'D	BK REV'D
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APPENDIX B1.4
 CRATON MINING AND EXPLORATION (PTY) LTD
 OMITIOMIRE COPPER PROJECT
 OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
 SUMMARY OF WASTE ROCK DUMP TEST PIT PROFILES

STRUCTURE ID.	TEST PIT No.	AS BUILT COORDINATES		TOTAL DEPTH (M)	THICKNESS OF LAYER											GROUNDWATER LEVEL (m)	
		UTM 33K			(m)												
		X-COORD	Y-COORD		Transported		Pedogenic				Residual Soil	Bedrock					
					Fine Alluvium	Coarse Alluvium	Ferruginous/ "Ferruginised Alluvium"	Calcified Alluvium	Nodular Calcrete	Honeycomb Calcrete/ Ferricrete	Hardpan Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock		
Waste Rock Dump (WRD)	TP1-WRD	801782.00	7582015.00	3.50 + R	0 - 3.50	-	-	-	-	-	-	-	3.50 + R	-	-	-	1.40
	TP2-WRD	802343.00	7582645.00	1.80 + R	0 - 1.20	1.20 - 1.40	-	-	-	-	-	-	1.40 - 1.60	1.60 - 1.80	1.80 + R	-	-
	TP3-WRD	802332.80	7582281.00	3.30 + R	0 - 0.30	-	-	0.80 - 3.30 + R	-	-	-	-	-	-	-	-	-
	TP4-WRD	801993.39	7582129.46	2.40 + R	0 - 0.40 & 1.50 - 2.40 + R	-	-	0.40 - 1.50	-	-	-	-	-	-	-	-	-
	TP5-WRD	802728.00	7582301.00	3.60 + R	0 - 0.90	-	0.90 - 2.10	2.10 - 3.60 + R	-	-	-	-	-	-	-	-	-
	TP6-WRD	802341.11	7582119.35	4.30 + R	-	0 - 0.45	*0.45 - 2.35	3.10 - 4.30	2.35 - 3.10	-	-	-	-	-	-	4.30 + R	-
	TP7-WRD	802532.00	7581769.00	0.80 + R	0 - 0.80 + R	-	-	-	-	-	-	-	-	-	-	-	-
	TP8-WRD	802820.40	7582154.58	1.70 + R	0 - 0.60	-	-	0.60 - 1.70 + R	-	-	-	-	-	-	-	-	-
	TP9-WRD	802802.00	7582018.00	1.80 + R	0 - 0.80	0.80 - 1.80 + R	-	-	-	-	-	-	-	-	-	-	-
	TP10-WRD	803055.00	7581743.00	4.40 +	0 - 4.00	-	-	4.00 - 4.40 +	-	-	-	-	-	-	-	-	-
	TP11-WRD	803420.00	7581961.00	3.70 + R	0 - 0.30	-	*0.30 - 1.50	2.70 - 3.70 + R	1.50 - 2.70	-	-	-	-	-	-	-	-
	TP12-WRD	803082.00	7582525.00	1.80 + R	0 - 1.00	1.00 - 1.20	-	-	-	-	-	-	1.20 - 1.30	1.30 - 1.80	-	1.80 + R	-
	TP13-WRD	803424.70	7582333.22	3.40 + R	0 - 1.10	1.40 - 3.10	-	1.10 - 1.40	-	-	-	-	-	3.10 - 3.40	3.40 + R	-	-
	TP14-WRD	803530.99	7582575.57	4.90 +	-	2.80 - 4.90 +	0.00 - 0.80 *0.80 - 2.80	-	-	-	-	-	-	-	-	-	-
	TP15-WRD	803963.75	7582815.05	3.60 + R	0 - 0.70	-	-	0.70 - 3.60 + R	-	-	-	-	-	-	-	-	-
	TP16-WRD	803563.99	7582976.05	5.00 +	0 - 0.50	-	-	1.80 - 5.00 +	0.50 - 1.80	-	-	-	-	-	-	-	-
	TP17-WRD	804031.00	7583179.00	4.10 + R	0 - 0.90	3.60 - 4.10 + R	-	-	0.90 - 3.60	-	-	-	-	-	-	-	-
	TP18-WRD	804076.74	7583671.36	2.20 + R	0 - 1.60	-	-	-	1.60 - 2.20	-	2.20 + R	-	-	-	-	-	-
	TP19-WRD	804095.95	7583420.70	1.40 + R	0 - 0.30	-	*0.30 - 1.40 + R	-	-	-	-	-	-	-	-	-	-
	TP20-WRD	804927.00	7583457.00	2.70 + R	0 - 0.50	-	-	-	-	-	-	-	0.50 - 2.70	-	-	2.70 + R	-
	TP21-WRD	804187.03	7584237.04	3.10 + R	0 - 0.90	-	-	-	1.10 - 3.00	-	-	-	-	0.90 - 1.10	-	3.00 - 3.10 + R	-
	TP22-WRD	805033.00	7584029.00	1.20 + R	0 - 1.20	-	-	-	-	-	-	-	-	-	-	-	-
	TP23-WRD	804153.99	7584615.84	1.50 + R	0 - 0.85	-	-	-	-	-	0.85 - 1.50 + R	-	-	-	-	-	-
	TP24-WRD	805039.00	7584921.00	1.30 + R	0 - 1.00	1.00 - 1.30	*1.30 + R	-	-	-	-	-	-	-	-	-	-
	TP26-WRD	803030.00	7584941.00	2.60 + R	0 - 1.00	1.00 - 1.20	-	-	-	-	1.20 - 2.60 + R	-	-	-	-	-	-
	TP27-WRD	802994.92	7585563.53	0.25 + R	0 - 0.25	-	-	-	-	-	0.25 + R	-	-	-	-	-	-
	TP28-WRD	802706.77	7586159.95	1.20 + R	0 - 0.45	-	-	-	0.45 - 1.20	1.20 + R	-	-	-	-	-	-	-
	TP29-WRD	803562.23	7586522.25	2.90 + R	0 - 1.25	-	-	-	1.25 - 1.60	-	-	-	1.60 - 2.07	2.07 - 2.90	-	2.90 + R	-
	TP30-WRD	803710.59	7585929.08	1.55 + R	0 - 1.25	1.25 - 1.40	-	-	-	-	-	-	-	-	1.40 - 1.55	1.55 + R	-
	TP31-WRD	804445.00	7586187.00	1.40 + R	0 - 0.60	0.60 - 1.20	-	-	-	-	*1.20 - 1.40 + R	-	-	-	-	-	-
	TP32-WRD	803973.00	7585217.00	1.45 + R	0 - 1.15	-	-	-	-	-	1.15 - 1.45 + R	-	-	-	-	-	-
	TP33-WRD	804832.00	7585645.00	1.30 + R	0 - 1.10	1.10 - 1.20	-	-	-	-	-	-	1.20 - 1.30 + R	-	-	-	-
	TP34-WRD	804786.59	7583725.60	3.55 + R	0 - 1.00	-	-	-	1.00 - 2.35	2.35 - 3.55 + R	-	-	-	-	-	-	-
	TP35-WRD	803149.00	7582266.00	4.10 +	0 - 0.90	1.30 - 2.50	2.50 - 4.10 +	0.90 - 1.30	-	-	-	-	-	-	-	-	-
	TP36-WRD	801345.00	7582051.00	1.60 + R	0 - 0.60	-	*0.60 - 1.60 + R	-	-	-	-	-	-	-	-	-	-
	TP37-WRD	801515.00	7582442.00	2.70 + R	0 - 0.90	0.90 - 1.10	-	-	-	-	-	-	1.10 - 2.40	-	2.40 - 2.70	2.70 + R	-
	TP38-WRD	802264.00	7583006.00	1.30 + R	0 - 1.00	1.00 - 1.20	-	-	-	-	-	-	1.20 - 1.30	-	1.30 + R	-	-
	TP39-WRD	801254.00	7582470.00	1.50 + R	0 - 0.60	-	*0.60 - 1.40	-	-	-	-	-	1.40 - 1.50 + R	-	-	-	-
	TP40-WRD	801382.00	7582856.00	2.10 + R	0 - 1.40	1.40 - 1.60	-	-	-	-	-	-	1.60 - 1.90	-	-	1.90 - 2.10 + R	-
	TP41-WRD	801860.00	7582706.00	1.20 + R	0 - 0.70	0.70 - 1.20 + R	-	-	-	-	-	-	-	-	-	-	-

NOTES:
 1. R = REFUSAL.
 2. + = MATERIAL CONTINUES AS ABOVE.
 3. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.
 4. TP25-WRD WAS NOT EXCAVATED DUE TO ACCESS CONSTRAINTS.

A	REV	DATE	ISSUED WITH LETTER	DESCRIPTION	BY	CHK
					PREP	REV'D

APPENDIX B1.5

CRATON MINING AND EXPLORATION (PTY) LTD
OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
SUMMARY OF ADDITIONAL MATERIAL SOURCES TEST PIT PROFILES

STRUCTURE ID.	TEST PIT No.	AS BUILT COORDINATES		TOTAL DEPTH (M)	THICKNESS OF LAYER							GROUNDWATER LEVEL (M)
		UTM 33K			(m)							
		X-COORD	Y-COORD		Transported		Pedogenic			Bedrock		
					Fine Alluvium	Coarse Alluvium	Ferruginous Alluvium	Calcareous Alluvium	Calcified Alluvium	Very Soft Rock	Soft Rock	
Additional Materials (AM)	AM-TP01	807088.00	7583751.00	0.60 + R	0 - 0.40	-	0.40 -0.60 + R	-	-	-	-	-
	AM-TP02	806656.00	7583660.00	1.10 + R	0 - 0.70	0.70 - 0.90 + R	-	-	-	0.90 - 1.10	1.10 + R	-
	AM-TP03	806392.00	7583799.00	1.00 + R	0 - 0.70	0.70 - 1.00 + R	1.00 + R	-	-	-	-	-
	AM-TP04	806025.00	7583714.00	1.50 + R	-	0.00 - 0.30	-	0.30 - 1.10	1.10 - 1.20 + R	1.20 - 1.50	1.50 + R	-
	AM-TP05	806080.00	7583500.00	1.20 + R	0 - 1.20	-	-	-	-	-	-	-
	AM-TP06	806402.00	7583603.00	1.00 + R	0 - 1.00 + R	-	-	-	-	-	-	-
	AM-TP06-B	806390.00	7583549.00	1.00 + R	0 - 1.00 + R	-	-	-	-	-	-	-
	AM-TP07	806585.00	7583438.00	1.10 + R	0 - 1.10 + R	-	-	-	-	-	-	-
	AM-TP08	807041.00	7583485.00	0.60 + R	0 - 0.60 + R	-	-	-	-	-	-	-
	AM-TP08-B	807035.00	7583446.00	0.70 + R	0 - 0.70 + R	-	-	-	-	-	-	-
	AM-TP09	807324.00	7583323.00	1.10 + R	0 - 0.70	-	0.70 - 1.10 + R	-	-	-	-	-
AM-TP10	807340.00	7583639.00	0.80 + R	0 - 0.80 + R	-	-	-	-	-	-	-	

NOTES:

1. R = REFUSAL.
2. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.

A	2023/05/31	ISSUED WITH LETTER		SY	BK
REV	DATE	DESCRIPTION		PREP'D	REV'D

APPENDIX B2

Summary of Boreholes

APPENDIX B2

CRATON MINING AND EXPLORATION (PTY) LTD
OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
SUMMARY OF BOREHOLE LOGS

STRUCTURE ID.	Borehole No.	AS BUILT COORDINATES		PLANNED DRILL DEPTH (M)	ACTUAL DRILL DEPTH (M)	THICKNESS OF LAYERS										GROUNDWATER LEVEL (*PERCHED) (m)
		UTM 33S				(m)										
		X-COORD	Y-COORD			Transported Soils		Pedogenic Soils		Residual Soil	Bedrock Gneiss (*with Pegmatite Zones)					
						Fine Alluvium	Coarse Alluvium	Calcareous/*Calcified/** Powdery calcrete Alluvium	Hardpan Calcrete		Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock	Hard Rock	
Stormwater Diversion Channel (SWD) Inlet	BH01-SWD	801384.00	7581911.00	20.00	24.27	0 - 1.85	-	-	-	-	1.85 - 3.0 5.50 - 8.20	11.25 - 14.85	3.00 - 5.50 8.2 - 11.25	14.85 - 24.27+	-	7.4
	BH02-SWD	801681.00	7581852.00	20.00	25.11	0 - 1.05	1.05 - 2.08	-	-	2.08 - 2.34 10.34 - 12.40	-	2.34 - 10.34 12.40 - 18.20	-	18.20 - 19.35	19.35 - 25.11+	-
	BH03-SWD	801768.00	7581835.00	20.00	20.23	0 - 1.33	-	-	-	-	7.83 - 9.35	1.33 - 2.20	2.2-7.83	9.35 - 12.30	12.30 - 20.23+	-
	BH04-SWD	801678.00	7581770.00	20.00	21.03	0 - 2.20	-	-	-	2.20 - 2.95 4.52 - 6.60	-	2.95 - 4.52 6.60 - 10.80	10.80 - 15.50	15.50 - 21.03+	-	14.60
	BH05-SWD	801740.00	7581876.00	20.00	20.01	0 - 3.95	-	-	-	3.95 - 5.35	5.35 - 6.30	-	6.30 - 20.01+	-	-	8.05
	BH10-SWD	801538.00	7581868.00	10.00	10.44	0 - 0.80	0.80 - 2.00	-	-	2.00 - 3.30	-	-	3.30 - 10.44+	-	-	-
Stormwater Diversion Channel (SWD) Channel	BH06-SWD	803037.82	7581516.63	15.00	15.26	0 - 3.00	3.00 - 7.60	-	-	-	-	7.60 - 15.26+	-	-	-	-
	BH07-SWD	804077.00	7582410.00	15.00	15.22	0 - 1.10	1.10 - 3.00	-	-	-	-	-	3.00 - 15.22+	-	-	-
Stormwater Diversion Channel (SWD) Outlet	BH08-SWD	804591.00	7583229.00	20.00	20.23	0 - 4.60	-	-	-	-	-	4.60 - 7.97	7.97 - 13.70	*13.70 - 20.23+	-	-
	BH09-SWD	804688.00	7583254.00	20.00	20.23	0 - 4.70	-	-	-	4.70 - 5.17	5.17 - 7.80	7.80 - 11.76	11.76 - 12.80	12.80 - 20.23+	-	-
Waste Rock Dump (WRD)	BH01-WRD	803426.00	7585025.00	25.00	20.28	0 - 0.19	-	**0.19 - 0.84 1.50 - 3.54	0.84 - 1.50	3.54 - 4.80 5.47 - 7.75	4.80 - 7.75	-	7.75 - 14.35	14.35 - 20.28+	-	-
Processing Plant	BH01-Plant	802349.95	7583844.14	25.00	25.05	0 - 1.75	-	*1.75 - 2.00	-	-	2.00 - 2.5	9.14 - 10.97 20.67 - 25.05+	2.5 - 9.14 10.97 - 20.67	-	-	-
	BH02-Acid Plant	801481.43	7583235.41	25.00	25.16	0 - 1.02	-	-	-	1.02 - 1.52 2.60 - 3.90	1.52 - 2.60	3.90 - 5.25	5.25 - 11.95 22.92 - 25.16+	11.95 - 22.92	-	-
	BH03-Crusher	802365.16	7583468.49	25.00	25.00	0 - 1.27	1.27 - 1.78	-	-	-	14.90 - 16.33	1.78 - 6.35	6.35 - 12.60	12.60 - 14.90 16.33 - 25.00+	-	*5.7
	BH04-Plant	802119.00	7583643.00	10.00	10.09	0 - 1.70	-	-	-	-	-	-	1.70 - 5.10 6.30 - 10.09+	5.10 - 6.30	-	-
Heap Leach Pad	BH01-HLP	801721.00	7585666.00	20.00	20.07	0 - 0.69	-	0.69 - 3.00	3.00 - 4.03	4.03 - 6.18	-	*6.18 - 18.30	18.30 - 20.07+	-	-	*7.4
	BH02-HLP	801240.00	7584810.00	20.00	20.08	0 - 3.00	-	-	-	3.00 - 3.20	3.20 - 7.90	7.90 - 11.65	11.65 - 20.08+	-	-	-
	BH03-HLP	802101.00	7584148.00	20.00	20.13	0 - 1.30	-	1.30 - 1.95	-	-	-	1.95 - 8.20	8.20 - 20.13+	-	-	6.30
	BH04-HLP	801731.00	7583536.00	20.00	20.23	0 - 1.05	-	1.05 - 1.70	-	1.70 - 2.00	-	2.00 - 4.51	4.51 - 20.23+	-	-	-

NOTES:

1. + = PROFILE CONTINUES AS ABOVE
2. ALL DEPTH MEASUREMENTS ARE TAKEN WITH RESPECT TO GROUND SURFACE LEVEL.
3. BOREHOLE COORDINATES INDICATED IN WGS 84 UTM 33S.

A	5/31/2023	ISSUED WITH LETTER	SY	BK
REV	DATE	DESCRIPTION	PREP'D	REVD

APPENDIX B3

Summary of SPT Results

APPENDIX B3

CRATON MINING AND EXPLORATION (PTY) LTD
OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION
SUMMARY OF STANDARD PENETRATION TEST RESULTS

Print Sep/18/23 7:26:52

BH	DEPTH	BLOWS PER 75MM PENETRATION						N-VALUE	DRILLERS MATERIAL DESCRIPTION
SWD-BH01	1.80	10	13	11	11	8	R	R	Brown fine sand/pebbles
SWD-BH02	1.87	11	11	11	17	50	R	R	Light brown fine sand/pebbles
SWD-BH04	1.95	1	1	2	2	2	3	9	Light brown fine sand
SWD-BH05	1.95	1	1	1	1	2	3	7	Light brown sand and clay
	3.45	4	3	2	2	2	2	8	Light brown sand and clay
	3.65	2	1	1	50	R		R	Clay and pebbles
SWD-BH06	1.95	4	7	7	9	10	11	37	Red sand
	3.19	8	40	45	50	R		R	Red sand with pebbles
SWD-BH08	1.95	2	3	2	3	3	5	13	Dark brown sand and clay
	3.45	1	2	2	2	3	3	10	Dark brown sand and clay
SWD-BH09	1.95	1	1	1	2	2	3	8	Dark brown sand
	3.79	1	2	3	3	4	3	13	Dark brown sand
SWD-BH10	1.95	5	10	15	16	21	28	>50	Yellow sand
	3.15	18	29	50	R			R	Light brown sand and clay
HLP-BH02	0.45	1	1	1	2	2	2	7	Dark brown sand
	1.95	2	2	1	2	3	4	10	Dark brown sand
HLP-BH03	0.45	1/3	1/3	1/3	1	1	2	4	Red dark brown sand
	1.65	15	22	50	R			R	Red dark brown sand
HLP-BH04	0.45	1	2	4	5	5	6	20	Dark brown sand
	1.88	4	5	4	10	20	R	R	Dark brown sand and pebbles
PLANT-BH01	0.45	1	1	2	2	5	5	14	Red sand
	1.75	9	12	16	20	50	R	R	Clay and weathered rock
PLANT-BH02	1.52	25	50	R				R	Weathered soft rock
PLANT-BH03	0.45	1	1	1	2	2	3	8	Red sand
	1.78	7	16	16	30	50	R	R	Red sand and pebbles
PLANT-BH04	1.70	12	19	16	50	R		R	Light brown sand and pebbles

NOTES:

1. R = REFUSAL

A	2022/04/21	ISSUED WITH LETTER	SSYF	BK
REV	DATE	DESCRIPTION	PREPD	REVD

APPENDIX B4

Summary Laboratory Test Results

APPENDIX B4

CRATON MINING AND EXPLORATION (PTY) LTD

HEAP LEACH PAD AND PONDS FEASIBILITY DESIGN
 GEOTECHNICAL INVESTIGATION FOR HEAP LEACH PAD, WASTE ROCK DUMP, STORMWATER DIVERSION & PLANT
 SUMMARY OF ROCK LABORATORY TEST RESULTS

STRUCTURE ID.	SAMPLE		Rock Description	Uniaxial Compressive Strength (UCS)		Point Load Index Test				
	Borehole No.	Depth (m)		UCS (MPa)	Failure Type	Type	Orientation	Failure Load P	I _{s(50)} (MPa)	Estimated UCS (MPa)
Stormwater Diversion Channel (SWD)	SWD-BH01	2.04 - 2.26	Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	8.0	normal	-	-	-	-	-
		2.39 - 2.51	Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	-	-	a	p	0.97	0.48	12.01
		3.14 - 3.48	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	23.5	normal	d	p	1.09	0.50	12.51
		3.36 - 3.95	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	-	-	a	p	1.14	1.03	25.53
	SWD-BH02	2.84 - 2.98	Highly to moderately weathered, brown to grey speckled black banded white, soft to medium hard rock, gneiss	-	-	d	p	1.09	0.84	20.79
		7.96 - 8.16	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	33.5	normal	a	p	1.14	1.57	39.02
		8.16 - 8.35	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	-	-	d	p	1.09	1.08	26.83
		8.16 - 8.35	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	-	-	a	p	1.04	0.92	22.87
	SWD-BH03	2.84 - 2.98	Highly to moderately weathered, brown to grey speckled black banded white, soft to medium hard rock, gneiss	-	-	d	p	1.08	0.47	11.59
		7.96 - 8.16	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	33.5	normal	-	-	-	-	-
	SWD-BH04	3.82 - 4.13	Moderately weathered, grey speckled black and white, soft to medium hard rock, gneiss	12.0	normal	a	p	1.05	1.15	28.67
		3.82 - 4.13	Moderately weathered, grey speckled black and white, soft to medium hard rock, gneiss	12.0	normal	d	p	1.09	1.01	24.87
	SWD-BH05	3.71 - 3.88	Moderately weathered, grey speckled black streaked white, soft to medium hard rock, gneiss	-	-	a	p	1.04	0.88	21.86
		8.23 - 8.39	Moderately weathered, grey speckled black streaked white, soft to medium hard rock, gneiss	-	-	d	p	1.09	0.48	11.92
		5.88 - 6.02	Highly to completely weathered, dark grey to dark brown speckled black, very soft rock, gneiss	-	-	a	p	1.13	4.91	120.83
		9.69 - 9.93	Highly to moderately weathered, grey to brown speckled black streaked white, medium hard rock, gneiss	25.5	normal	d	p	1.08	0.18	4.55
	SWD-BH08	5.88 - 6.02	Highly to completely weathered, dark grey to dark brown speckled black, very soft rock, gneiss	-	-	a	p	1.06	4.07	100.76
		9.69 - 9.93	Highly to moderately weathered, grey to brown speckled black streaked white, medium hard rock, gneiss	25.5	normal	d	p	1.09	1.64	40.62
	SWD-BH08	5.07 - 5.21	Highly weathered, grey to brown speckled black streaked white, soft rock, gneiss	-	-	a	p	1.07	0.49	12.23
		5.07 - 5.21	Highly weathered, grey to brown speckled black streaked white, soft rock, gneiss	-	-	d	p	1.09	0.10	2.58
Heap Leach Pad (HLP)	HLP-BH01	6.18 - 6.31	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	a	p	0.96	0.39	9.67
		6.18 - 6.31	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	d	p	1.09	0.15	3.67
	HLP-BH02	8.35 - 8.60	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	a	p	1.04	0.62	15.31
		8.35 - 8.60	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	d	p	1.08	0.49	12.05
	HLP-BH03	5.35 - 5.65	Highly to completely weathered, brown to grey, banded white, very soft rock gneiss	8.0	normal	a	p	1.02	0.35	8.73
		5.35 - 5.65	Highly to completely weathered, brown to grey, banded white, very soft rock gneiss	8.0	normal	d	p	1.09	0.19	4.80
	HLP-BH04	2.27 - 2.41	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	a	p	0.95	0.19	4.72
		2.27 - 2.41	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	d	p	1.09	0.11	2.61
HLP-BH04	3.73 - 3.93	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	a	p	1.11	0.41	10.12	
	3.73 - 3.93	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	d	p	1.09	0.3	7.43	
Plant Area (PLT)	PLT-BH01	2.66 - 2.86	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	7.0	normal	-	-	-	-	-
		2.66 - 2.86	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	7.0	normal	a	p	1.04	0.68	16.97
	PLT-BH02	5.19 - 5.35	Moderately weathered, grey speckled black streaked white, medium hard rock, gneiss	-	-	d	p	1.09	0.31	7.75
		5.19 - 5.35	Moderately weathered, grey speckled black streaked white, medium hard rock, gneiss	-	-	a	p	1.04	0.68	16.97
	PLT-BH03	2.80 - 9.00	Slightly weathered, grey speckled black streaked white, medium hard rock, granitic gneiss	-	-	a	p	1.14	1.67	41.27
		2.80 - 9.00	Slightly weathered, grey speckled black streaked white, medium hard rock, granitic gneiss	-	-	d	p	1.09	1.58	39.13
PLT-BH03	4.73 - 4.94	Highly weathered, brown to grey, soft rock, gneiss	-	-	a	p	1.05	0.54	13.41	
	4.73 - 4.94	Highly weathered, brown to grey, soft rock, gneiss	-	-	d	p	1.08	0.28	6.94	
PLT-BH03	8.09 - 8.37	Moderately weathered, light brown to grey speckled black and streaked white, medium hard rock, granitic gneiss	42.0	normal	a	p	1.14	4.04	100.56	
	8.09 - 8.37	Moderately weathered, light brown to grey speckled black and streaked white, medium hard rock, granitic gneiss	42.0	normal	a	p	1.08	0.48	14.06	
PLT-BH03	1.78 - 1.98	Completely weathered, yellow brown speckled black, calcified soft rock, gneiss	-	-	d	p	1.27	0.58	16.89	
	1.78 - 1.98	Completely weathered, yellow brown speckled black, calcified soft rock, gneiss	-	-	a	p	0.98	1.65	40.84	
PLT-BH03	6.33 - 6.55	Moderately weathered, dark grey to green speckled black streaked white, medium hard rock, gneiss	10.5	normal	d	p	1.09	0.81	20.09	
	6.33 - 6.55	Moderately weathered, dark grey to green speckled black streaked white, medium hard rock, gneiss	10.5	normal	a	p	0.98	1.65	40.84	
Waste Rock Dump (WRD)	WRD-BH01	1.12 - 1.35	Pale brown blotched white, hardpan calcrete of soft rock strength	5.0	normal	-	-	-	-	-
	WRD-BH01	5.13 - 5.30	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	-	-	a	p	0.97	0.22	5.48
		5.13 - 5.30	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	-	-	d	p	1.09	0.27	6.73
WRD-BH01	7.75 - 7.88	Slightly weathered to moderately weathered, dark brown to grey streaked and banded white, medium hard rock, gneiss	-	-	a	p	1.06	0.13	3.24	

NOTES:

1. D = DIAMETRICAL LOADING; 2. A = AXIAL LOADING; 3. UCS = UNIAXIAL COMPRESSIVE STRENGTH; 4. P = PERPENDICULAR.

A	8/1/2023	ISSUED WITH LETTER	SY	BK
REV	DATE	DESCRIPTION	PREP'D	REV'D

Evaluation of unpaved wearing course materials according to THR20

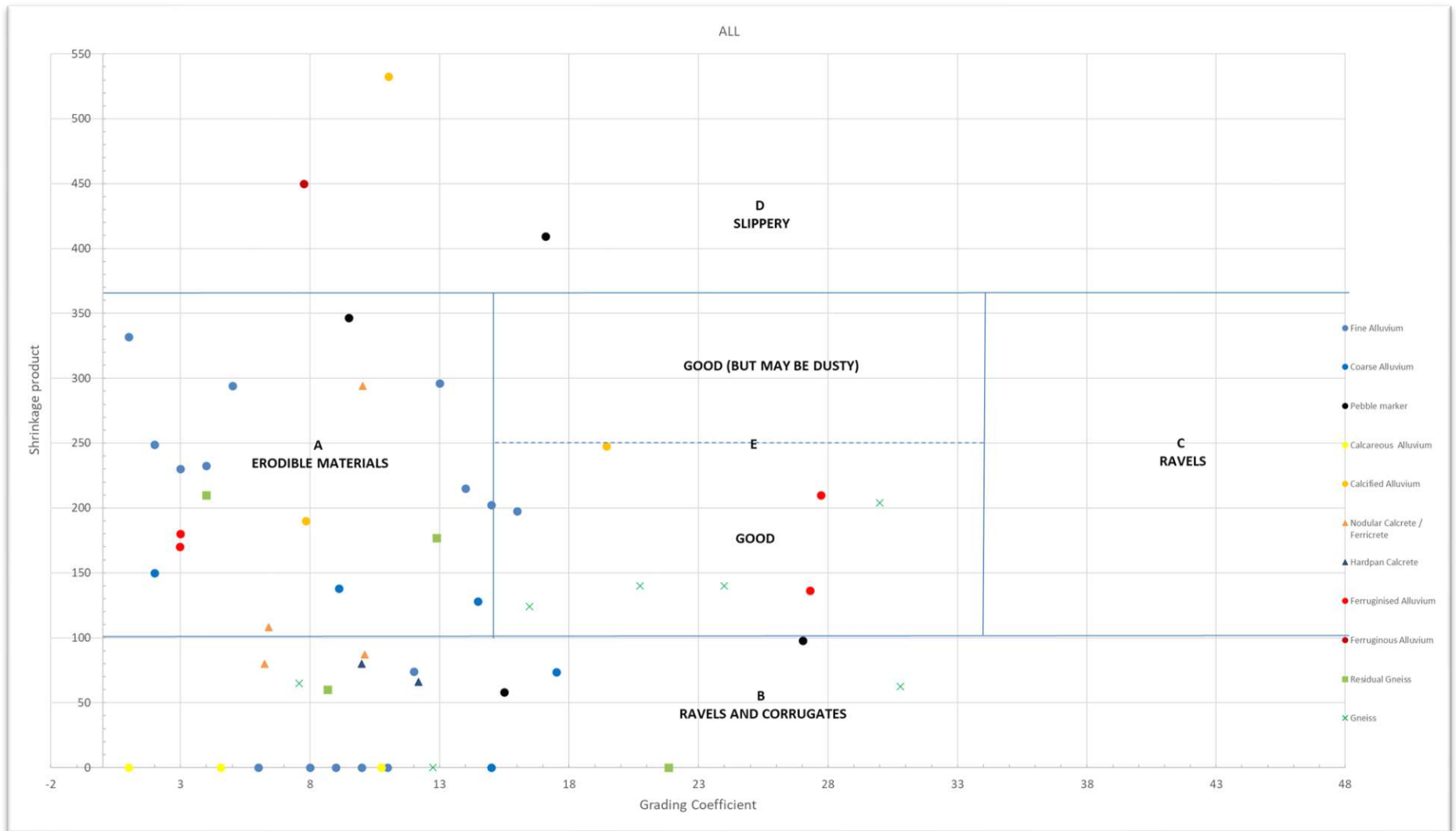


Figure 1 Evaluation of all materials unpaved wearing course according to THR20

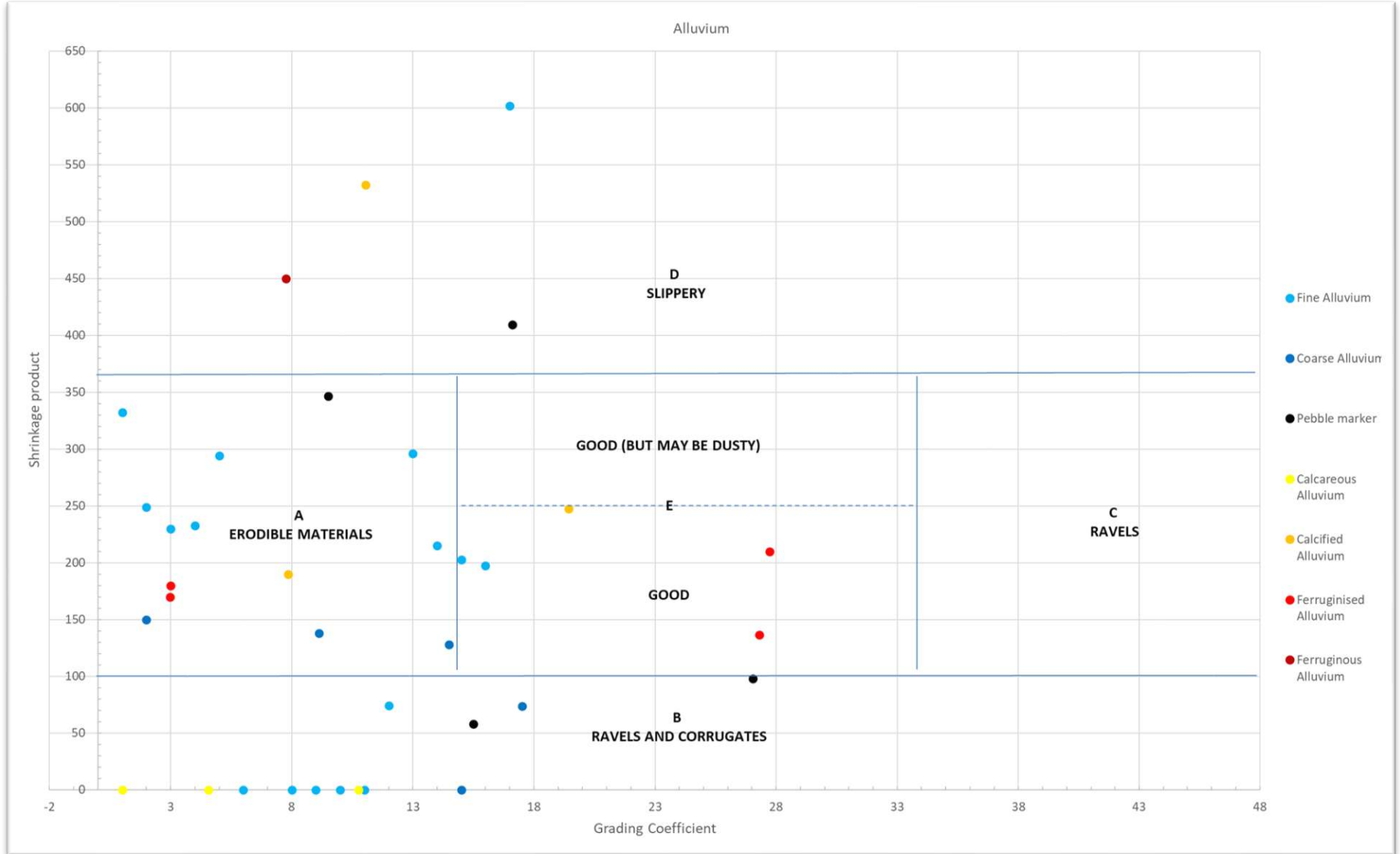


Figure 2 Evaluation of alluvium materials unpaved wearing course according to THR20

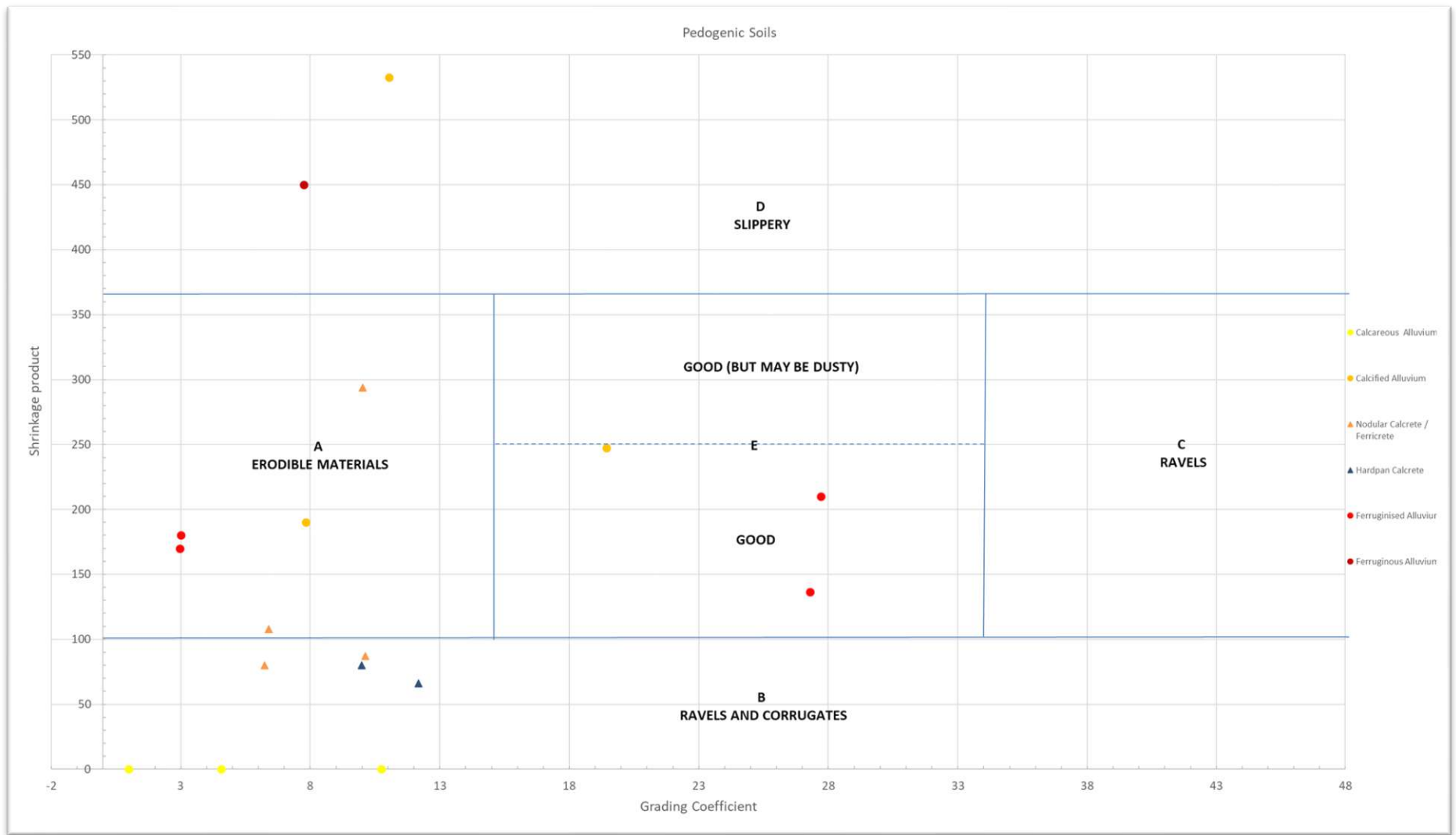


Figure 3 Evaluation of pedogenic materials unpaved wearing course according to THR20

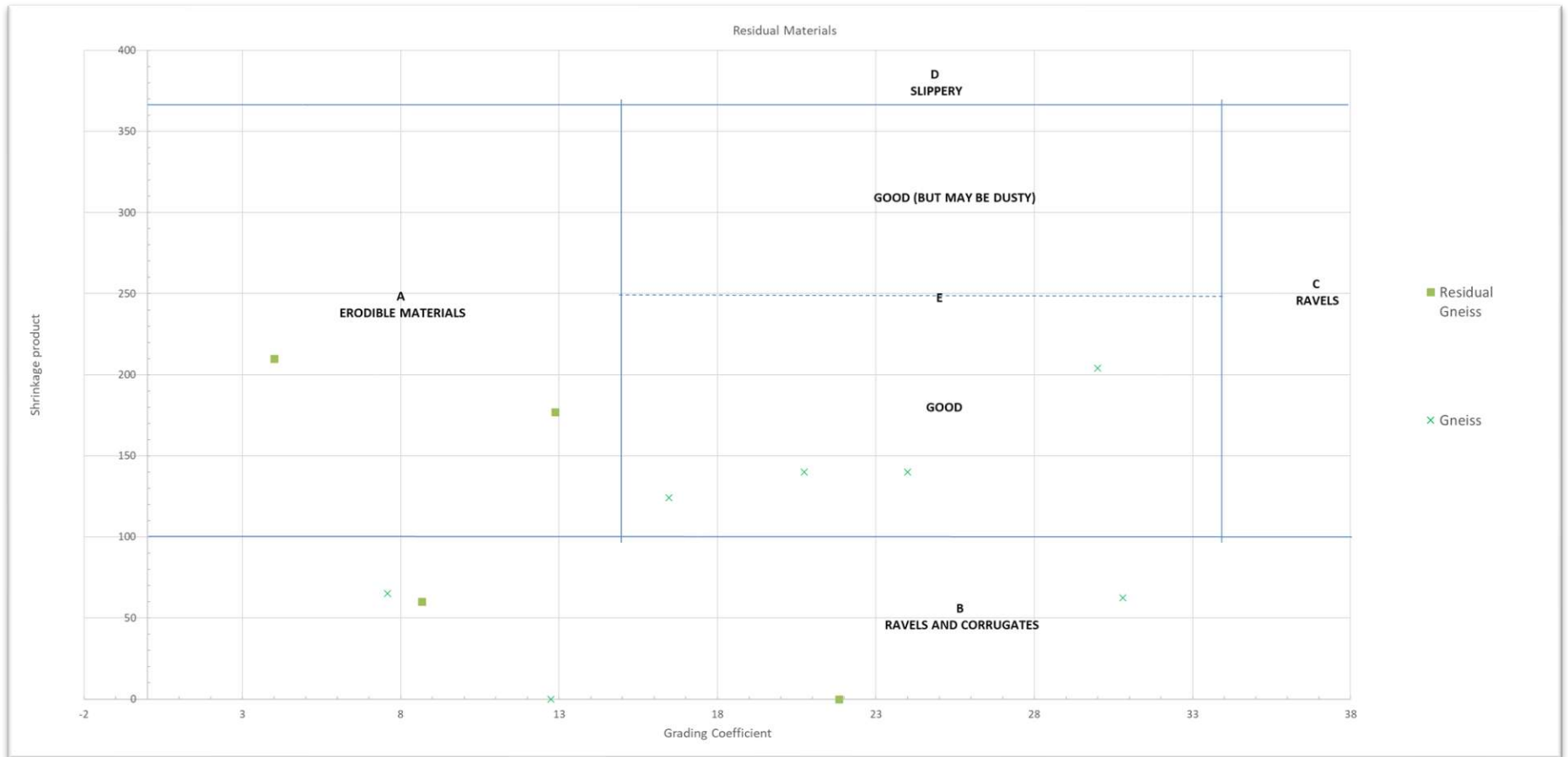


Figure 4 Evaluation of residual and very soft rock materials unpaved wearing course according to THR20

APPENDIX C

Test Pit and Borehole Logs

Appendix C1

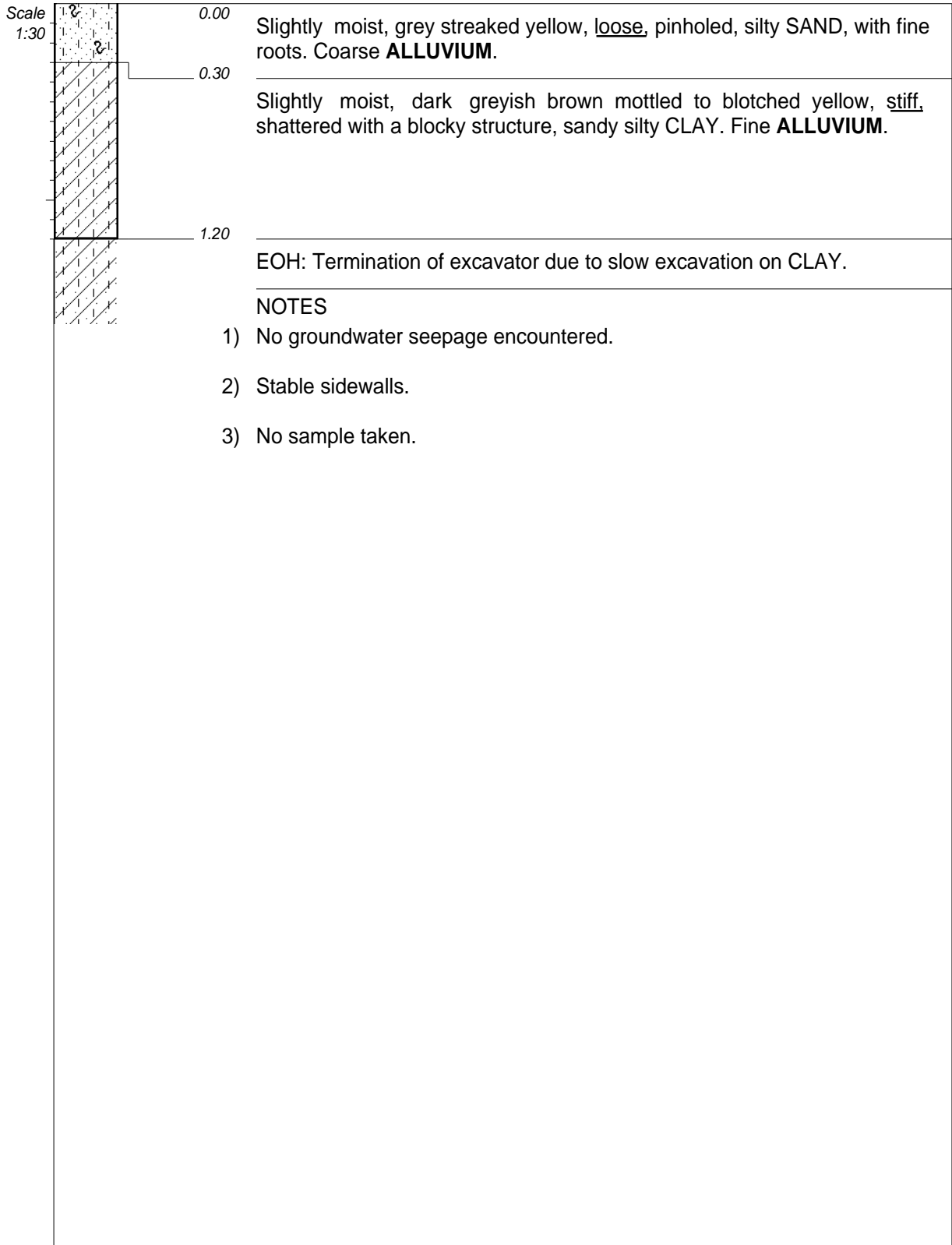
Test Pit Profiles

Appendix C2

Boreholes Logs

APPENDIX C1

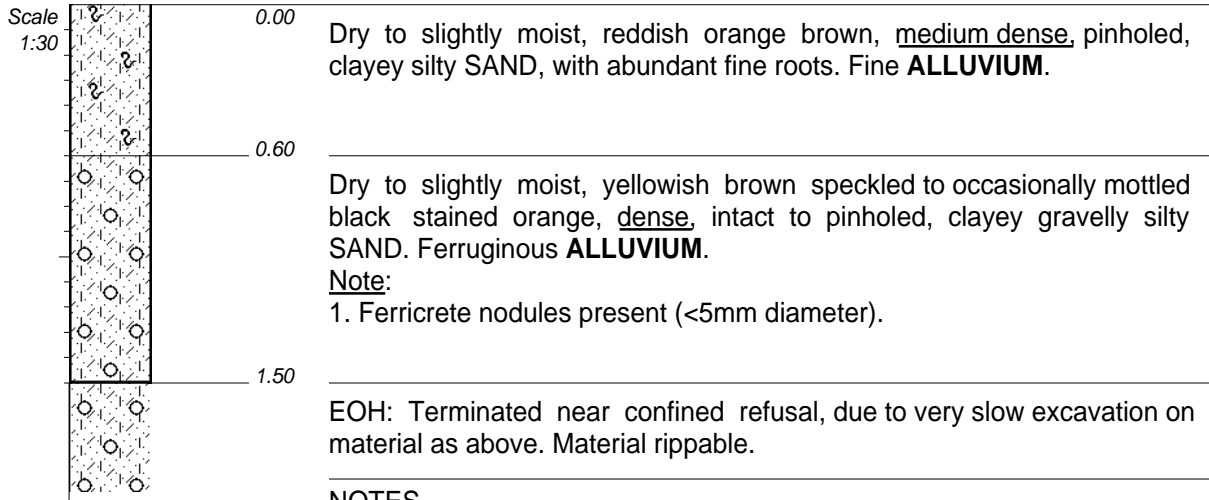
Test Pit Profiles



CONTRACTOR : Sandworx
 MACHINE : : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : Chris Homan
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 22 May 2023
 DATE : 22 May 2023
 DATE : 04/07/2023 13:44
 TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 0800659
 Y-COORD : 7584200



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan

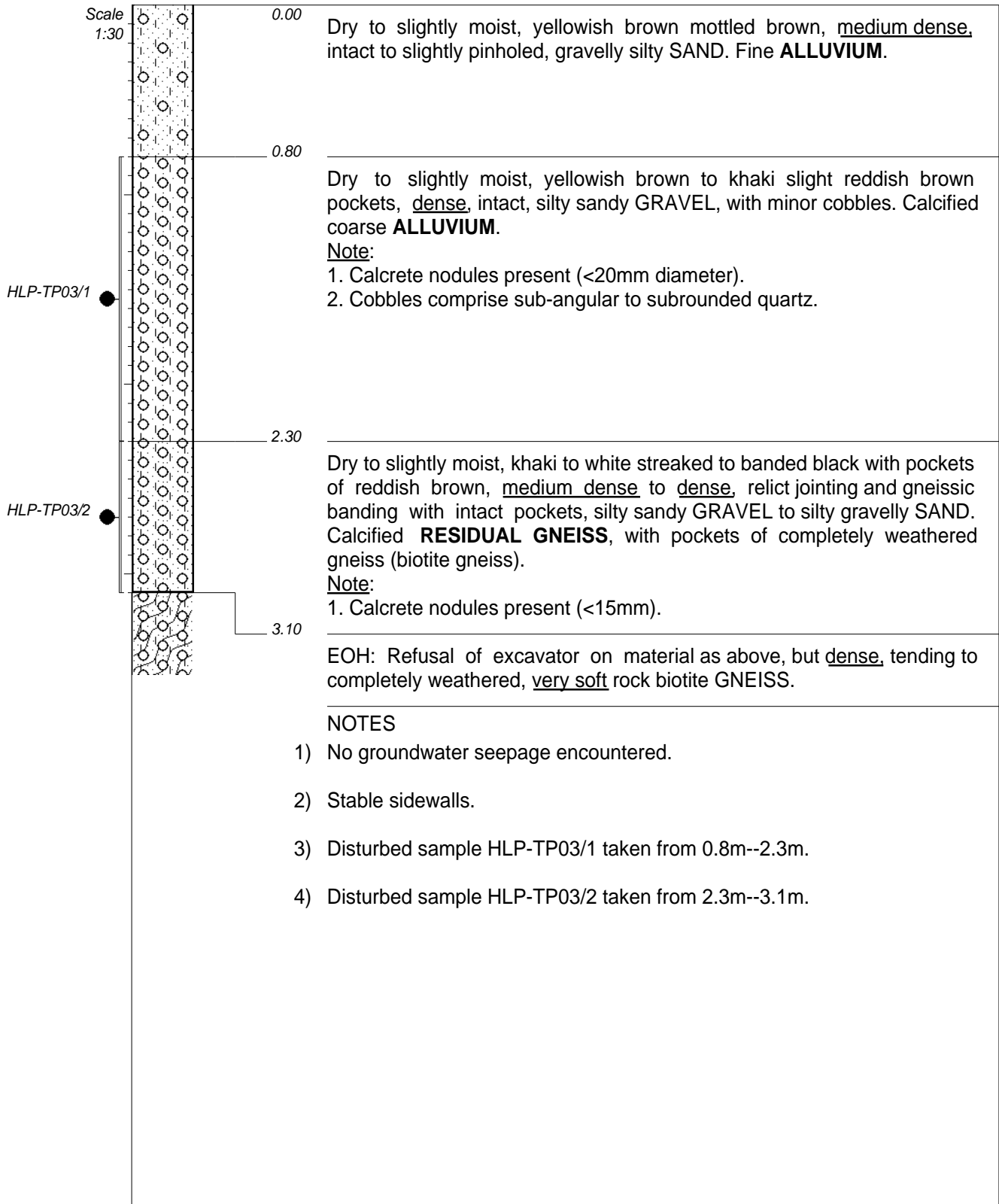
INCLINATION : Vertical
DIAM :
DATE : 23 May 2023
DATE : 23 May 2023

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0800189
Y-COORD : 7585668

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:44
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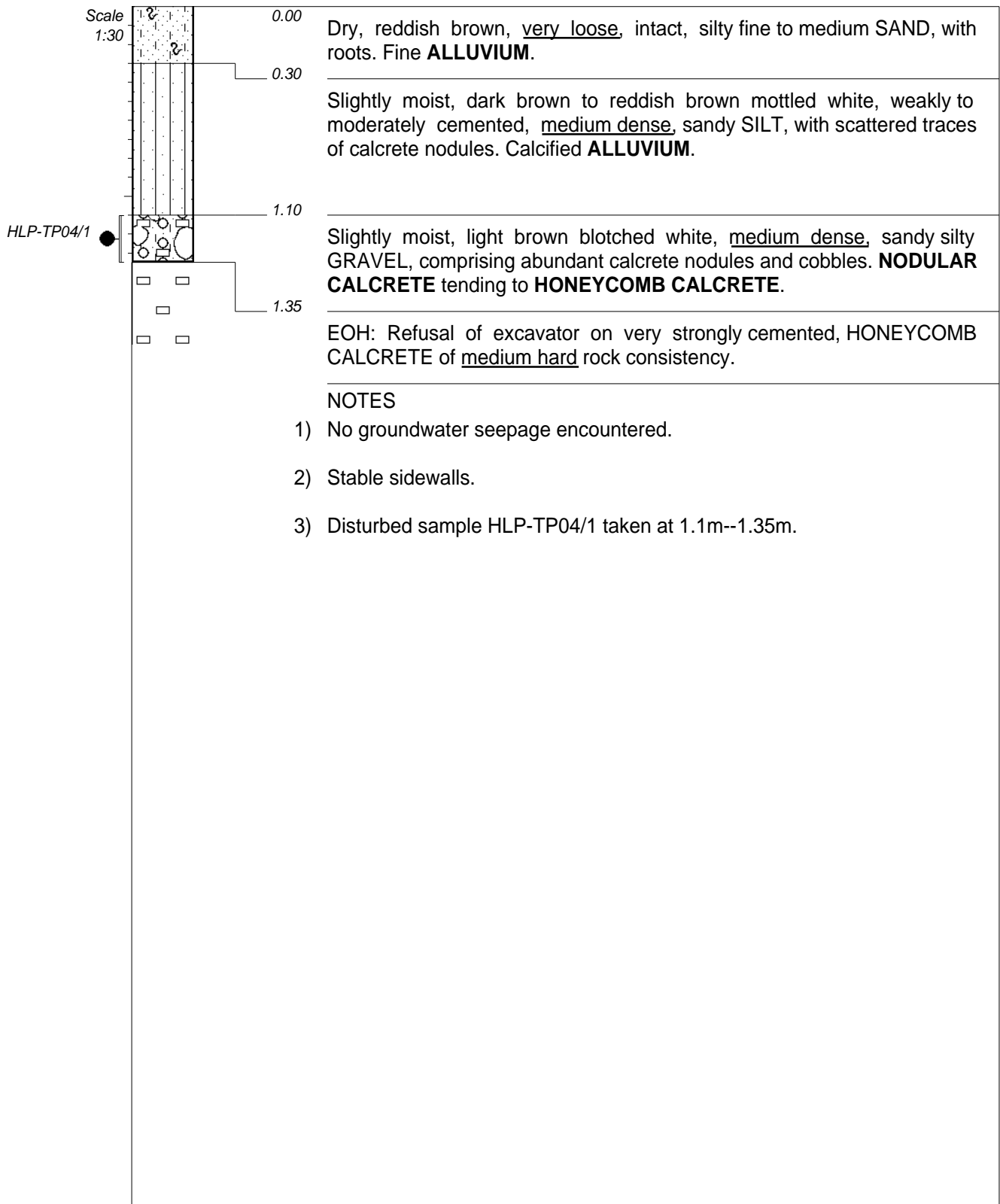
HOLE No: HLP-TP02



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 22 May 2023
DATE : 22 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0800632
Y-COORD : 7585869

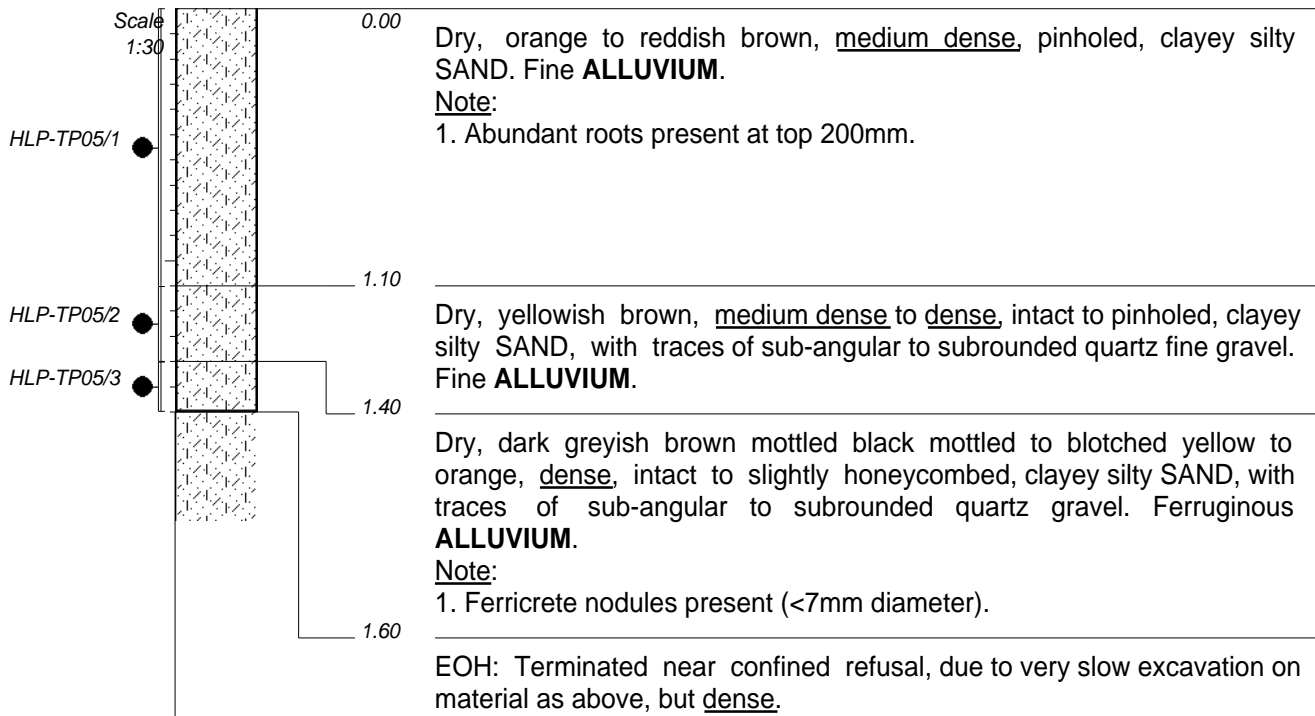


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 19 May 2023
DATE : 19 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801350.85
Y-COORD : 7586209.65

HOLE No: HLP-TP04



NOTES

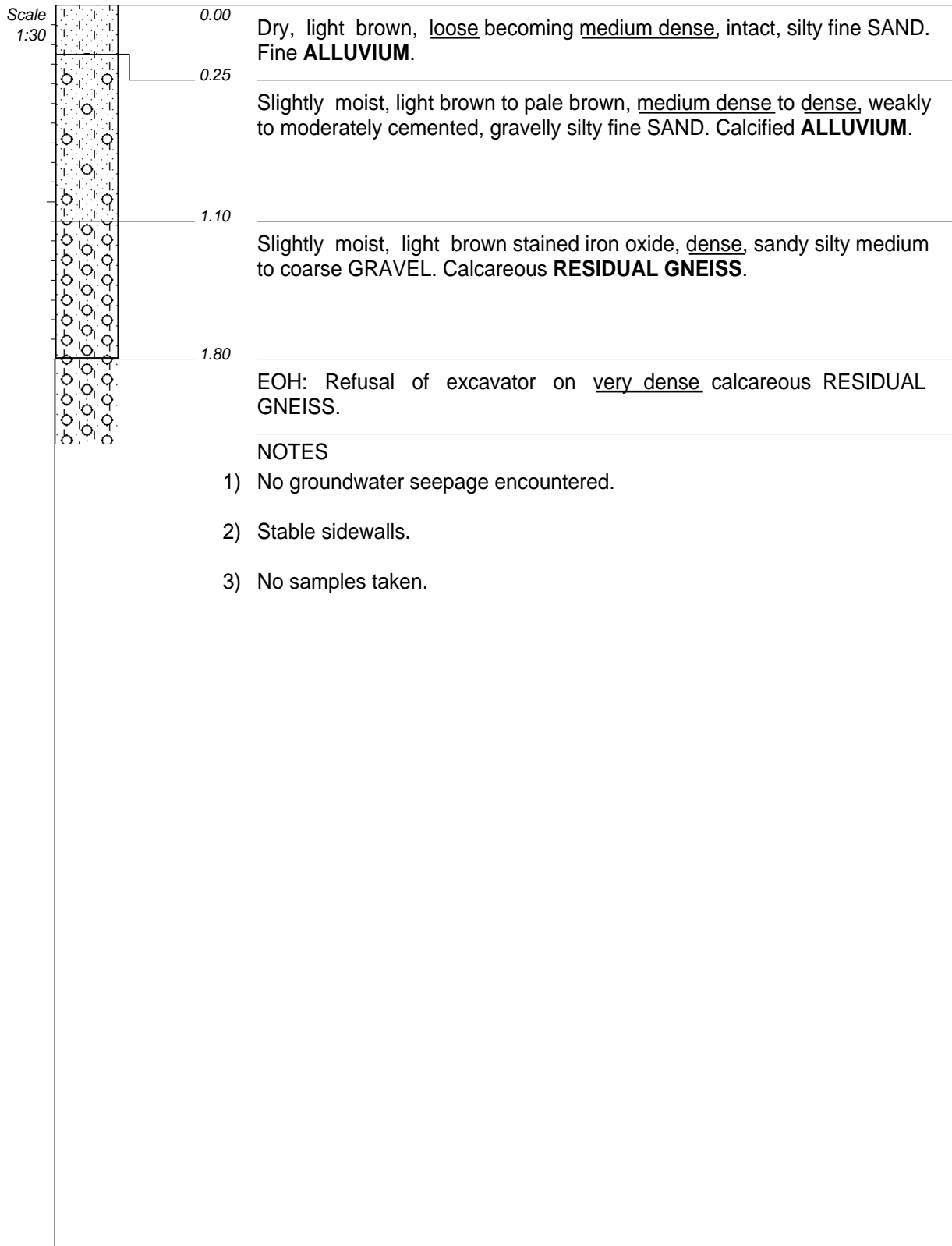
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP05/1 taken from 0.0m--1.1m.
- 4) Disturbed sample HLP-TP05/2 taken from 1.1m--1.4m.
- 5) Disturbed sample HLP-TP05/3 taken from 1.4m--1.6m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 23 May 2023
DATE : 23 May 2023
DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
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Y-COORD : 7585074

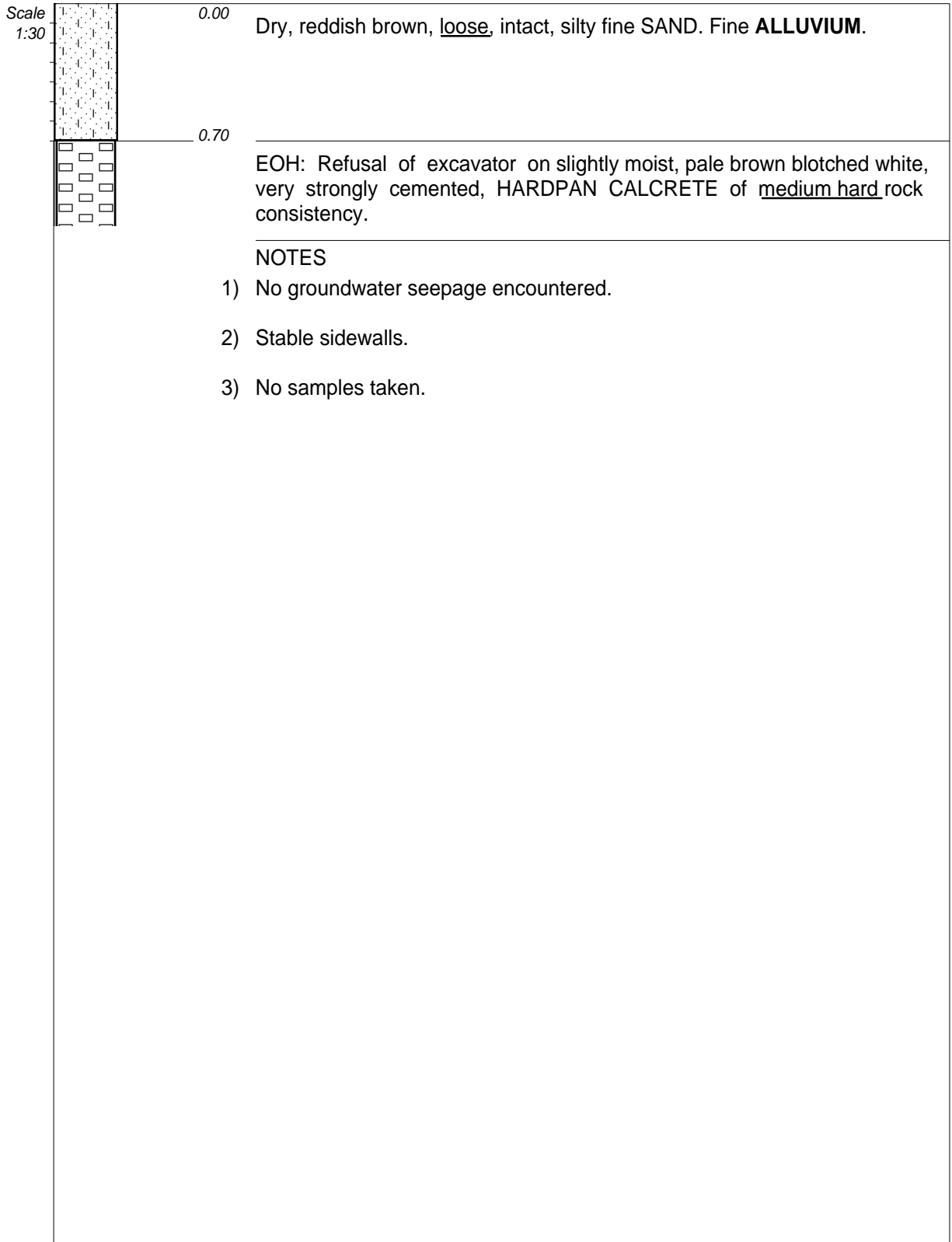
HOLE No: HLP-TP05



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 20 May 2023
DATE : 20 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

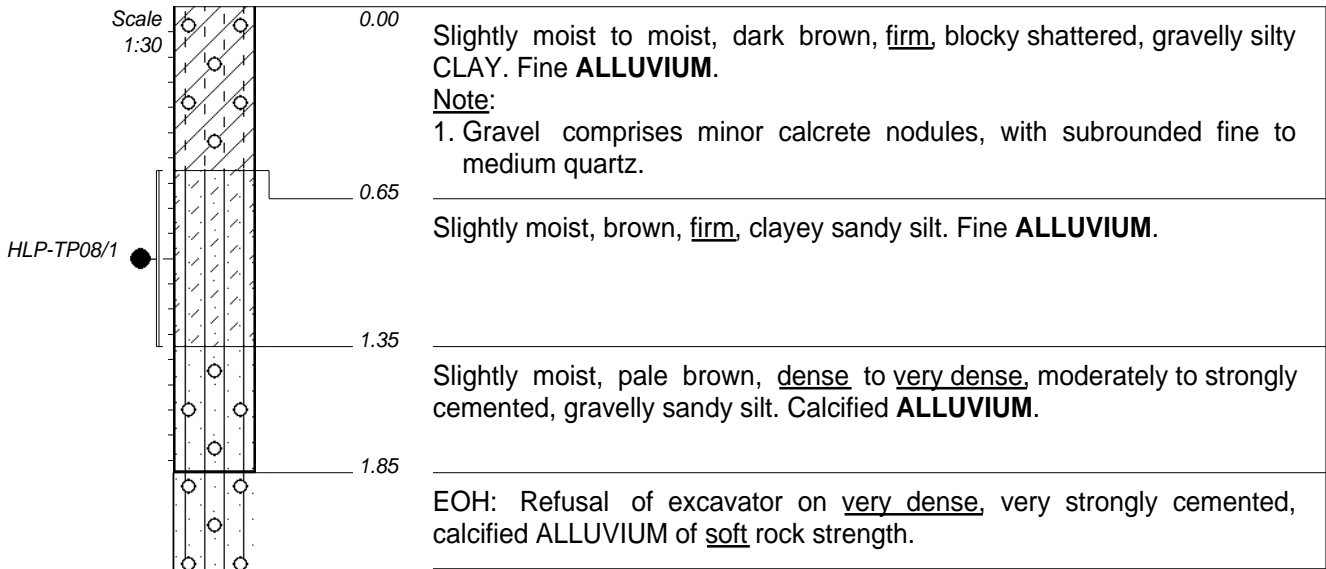
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 800597.01
Y-COORD : 7585212.77



CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 19 May 2023
 DATE : 19 May 2023
 DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 801701.70
 Y-COORD : 7585778.61



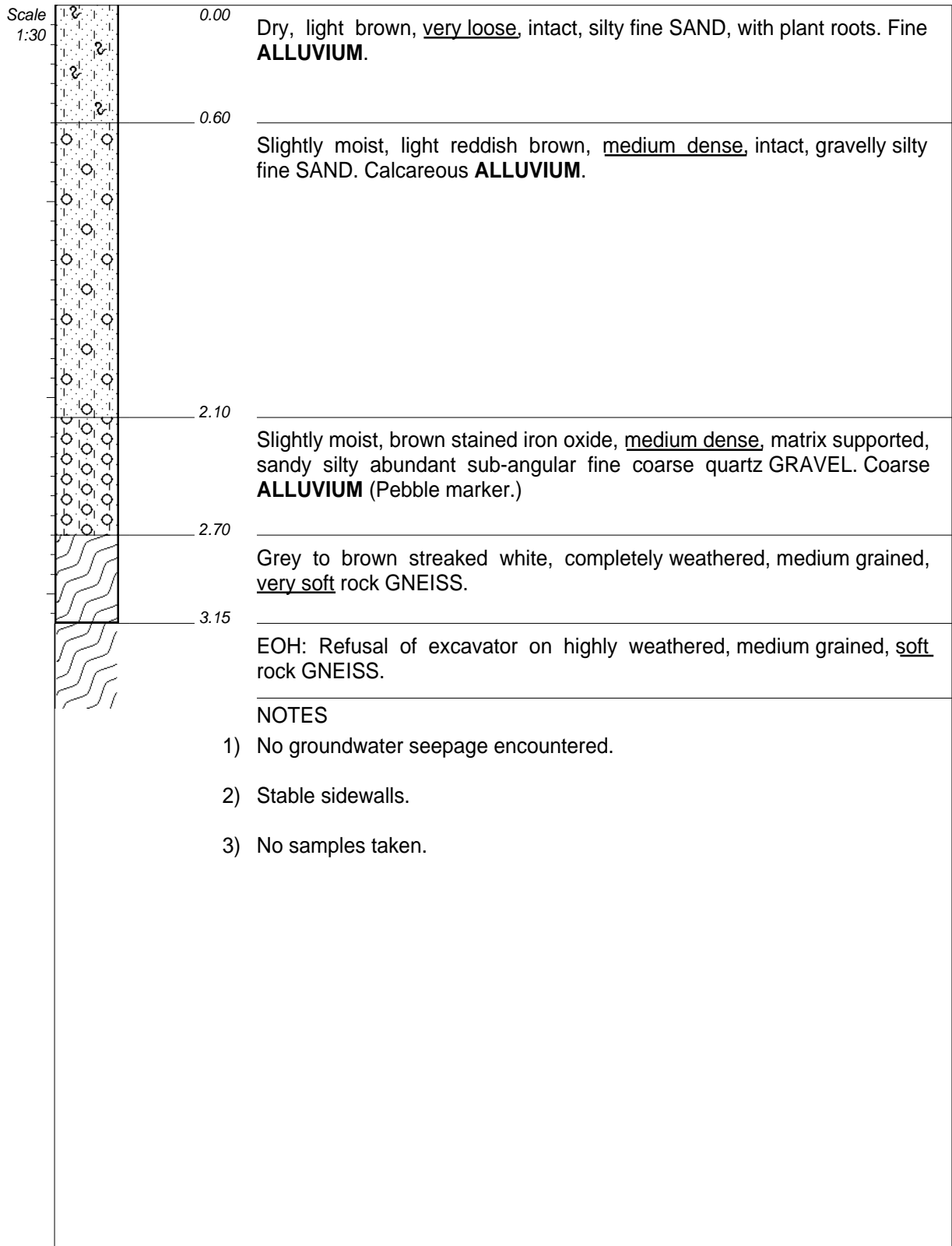
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP08/1 taken at 0.65m--1.35m.

CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 19 May 2023
 DATE : 19 May 2023
 DATE : 04/07/2023 13:44
 TEXT : ..51\PROFILES\PKFHLPTP.TXT

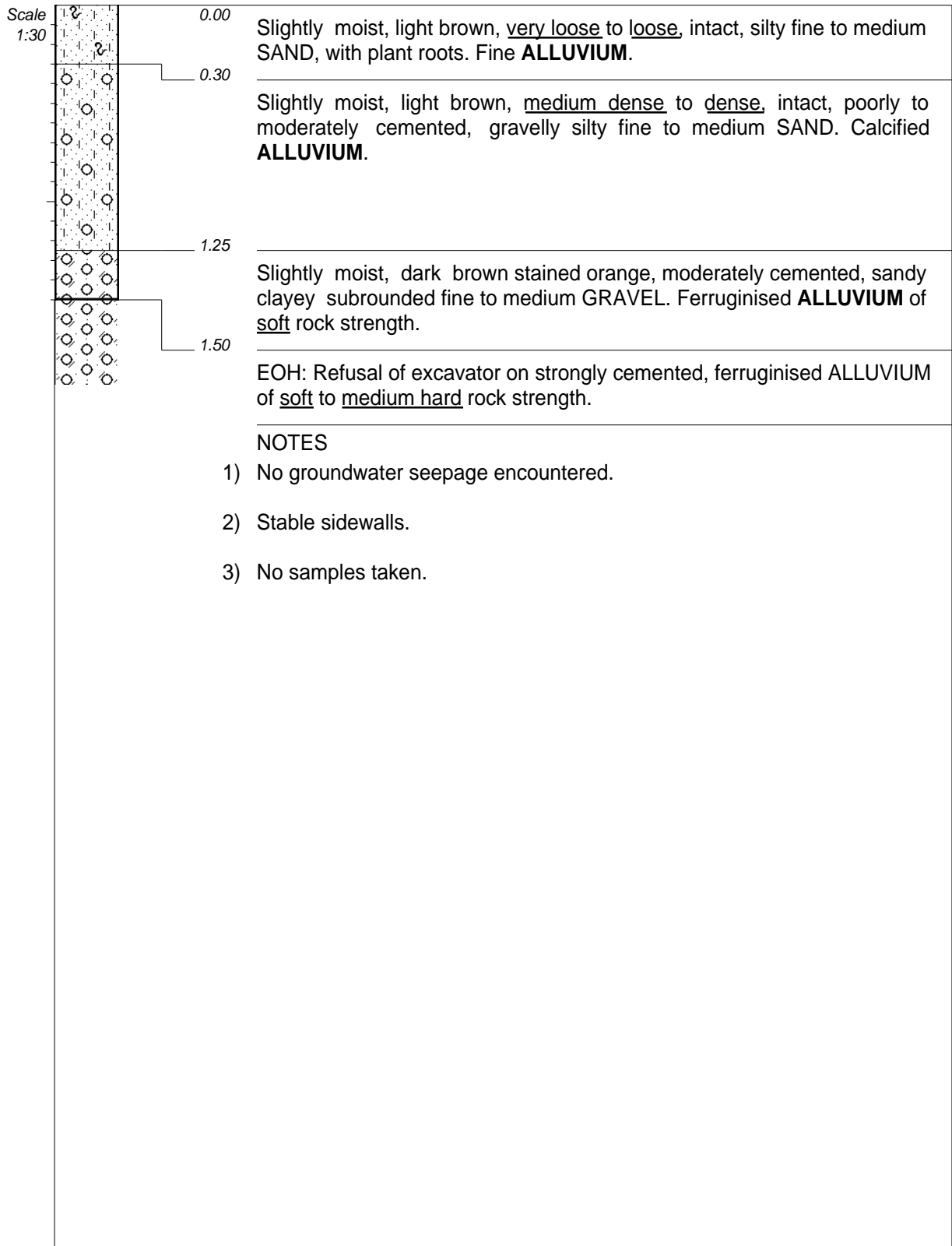
COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 801019.00
 Y-COORD : 7585750.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 20 May 2023
DATE : 20 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

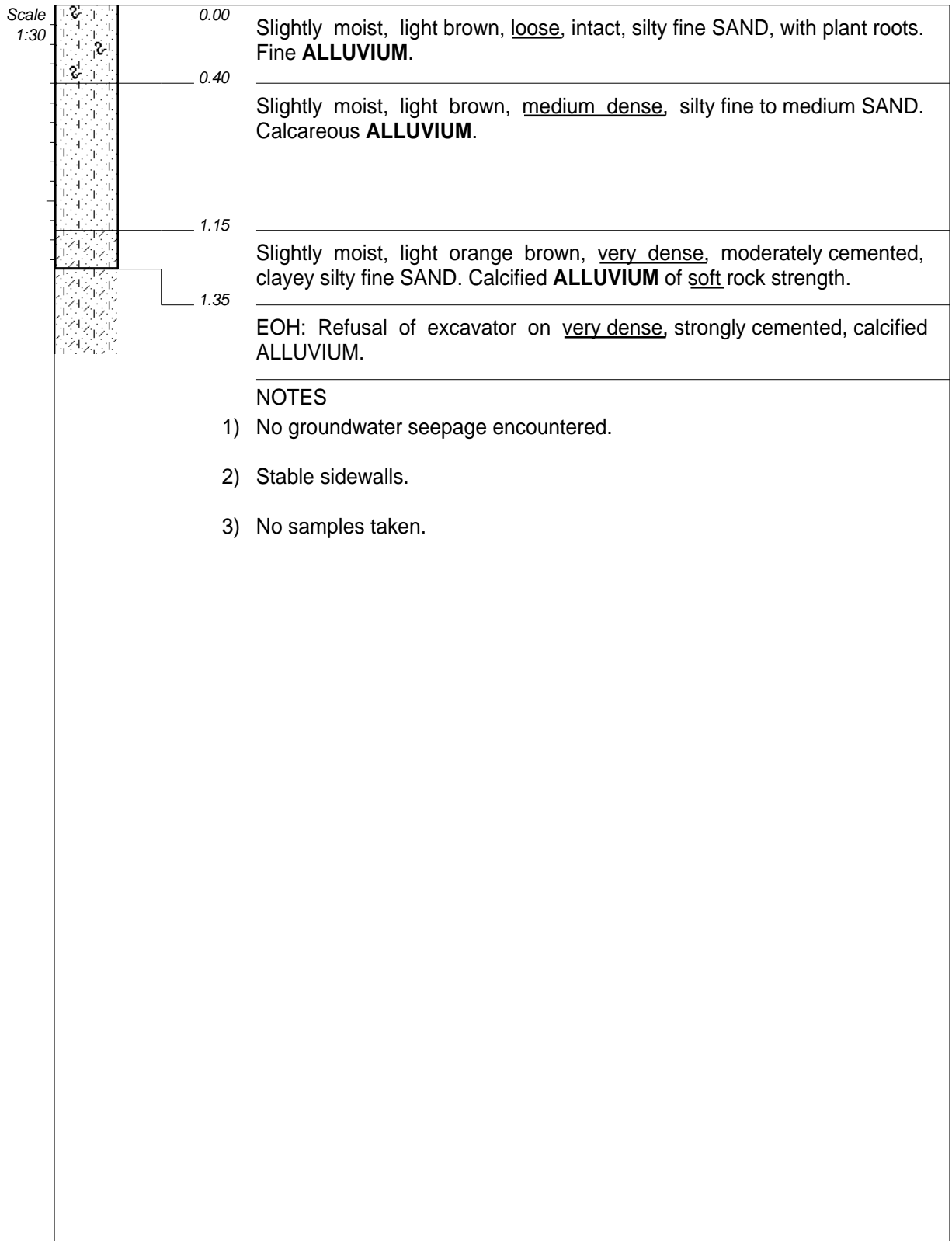
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 800837.00
Y-COORD : 7584690.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 19 May 2023
DATE : 19 May 2023
DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801915.33
Y-COORD : 7585227.33

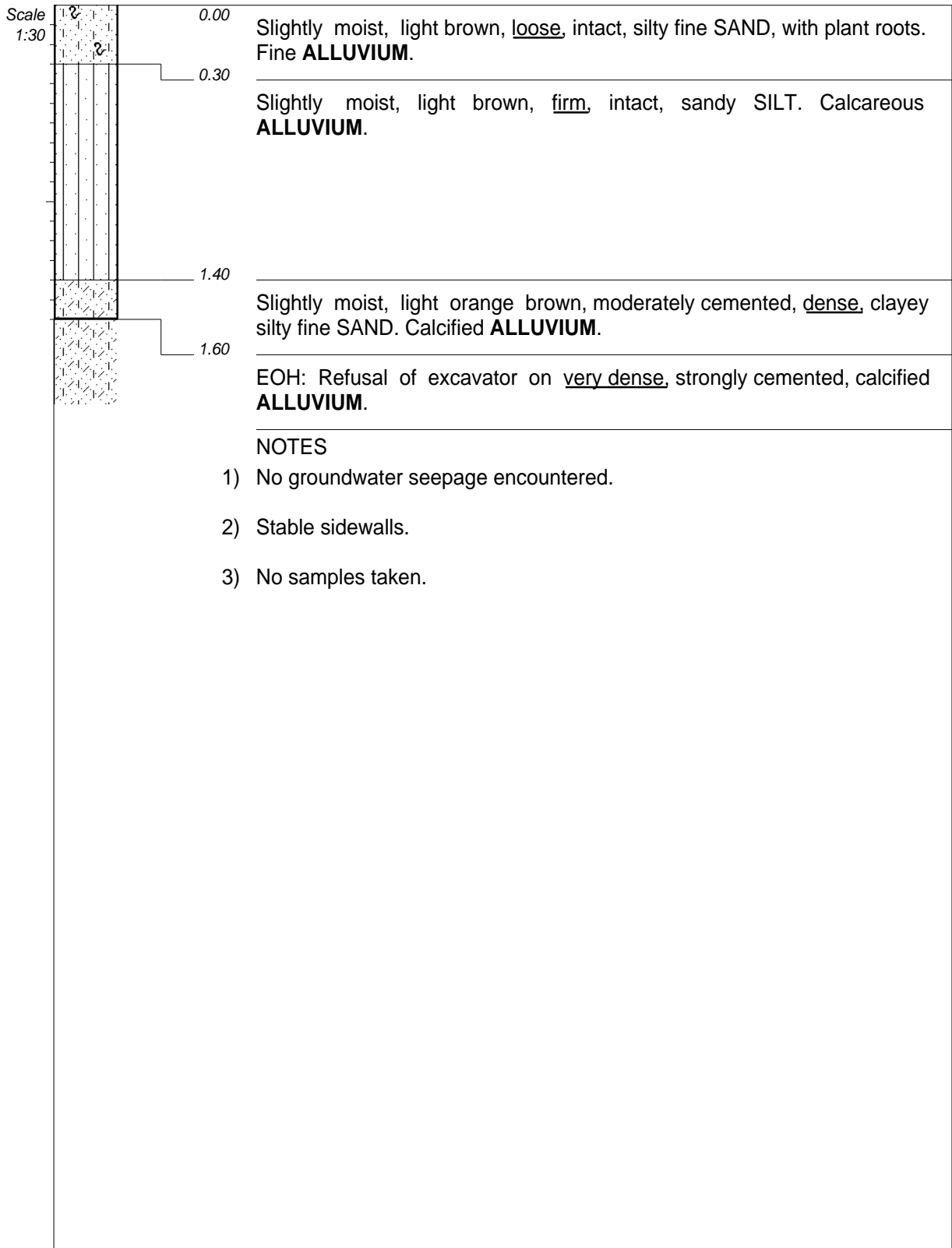


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 19 May 2023
DATE : 19 May 2023
DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801321.00
Y-COORD : 7585404.00

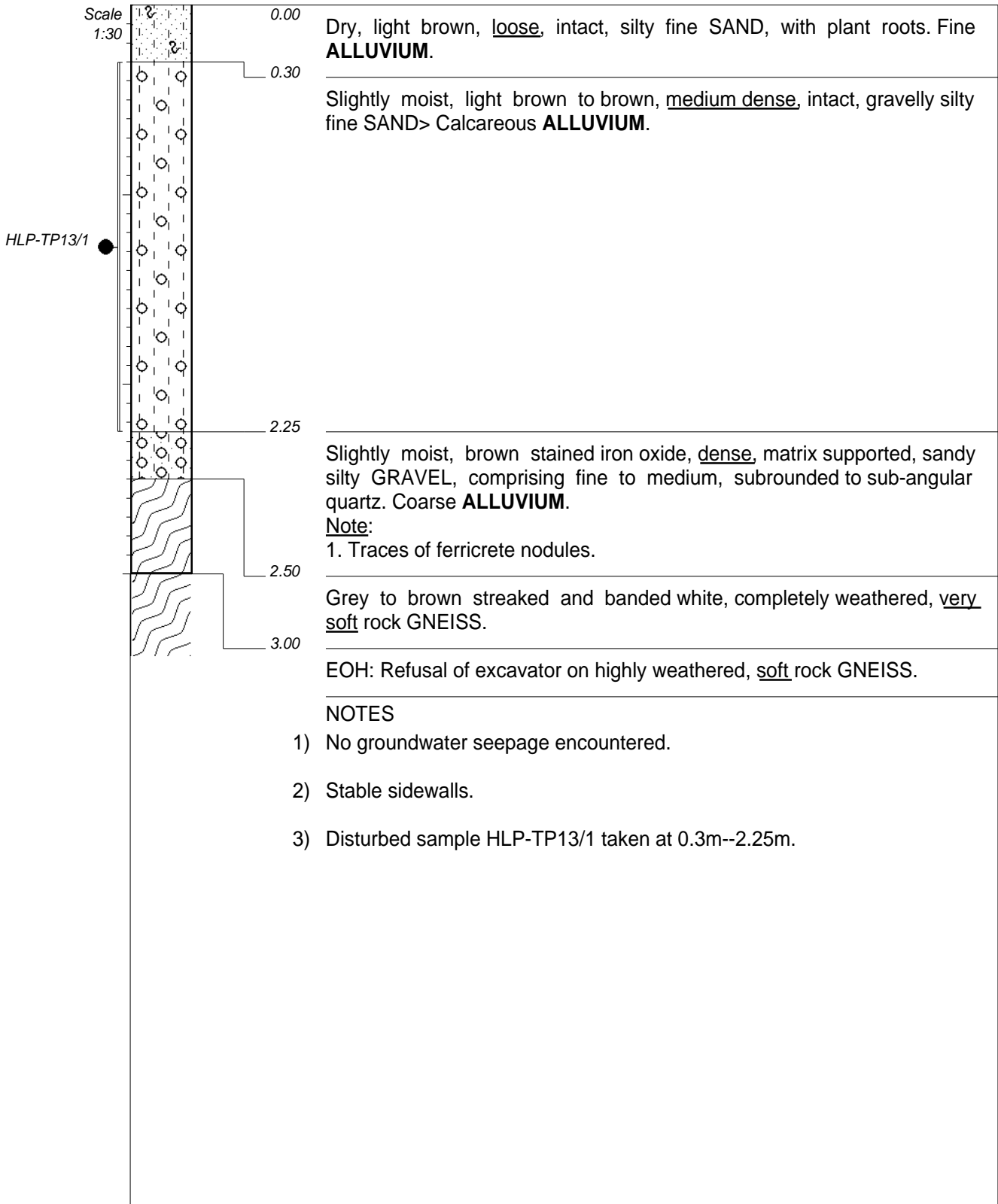
HOLE No: HLP-TP11



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 19 May 2023
DATE : 19 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

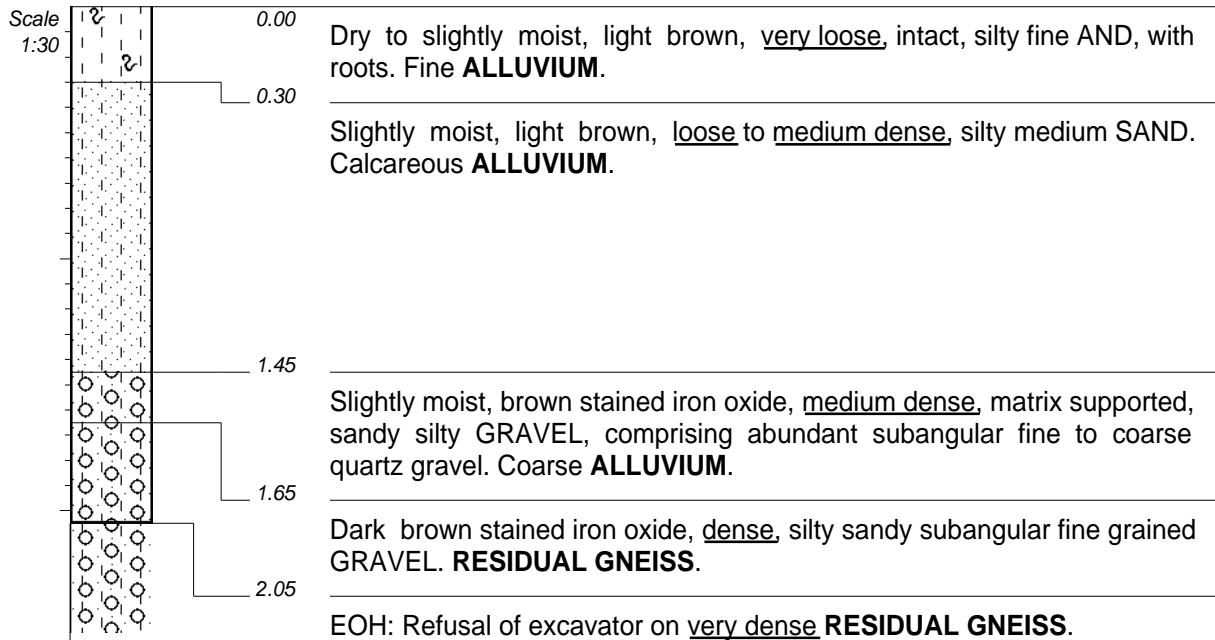
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801251.02
Y-COORD : 7585169.08



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 20 May 2023
DATE : 20 May 2023
DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801093.27
Y-COORD : 7584385.61



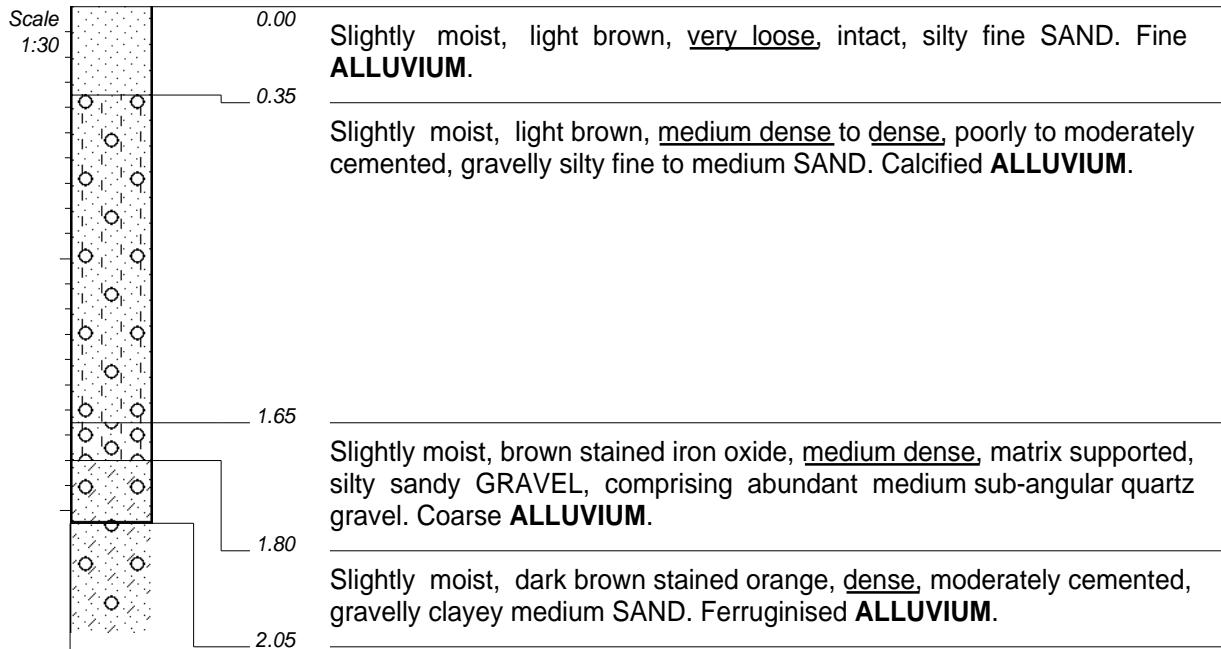
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 19 May 2023
 DATE : 19 May 2023
 DATE : 04/07/2023 13:44
 TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 801470.07
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NOTES

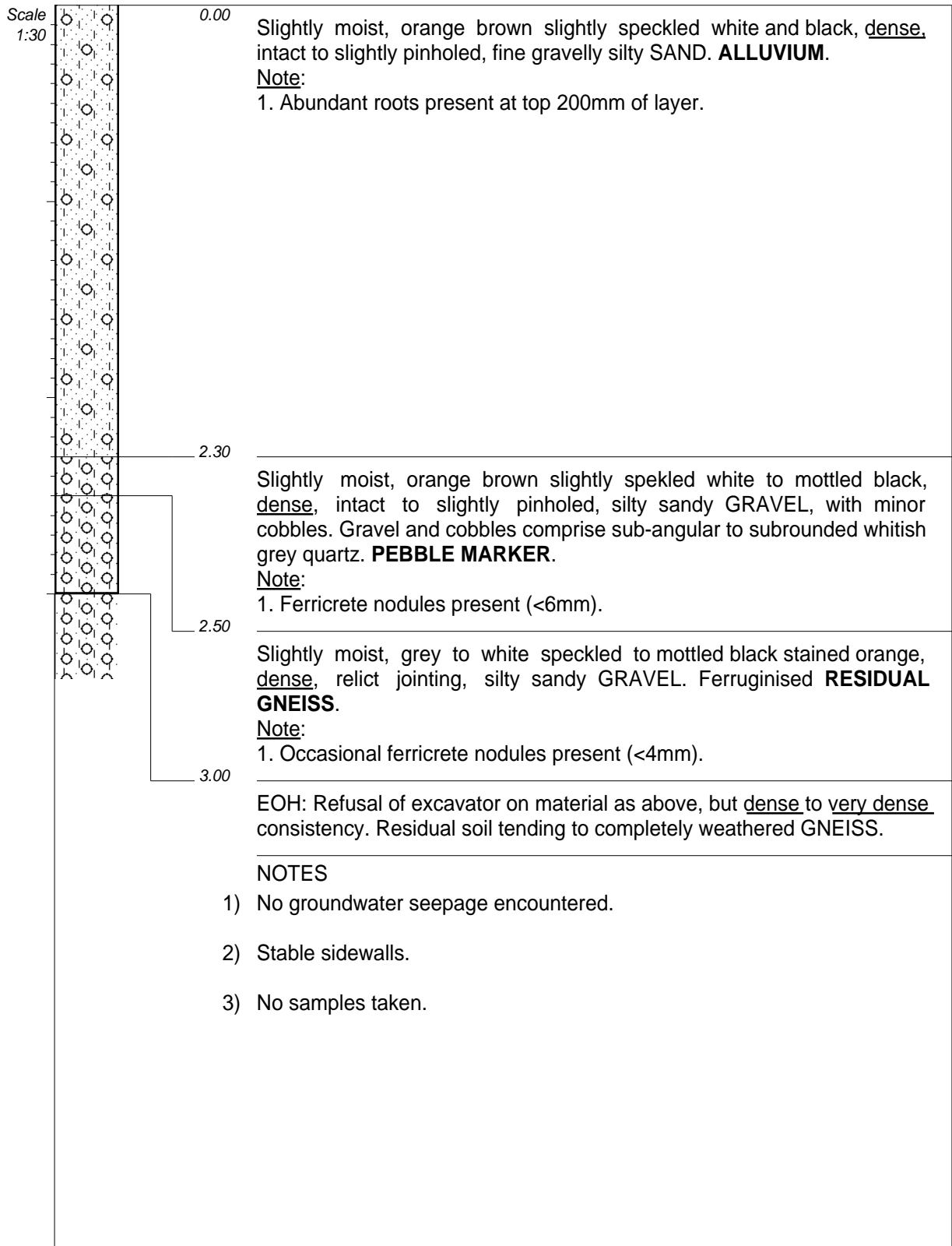
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
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COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802066.00
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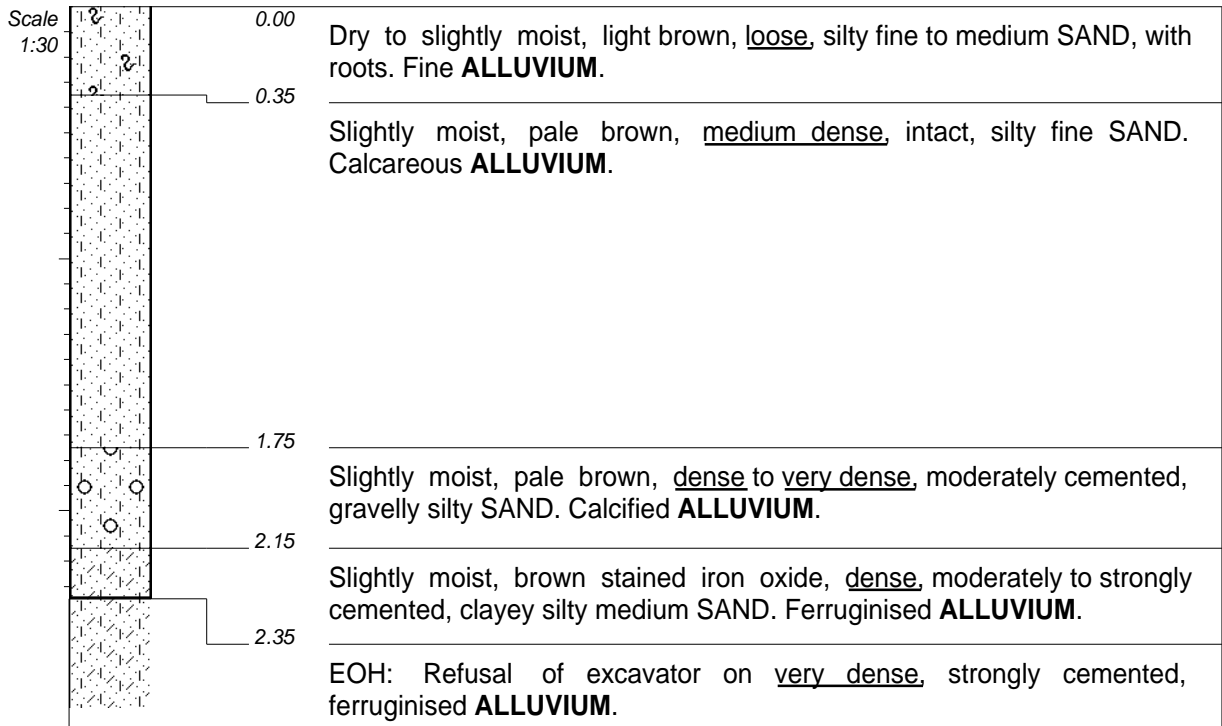
HOLE No: HLP-TP15



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 22 May 2023
DATE : 22 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801209
Y-COORD : 7584039



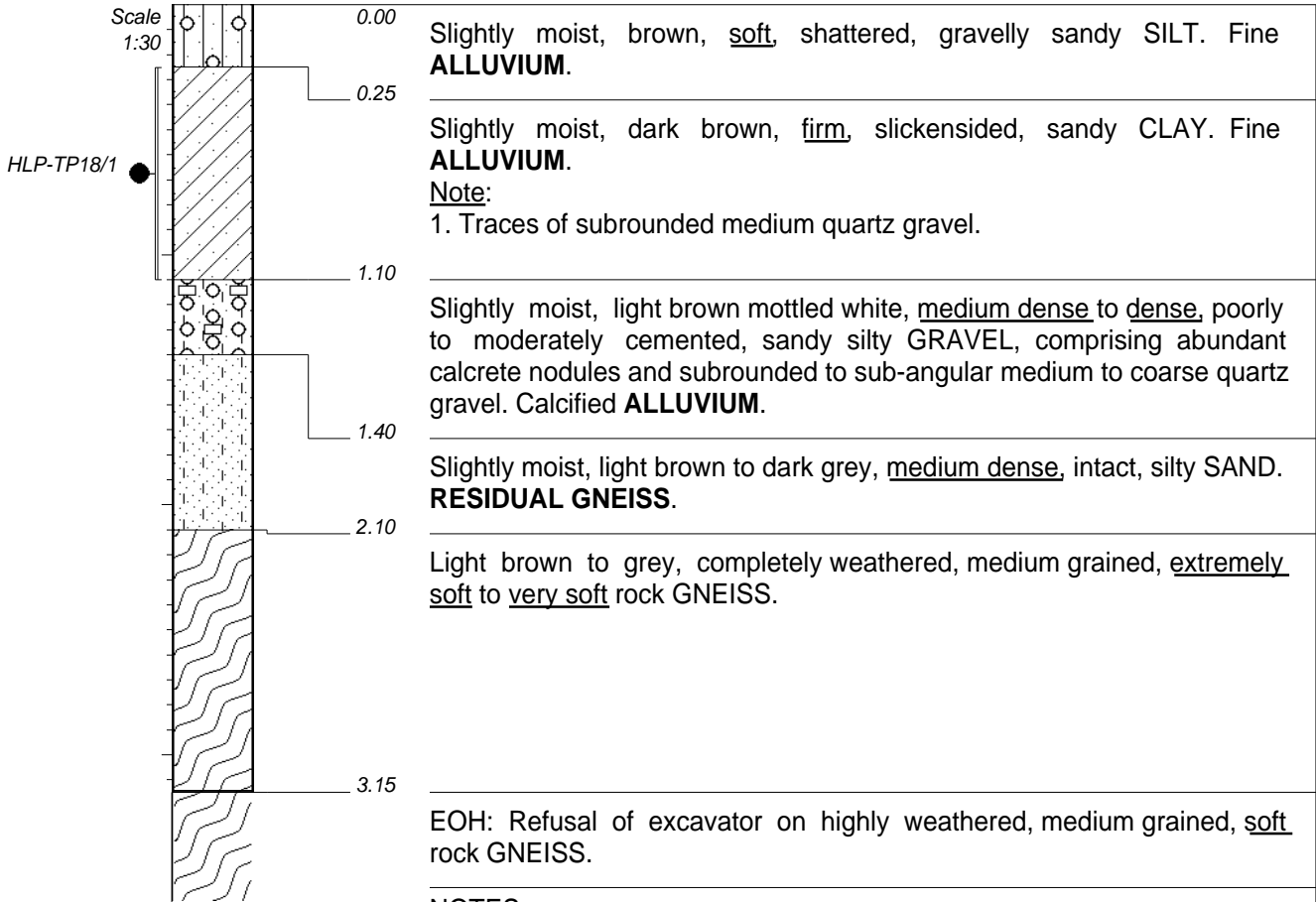
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 19 May 2023
DATE : 19 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801663.45
Y-COORD : 7584447.86



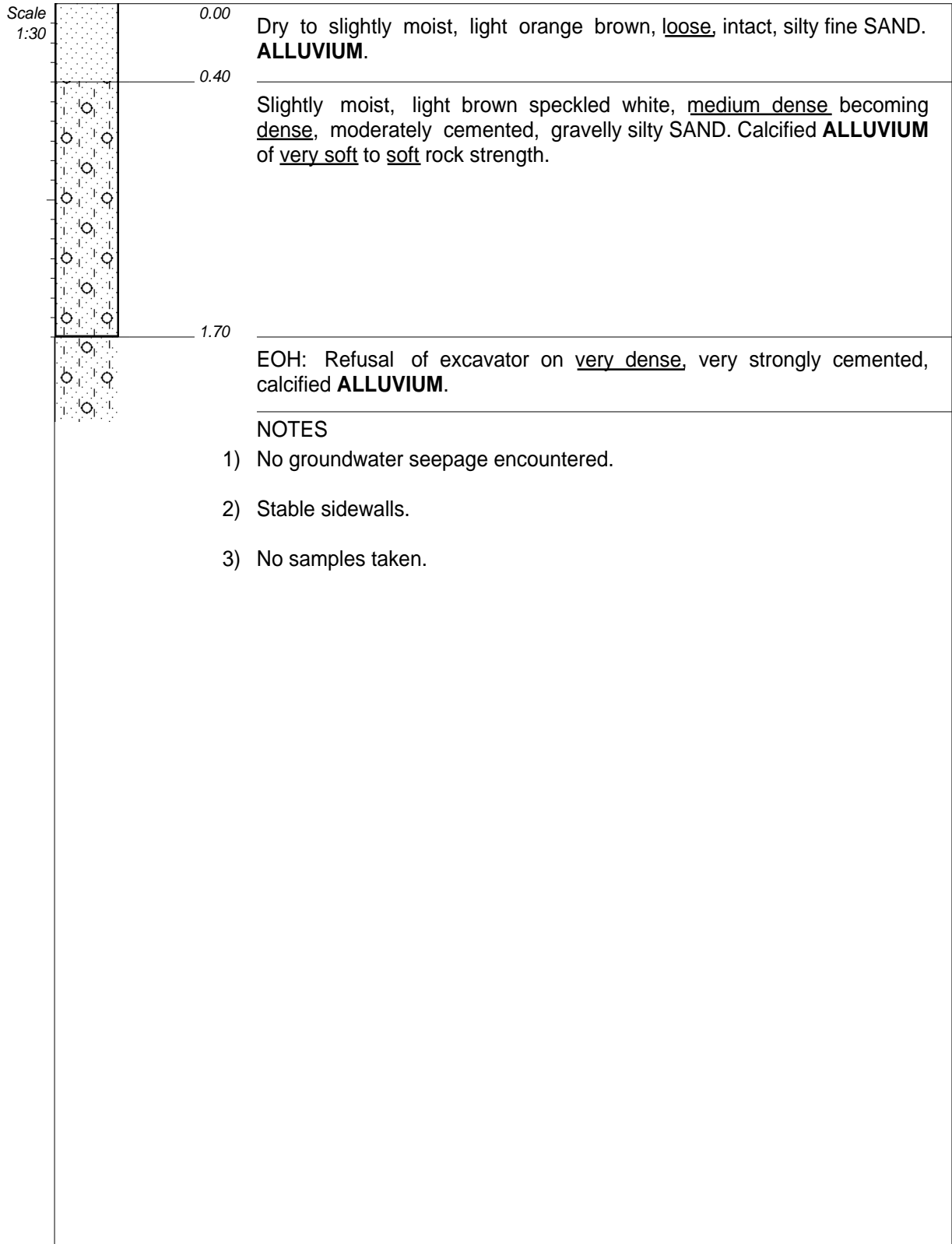
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP18/1 taken at 0.25m--1.1m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

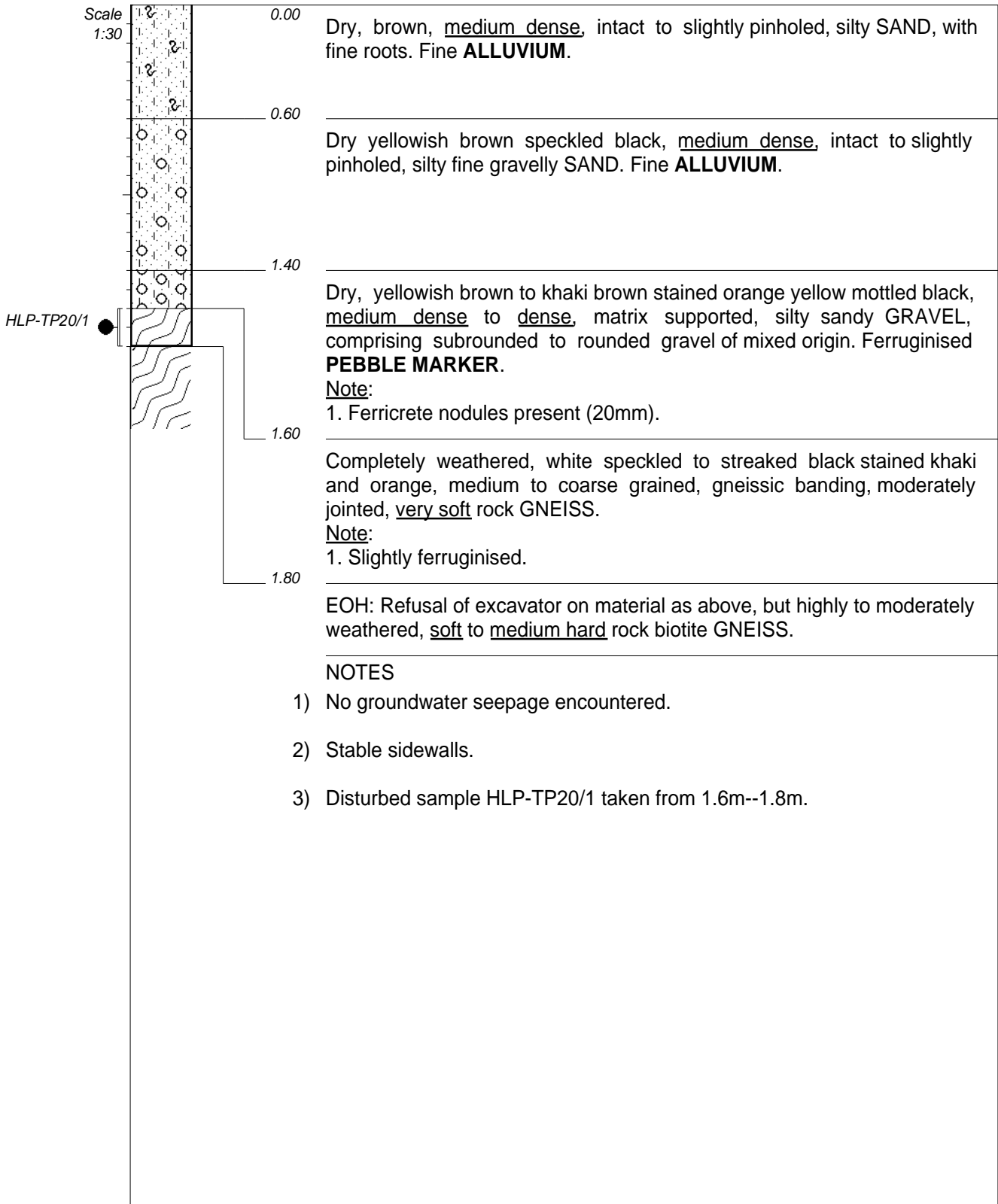
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802000.00
Y-COORD : 7584198.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

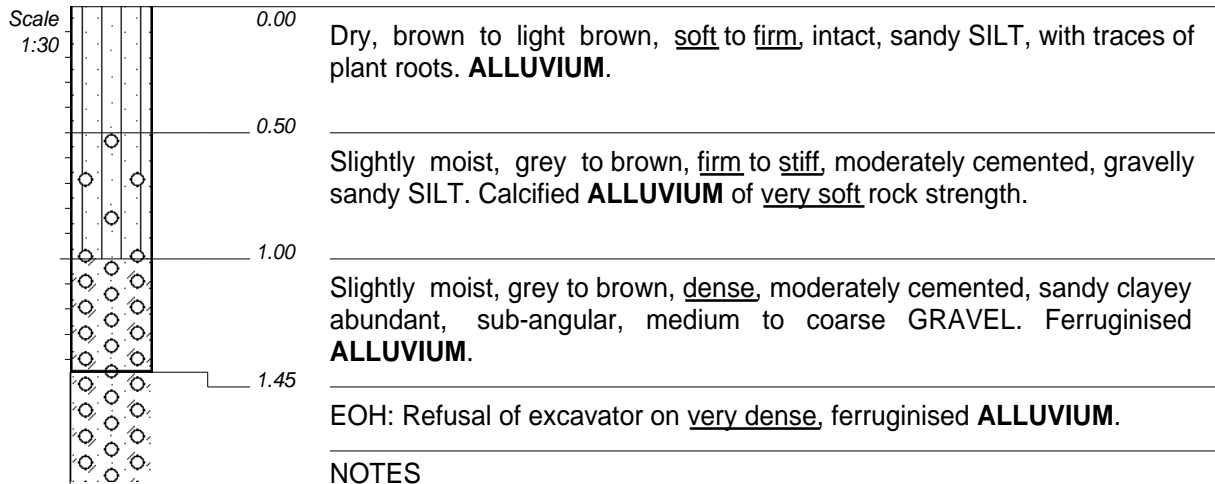
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802208.97
Y-COORD : 7584520.16



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 23 May 2023
DATE : 23 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801428
Y-COORD : 7583326



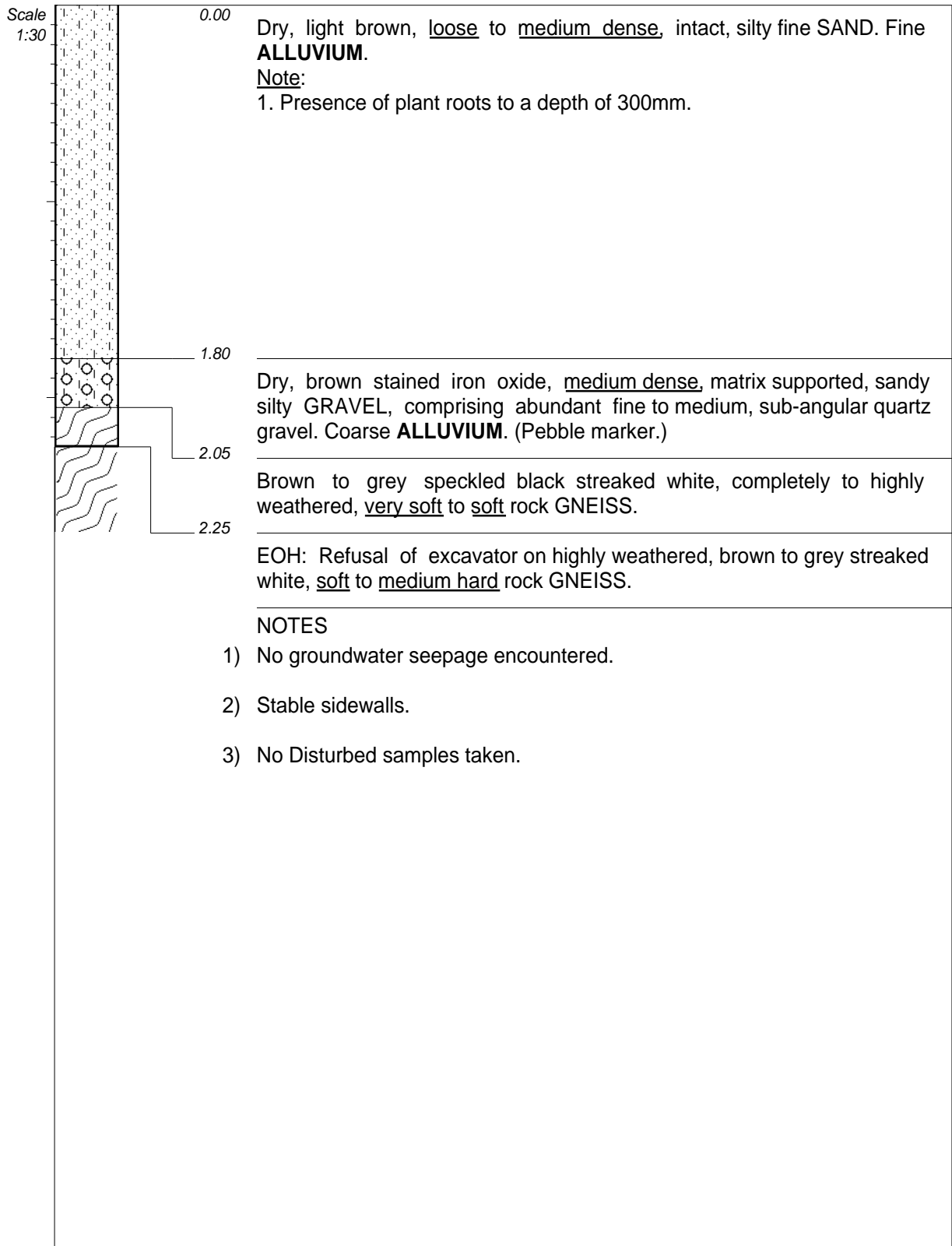
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 20 May 2023
 DATE : 20 May 2023
 DATE : 04/07/2023 13:44
 TEXT : ..51\PROFILES\PKFHLP.TXT

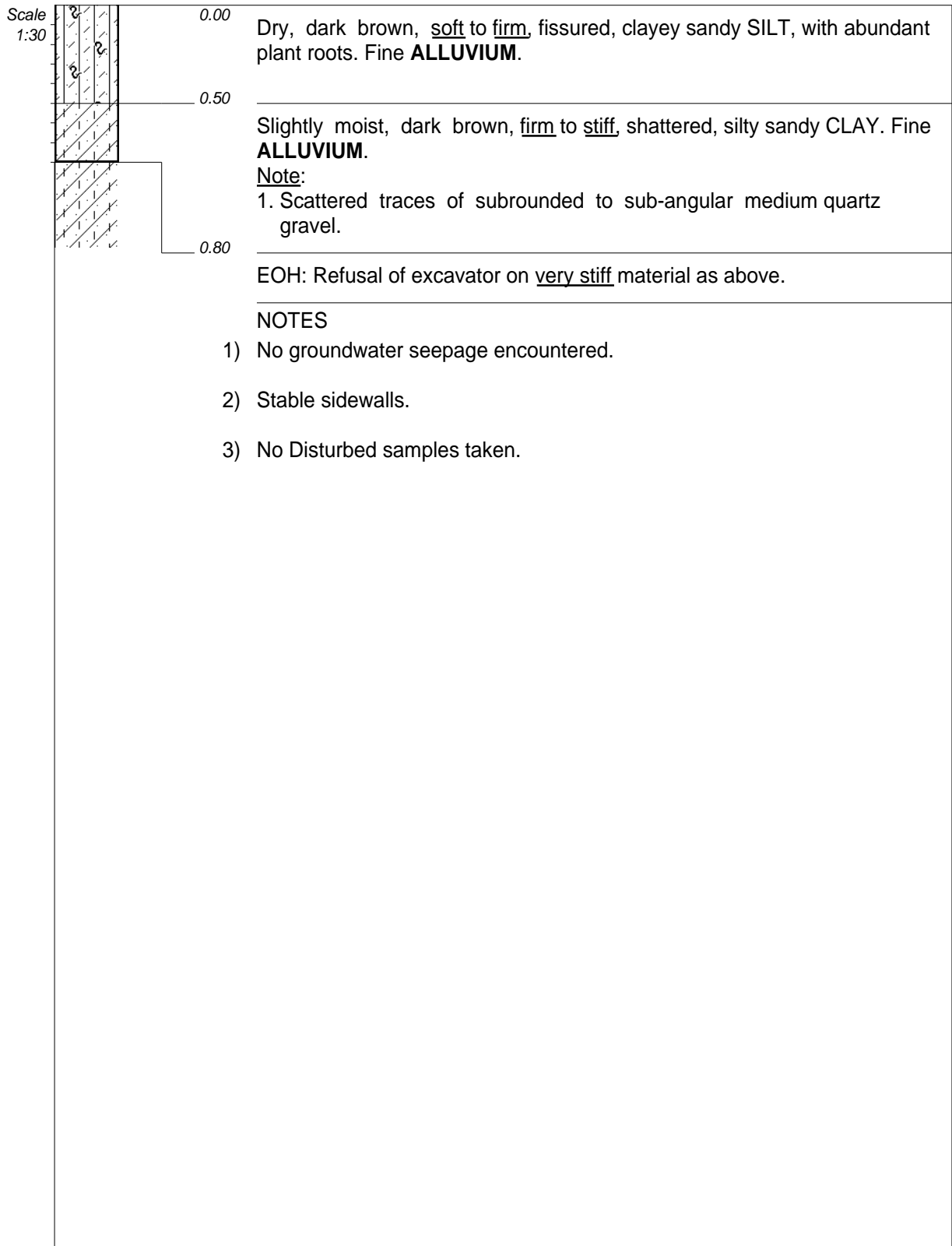
COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 801505.04
 Y-COORD : 7583835.86



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 20 May 2023
DATE : 20 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

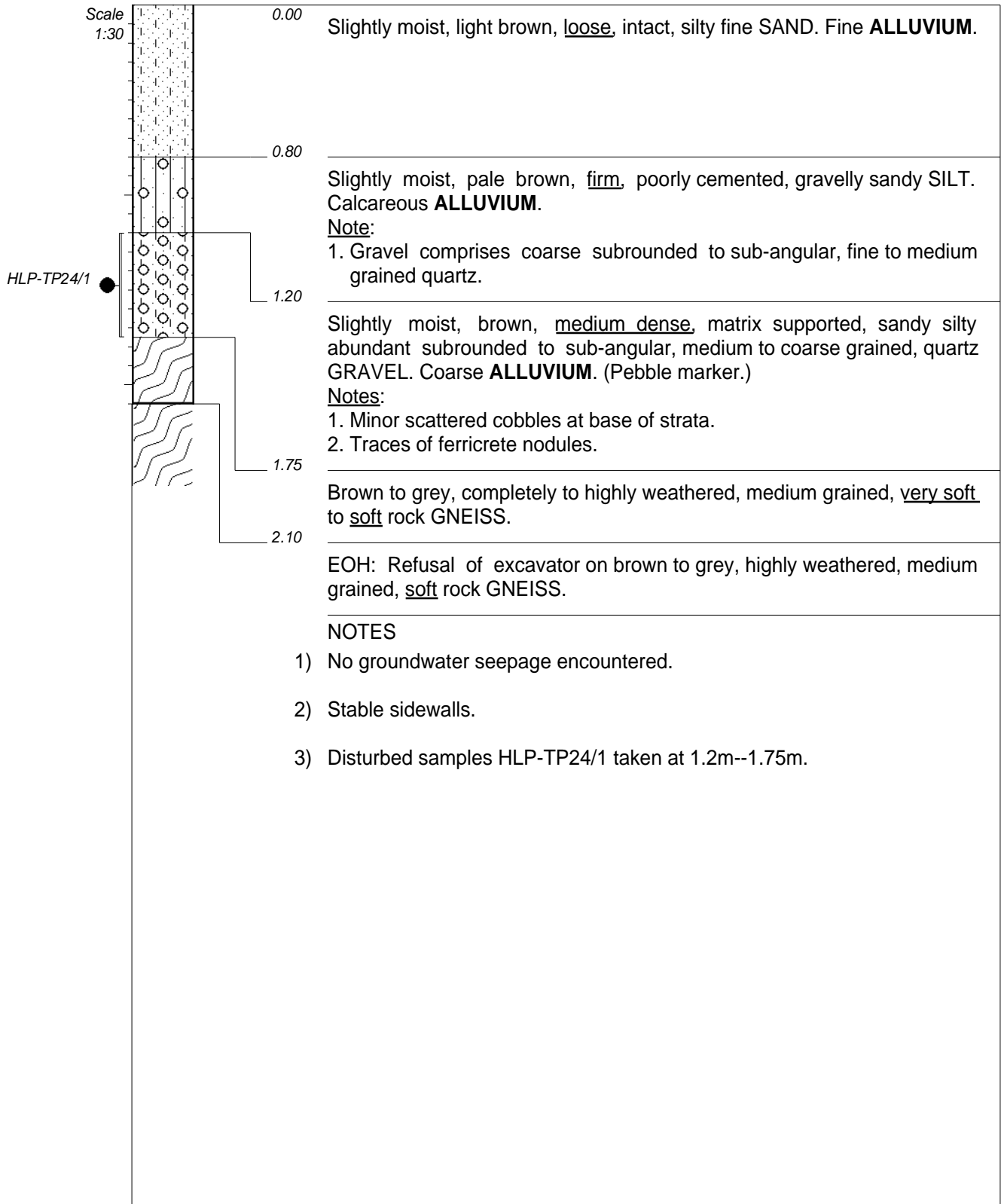
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801964.23
Y-COORD : 7583574.80



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 20 May 2023
DATE : 20 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801808.00
Y-COORD : 7583693.00

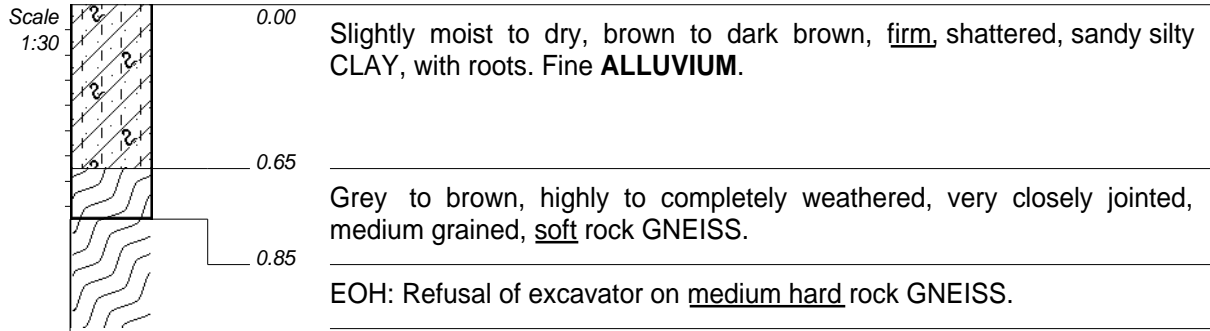


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802094.00
Y-COORD : 7583908.00

HOLE No: HLP-TP24



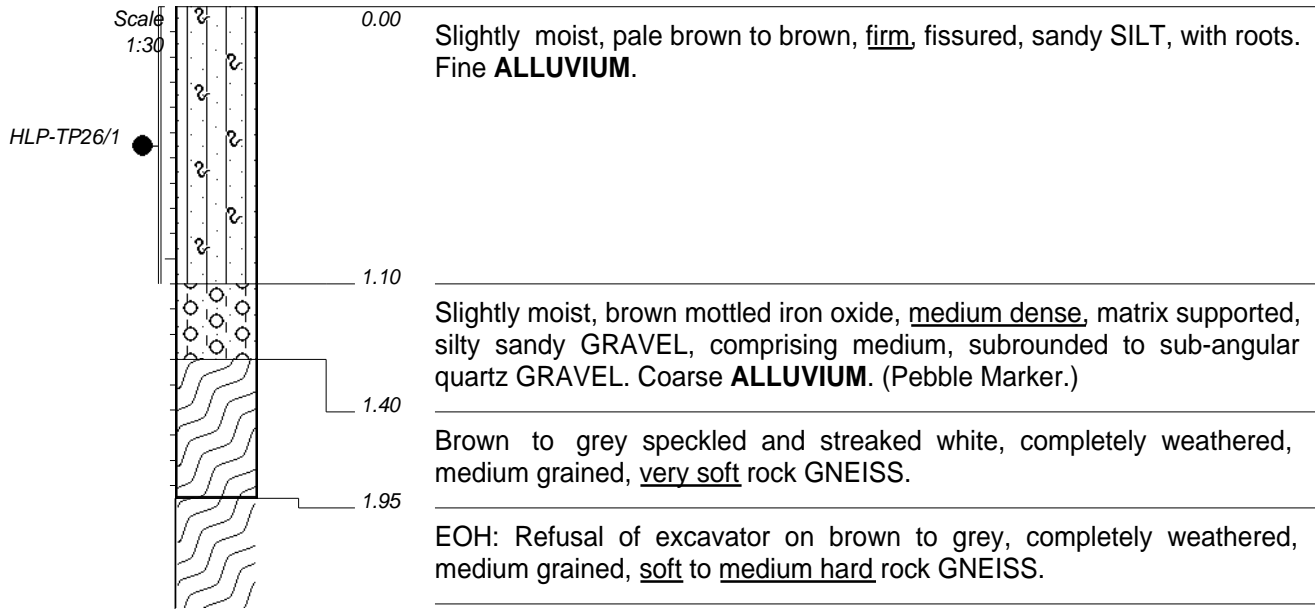
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802527.33
Y-COORD : 7584011.03



NOTES

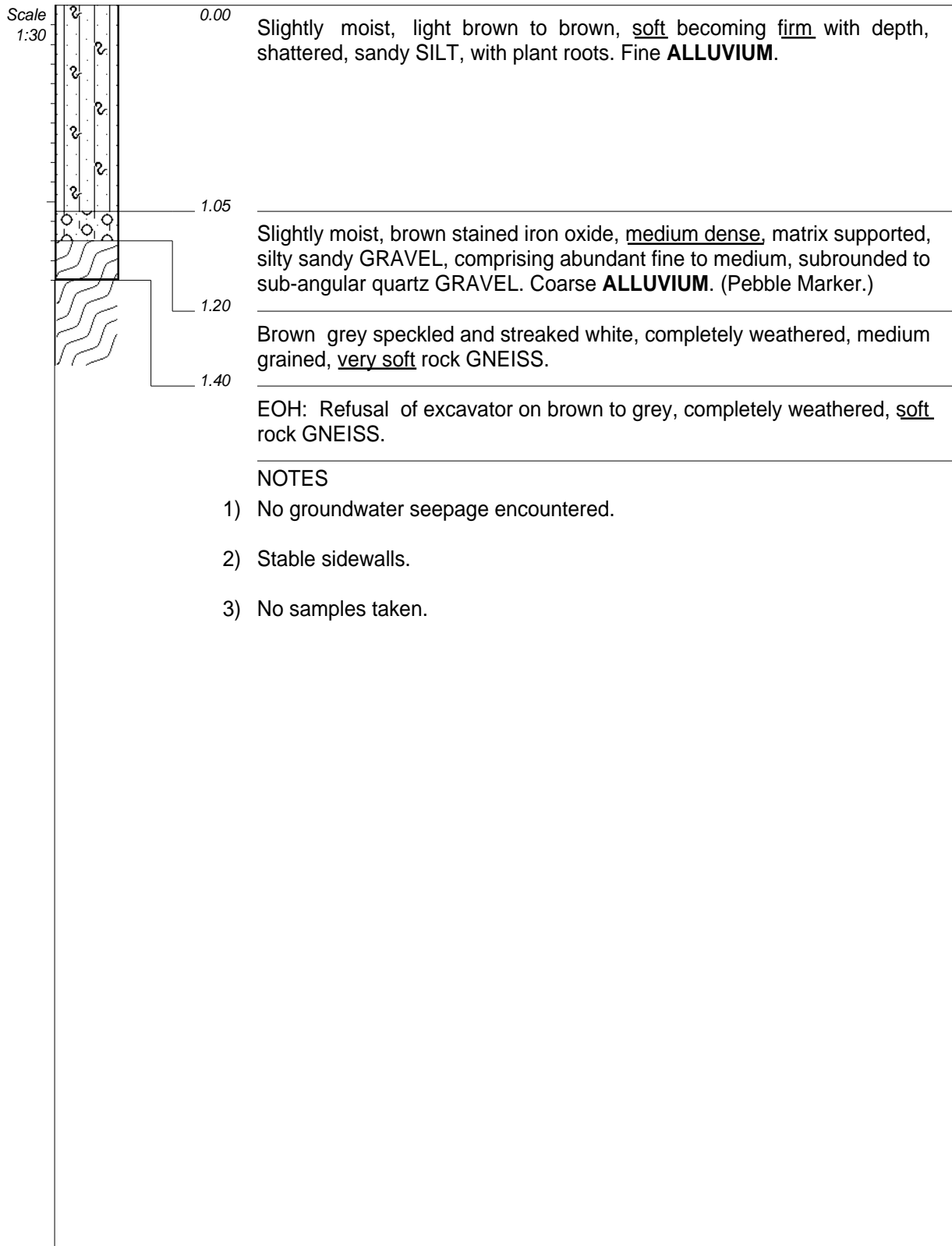
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP26/1 taken at 0.0m--1.1m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802563.00
Y-COORD : 7583862.00

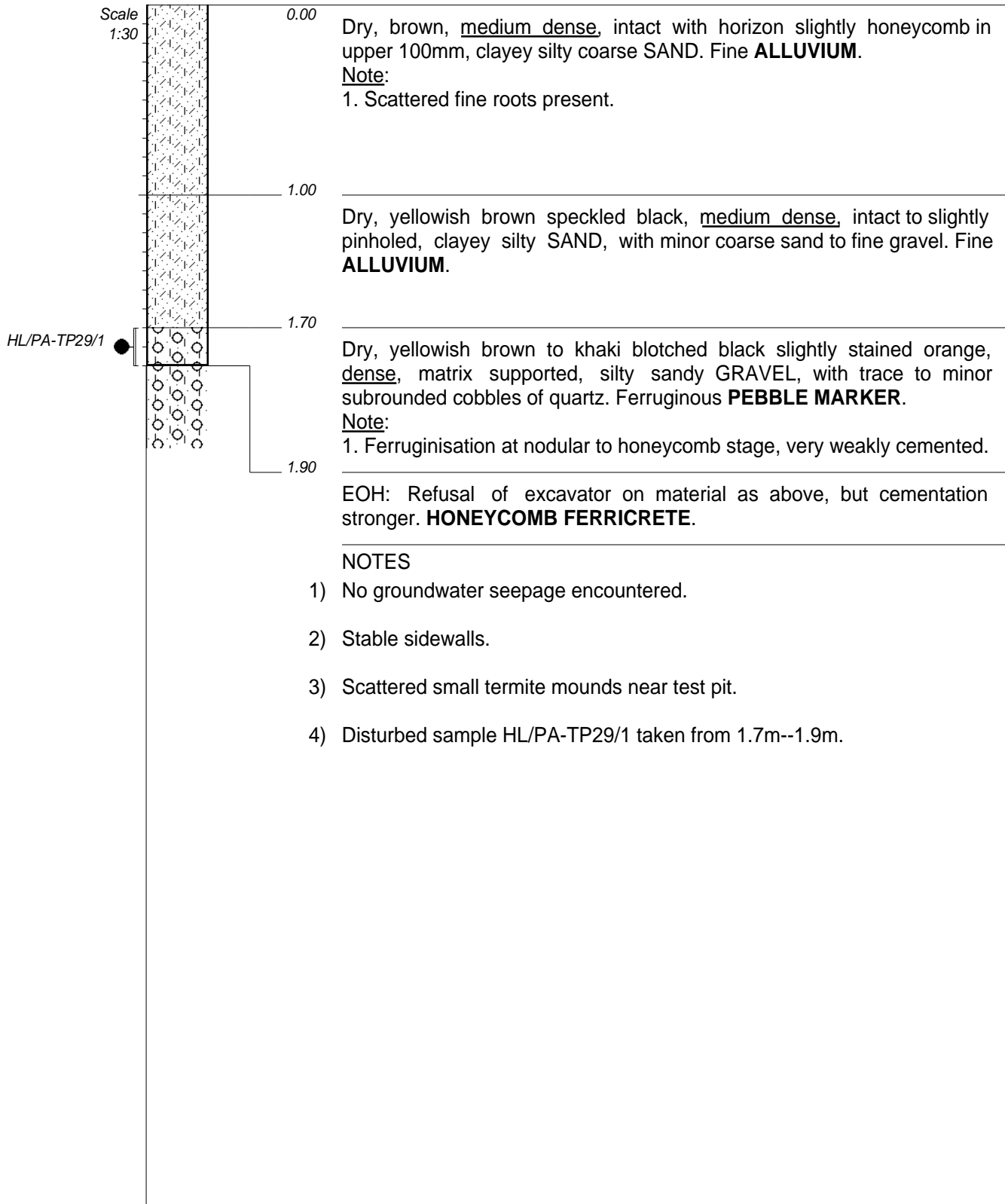
HOLE No: HLP-TP26



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 18 May 2023
DATE : 18 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

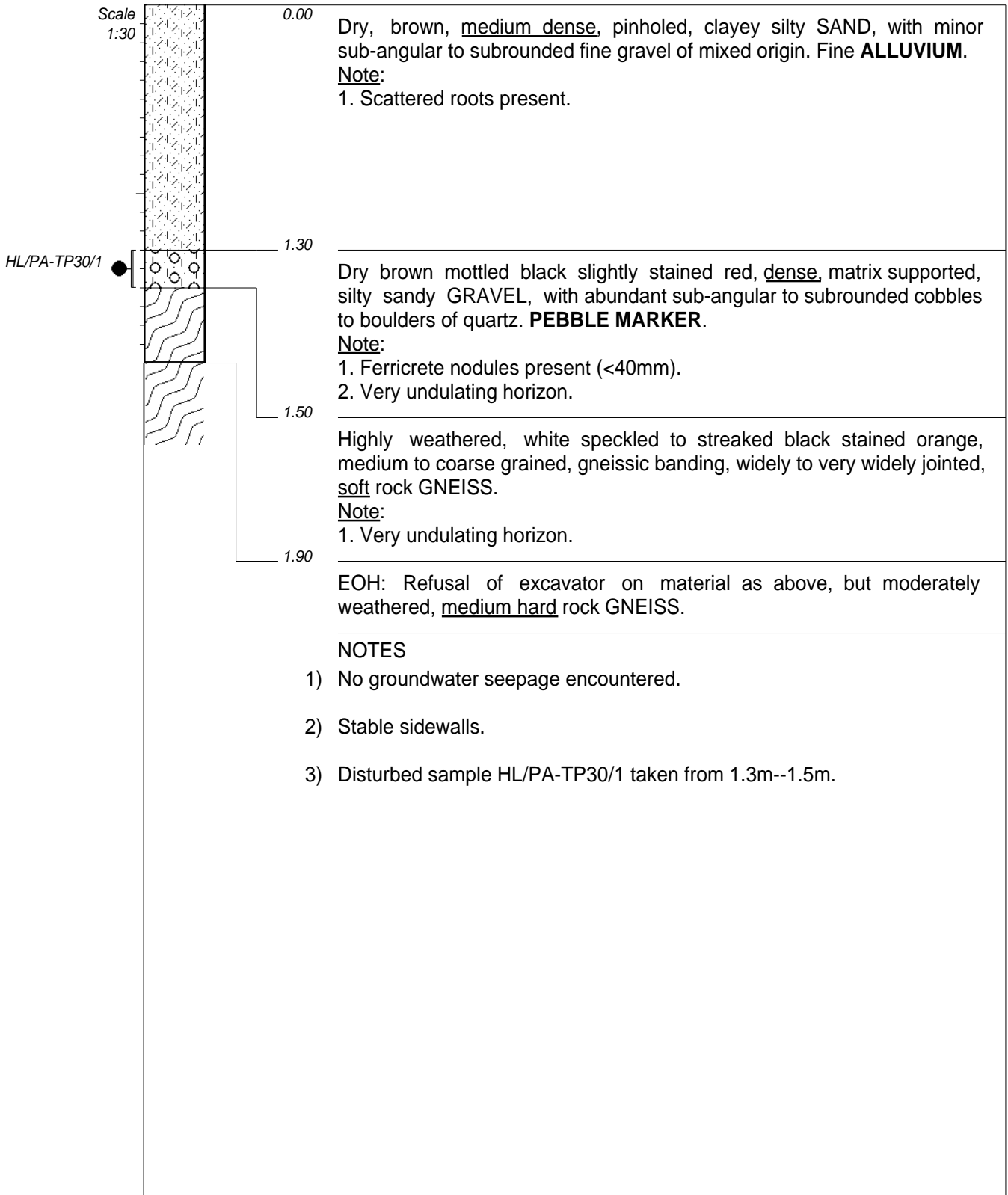
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802323.97
Y-COORD : 7584004.09



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 23 May 2023
DATE : 23 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801863
Y-COORD : 7583191

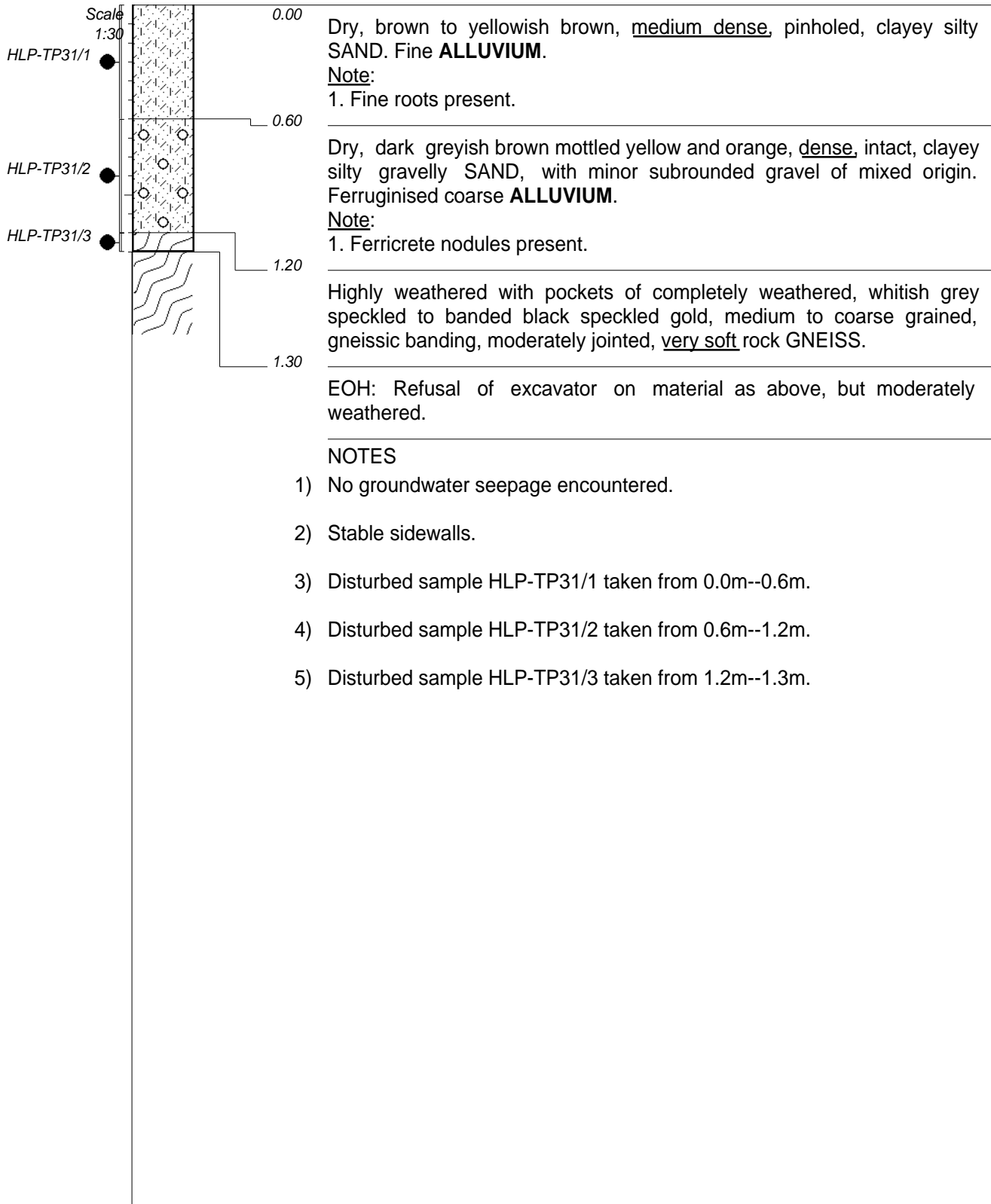


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 25 May 2023
DATE : 25 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0800808
Y-COORD : 7582253

HOLE No: HL/PA-TP30

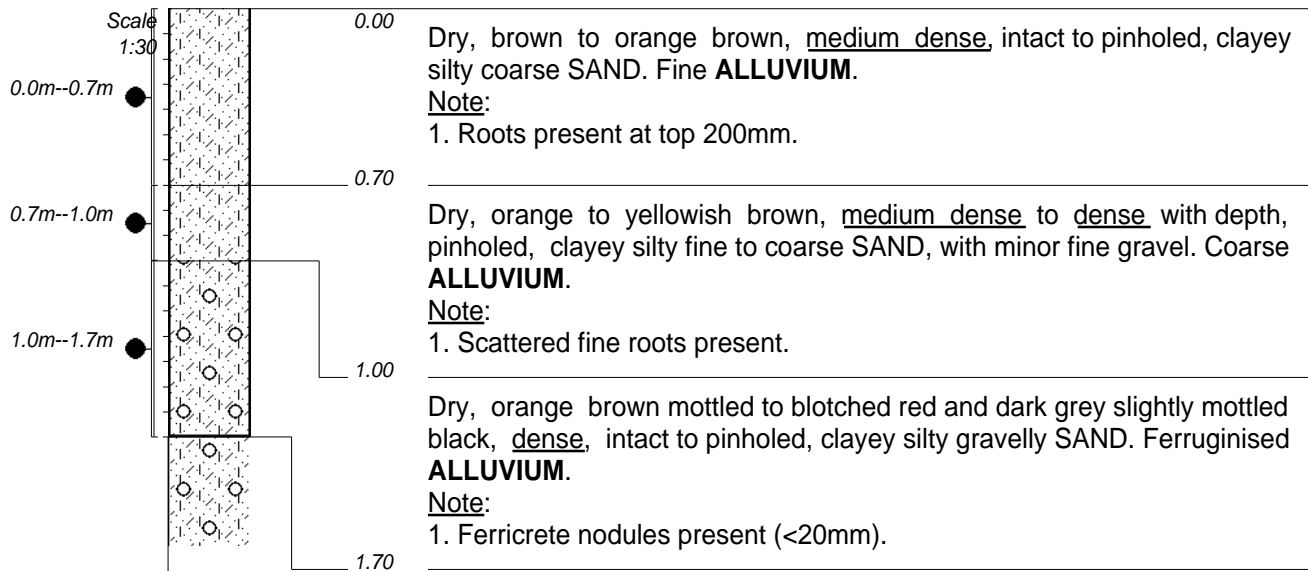


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 23 May 2023
DATE : 23 May 2023
DATE : 04/07/2023 13:44
TEXT : ..51\PROFILES\PKFHLP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0809373
Y-COORD : 7586039

HOLE No: HLP-TP31



EOH: Refusal of excavator due to slow excavation on material as above, but more dense/cemented. Nodular ferruginised **ALLUVIUM**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Sample Plant-TP32/1 taken from 0.0m--0.7m.
- 4) Sample Plant-TP32/2 taken from 0.7m--1.0m.
- 5) Sample Plant-TP32/3 taken from 1.0m--1.7m.

CONTRACTOR :
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan

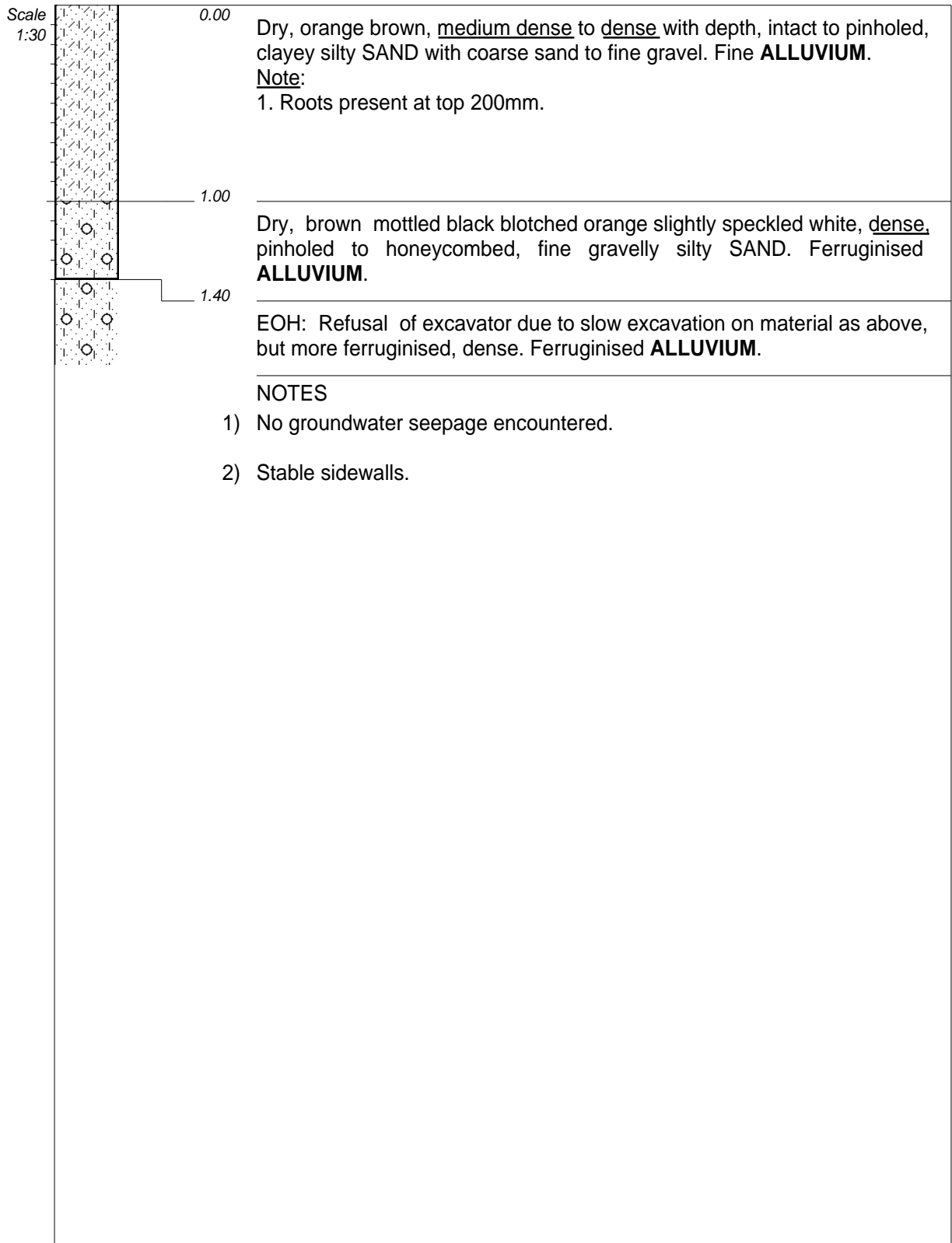
INCLINATION : Vertical
DIAM :
DATE :
DATE : 24 May 2023

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801124
Y-COORD : 7583069

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 10/07/2023 12:40
TEXT : ..\PROFILES\PKFPLANTTP.TXT

HOLE No: Plant-TP32

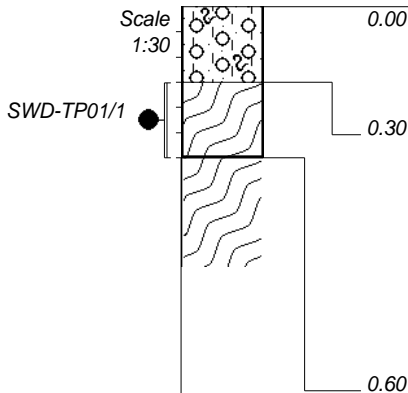


CONTRACTOR :
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE :
DATE : 24 May 2023
DATE : 10/07/2023 12:40
TEXT : ..\PROFILES\PKFPLANTTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801376
Y-COORD : 7583021

HOLE No: Plant-TP33



Dry, brown, dense, matrix supported, silty sandy GRAVEL, with minor sub-angular to subrounded quartz cobbles. Coarse **COLLUVIUM**, with roots. Interpreted as surface pebble marker.

Highly weathered, white speckled and streaked black stained orange, medium to coarse grained, gneissic banding, closely jointed, soft to medium hard rock with depth, GNEISS.

Note:

- 1 Very undulating horizon.
2. Brown, clayey silty sand infilling on joints, <2mm thick with occasional pockets in places.

EOH: Refusal of excavator on material as above, but moderately weathered, medium hard rock GNEISS.

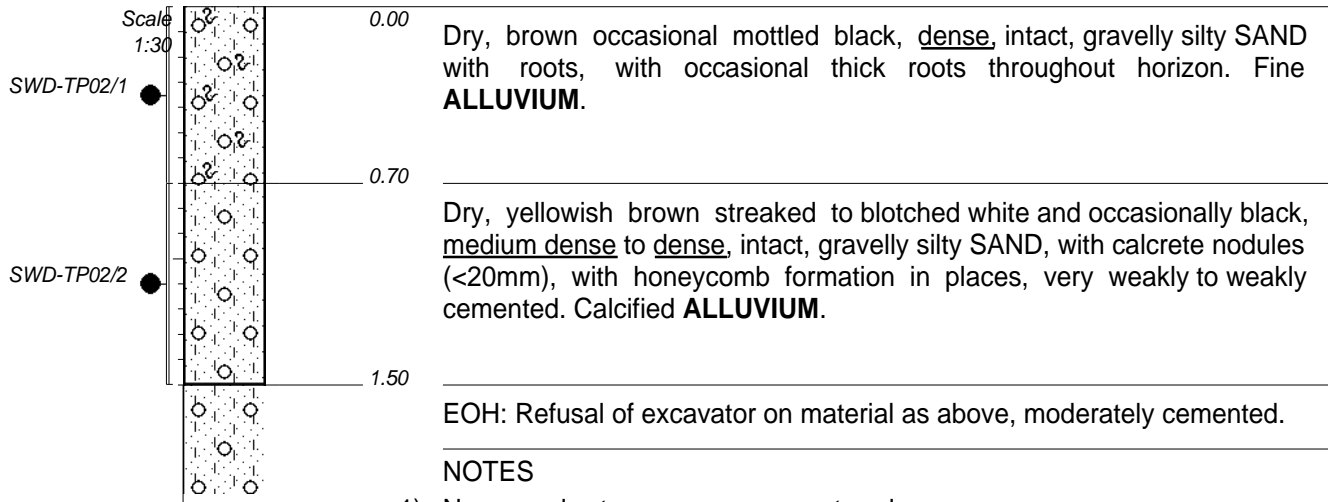
NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP01/1 taken at 0.3m--0.6m.

CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 25 May 2023
DATE : 25 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0800870
Y-COORD : 7582003



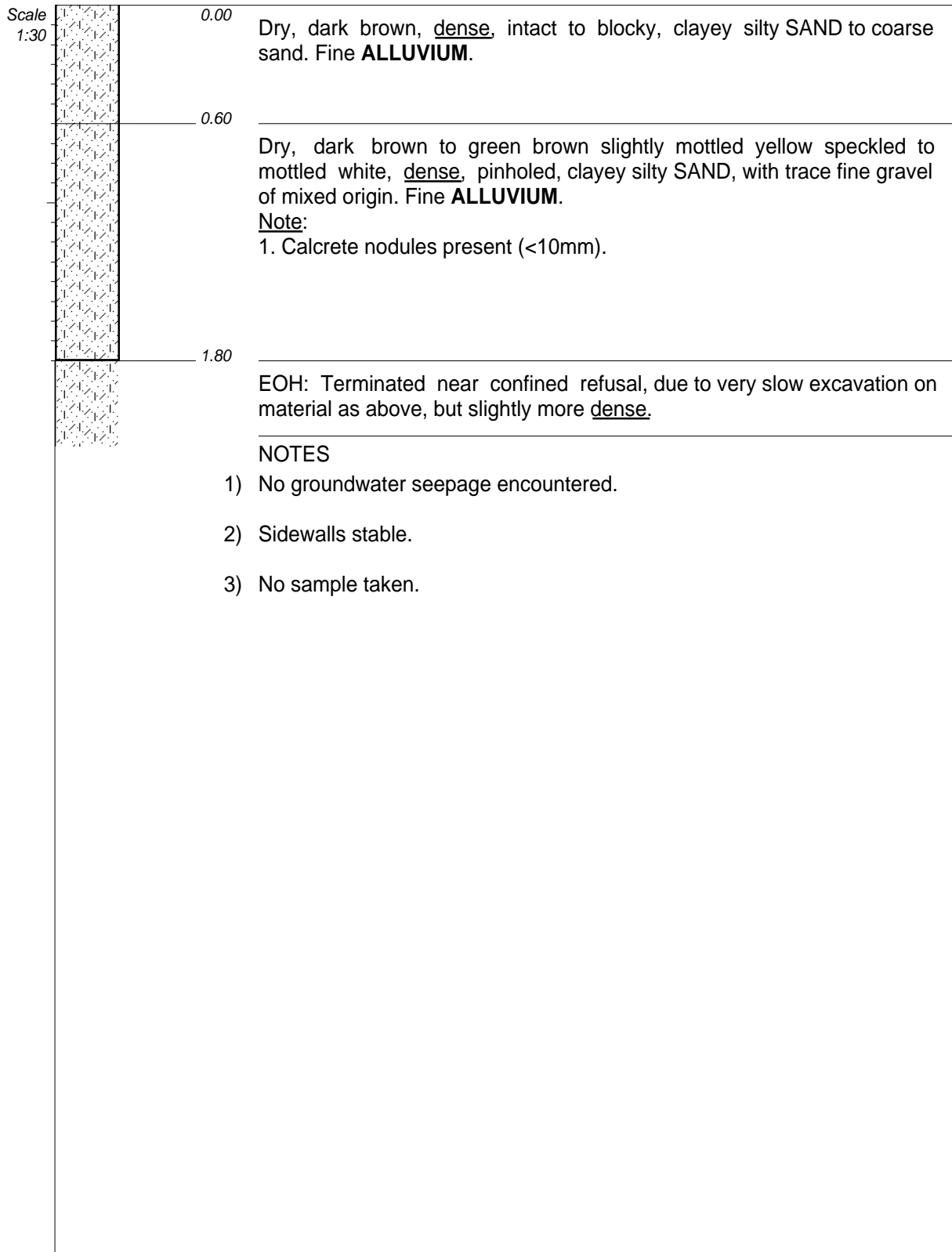
NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP02/1 taken at 0.0m--0.7m.
- 4) Disturbed sample SWD-TP02/2 taken at 0.7m--1.5m.

CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : C Homan
 TYPE SET BY : EM
 SETUP FILE : KTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : 24 May 2023
 DATE : 24 May 2023
 DATE : 07/07/2023 14:15
 TEXT : ..51\PROFILES\PKFSWDTP.TXT

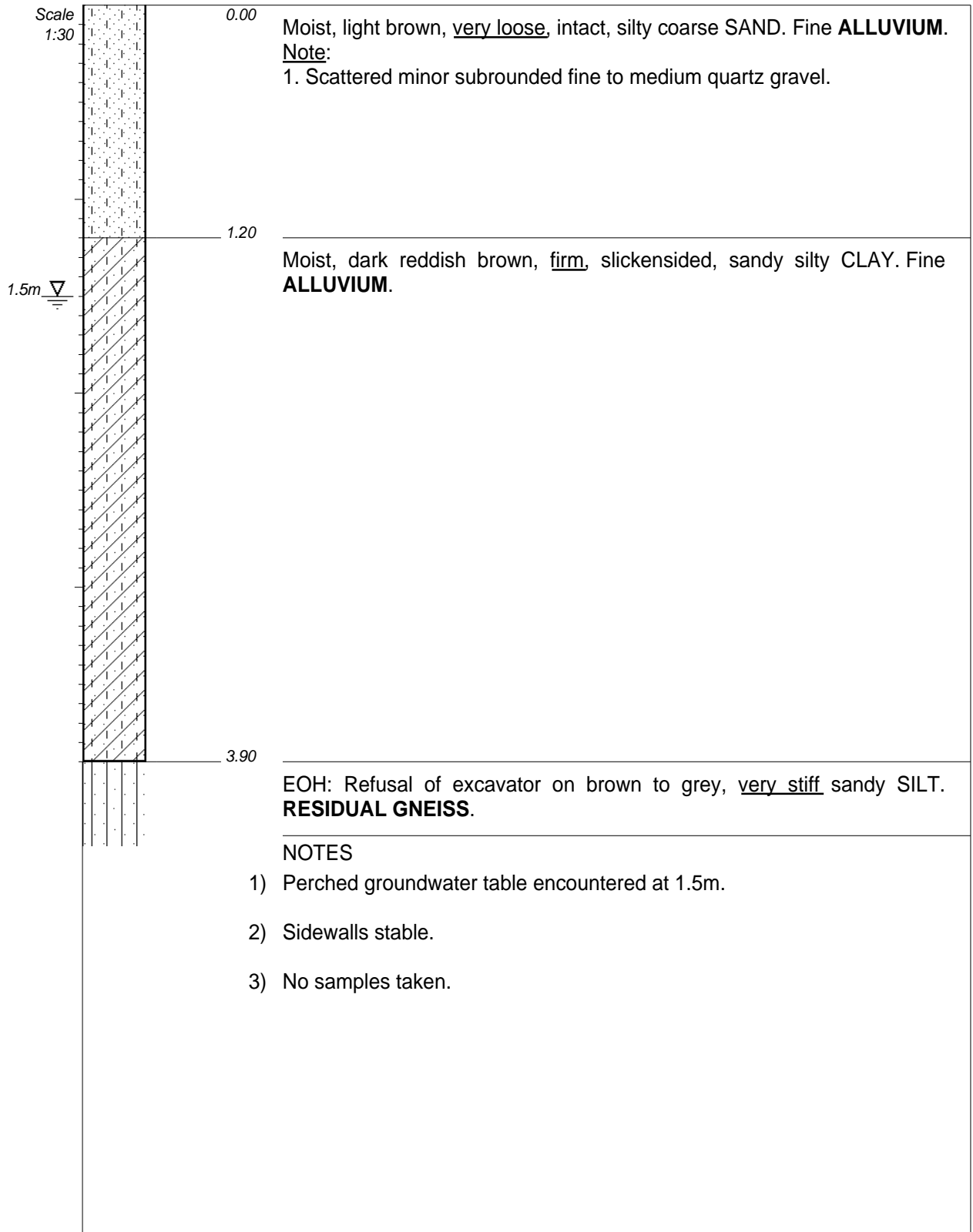
COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 801049
 Y-COORD : 7582030



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 24 May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

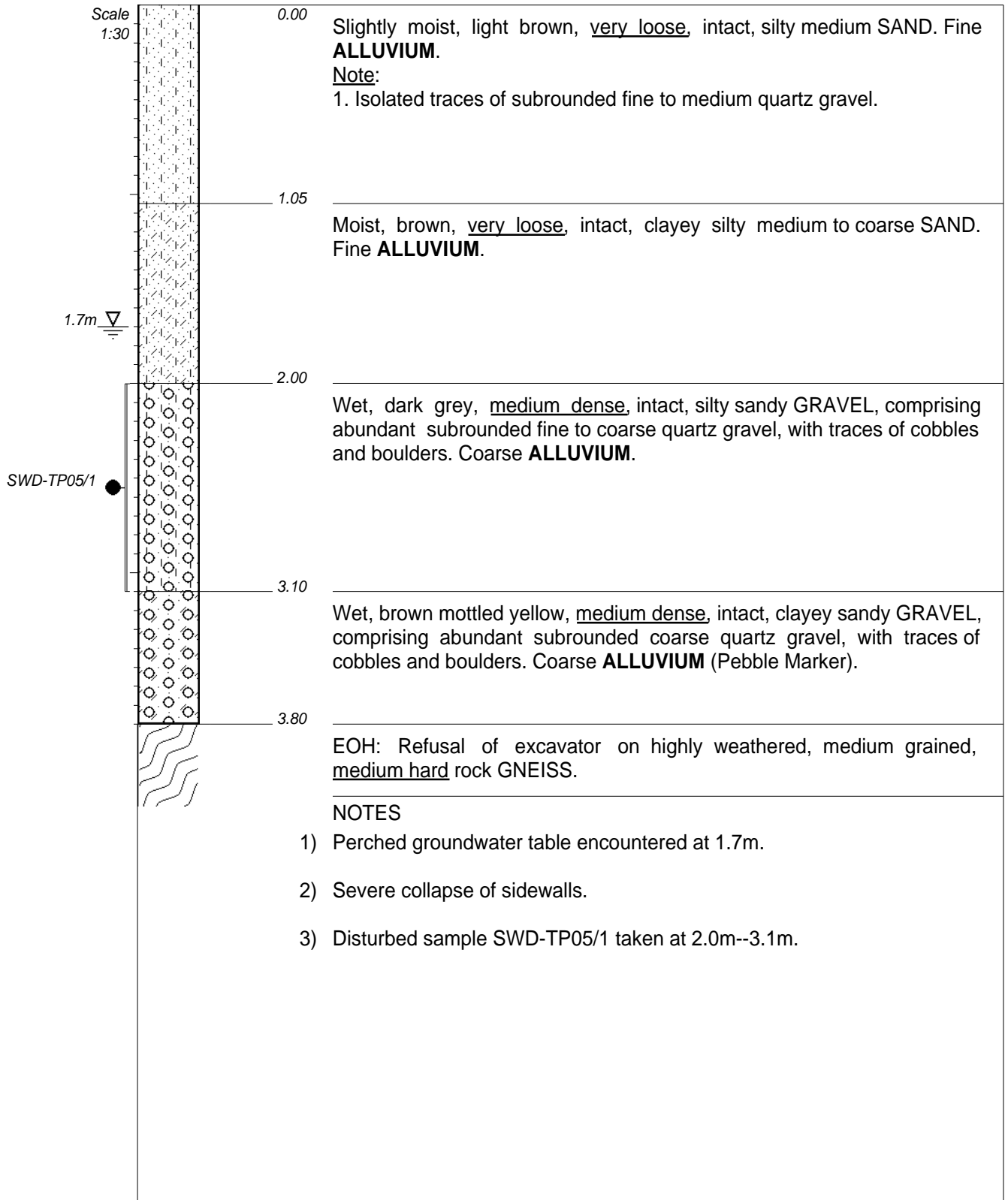
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801151
Y-COORD : 7581871



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKF5WDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801747
Y-COORD : 7581888

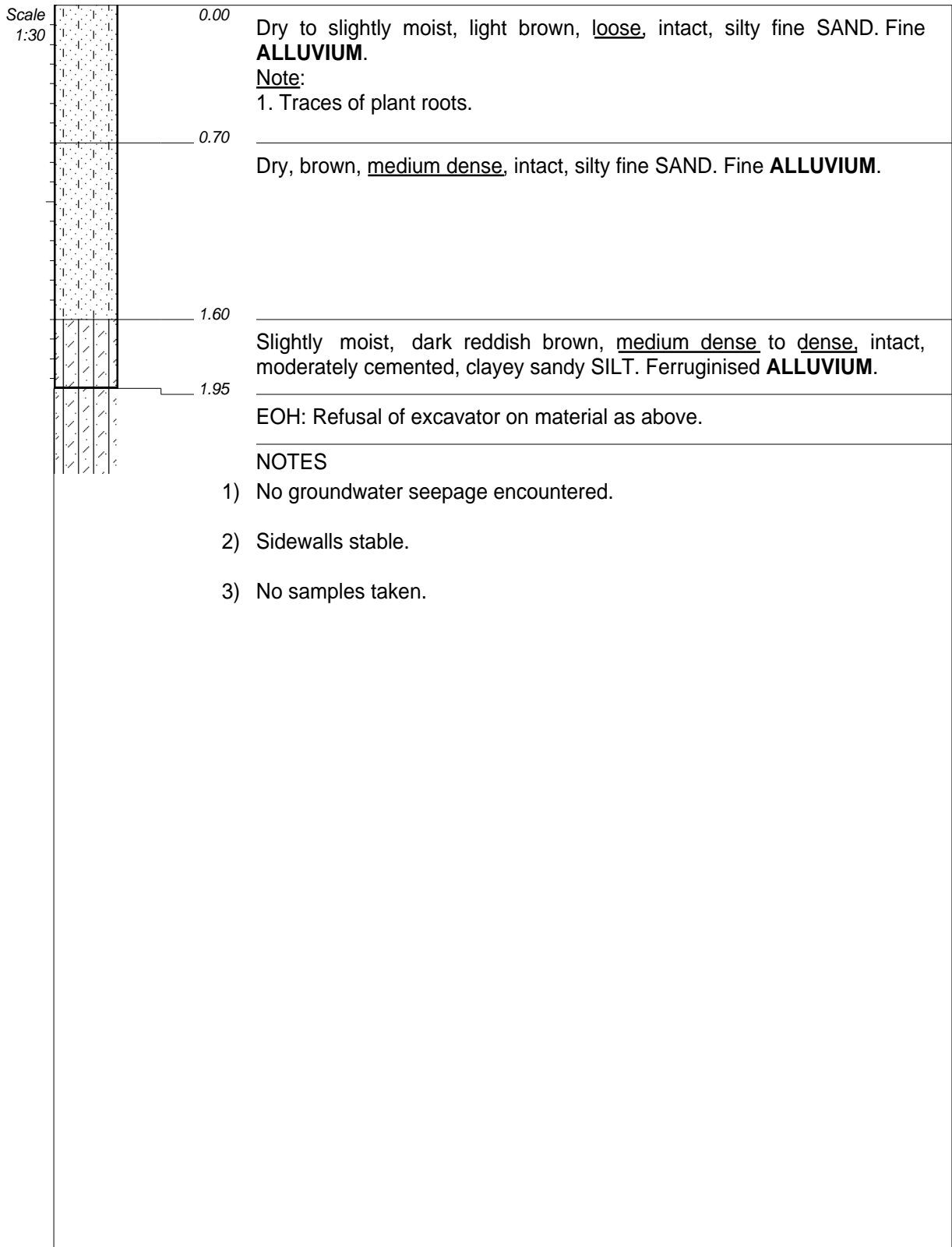


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKF5WDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801539
Y-COORD : 7581592

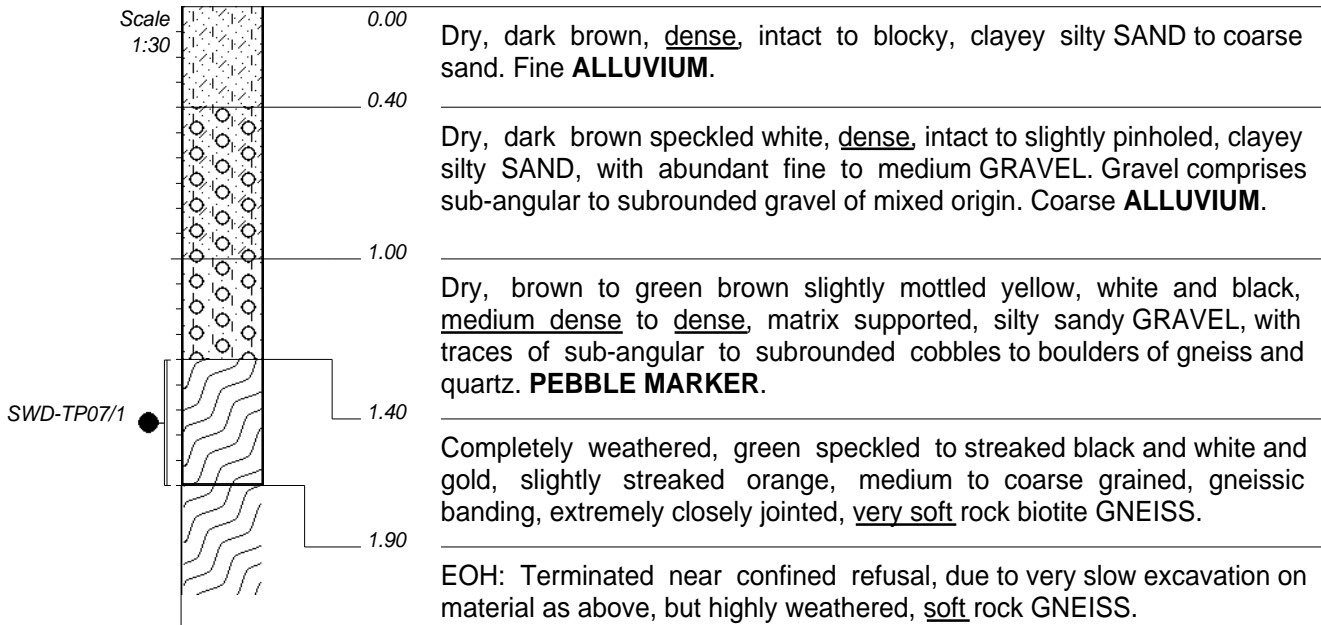
HOLE No: SWD-TP05



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801555
Y-COORD : 7581382



NOTES

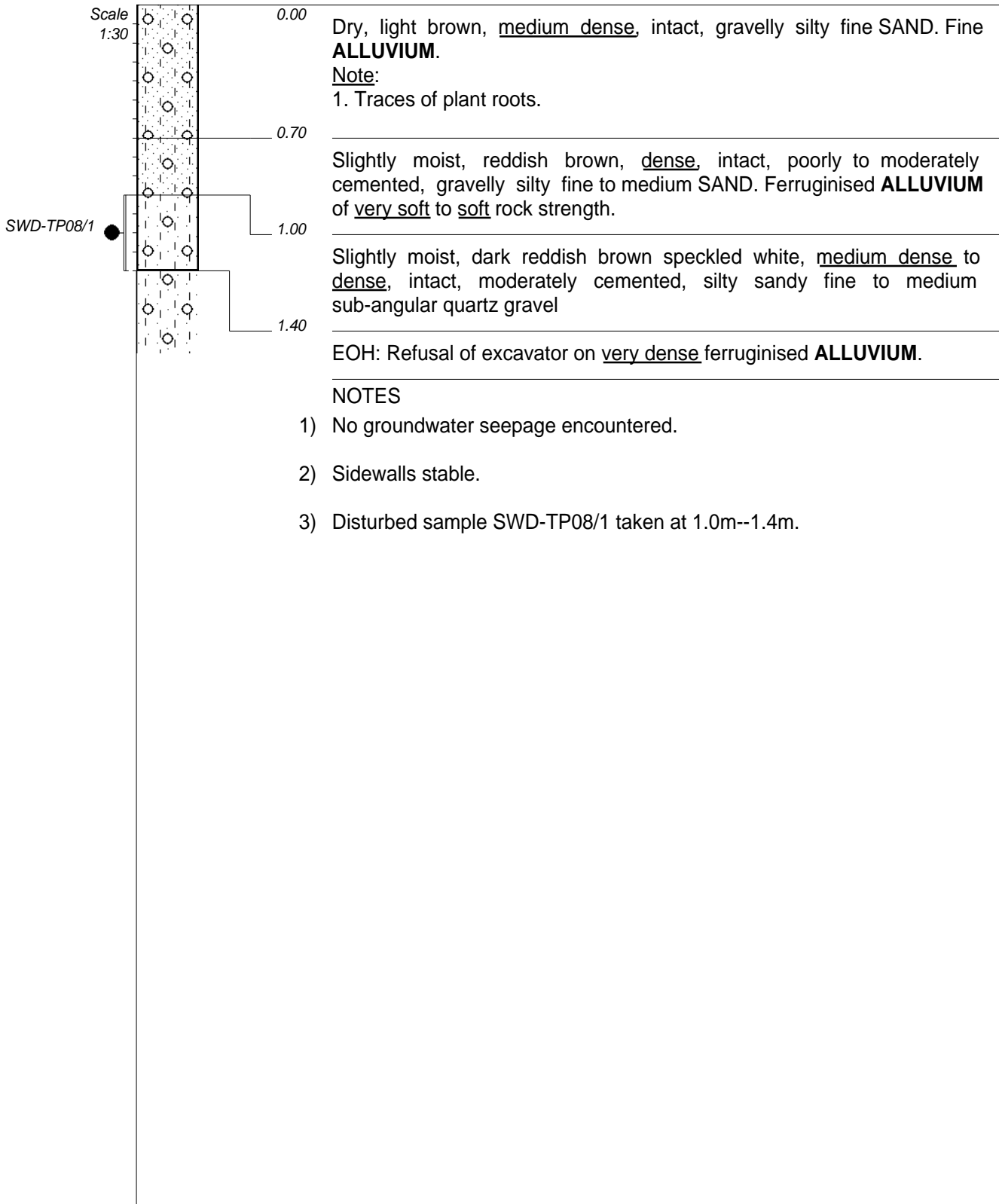
- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP07/1 taken at 1.4m--1.9m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 24 May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : k33 0801373
Y-COORD : 7581928

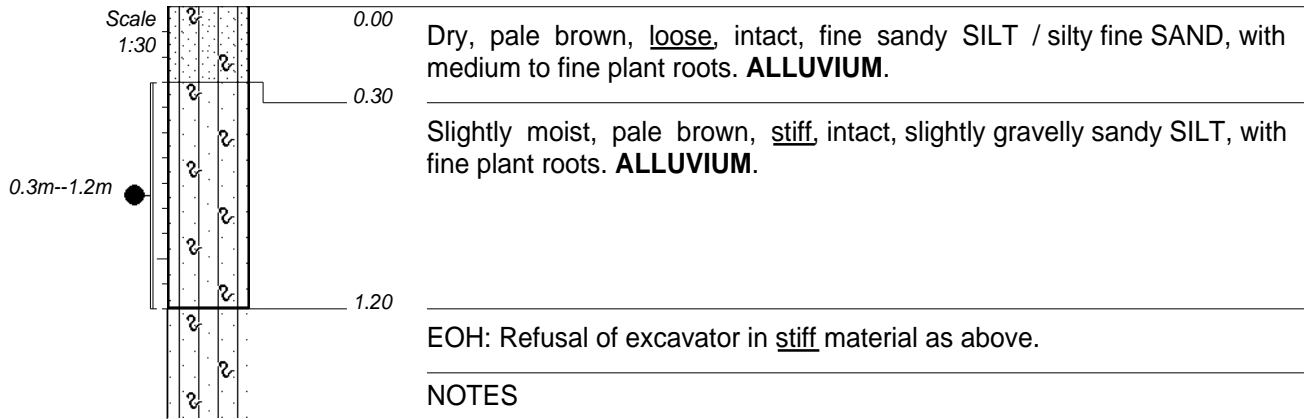
HOLE No: SWD-TP07



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801661
Y-COORD : 7581854



EOH: Refusal of excavator in stiff material as above.

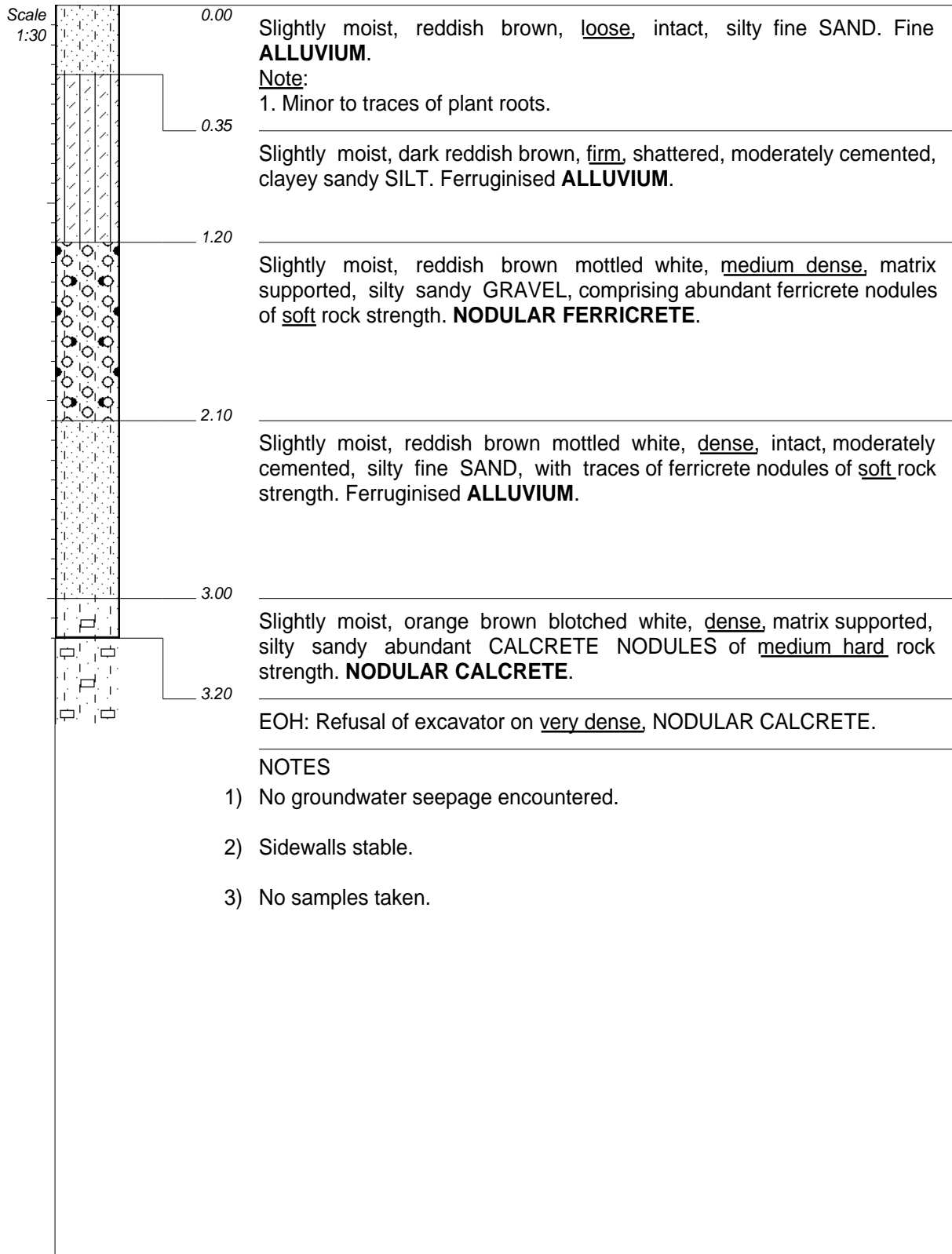
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample taken from 0.3m--1.2m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 10 May 2023
DATE : 10 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSDWTP.TXT

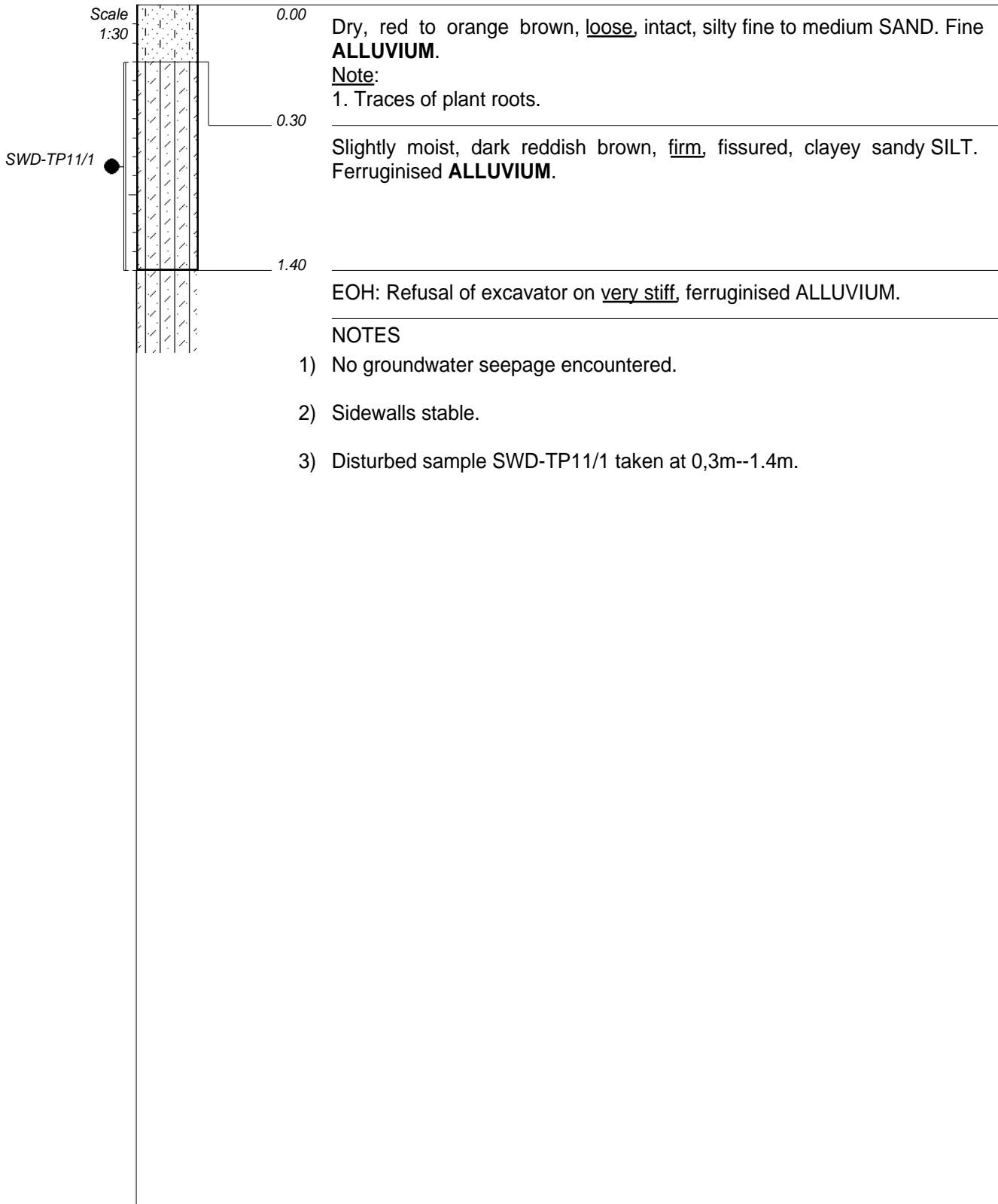
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801754
Y-COORD : 7581825



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802111
Y-COORD : 7581662

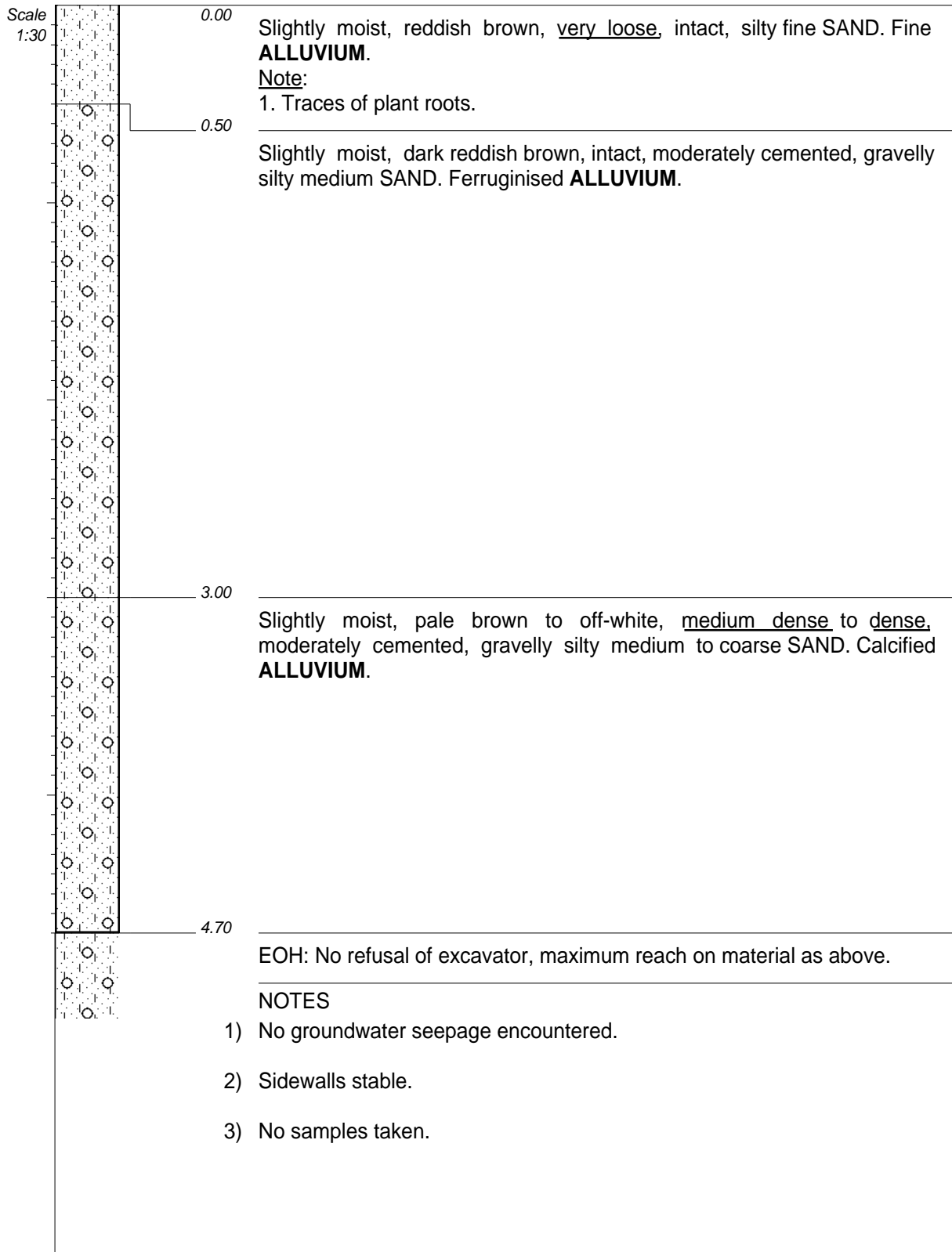


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802507
Y-COORD : 7581650

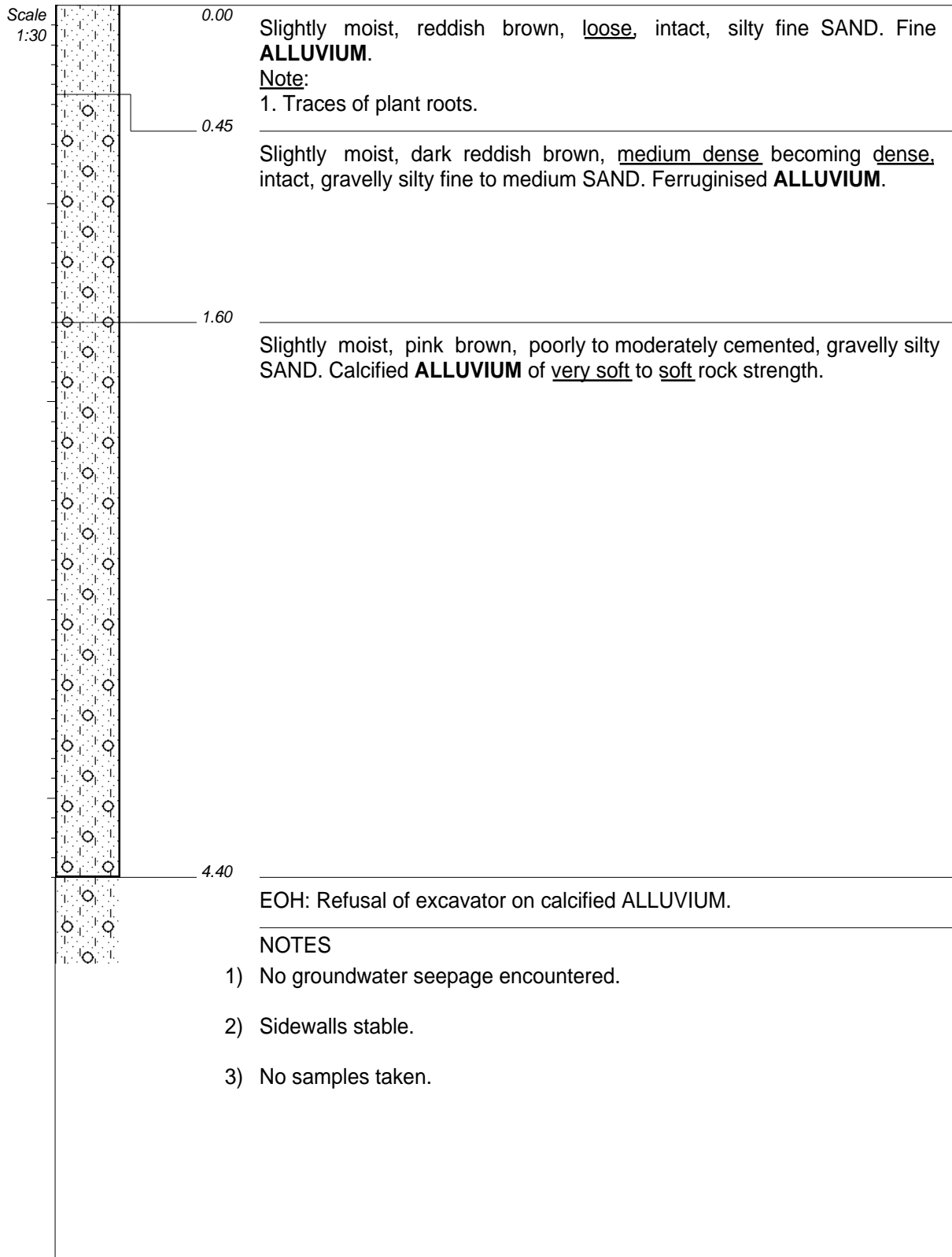
HOLE No: SWD-TP11



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSDWTP.TXT

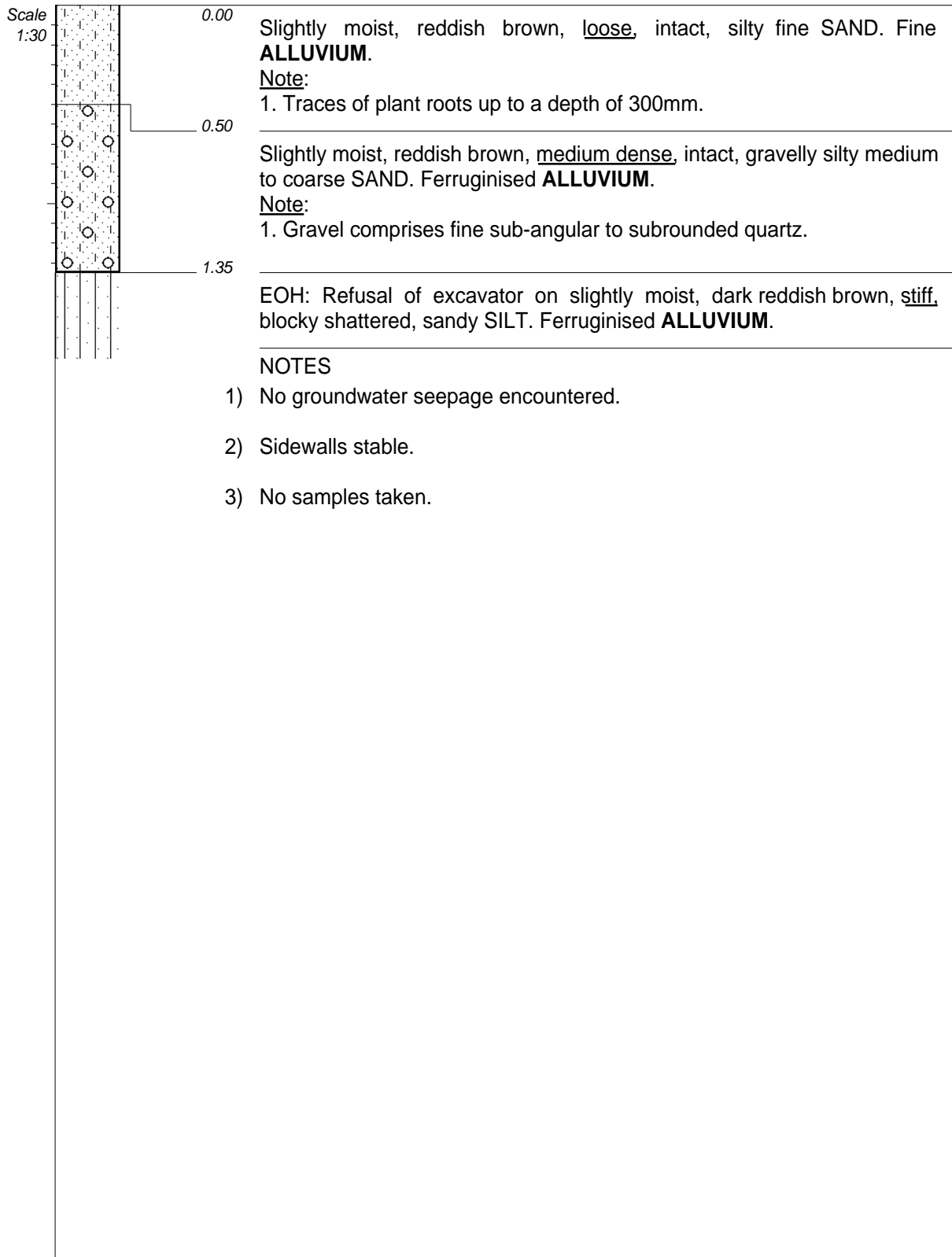
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802880
Y-COORD : 7581470



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSDWTP.TXT

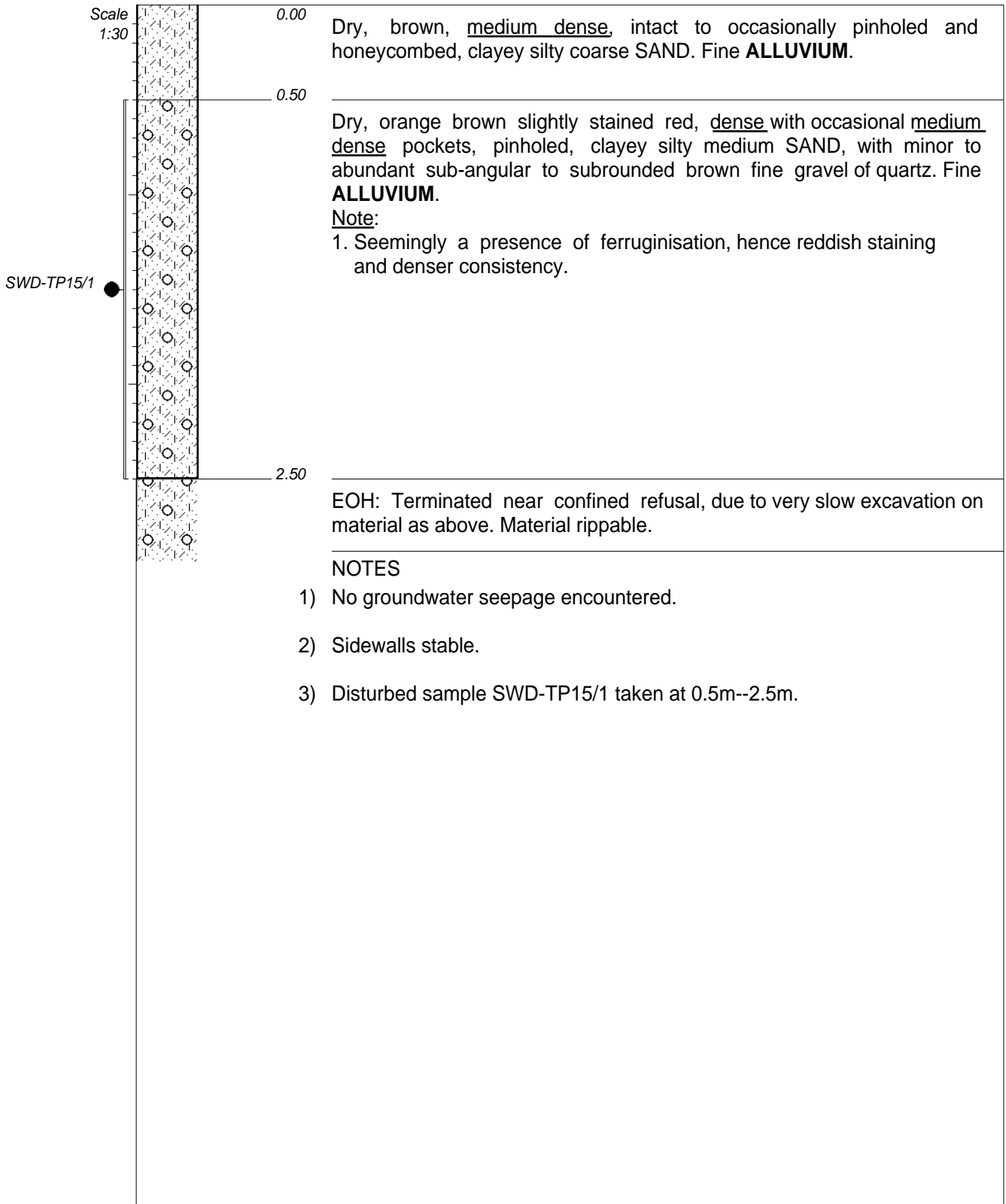
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803174
Y-COORD : 7581550



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSDTTP.TXT

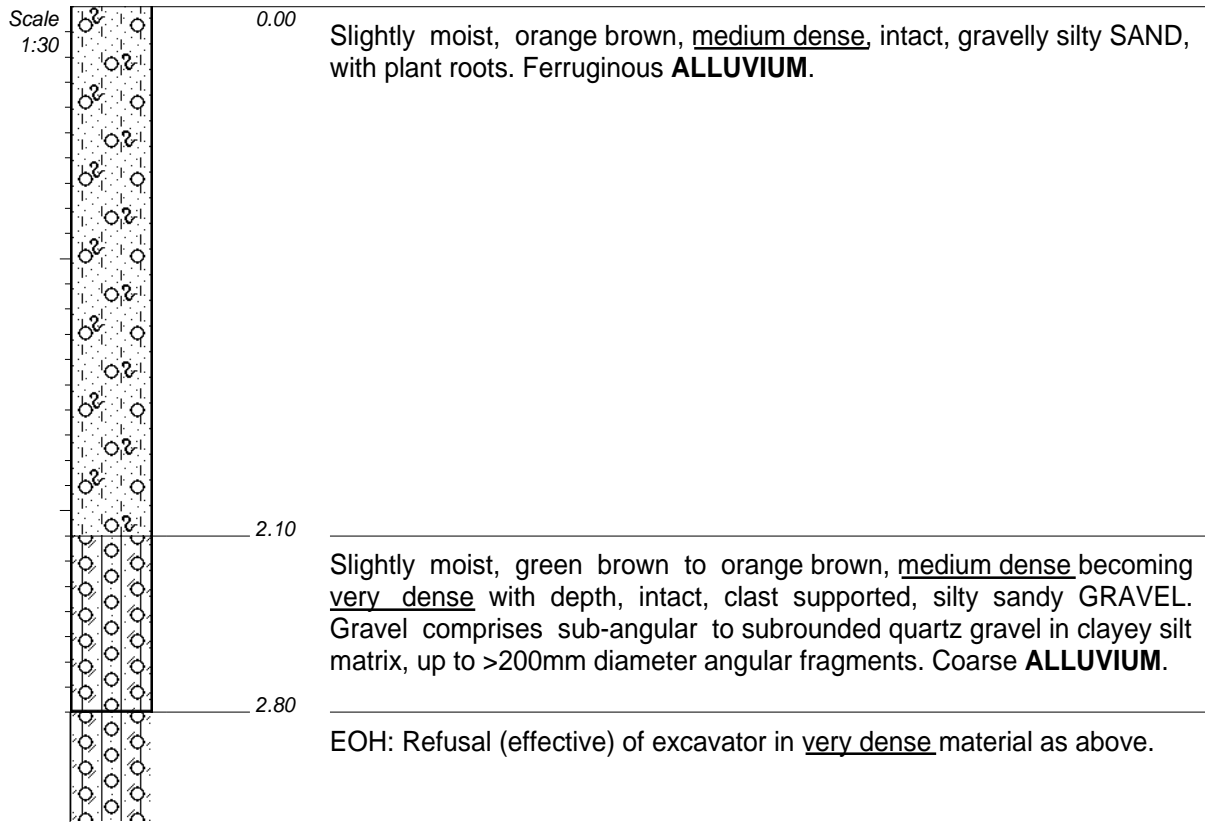
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803514
Y-COORD : 7581781



CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 25 May 2023
DATE : 25 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0803848
Y-COORD : 7581953



PLEASE CHECK THIS HOLE. I DON'T UNDERSTAND WHAT YOU WANT. ALSO CH TO ASK BK?

NOTES

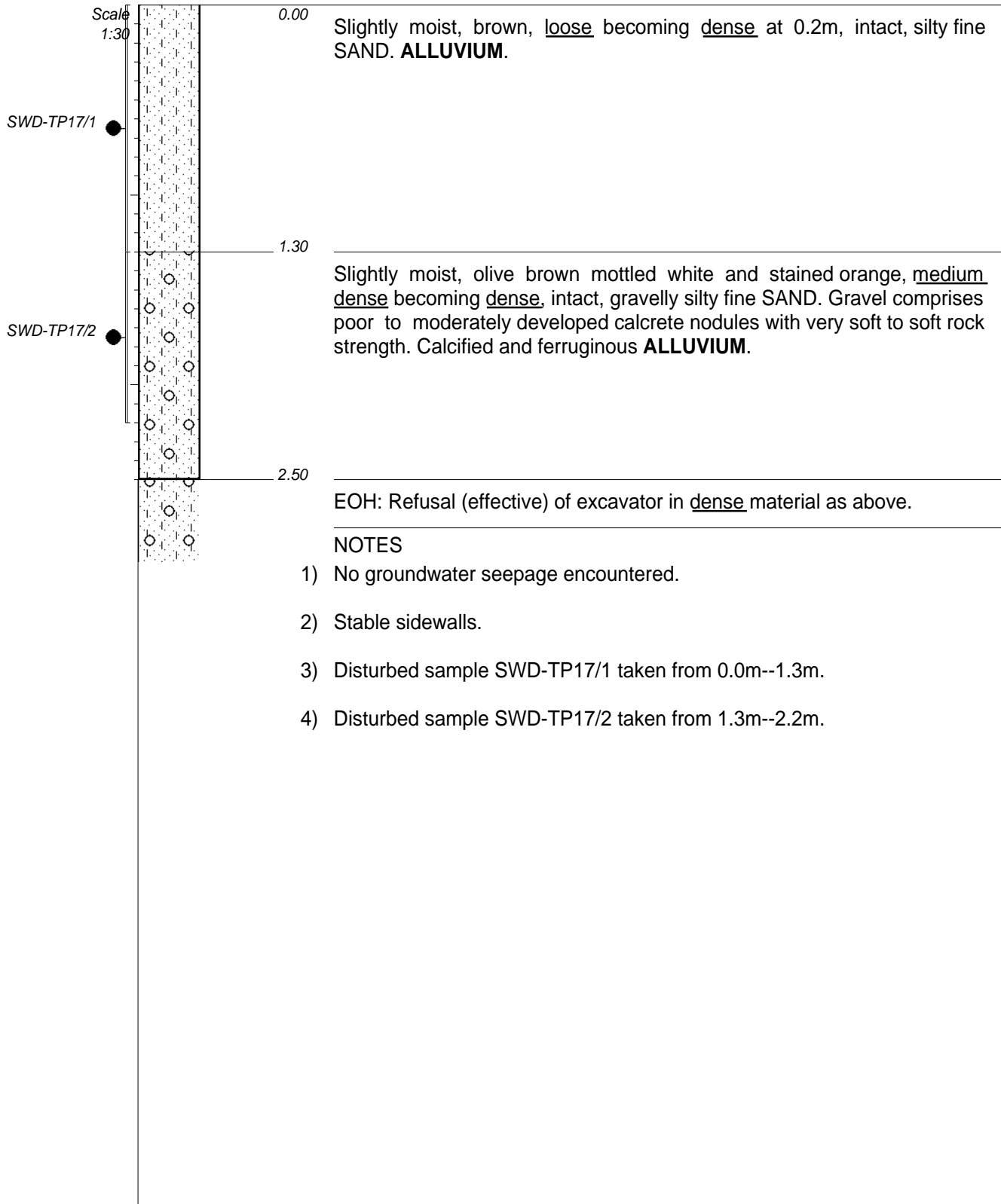
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 11 May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803965
Y-COORD : 7582214

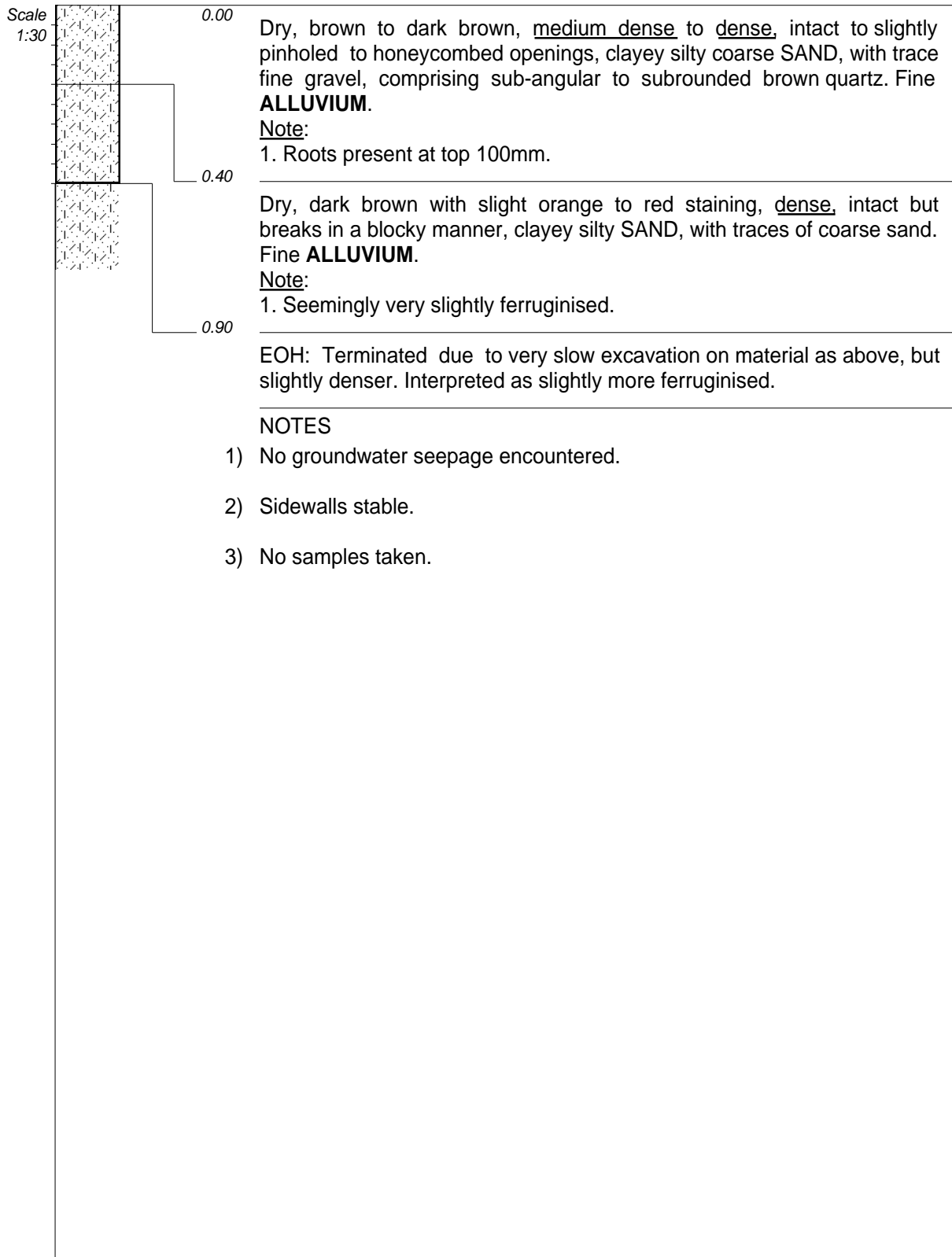
HOLE No: SWD-TP16



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 11 May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

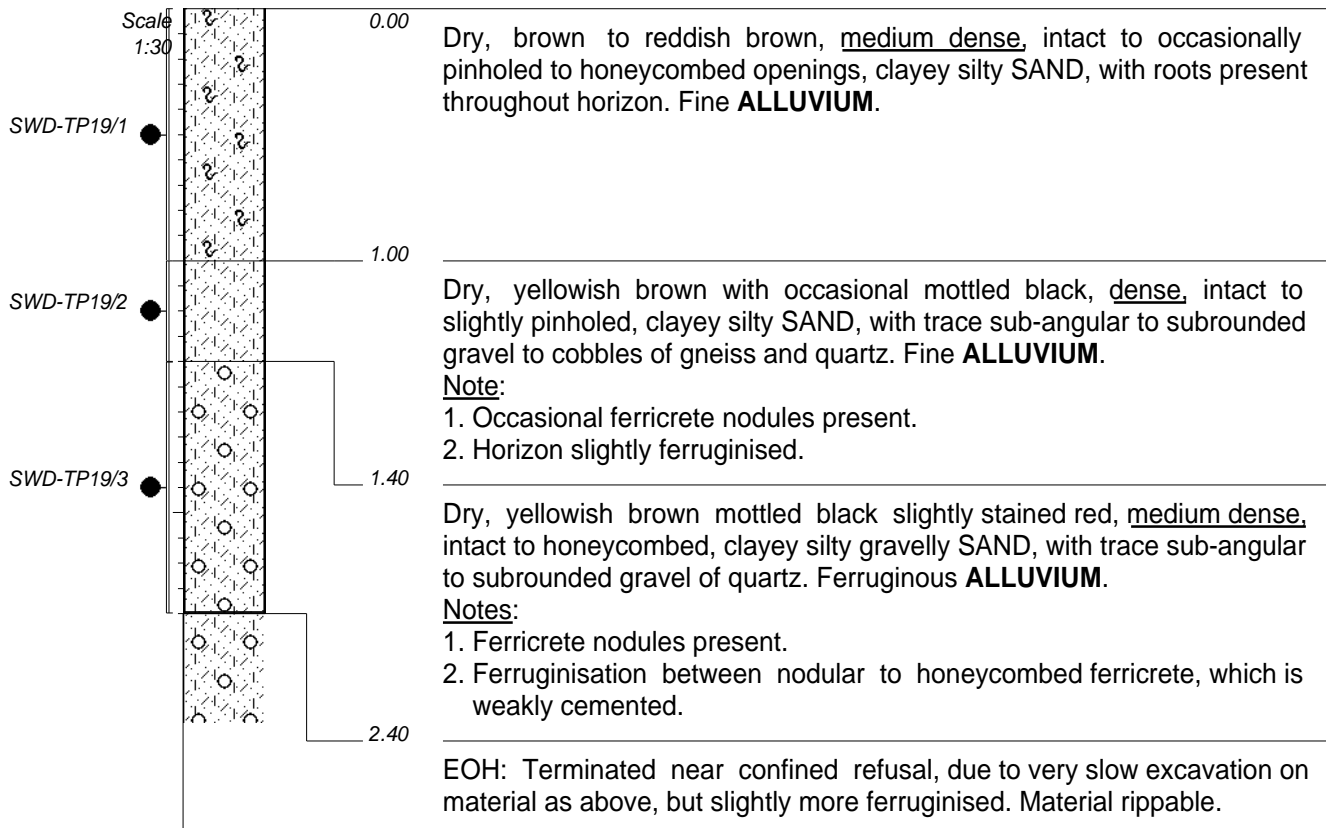
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804269
Y-COORD : 7582600



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 26 May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0804380
Y-COORD : 7582887



NOTES

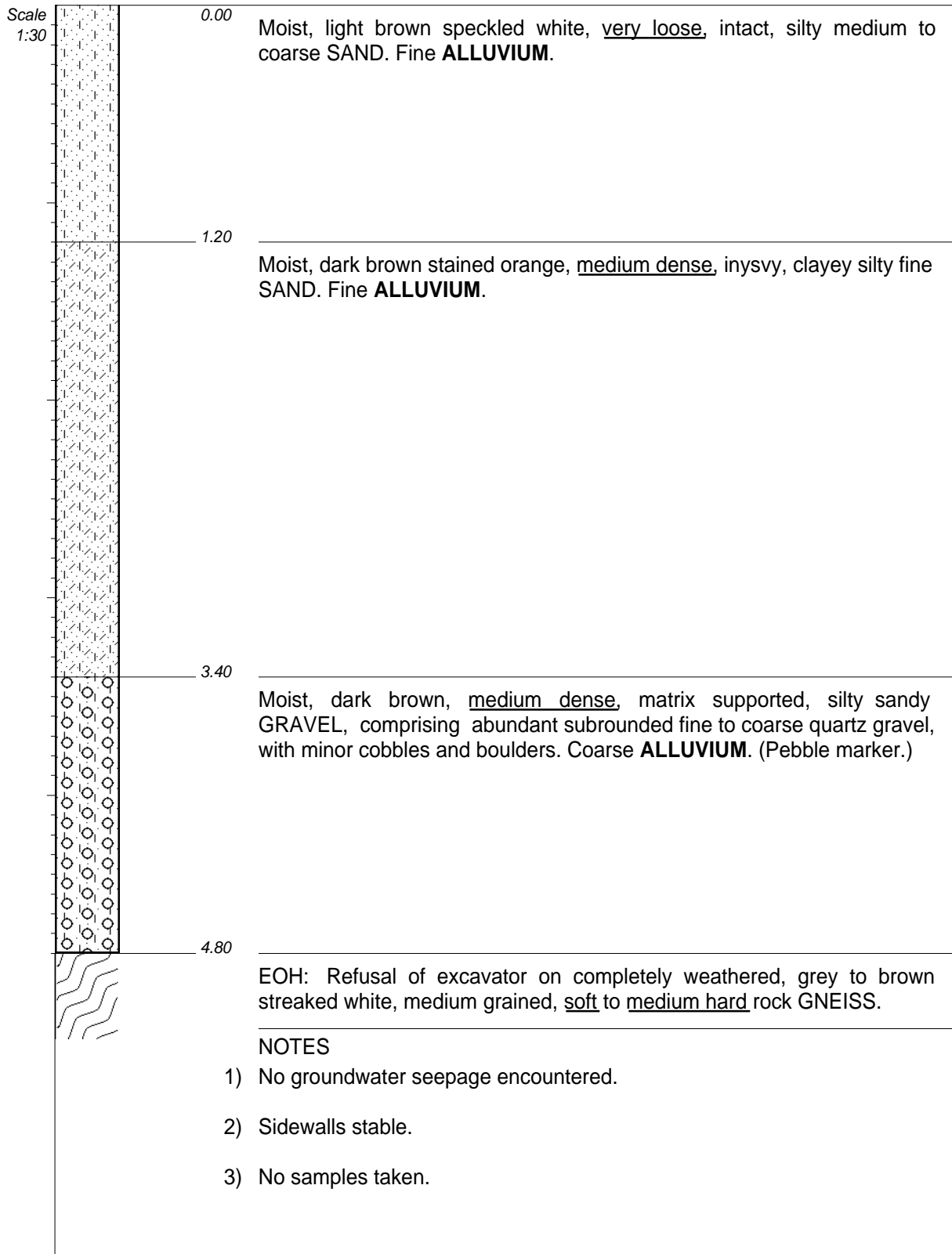
- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP19/1 taken at 0.0m--1.0m.
- 4) Disturbed sample SWD-TP19/2 taken at 1.0m--1.4m.
- 5) Disturbed sample SWD-TP19/3 taken at 1.4m--2.4m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 26 May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0804615
Y-COORD : 7583010

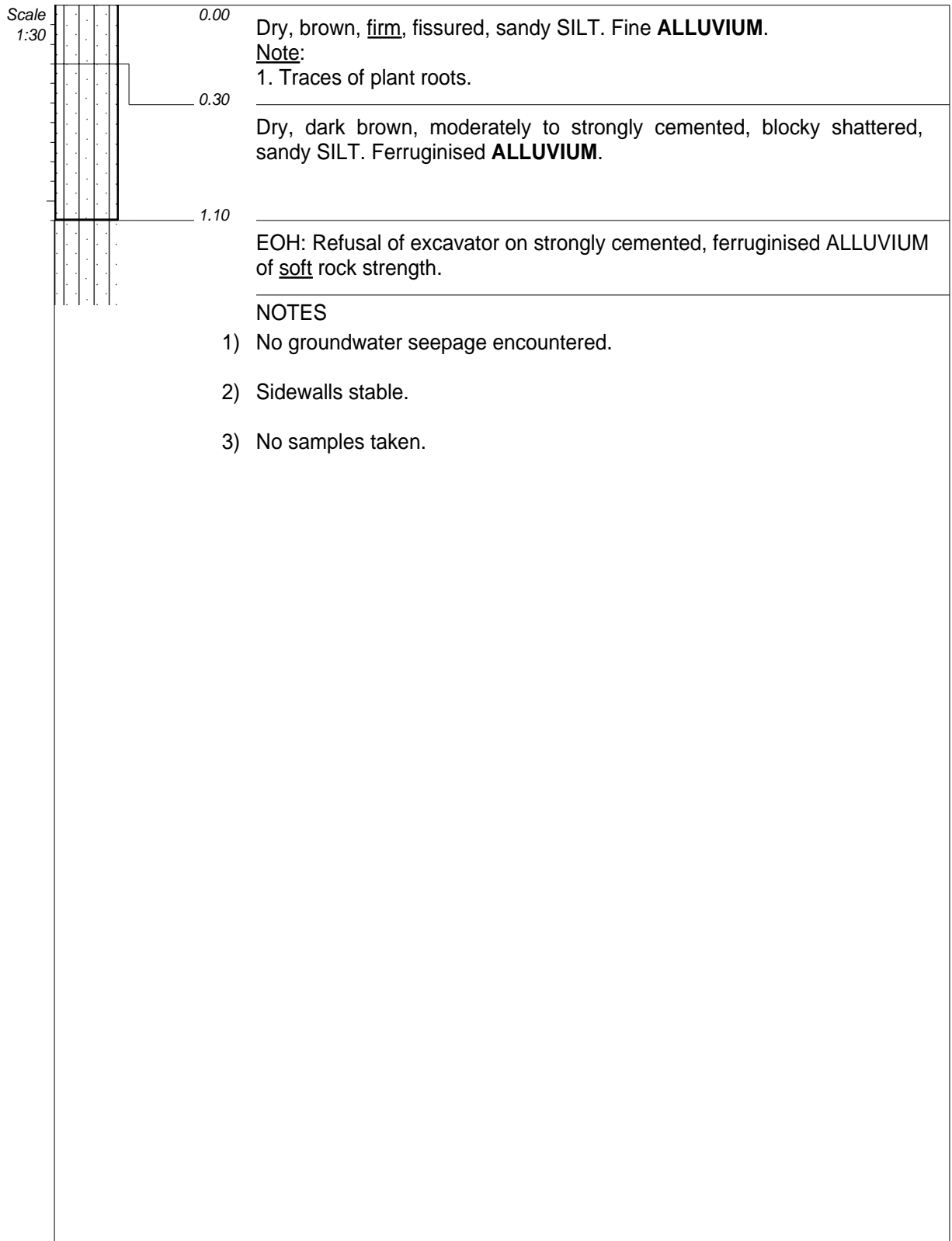
HOLE No: SWD-TP19



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

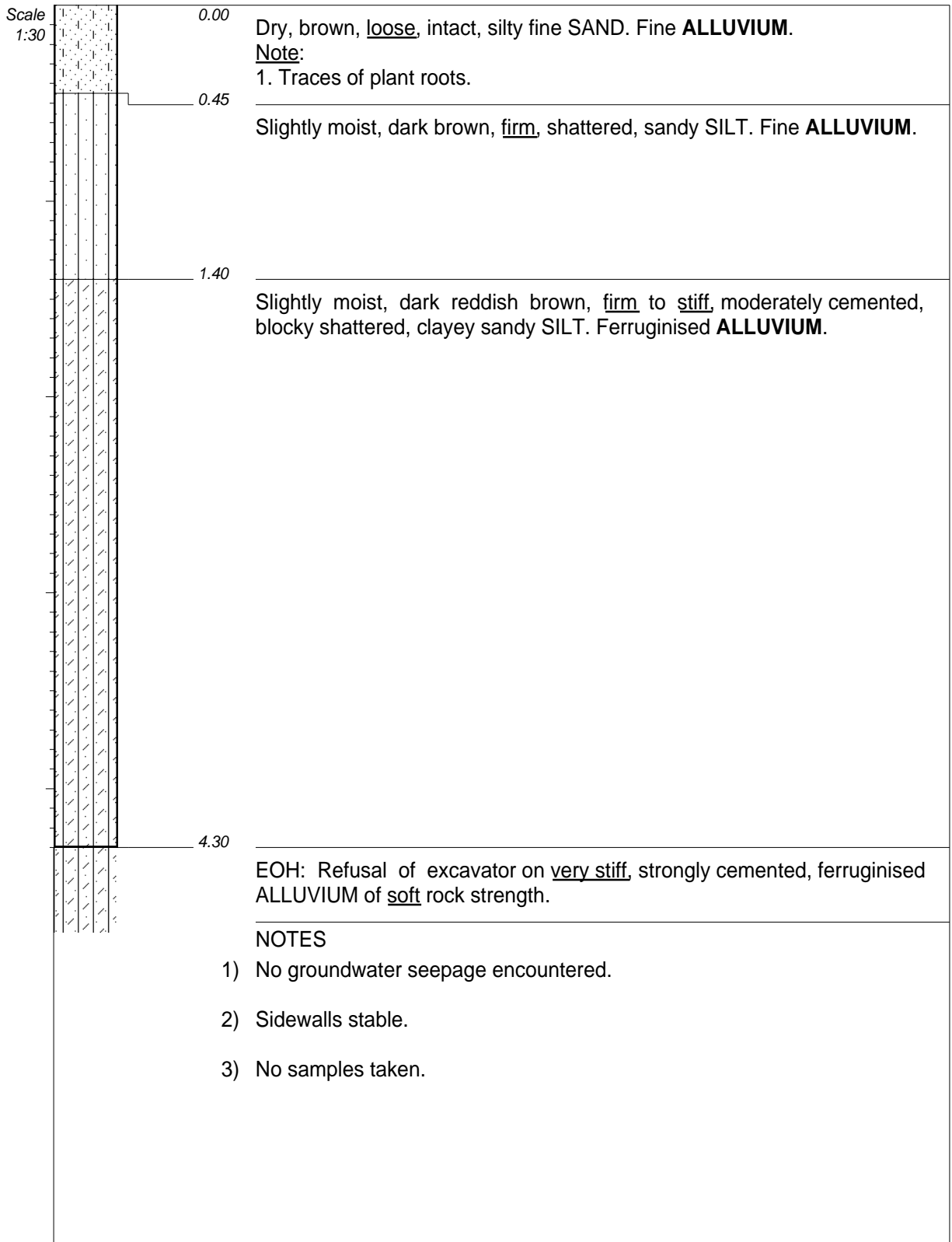
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804622
Y-COORD : 7583230



CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

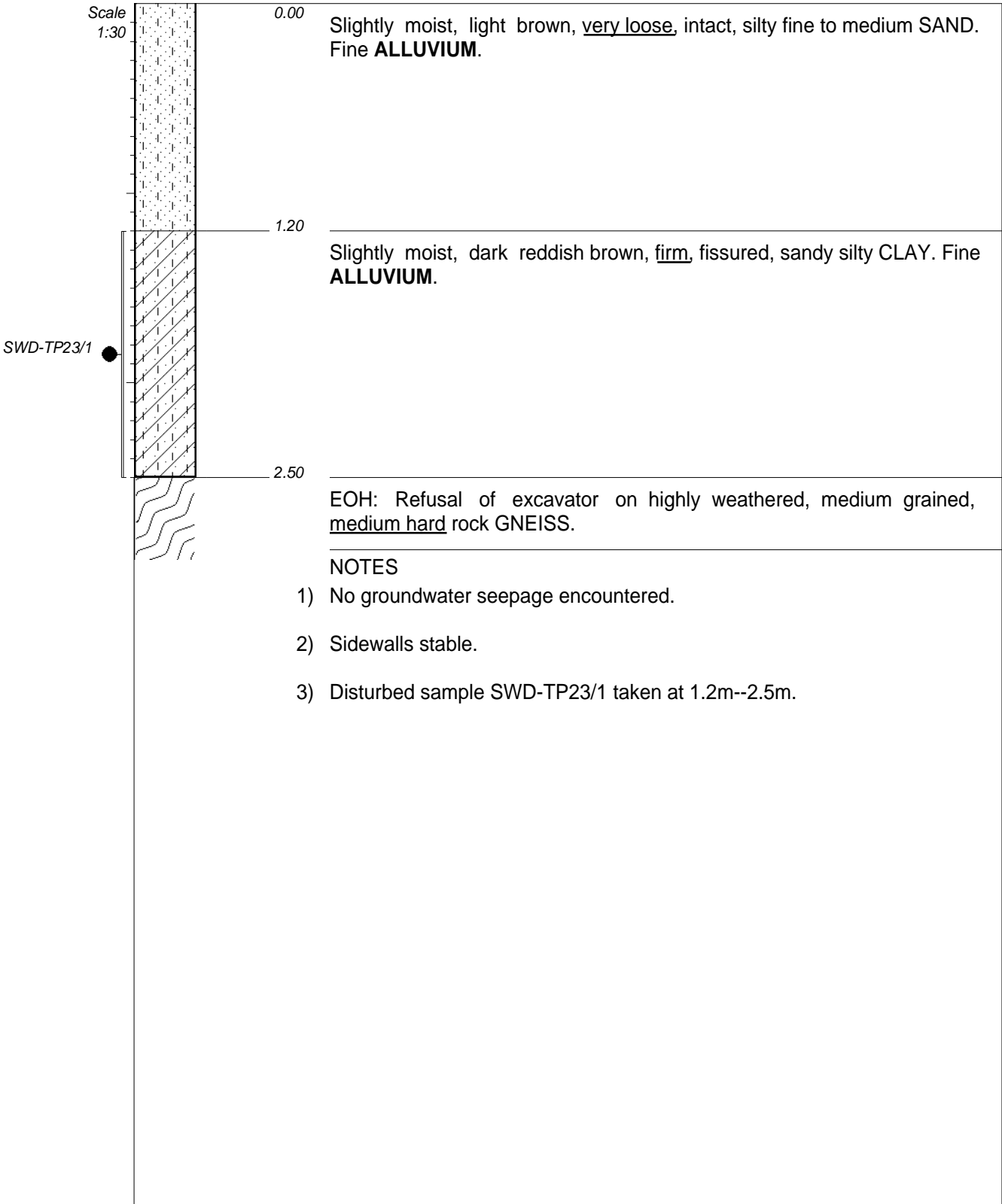
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804592
Y-COORD : 7583193



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

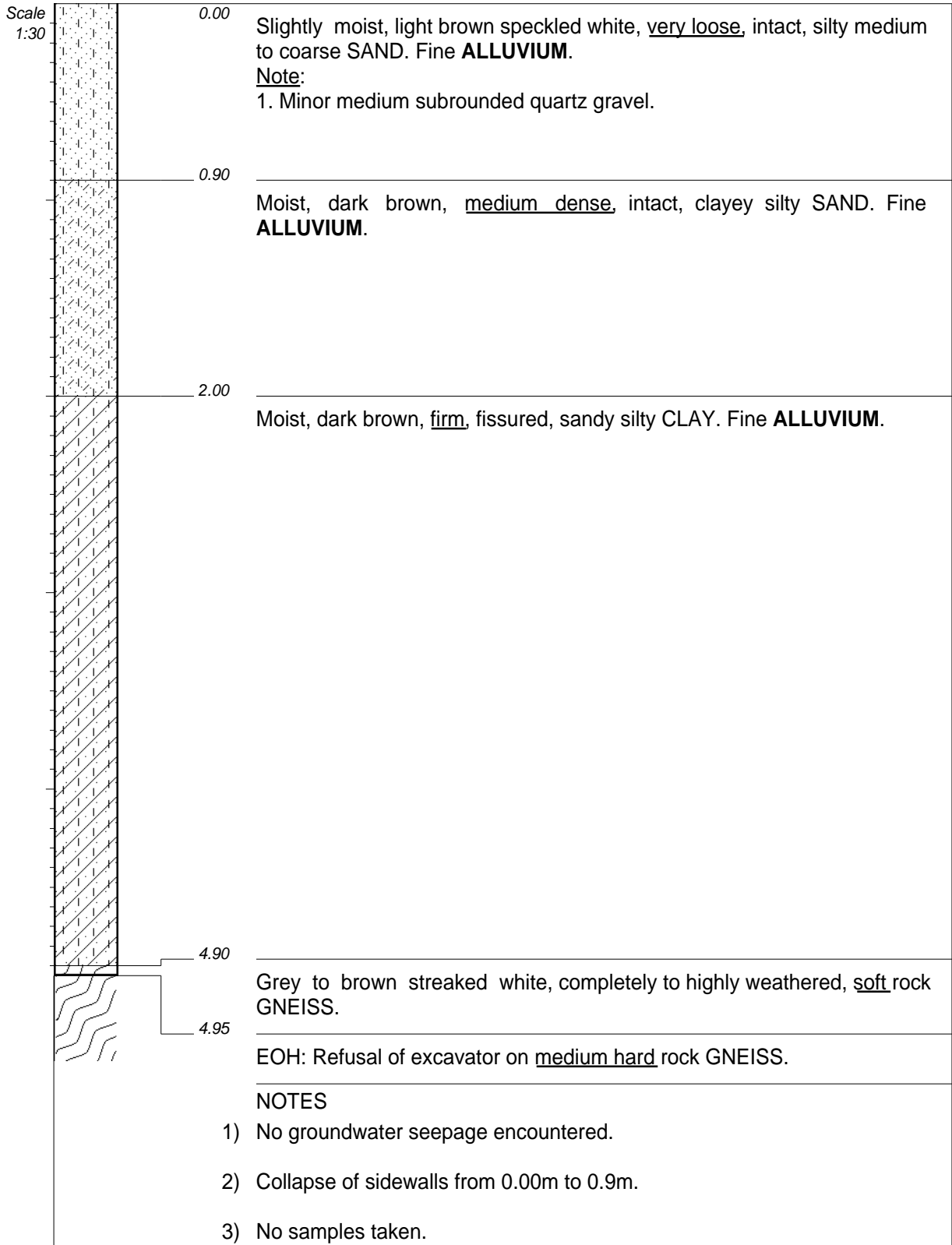
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804685
Y-COORD : 7583279



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

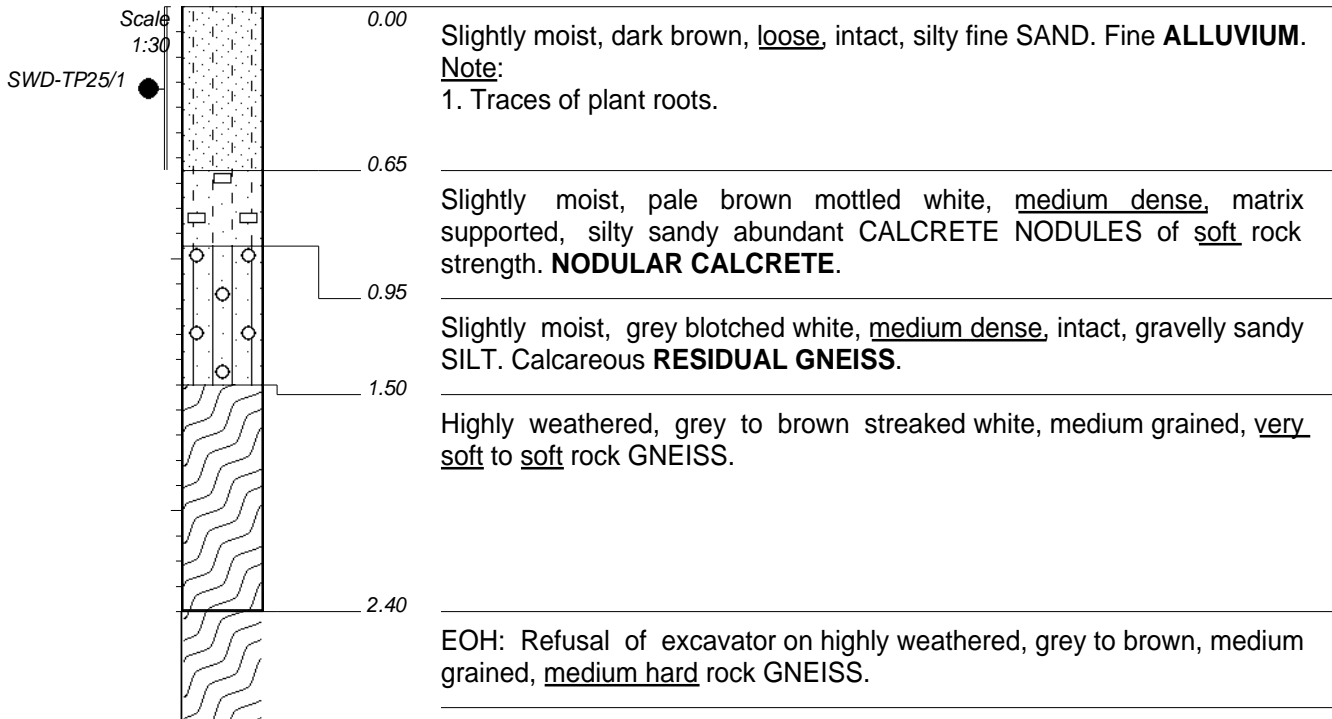
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801706
Y-COORD : 7581801



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804762
Y-COORD : 7583299



NOTES

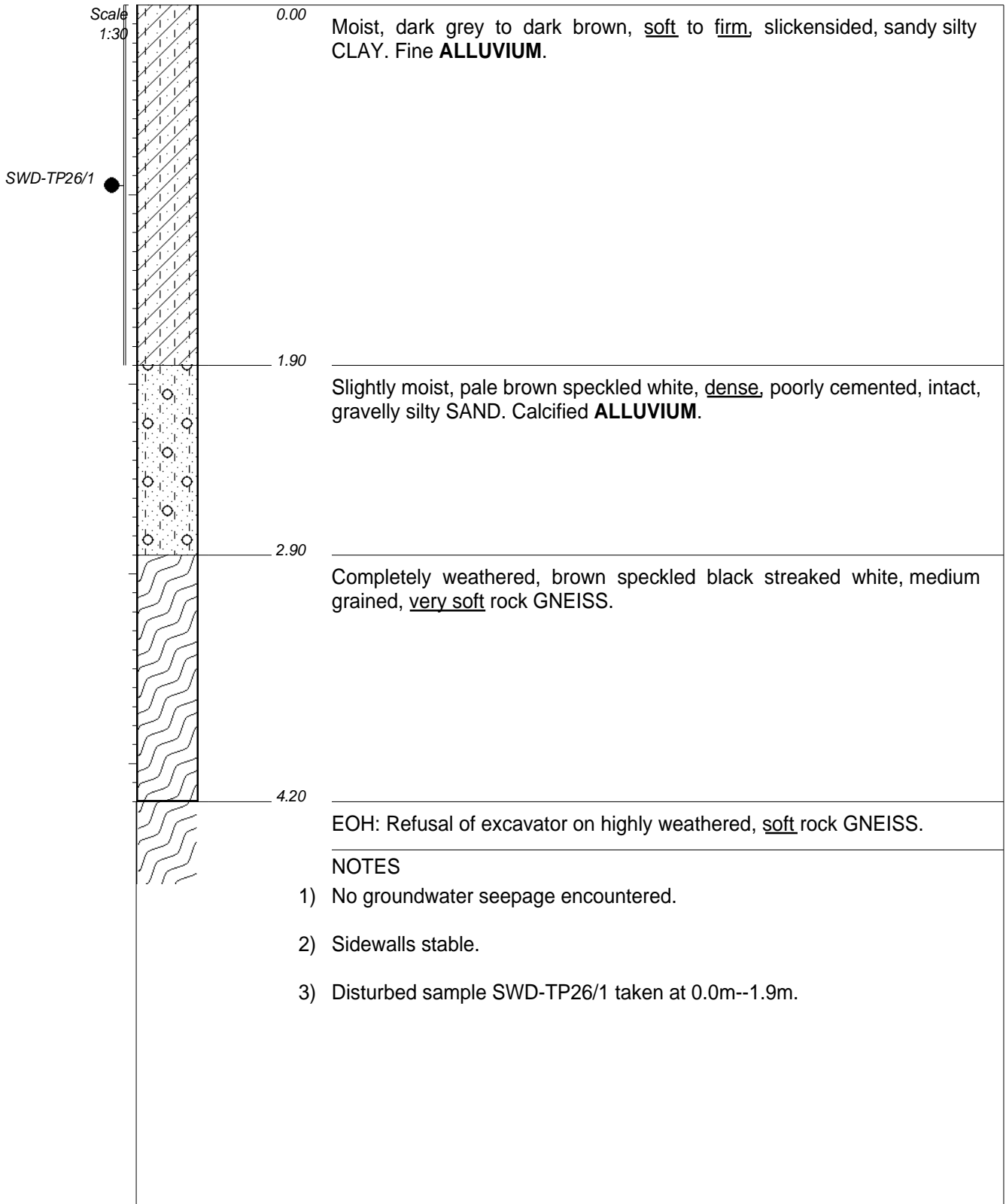
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample SWD-TP25/1 taken at 0.0m--0.65m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 16 May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:15
TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804829
Y-COORD : 7583474

HOLE No: SWD-TP25



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical

DIAM :
DATE : 16 May 2023
DATE : 16 May 2023

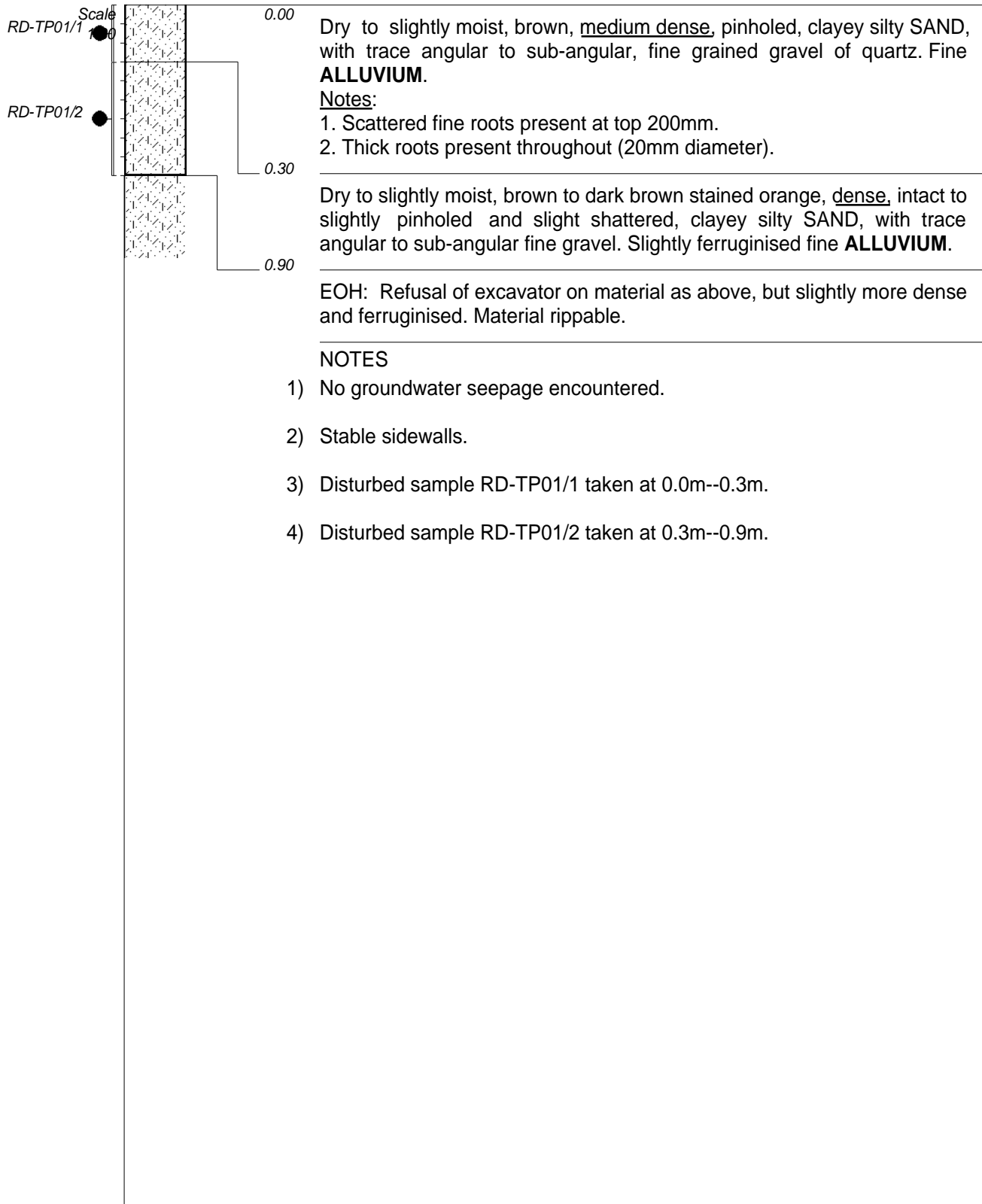
DATE : 07/07/2023 14:15

TEXT : ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)

X-COORD : 803392
Y-COORD : 7581470

HOLE No: SWD-TP26

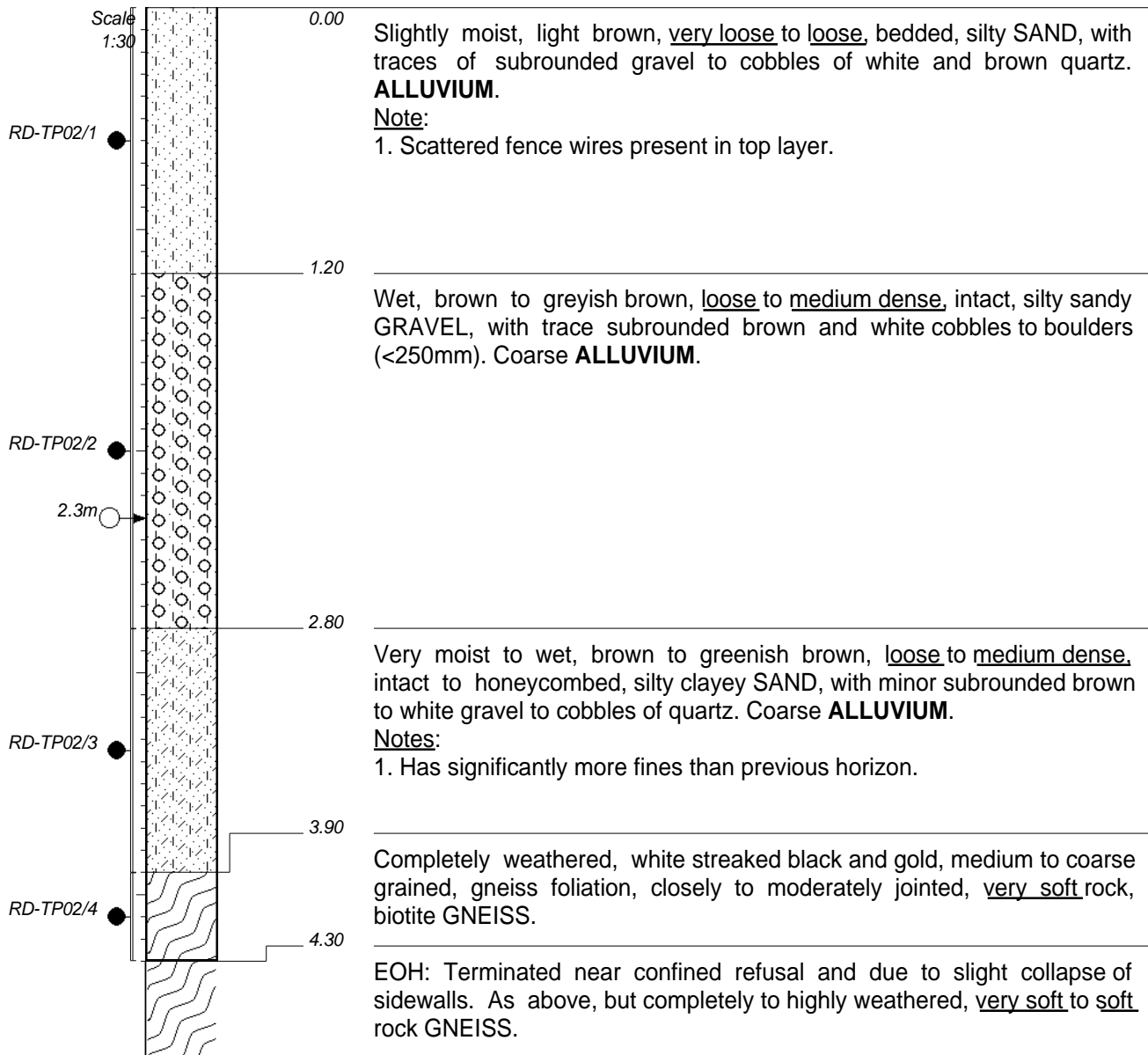


CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 30 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0800971
Y-COORD : 7579851

HOLE No: RD-TP01



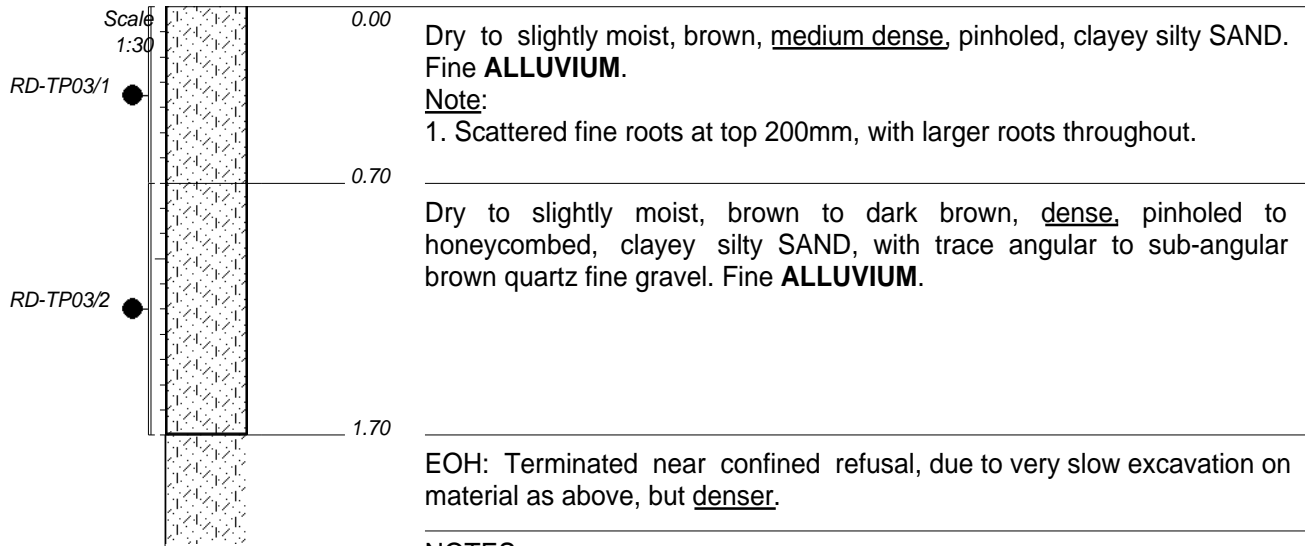
NOTES

- 1) Groundwater seepage encountered at 2.3m at 0.5 litre/minute.
- 2) Slightly unstable sidewalls.
- 3) Disturbed sample RD-TP02/1 taken at 0.0m--1.2m.
- 4) Disturbed sample RD-TP02/2 taken at 1.2m--2.8m.
- 5) Disturbed sample RD-TP02/3 taken at 2.8m--3.9m.
- 6) Disturbed sample RD-TP02/4 taken at 3.9m--4.3m.

CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 30 May 2023
DATE : 30 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801584
Y-COORD : 7580544



NOTES

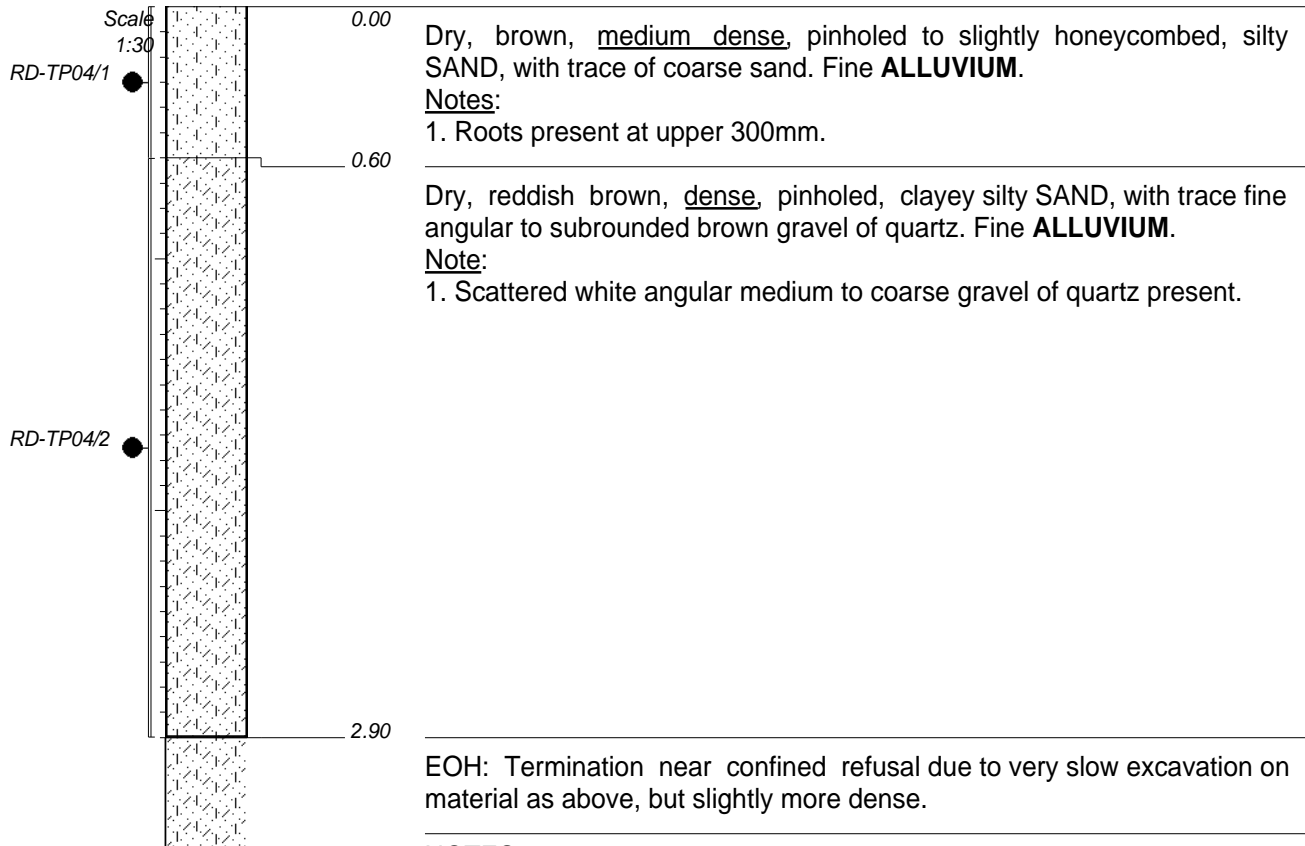
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP03/1 taken at 0.0m--0.7m.
- 4) Disturbed sample RD-TP03/2 taken at 0,7m--1.7m.

CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 29 May 2023
DATE : 29 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801605
Y-COORD : 7580544

HOLE No: RD-TP03



Dry, brown, medium dense, pinholed to slightly honeycombed, silty SAND, with trace of coarse sand. Fine **ALLUVIUM**.

Notes:

1. Roots present at upper 300mm.

Dry, reddish brown, dense, pinholed, clayey silty SAND, with trace fine angular to subrounded brown gravel of quartz. Fine **ALLUVIUM**.

Note:

1. Scattered white angular medium to coarse gravel of quartz present.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP04/1 taken at 0.0m--0.6m.
- 4) Disturbed sample RD-TP04/2 taken at 0.6m--2.9m.

CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan

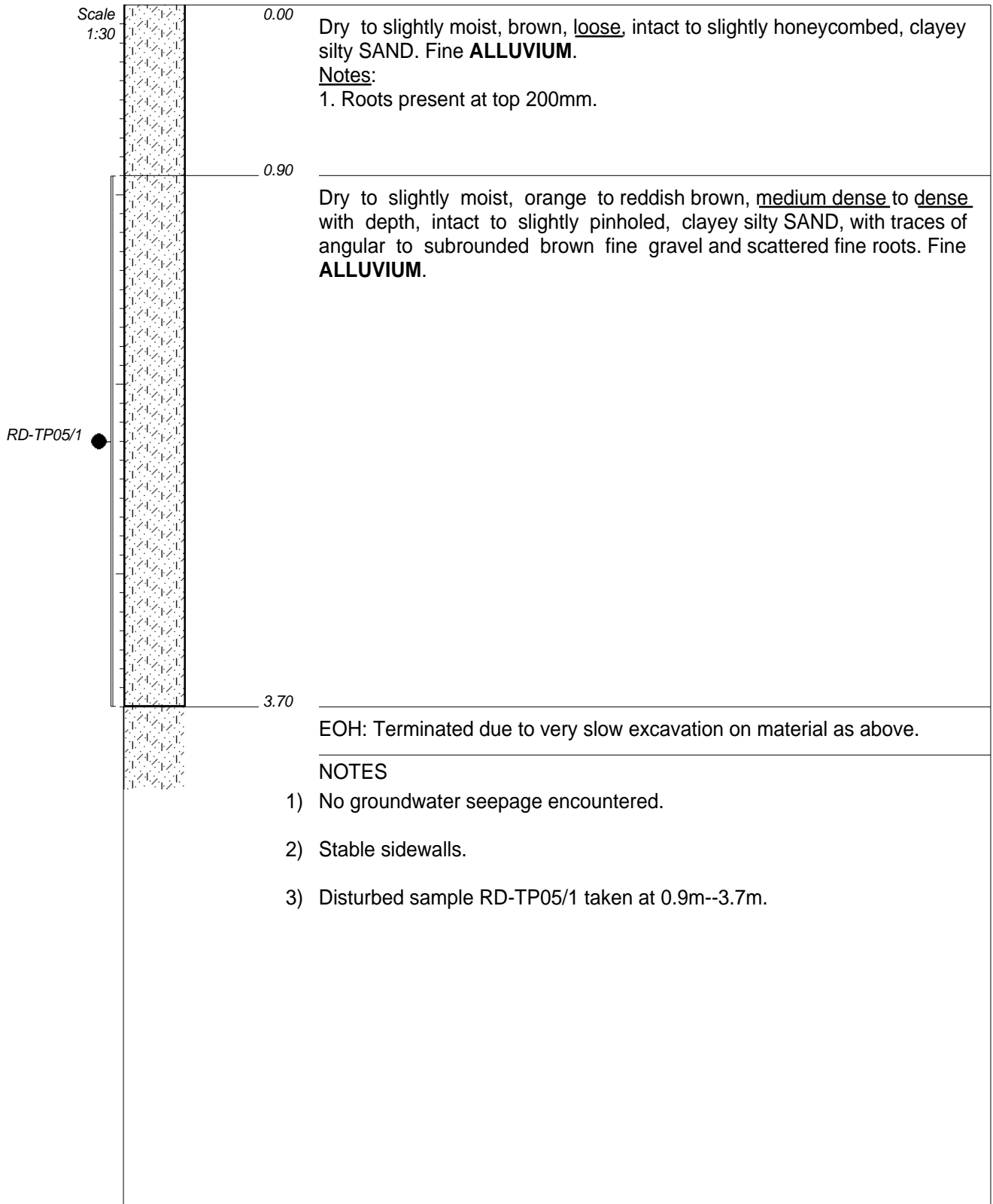
INCLINATION : Vertical
DIAM :
DATE : 30 May 2023
DATE : 30 May 2023

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0802497
Y-COORD : 7579862

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

HOLE No: RD-TP04

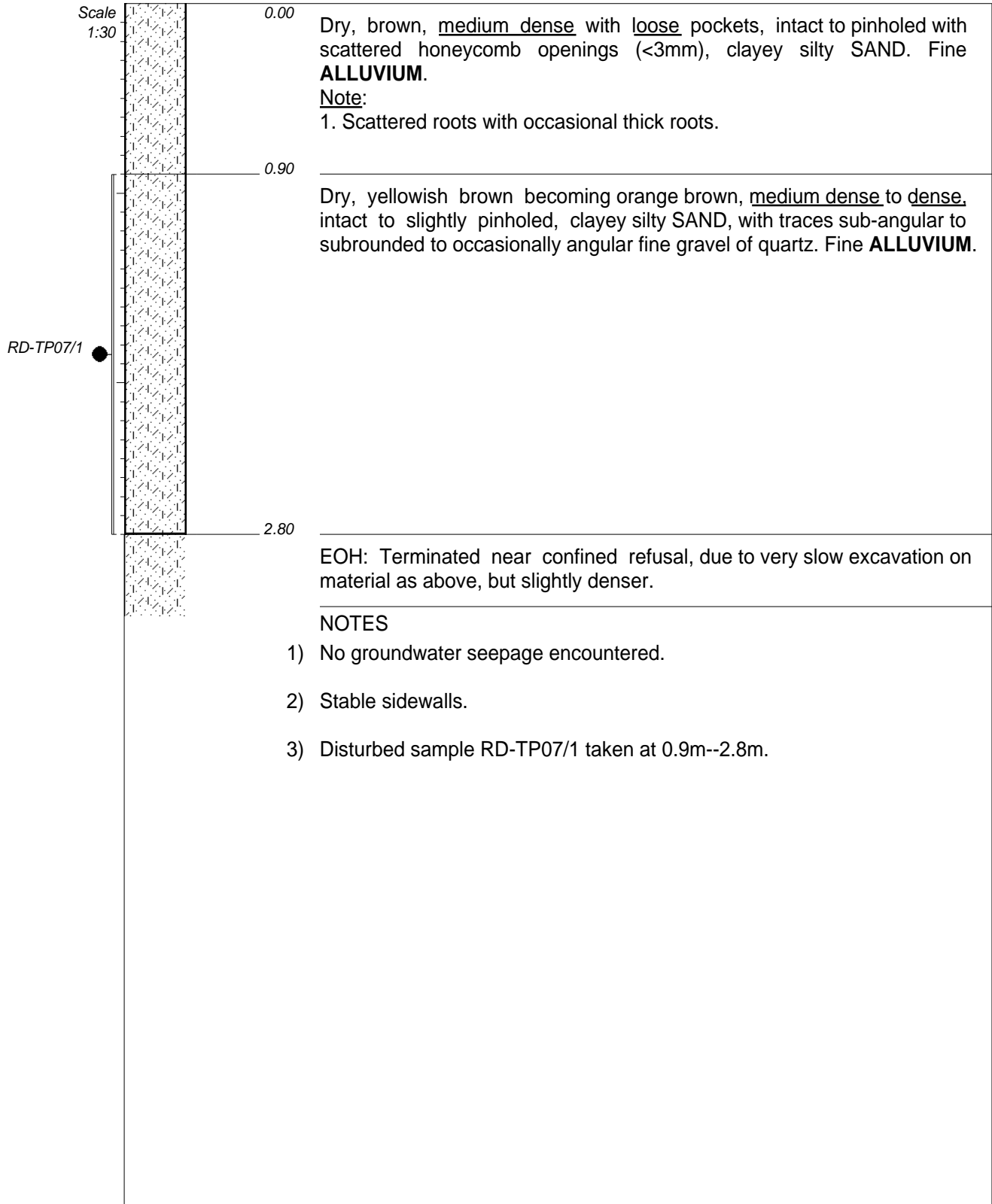


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 30 May 2023
DATE : 30 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 08033541
Y-COORD : 7580258

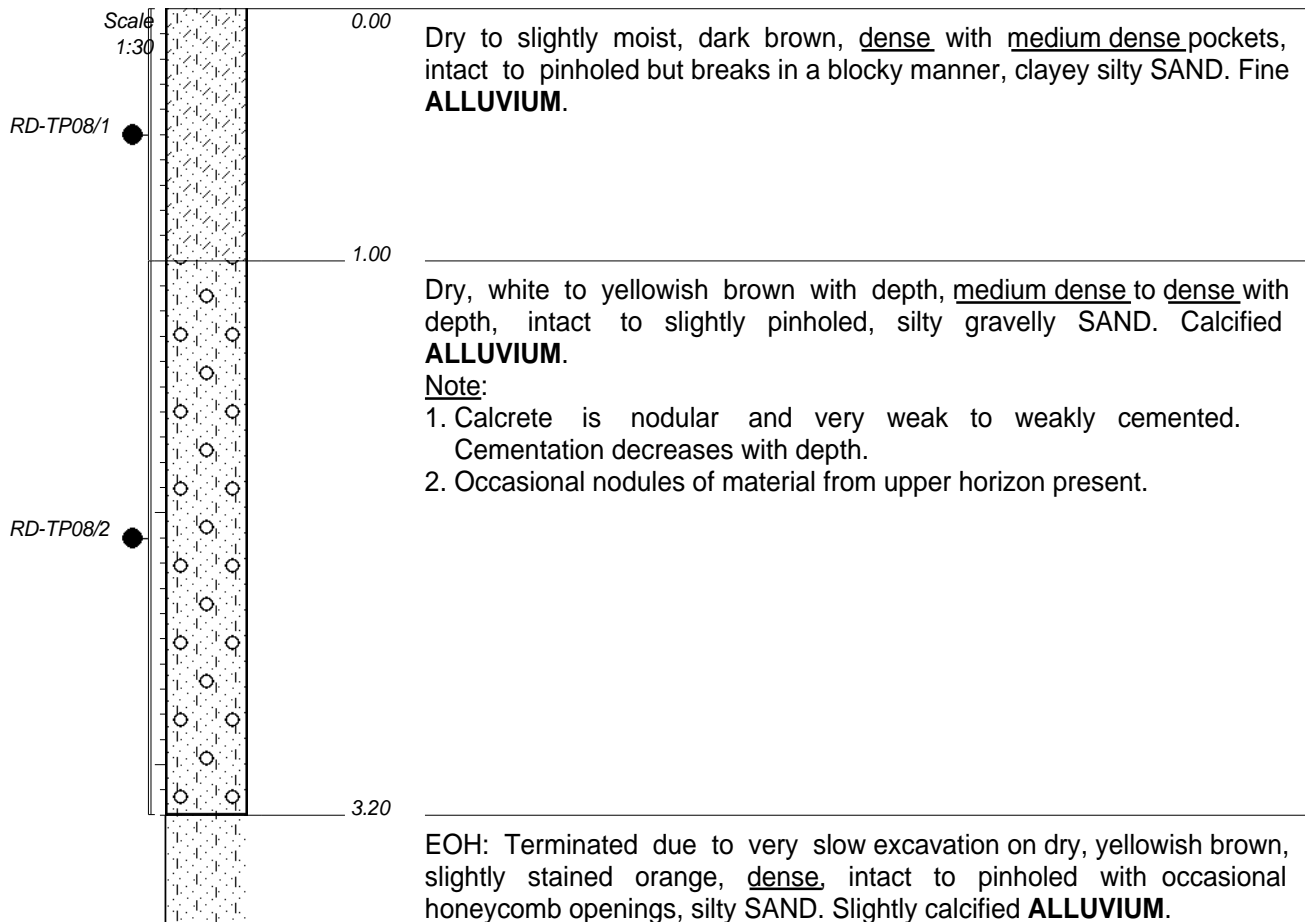
HOLE No: RD-TP05



CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 29 May 2023
DATE : 29 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0803985
Y-COORD : 7581279



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP08/1 taken at 0.0m--1.0m.
- 4) Disturbed sample RD-TP08/2 taken at 1.0m--3.2m.

CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan

TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical

DIAM :
DATE : 29 May 2023
DATE : 29 May 2023

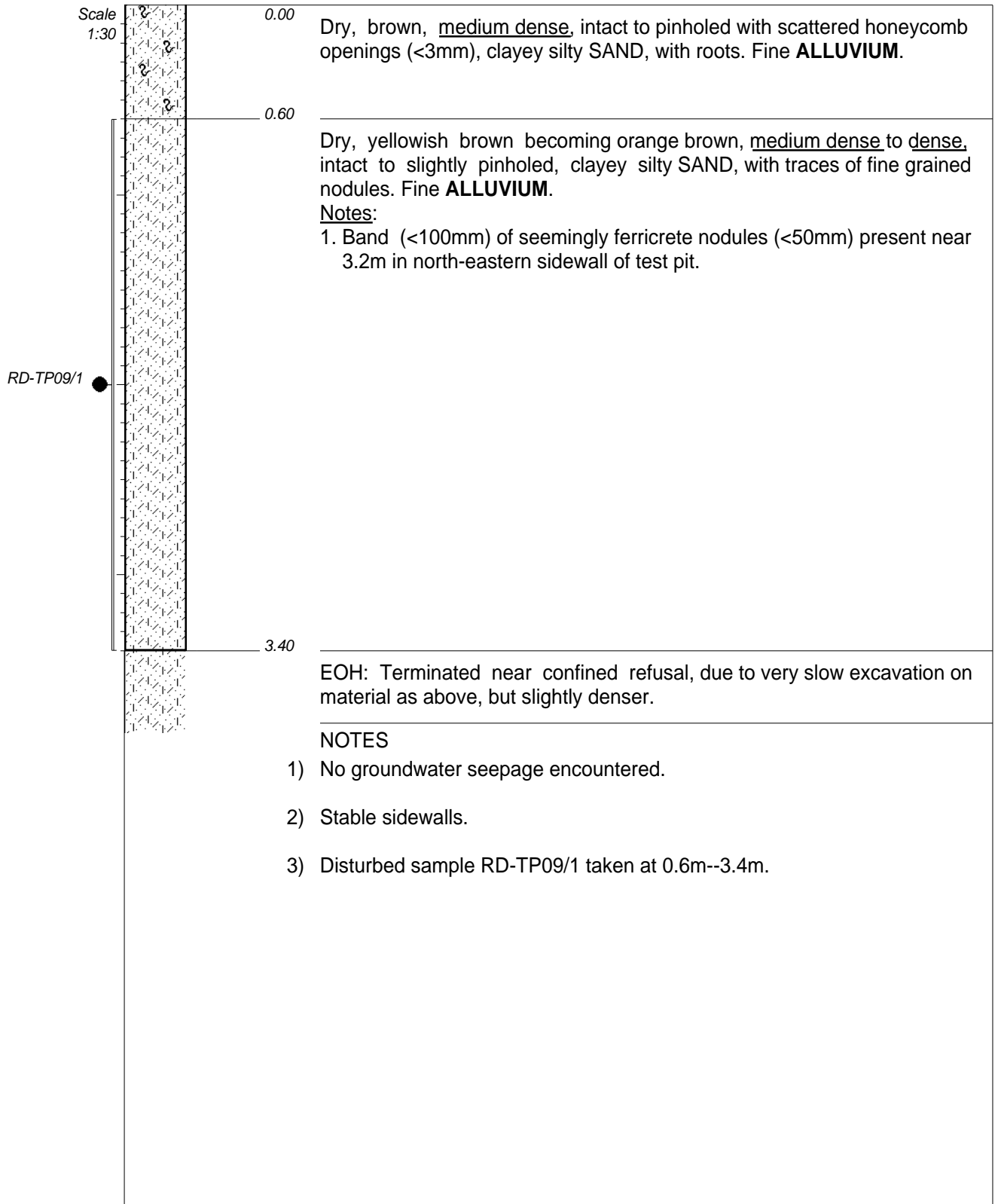
DATE : 04/07/2023 13:52

TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)

X-COORD : 0804425
Y-COORD : 7581074

HOLE No: RD-TP08

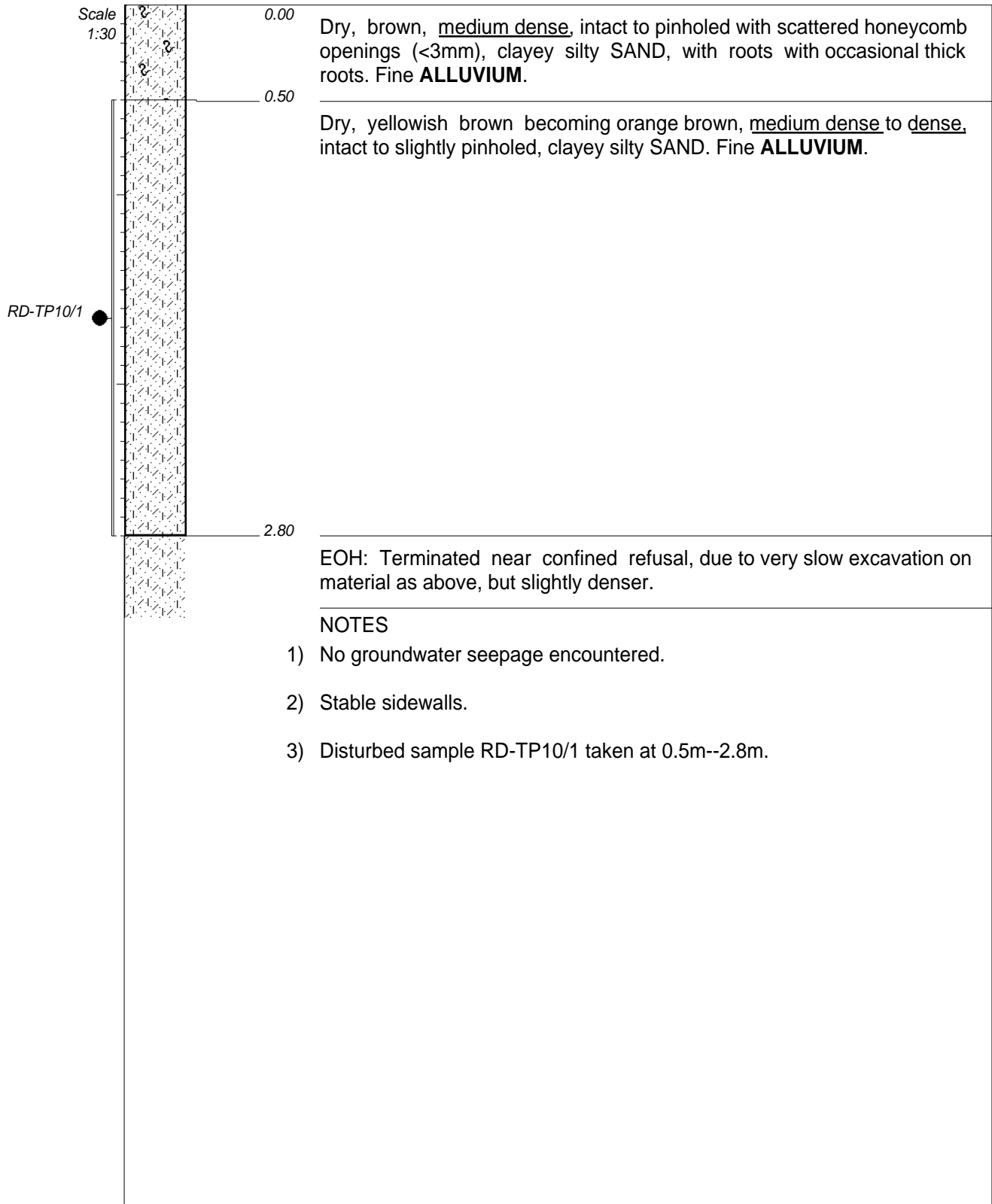


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 29 May 2023
DATE : 29 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0804750
Y-COORD : 7582018

HOLE No: RD-TP09

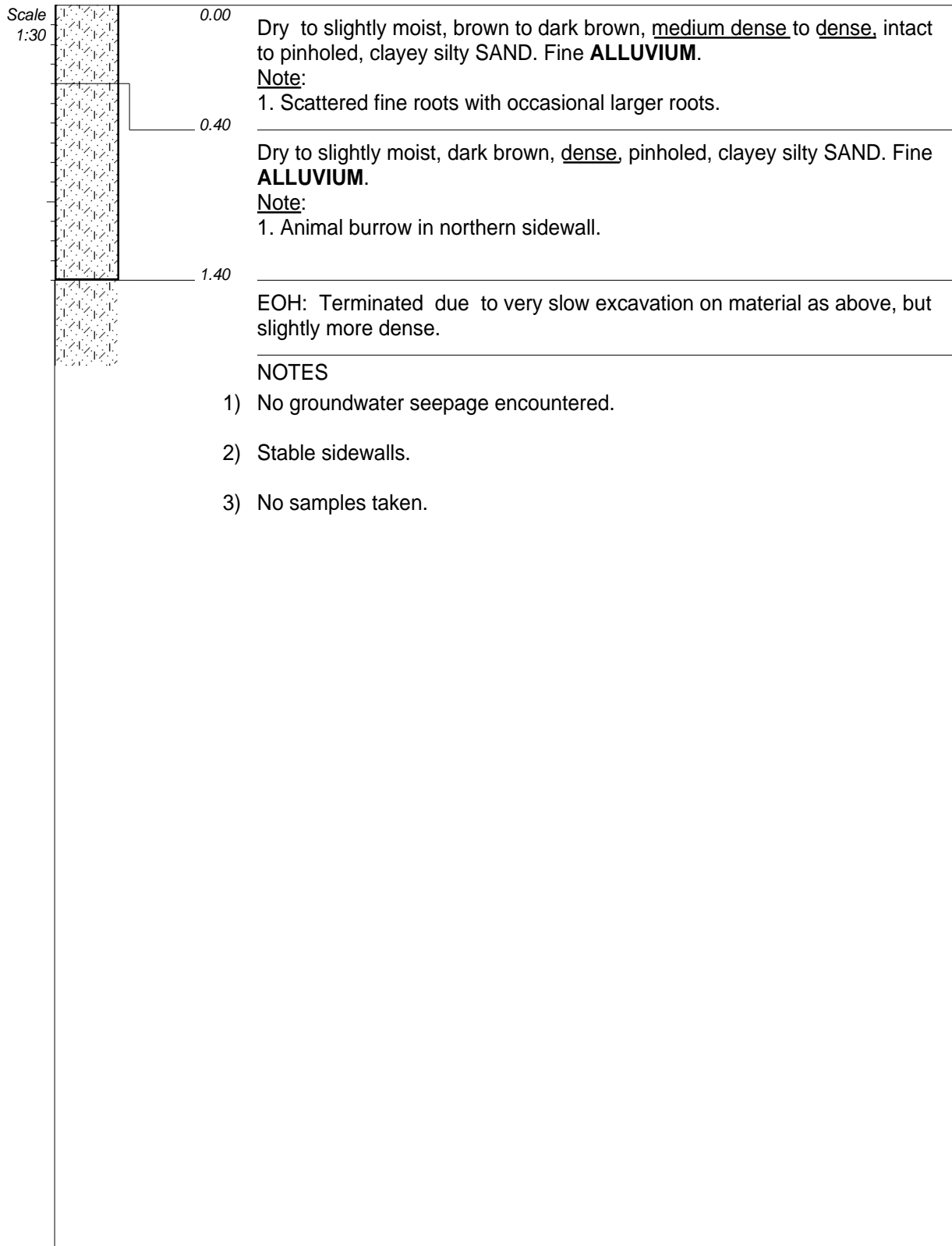


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 29 May 2023
DATE : 29 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0805515
Y-COORD : 7582133

HOLE No: RD-TP10

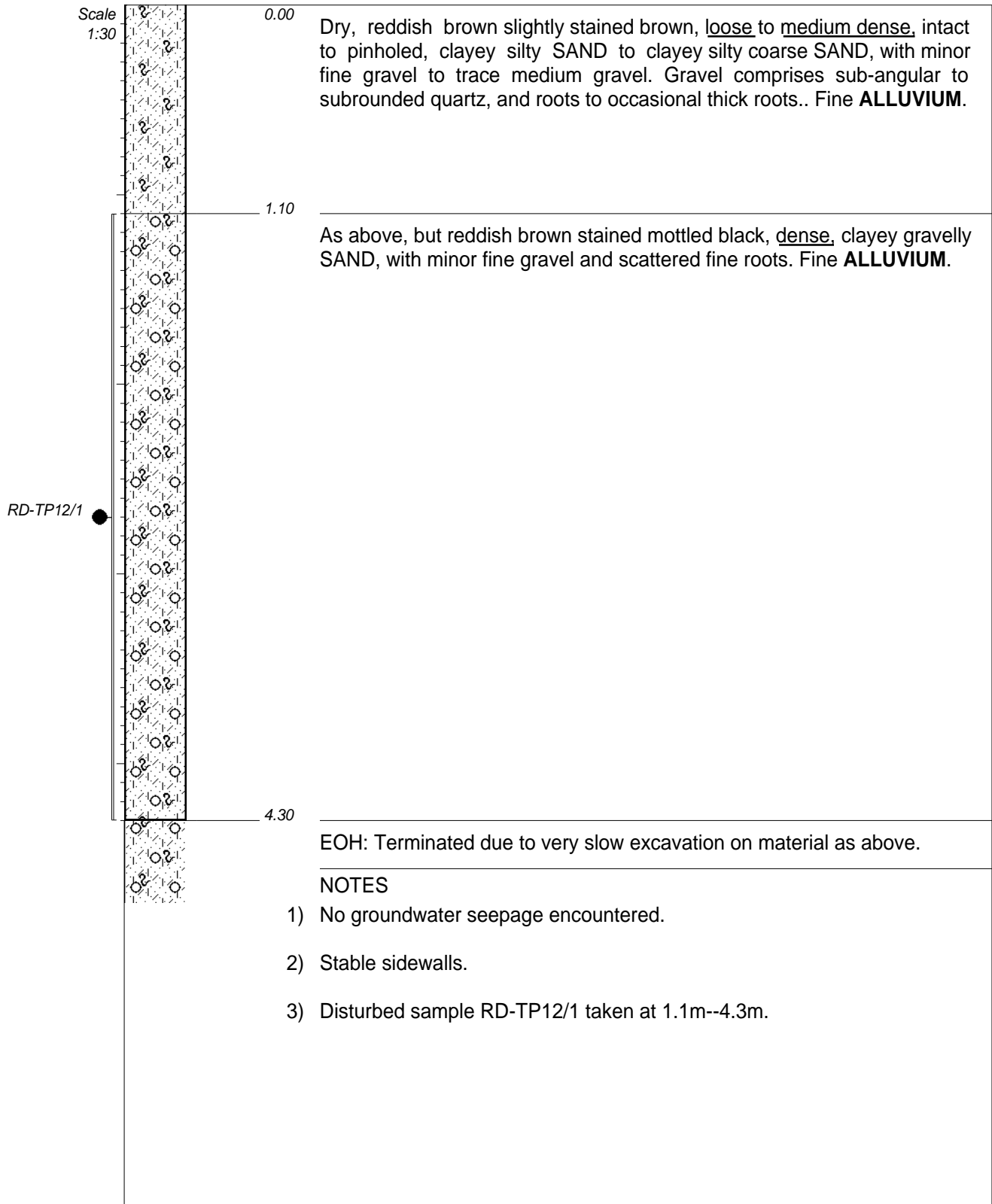


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0806850
Y-COORD : 7583349

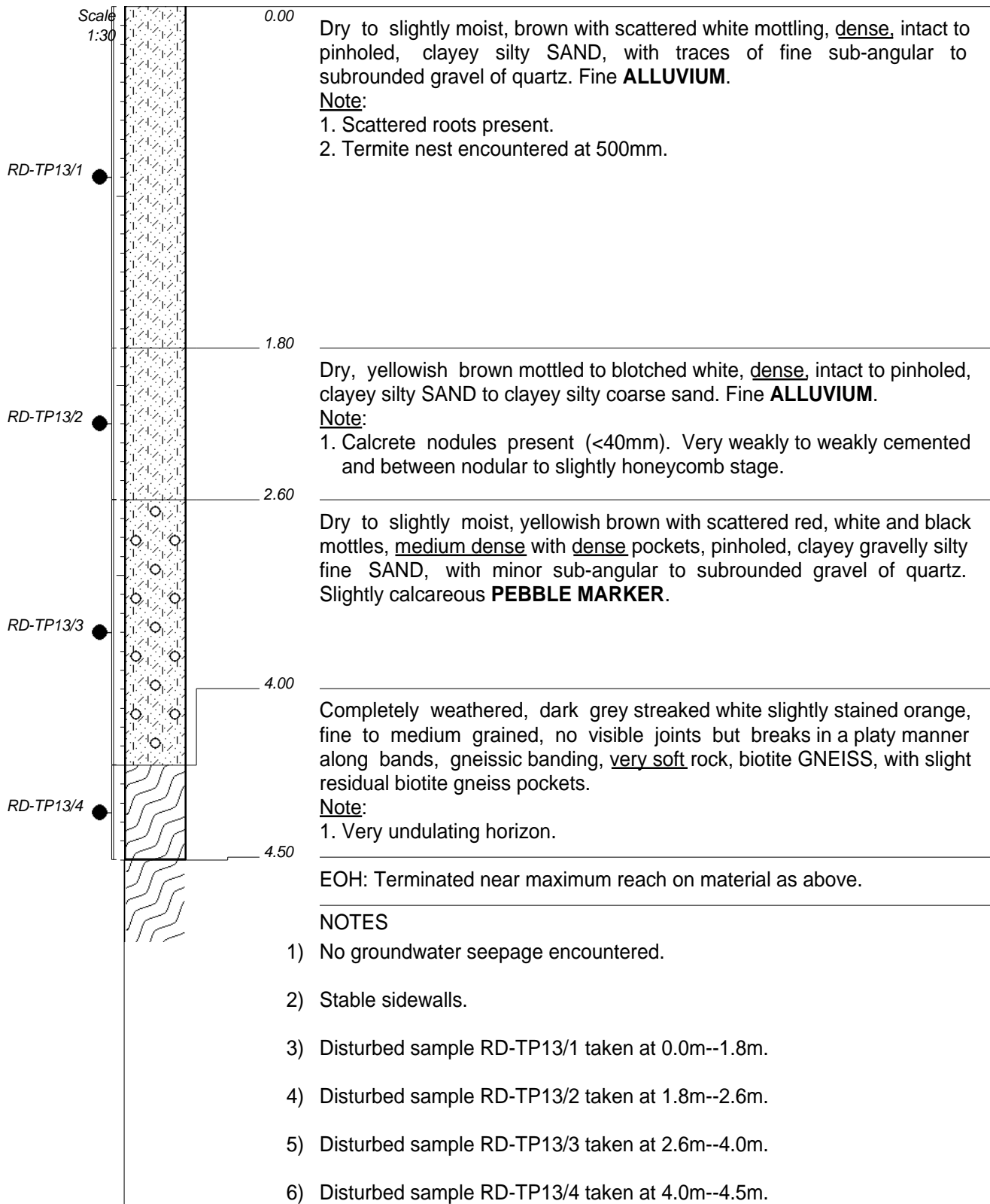
HOLE No: RD-TP11



CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0805797
Y-COORD : 7582981

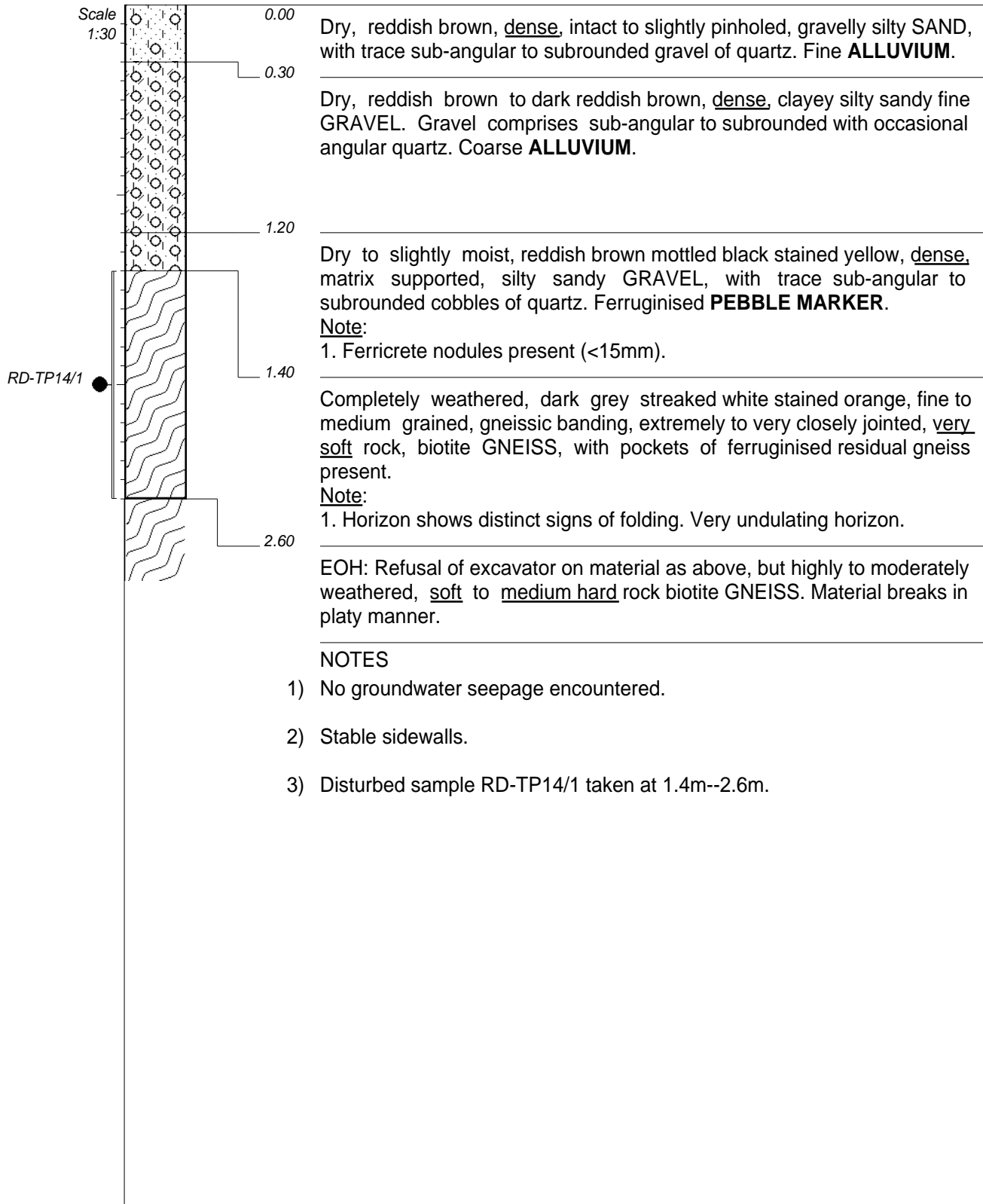


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0806772
Y-COORD : 7583302

HOLE No: RD-TP13

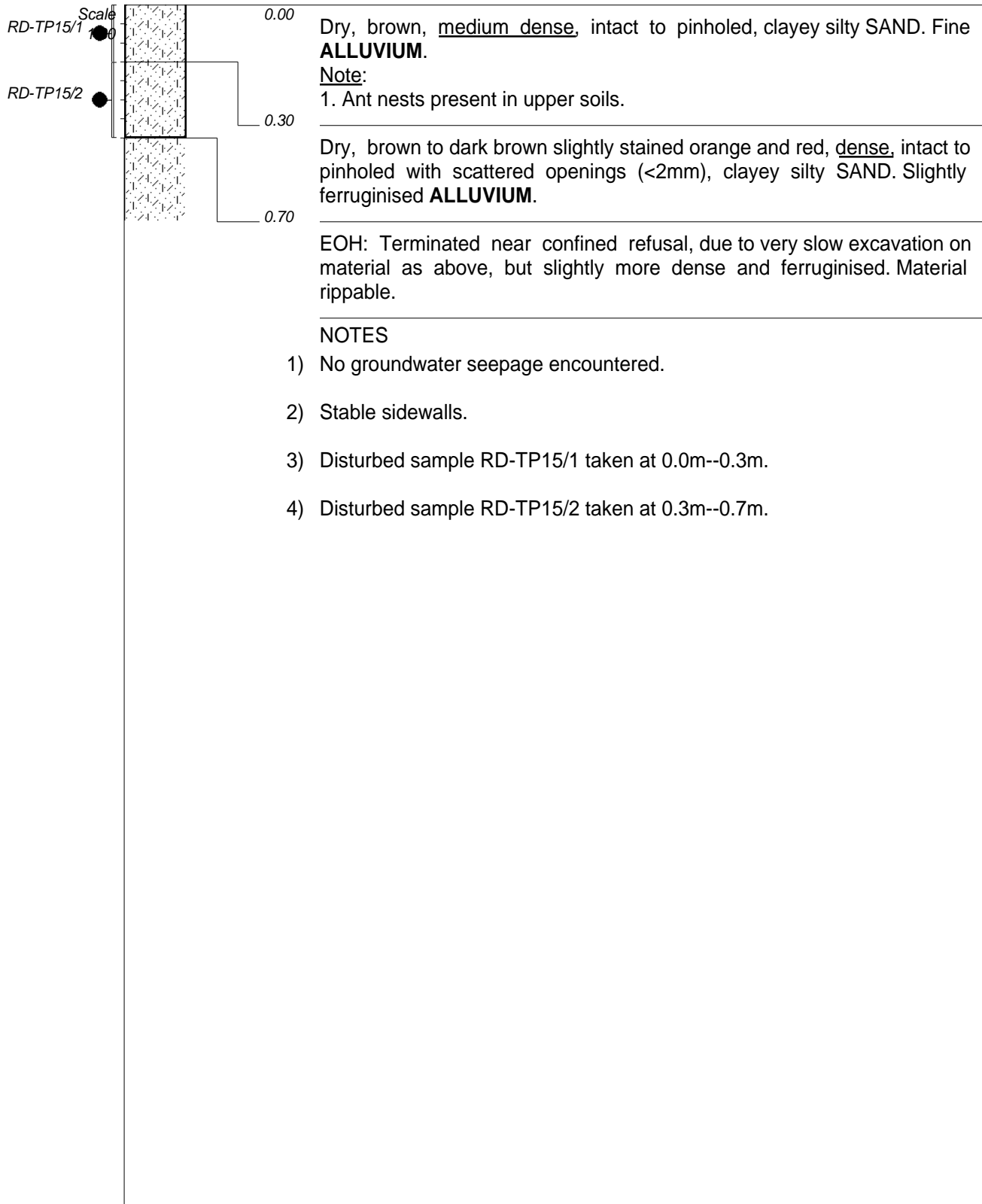


CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0806913
Y-COORD : 7583919

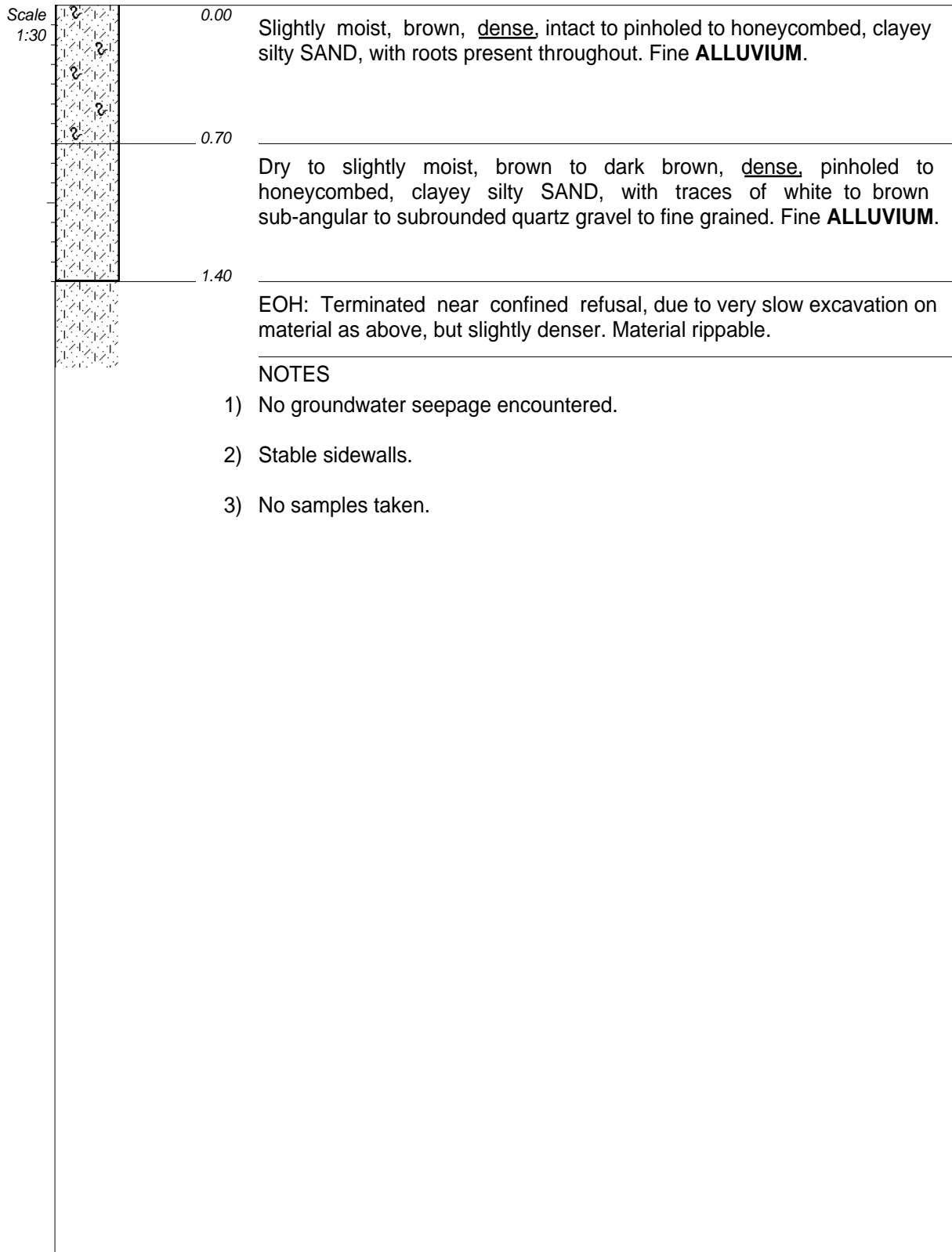
HOLE No: RD-TP14



CONTRACTOR : Sandworx
MACHINE : : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

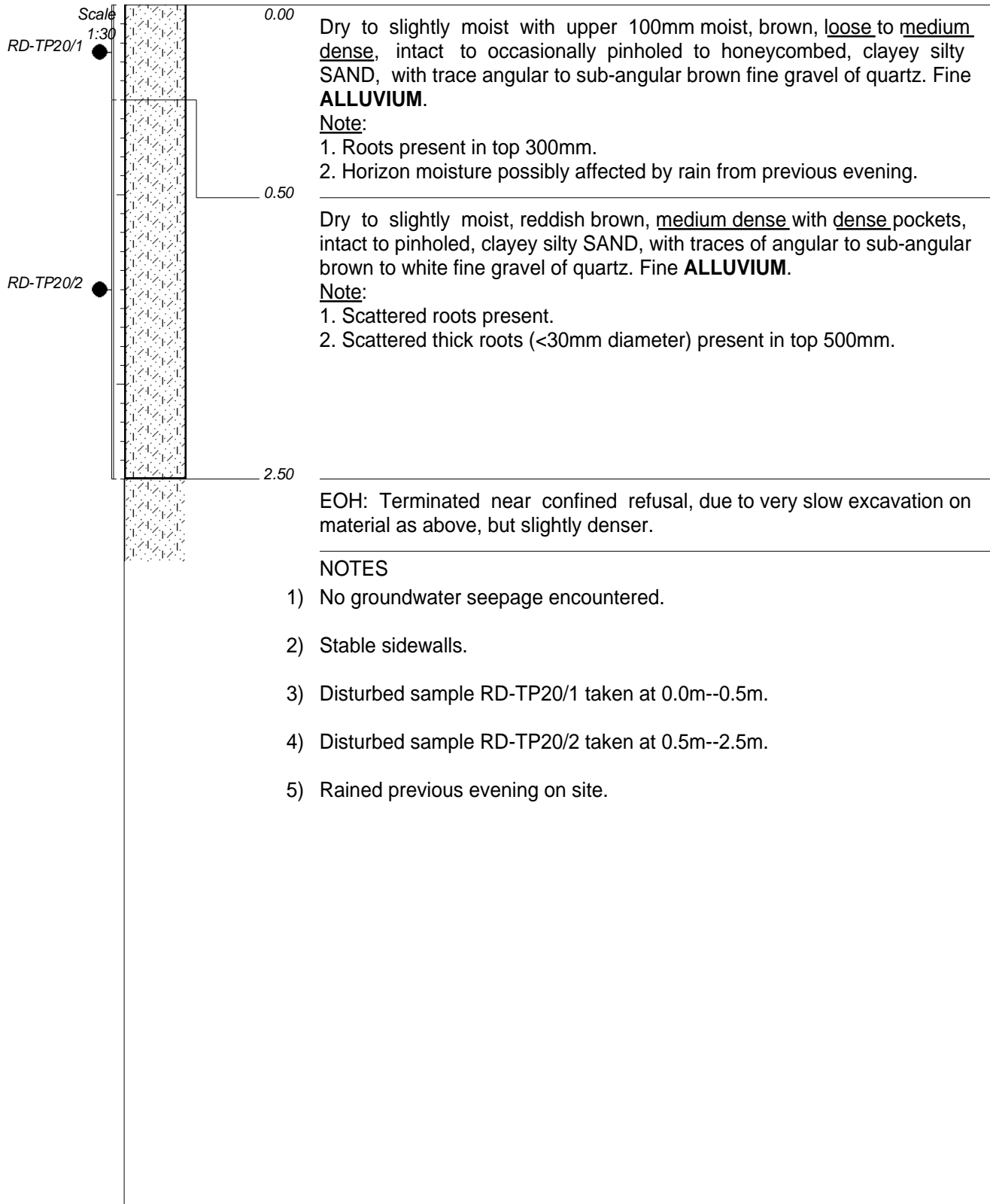
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0807324
Y-COORD : 7583709



CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 30 May 2023
DATE : 30 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801561
Y-COORD : 7580548

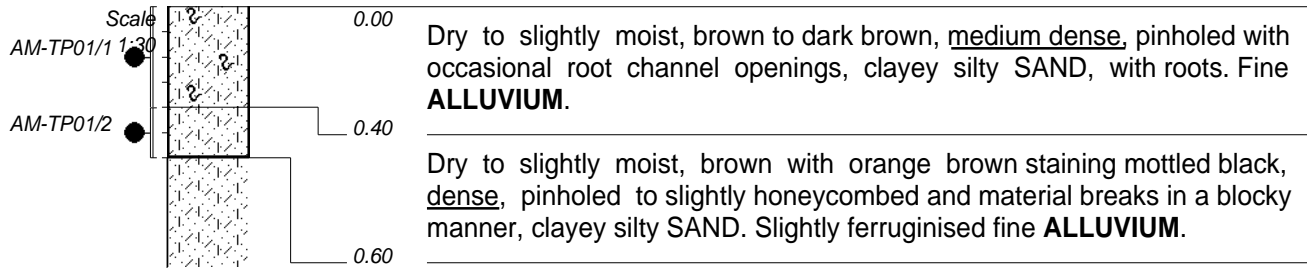


CONTRACTOR : Sandworx
MACHINE : Lovol Fr220D
DRILLED BY : Simon
PROFILED BY : Chris Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52
TEXT : ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0802541
Y-COORD : 7589491

HOLE No: RD-TP20



Dry to slightly moist, brown to dark brown, medium dense, pinholed with occasional root channel openings, clayey silty SAND, with roots. Fine **ALLUVIUM**.

Dry to slightly moist, brown with orange brown staining mottled black, dense, pinholed to slightly honeycombed and material breaks in a blocky manner, clayey silty SAND. Slightly ferruginised fine **ALLUVIUM**.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP01/1 taken at 0.0m--0.4m.
- 4) Disturbed sample AM-TP01/2 taken at 0.4m--0.6m.

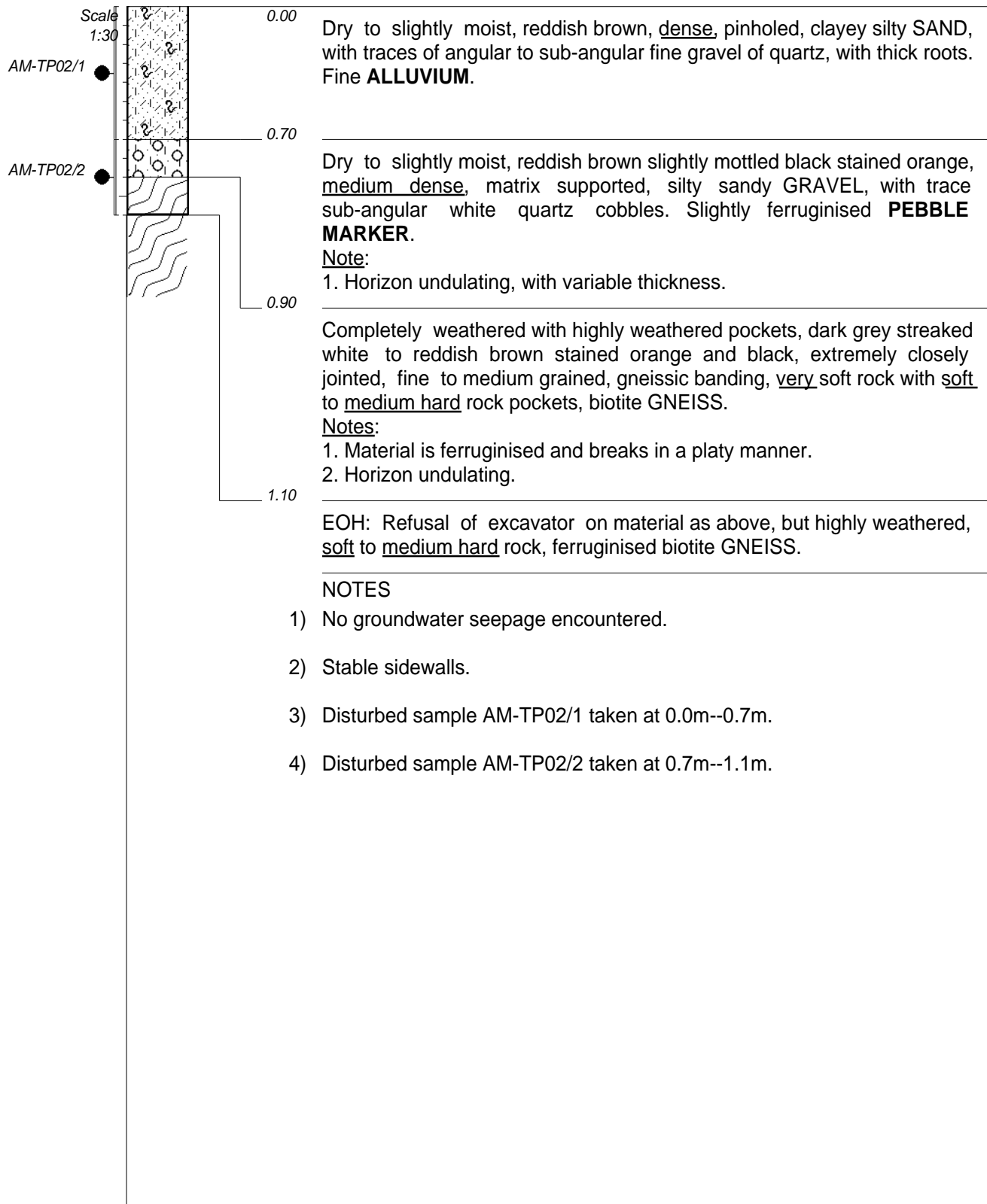
CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Simon
PROFILED BY : Chris Homan

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0807088
Y-COORD : 7583751

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT



CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

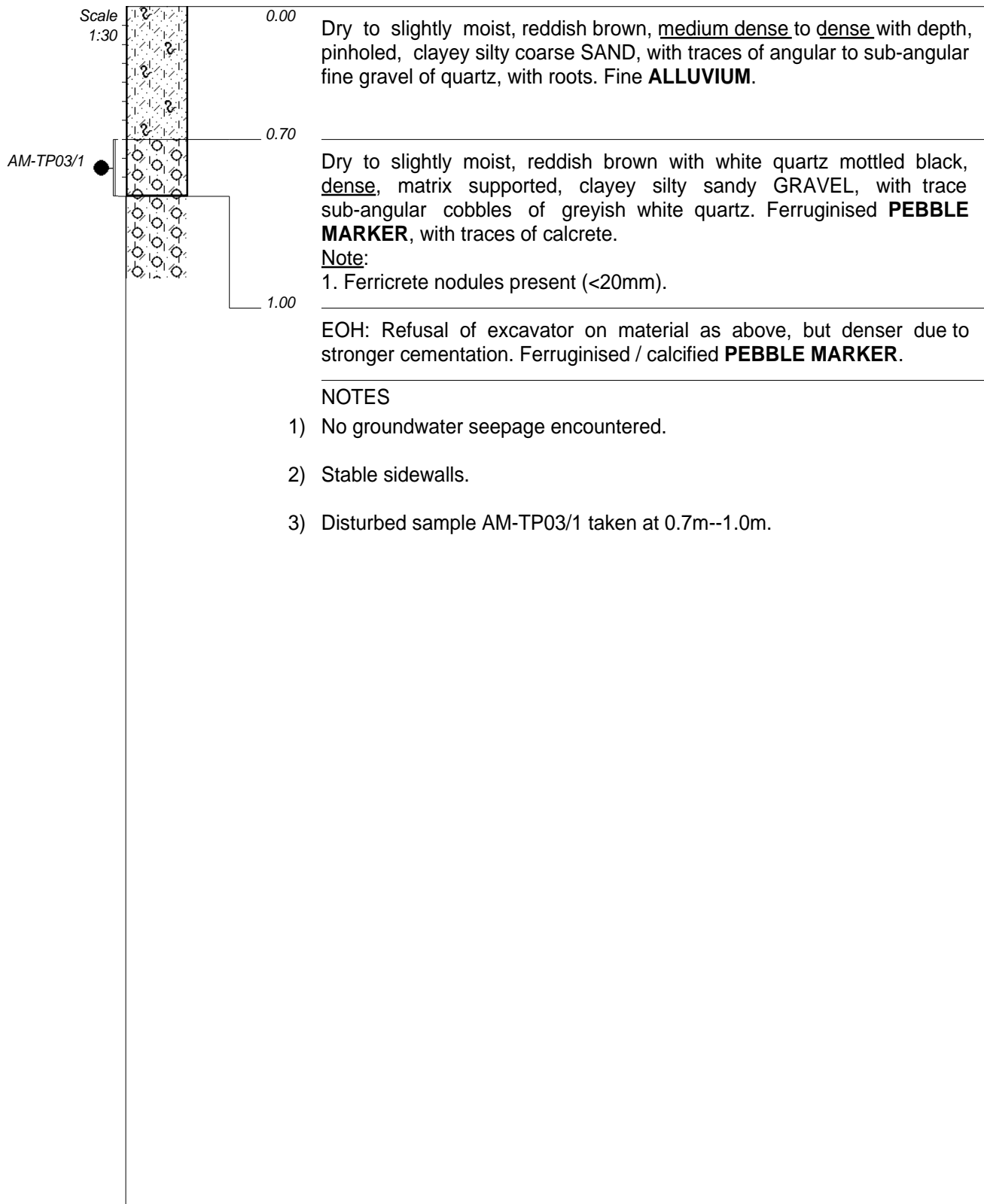
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806656
Y-COORD : 7583660

HOLE No: AM-TP02



CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

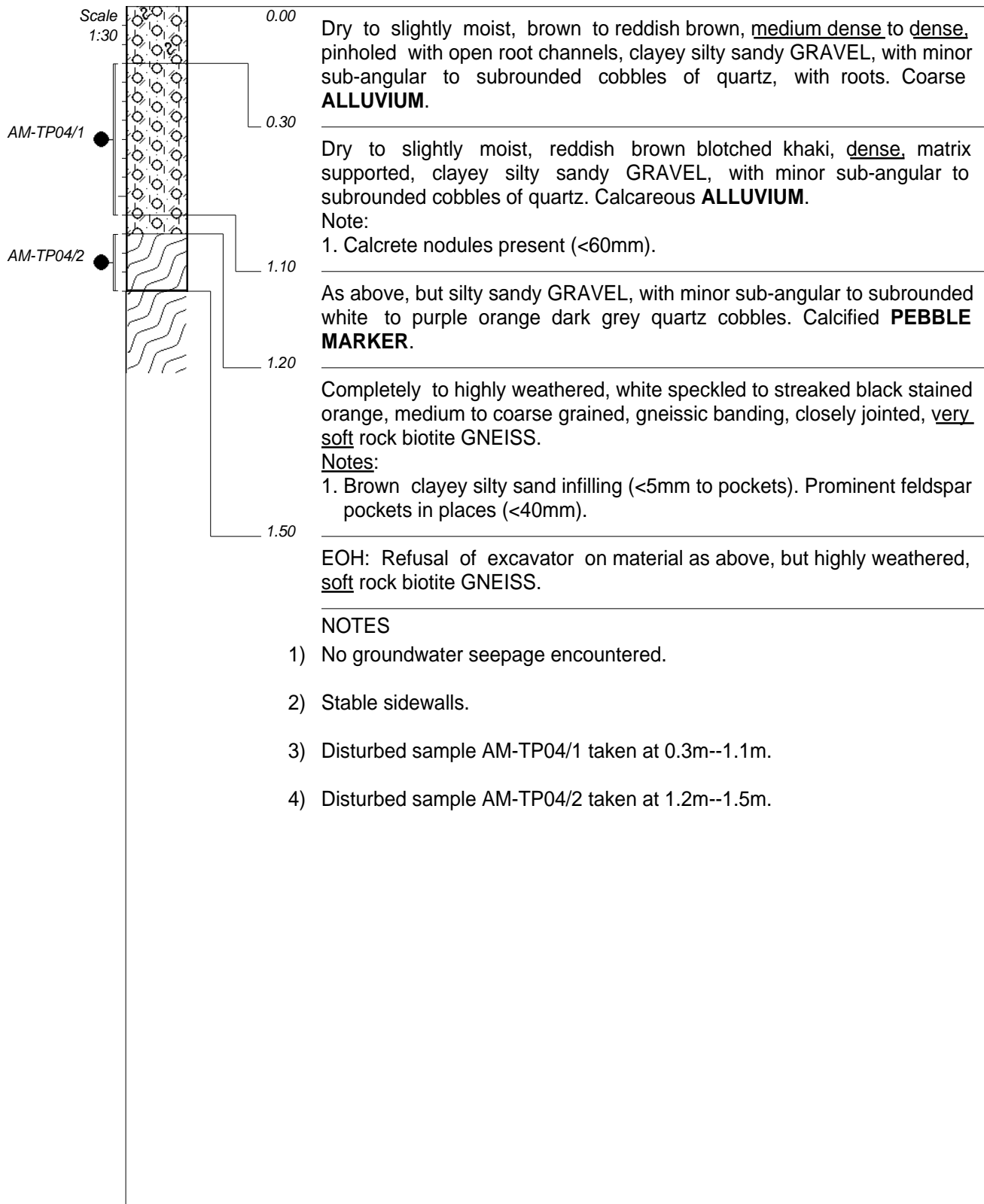
INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806392
Y-COORD : 7583799

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

HOLE No: AM-TP03



CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

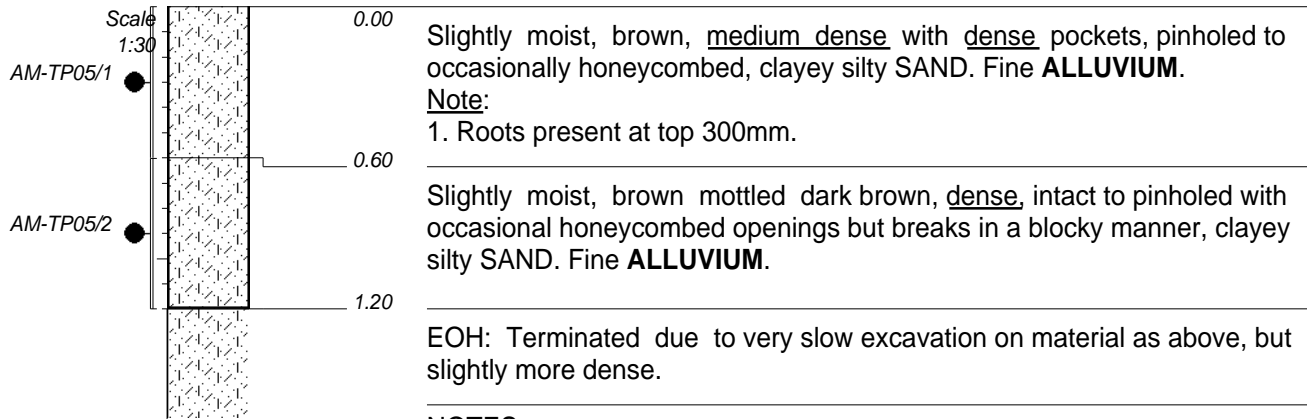
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806025
Y-COORD : 7583714

HOLE No: AM-TP04



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP05/1 taken at 0.0m--0.6m.
- 4) Disturbed sample AM-TP05/2 taken at 0.6m--1.2m.

CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

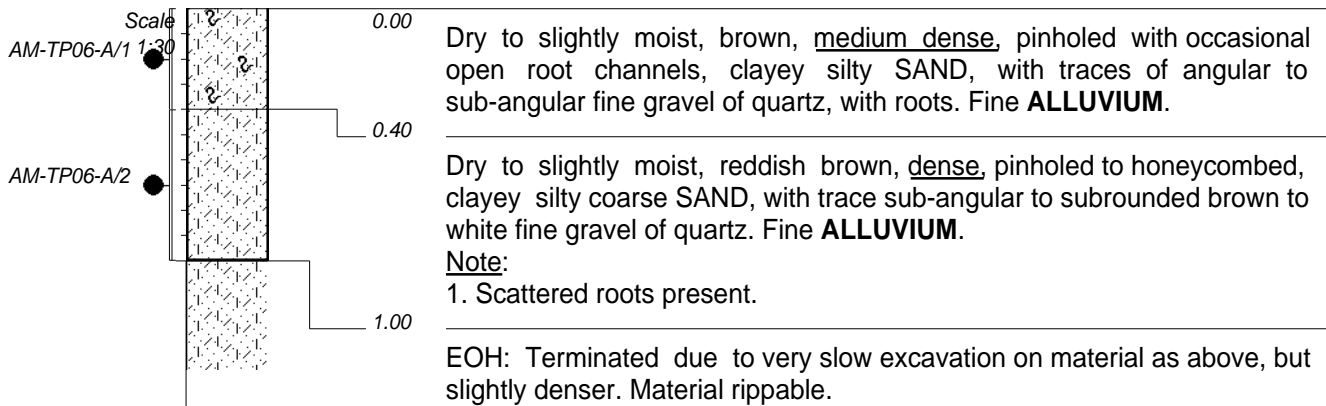
INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 30 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806080
Y-COORD : 7583500

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

HOLE No: AM-TP05



Note:

1. Scattered roots present.

EOH: Terminated due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP06-A/1 taken at 0.0m--0.4m.
- 4) Disturbed sample AM-TP06-A/2 taken at 0.4m--1.0m.

CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

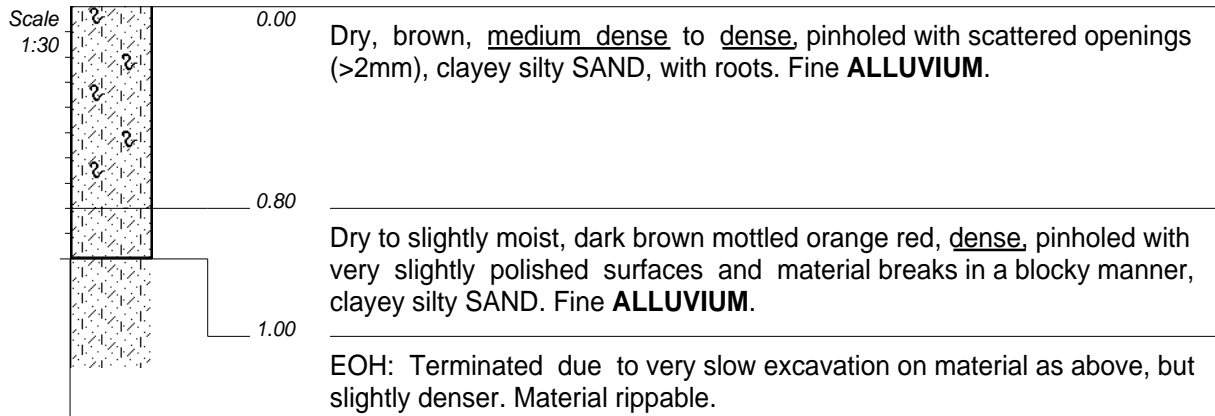
INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 30 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806402
Y-COORD : 7583603

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

HOLE No: AM-TP06-A



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

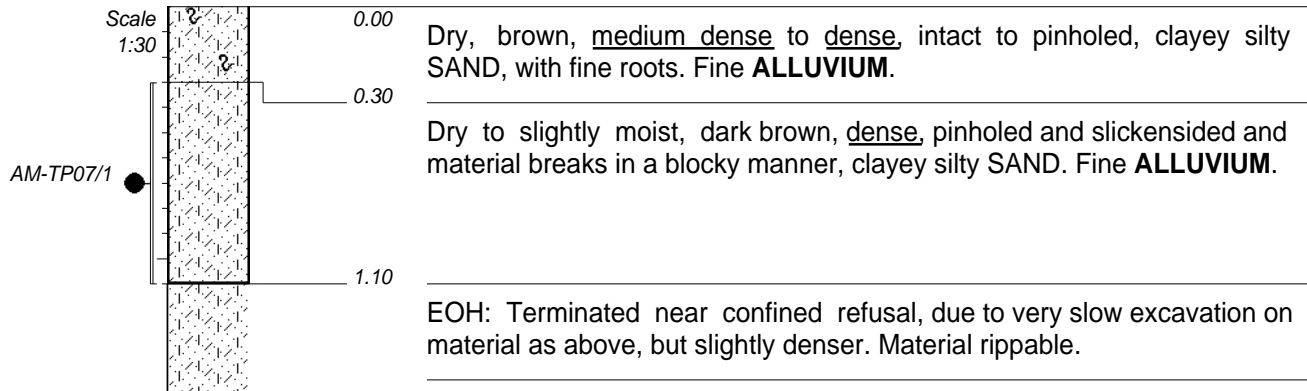
CONTRACTOR : Sandworx
MACHINE : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 30 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806389
Y-COORD : 7583550

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP07/1 taken at 0.3m--1.1m.

CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

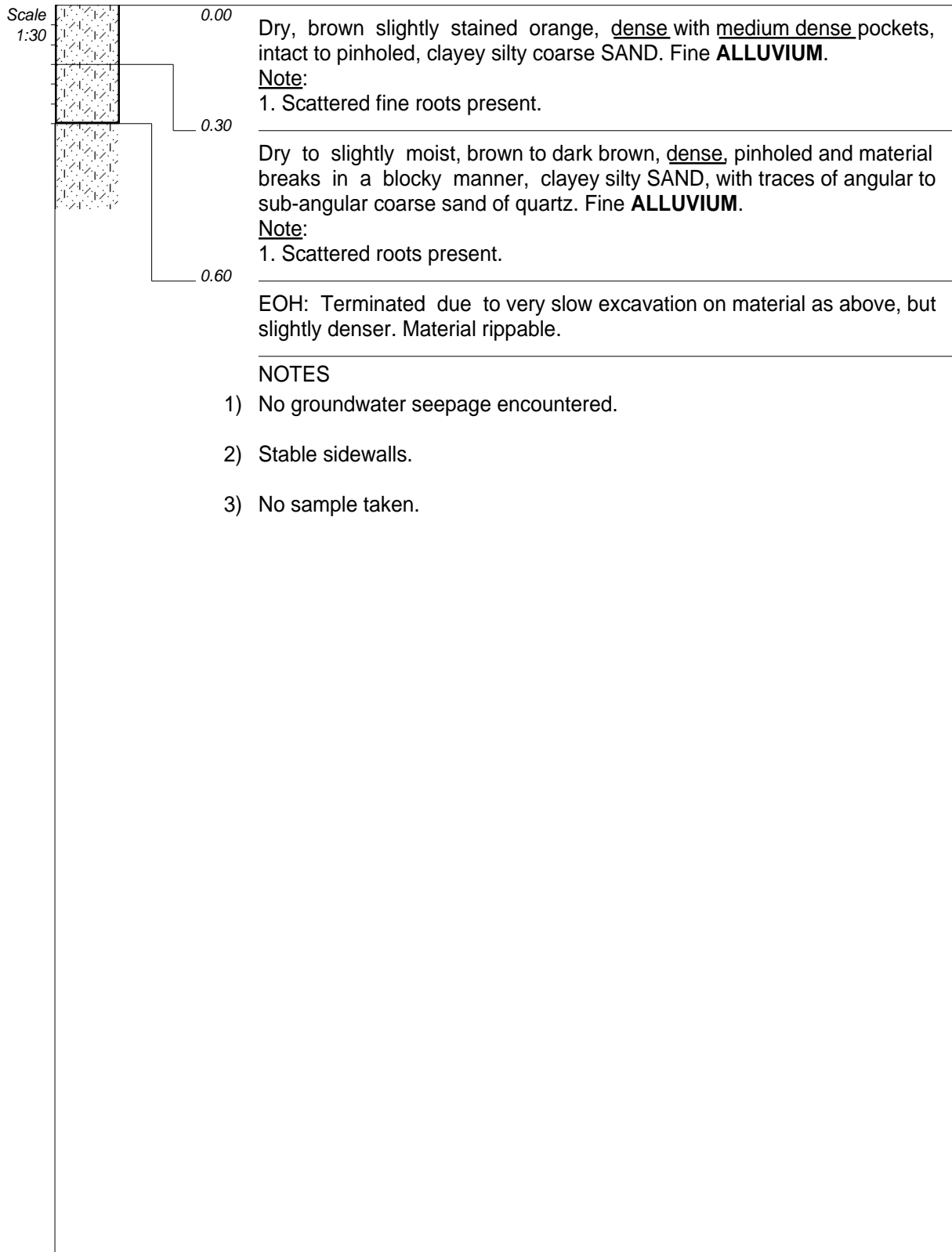
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 30 May 2023

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0806585
Y-COORD : 7583438

HOLE No: AM-TP07



CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

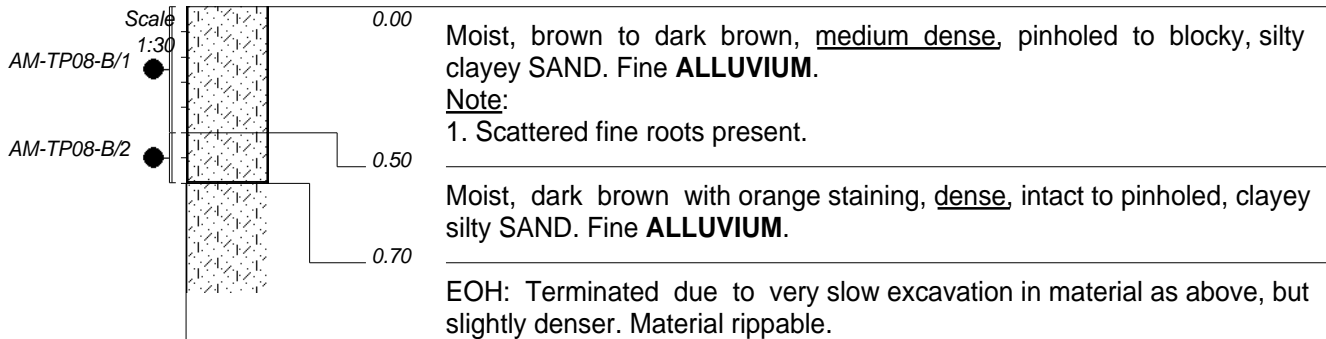
INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0807041
Y-COORD : 7583485

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

HOLE No: AM-TP08-A



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Sample AM-TP08-B/1 taken at 0.0m--0.5m.
- 4) Sample AM-TP08-B/2 taken at 0.5m--0.7m.

CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

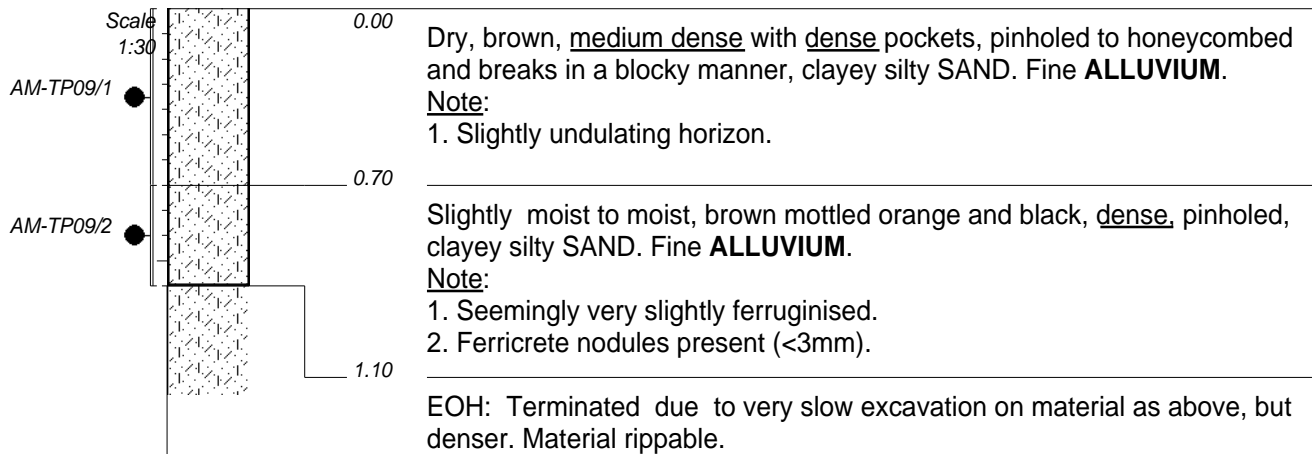
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0807035
Y-COORD : 7583446

HOLE No: AM-TP08-B



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP09/1 taken from 0.0m--0.7m.
- 4) Disturbed sample AM-TP09/2 taken from 0.7m--1.1m.

CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

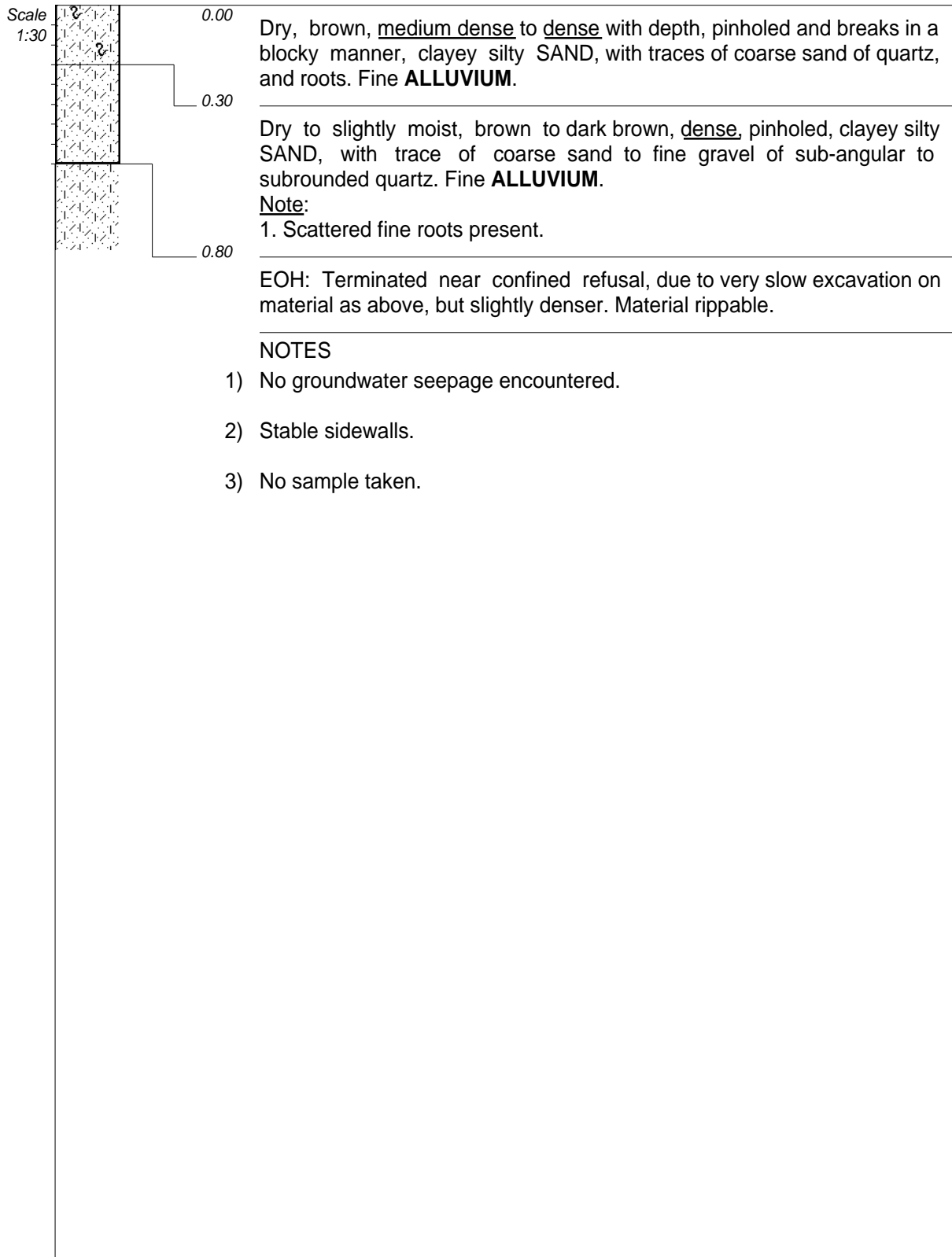
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0807324
Y-COORD : 7583323

HOLE No: AM-TP09



CONTRACTOR : Sandworx
MACHINE : : Lovol FR220D
DRILLED BY : Joseph Hosain
PROFILED BY : Chris Homan

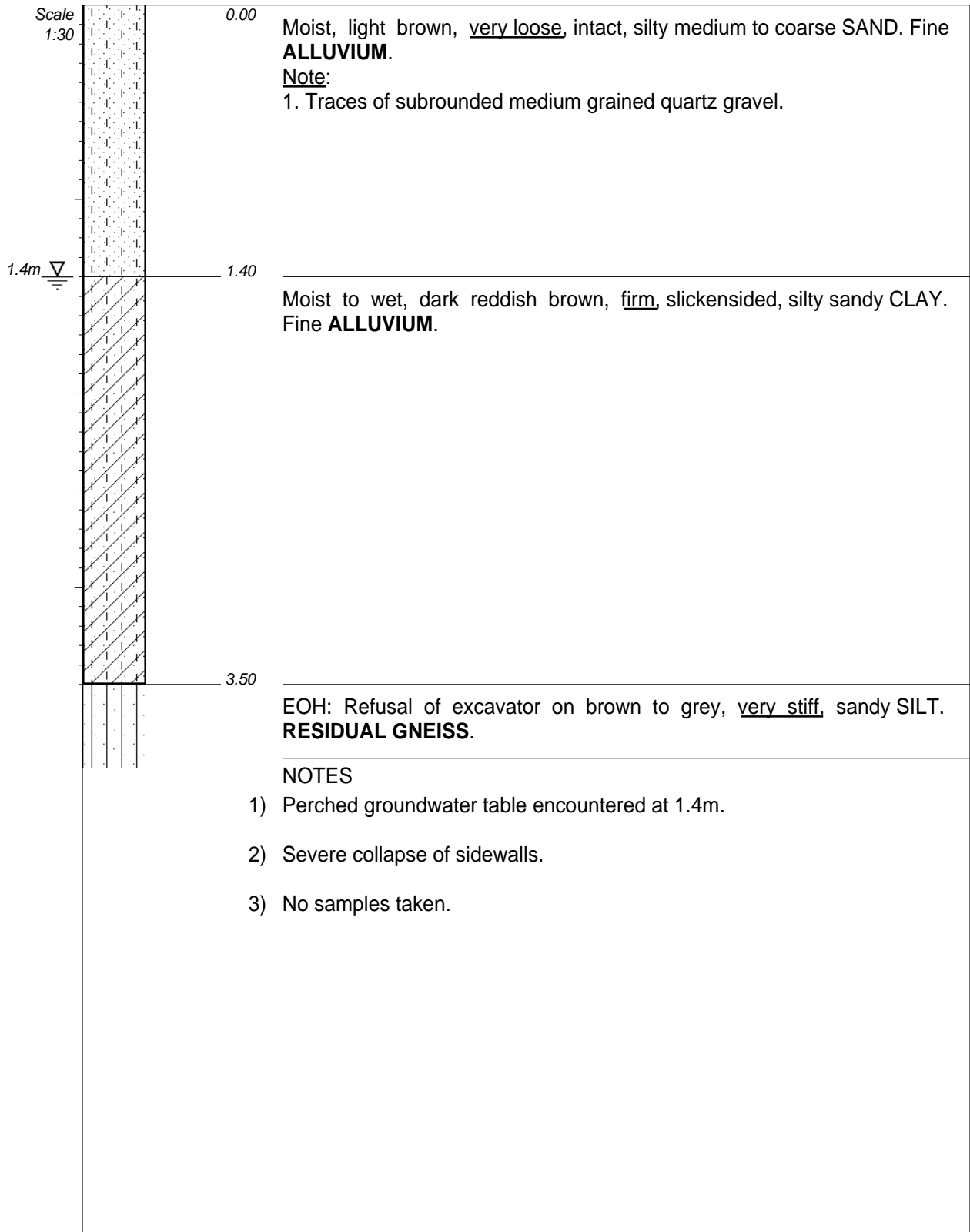
INCLINATION : Vertical
DIAM : 1200
DATE : 31 May 2023
DATE : 31 May 2023

COORDINATE SYSTEM : UTM,WGS84(33K)
X-COORD : 0807340
Y-COORD : 7583630

TYPE SET BY : EM
SETUP FILE : KPTP8.SET

DATE : 04/07/2023 13:47
TEXT : ..P51\PROFILES\PKFAMTP.TXT

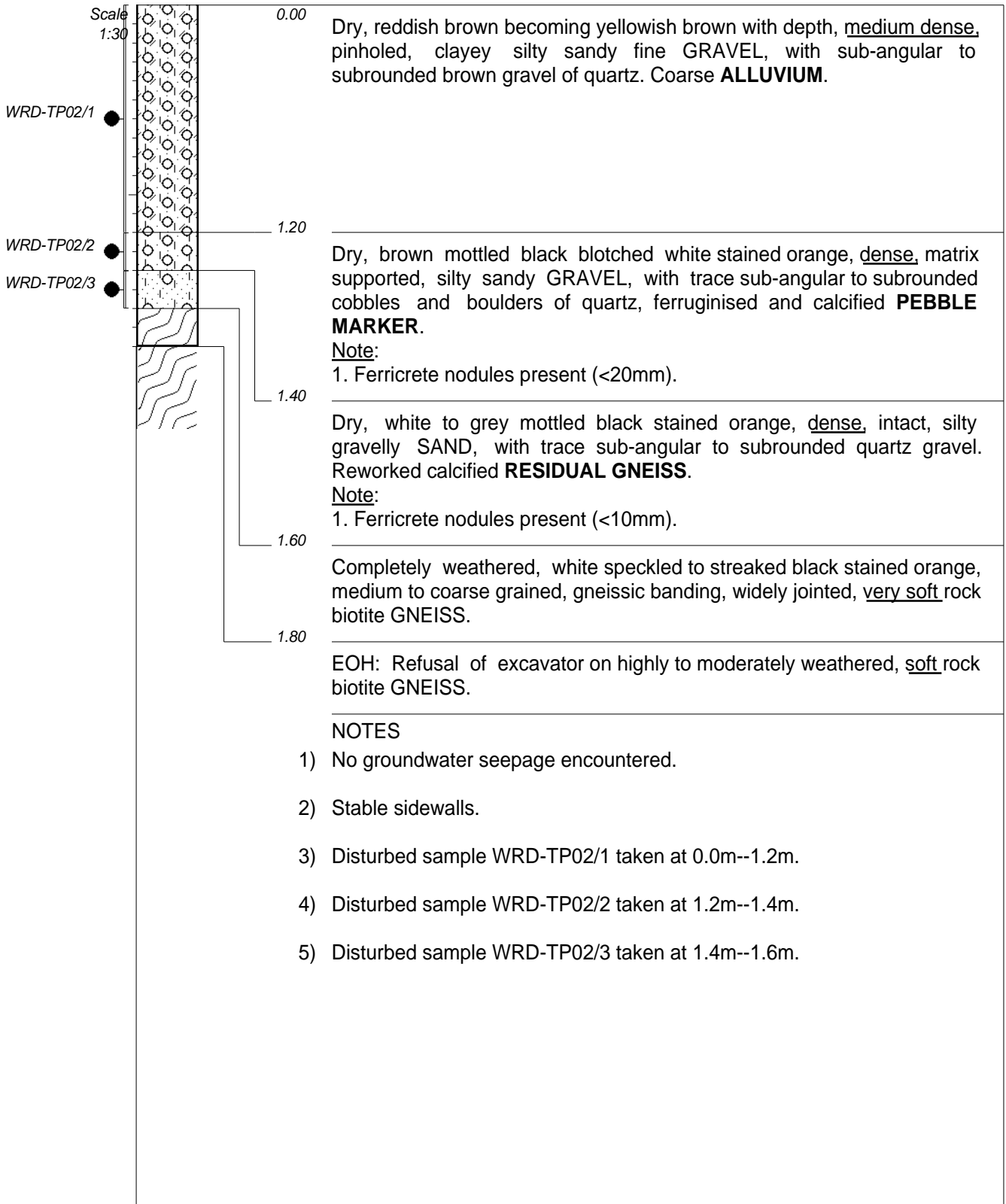
HOLE No: AM-TP10



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KTP8.SET

INCLINATION : Vertical
DIAM :
DATE : 13 May 2023
DATE : 13 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801782.00
Y-COORD : 7582015.00

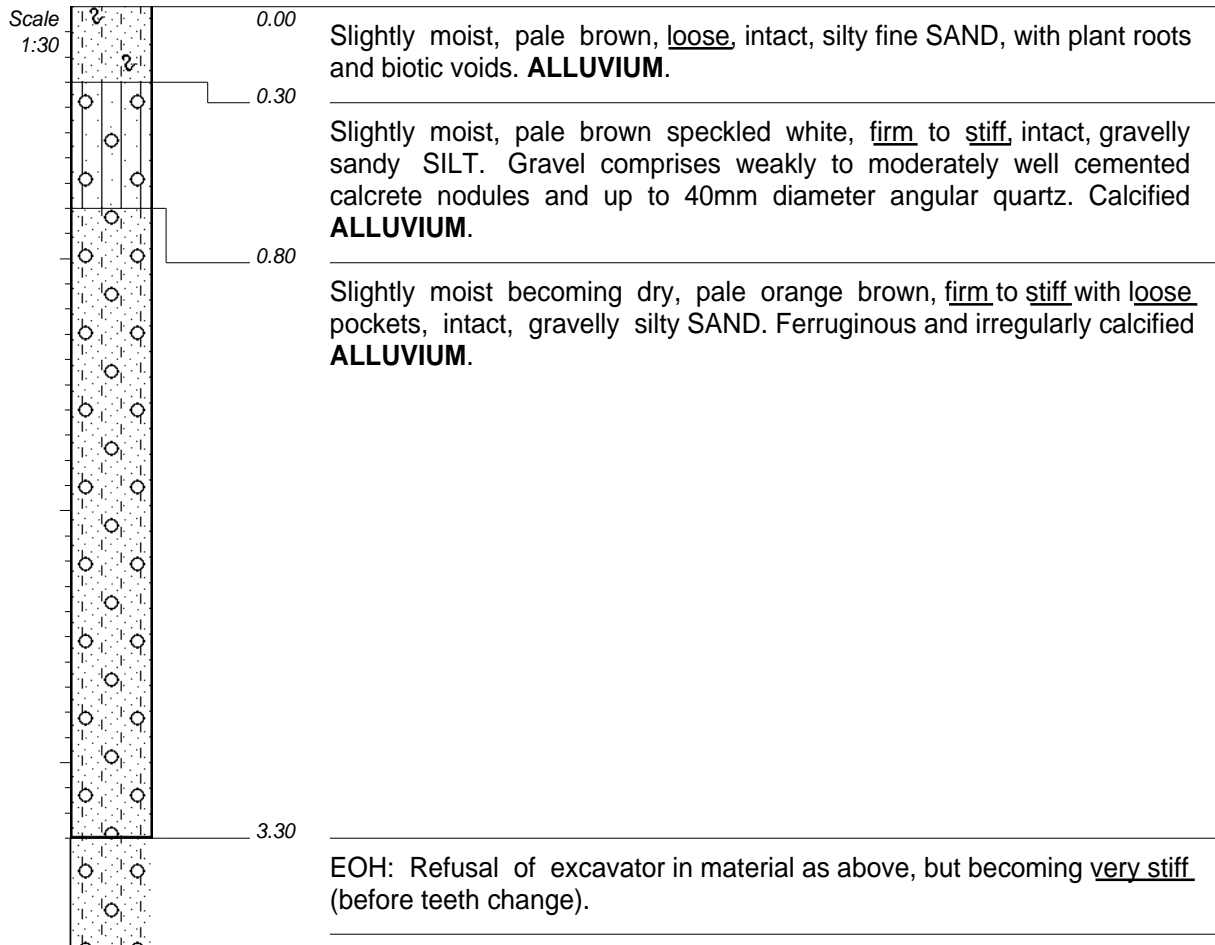


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0802343
Y-COORD : 7582645

HOLE No: WRD-TP02



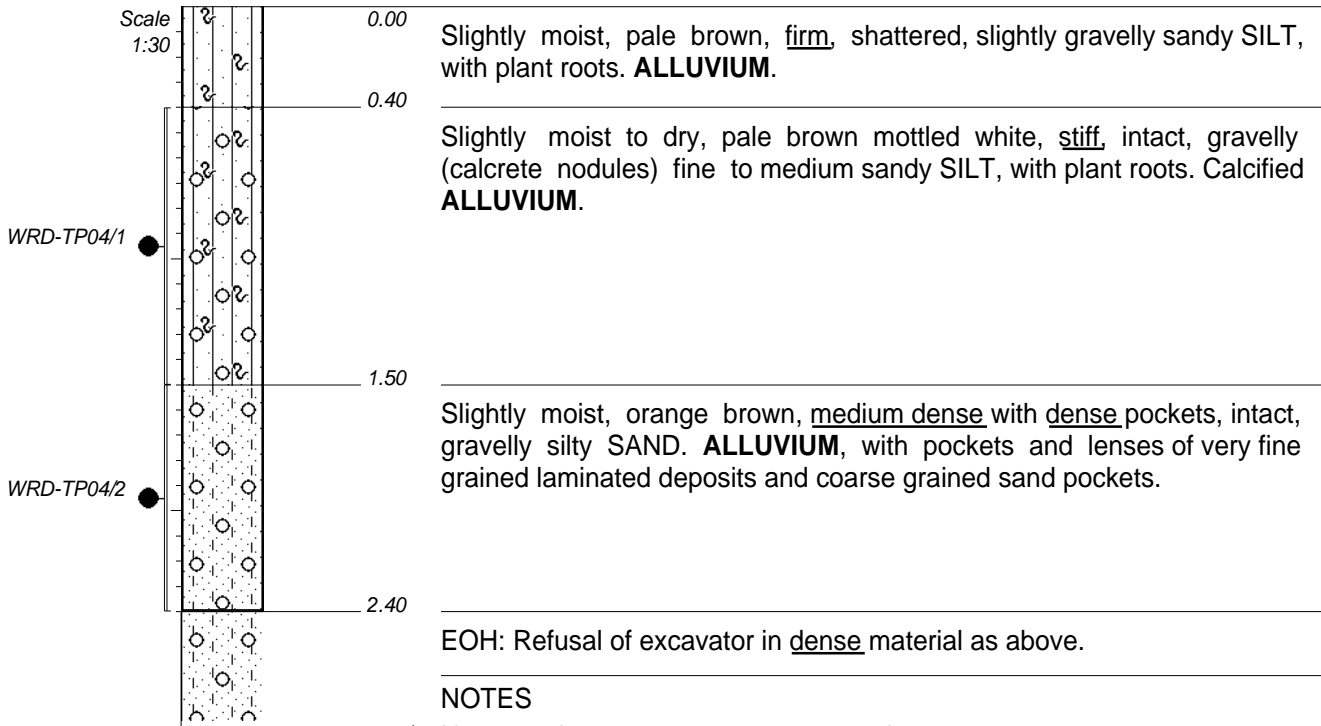
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.
- 4) Excavator teeth are not sharp causing premature refusal.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 10 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 80233280
Y-COORD : 7582281.00



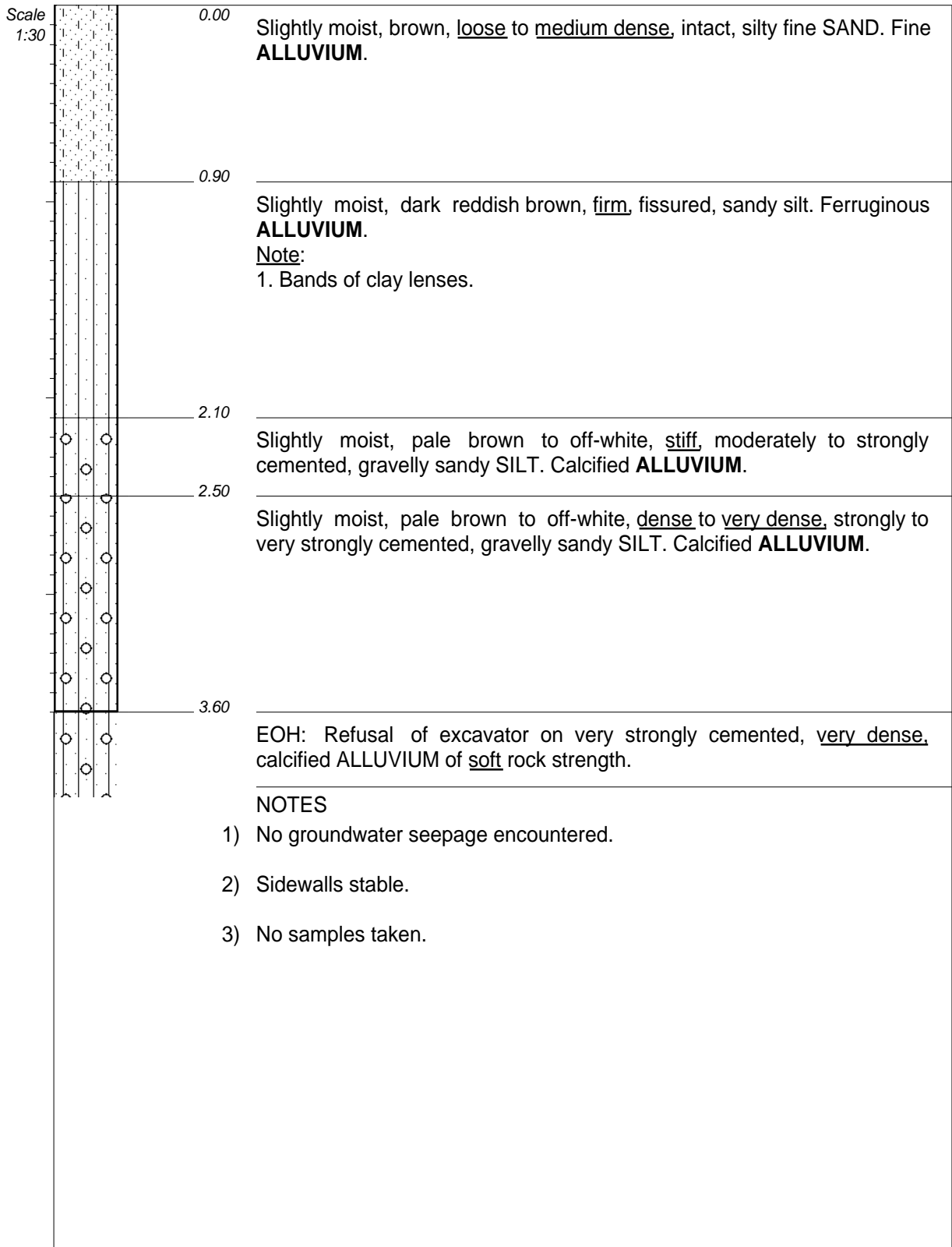
NOTES

- 1) No groundwater seepage encountered.
- 2) No further penetration by excavator (before teeth change).
- 3) Disturbed samples WRD-TP04/1 taken from 0.4m--1.5m.
- 4) Disturbed samples WRD-TP04/2 taken from 1.5m--2.4m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 10 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

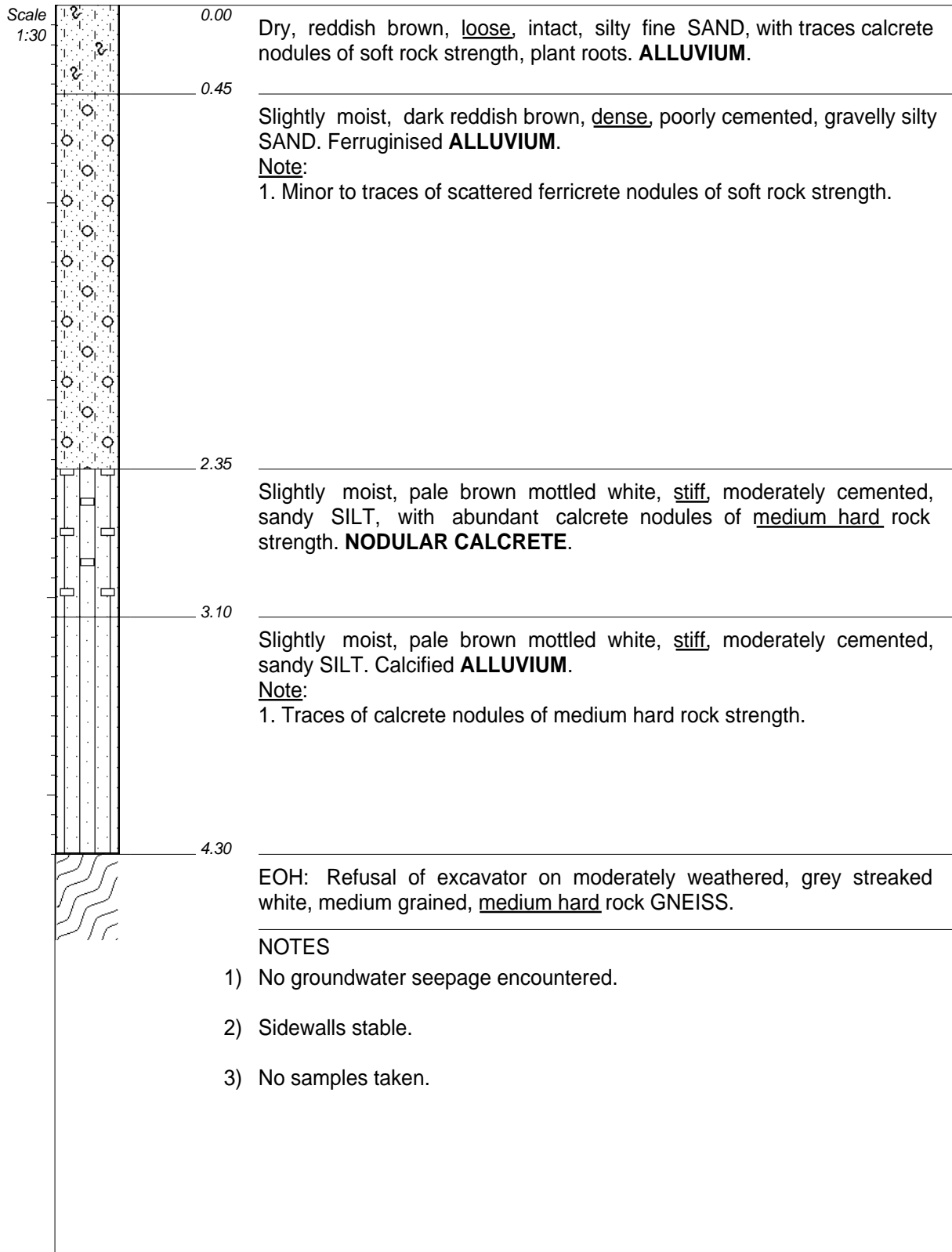
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 801993.39
Y-COORD : 7582129.46



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 13 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

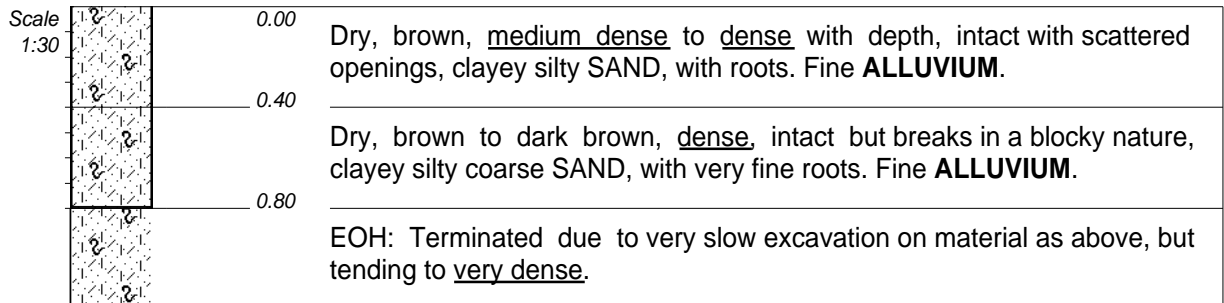
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802728.00
Y-COORD : 7582301.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPPT8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 13 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802341.11
Y-COORD : 7582119.35



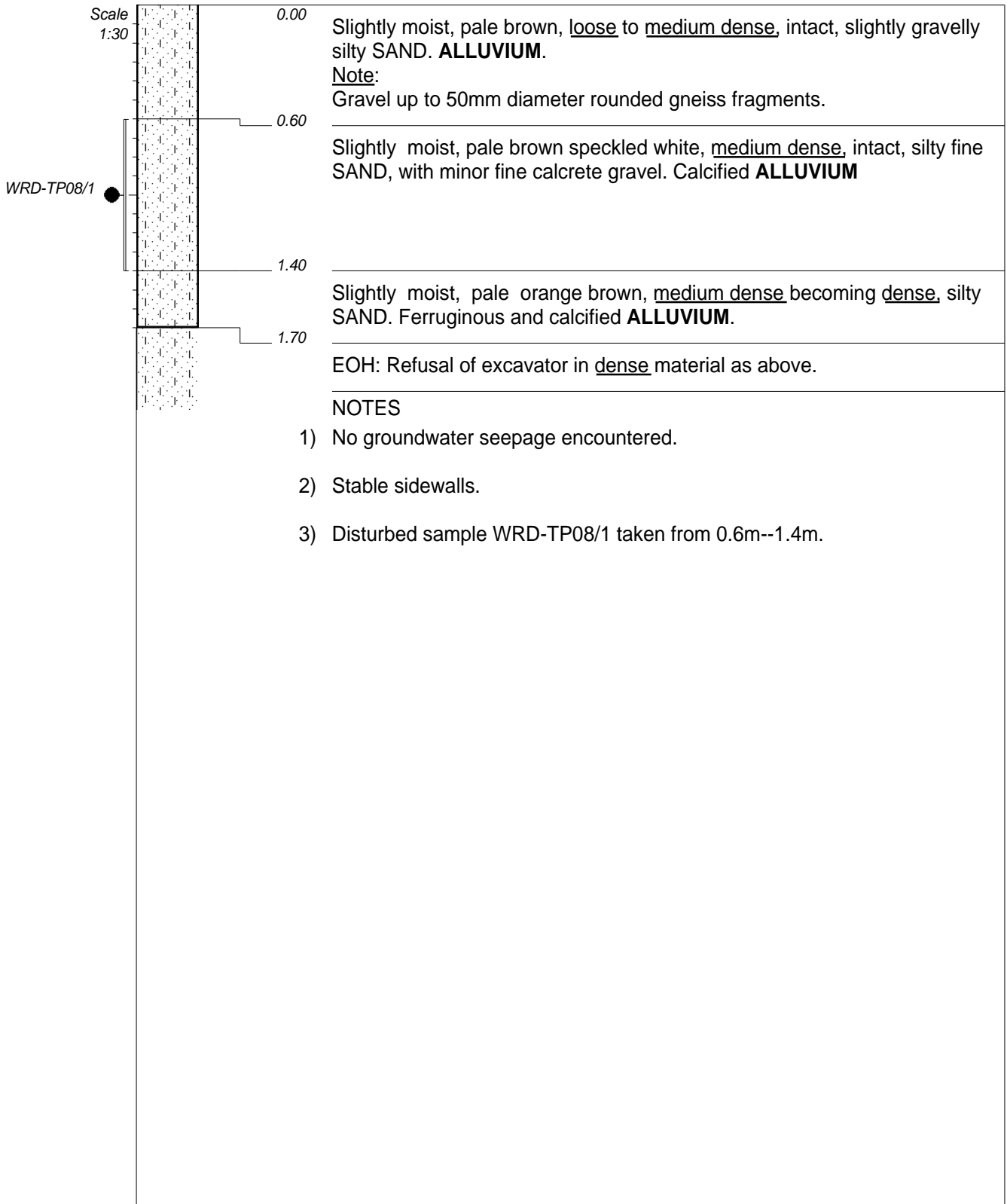
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

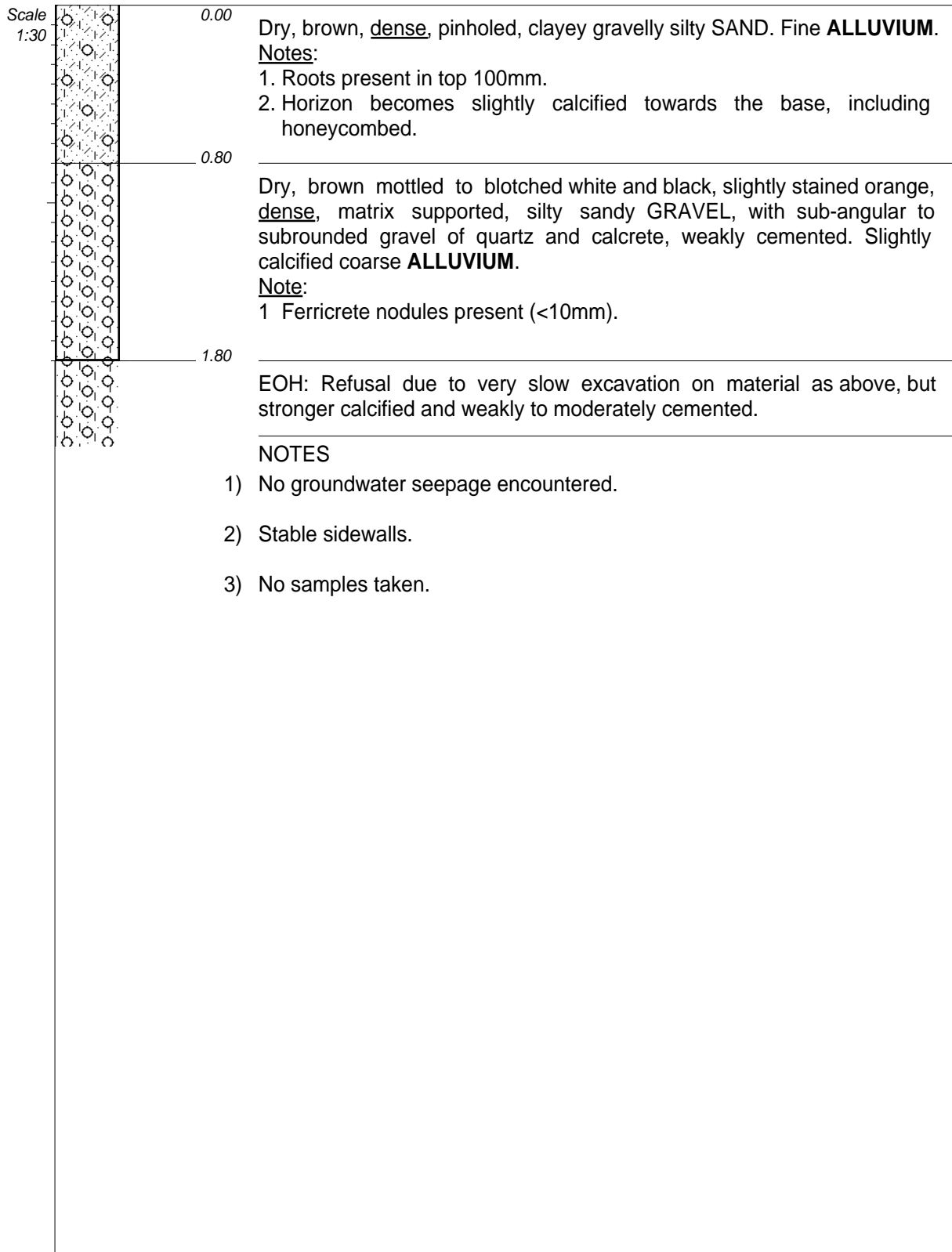
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0802532
Y-COORD : 7581769



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 10 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

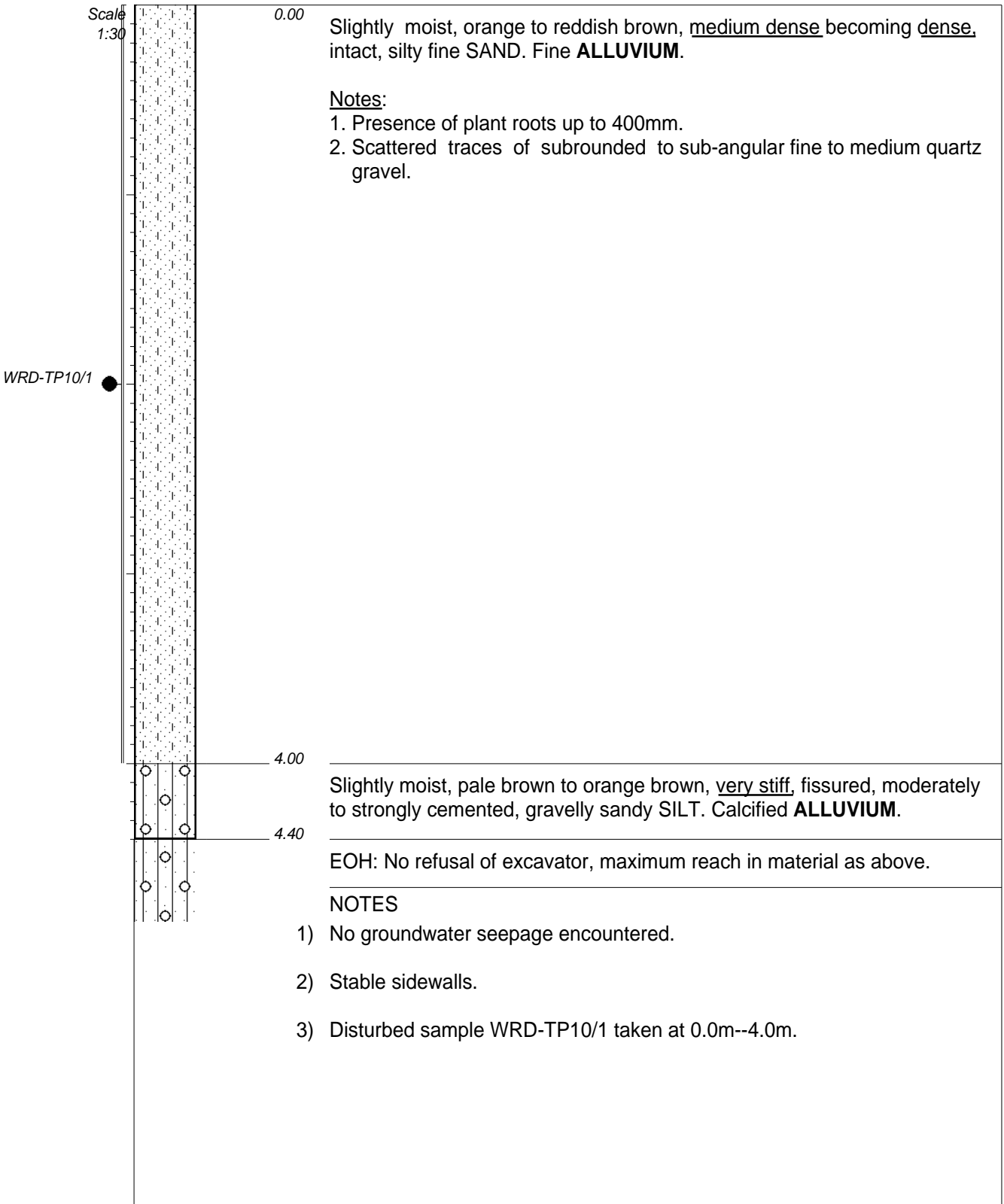
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802820.40
Y-COORD : 7582154.58



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

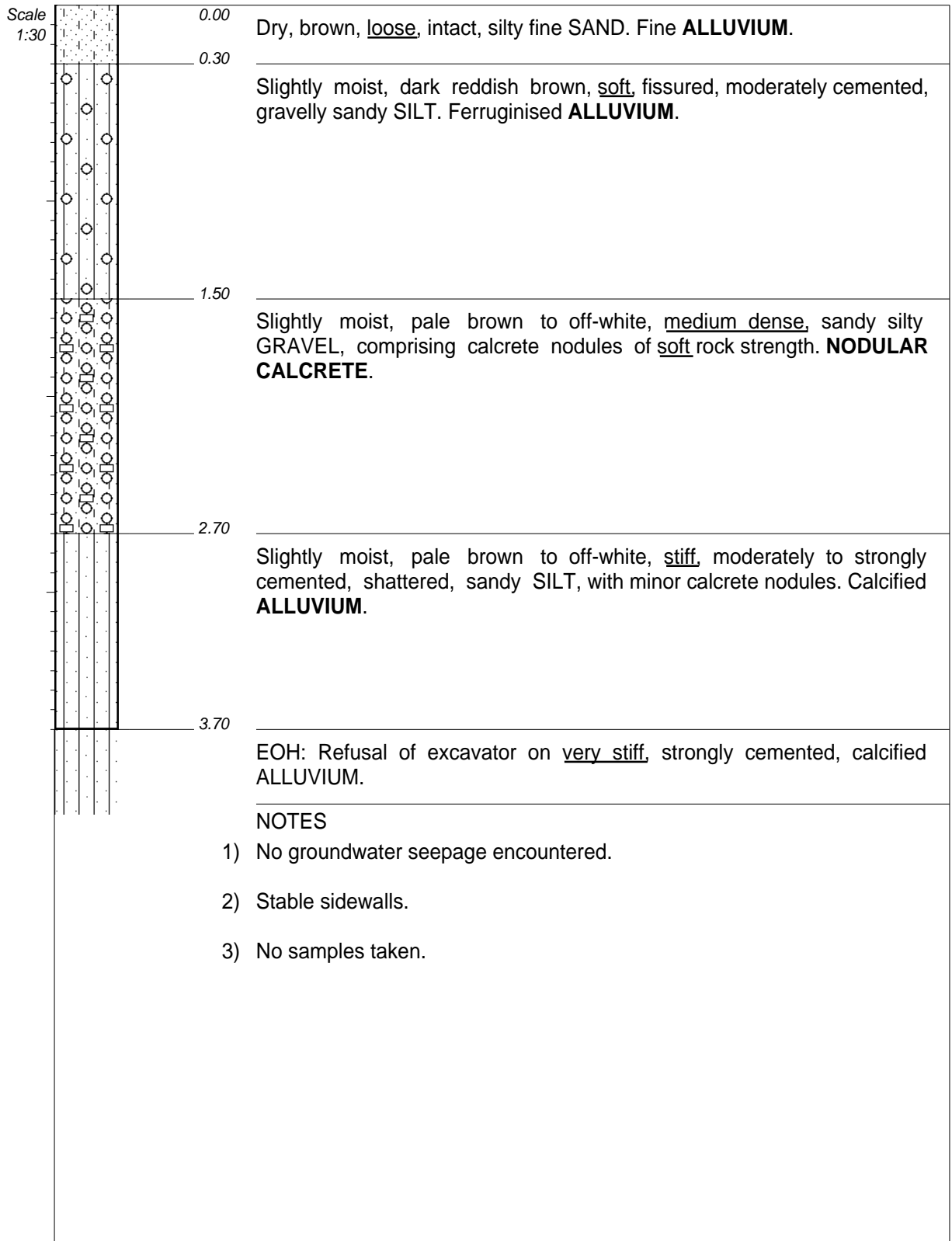
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0802802
Y-COORD : 7582018



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

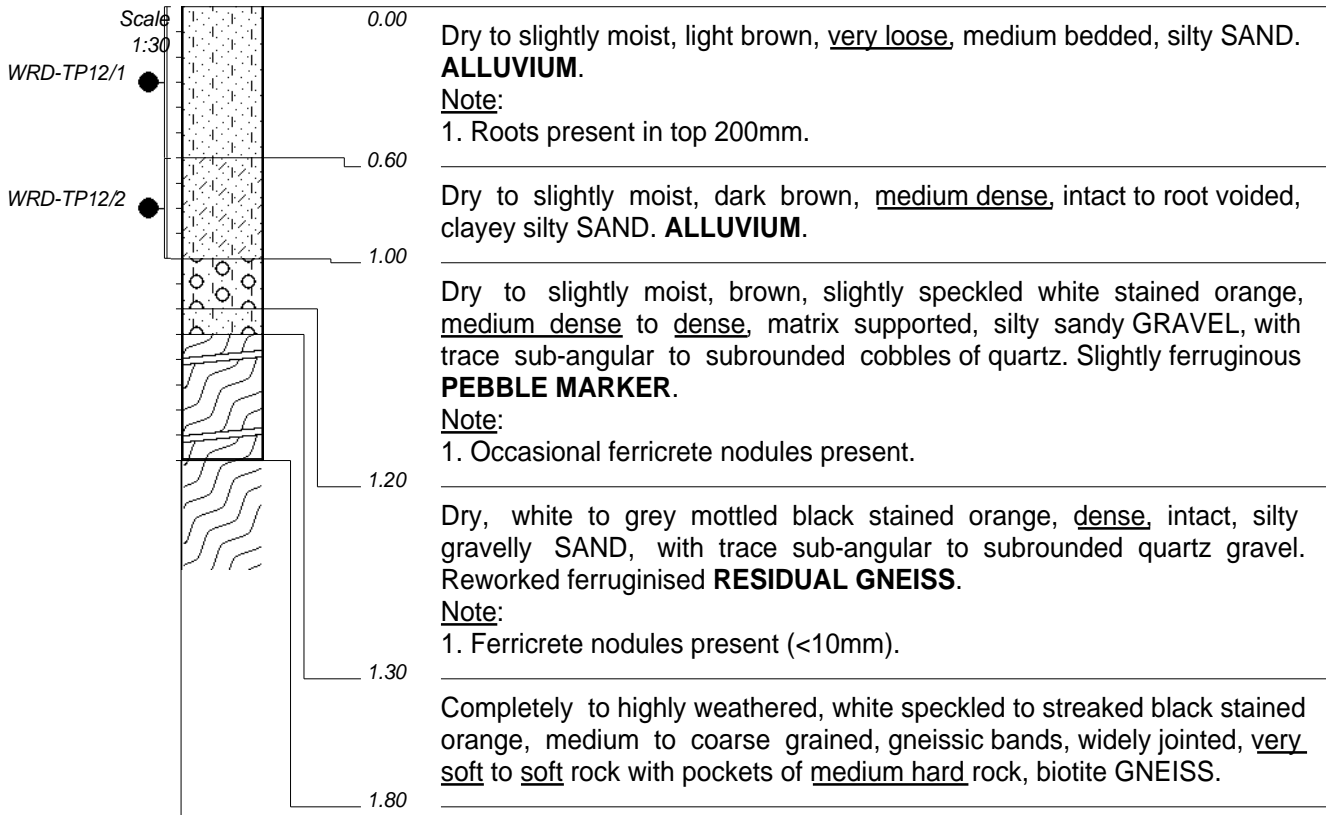
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803055.00
Y-COORD : 7581743.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803420.00
Y-COORD : 7581961.00



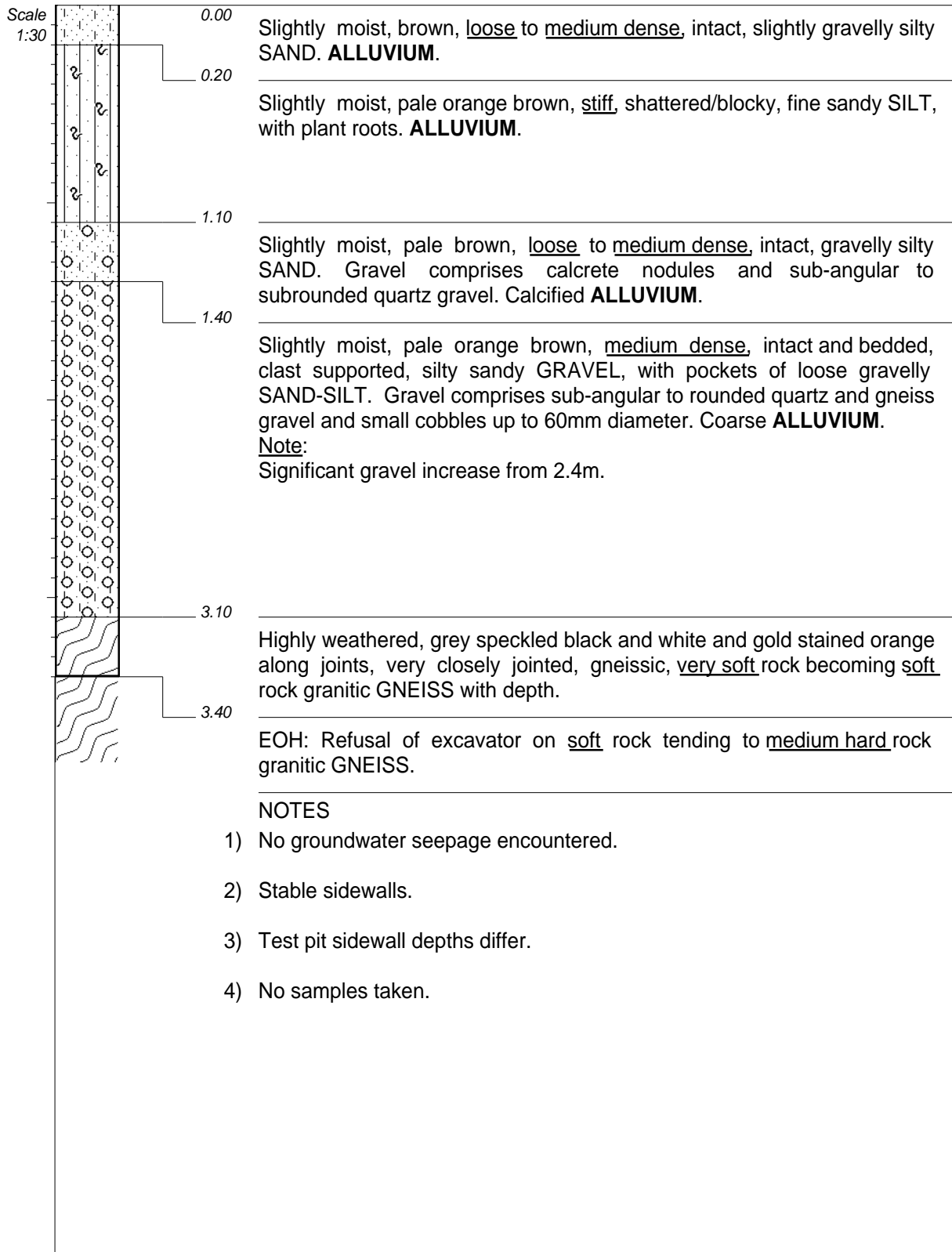
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP12/1 taken at 0.0m--0.6m.
- 4) Disturbed sample WRD-TP12/2 taken at 0.6m--1.0m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 26 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

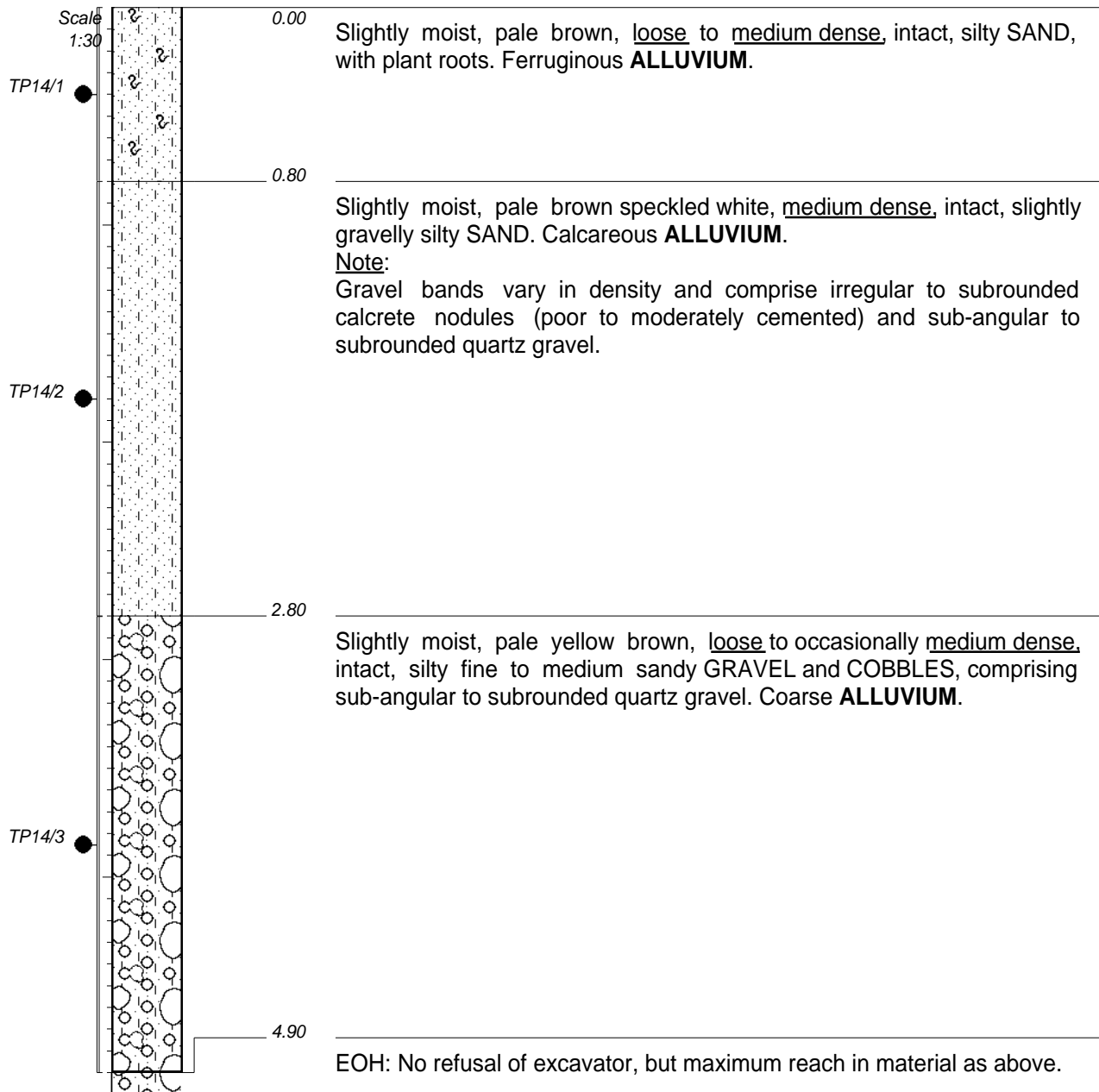
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0803082
Y-COORD : 7582525



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803424.70
Y-COORD : 7582333.22



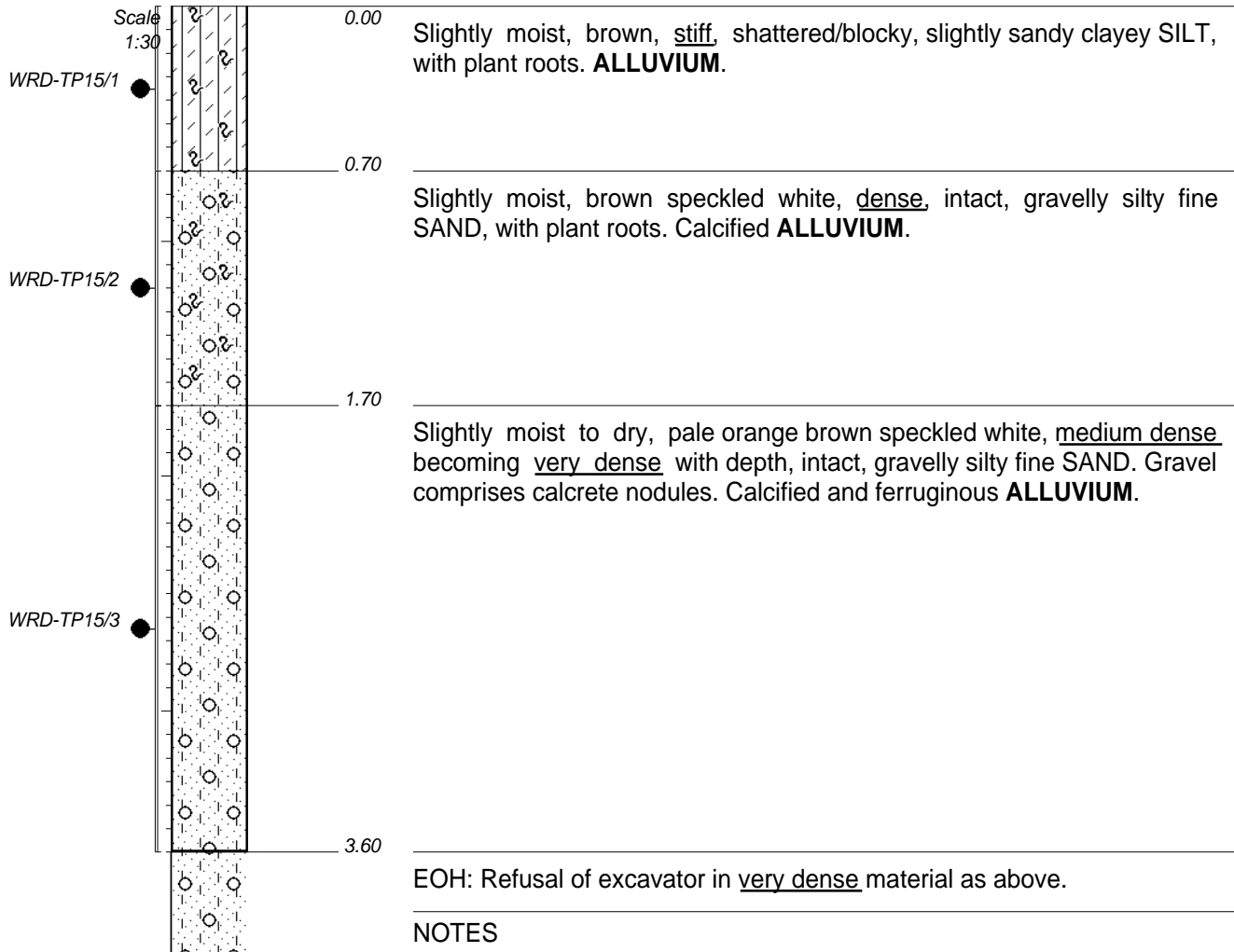
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed samples TP14/1 taken from 0.0m--0.8m.
- 4) Disturbed samples TP14/2 taken from 0.8m--2.8m.
- 5) Disturbed samples TP14/3 taken from 2.8m--4.9m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803530.99
Y-COORD : 7582575.57



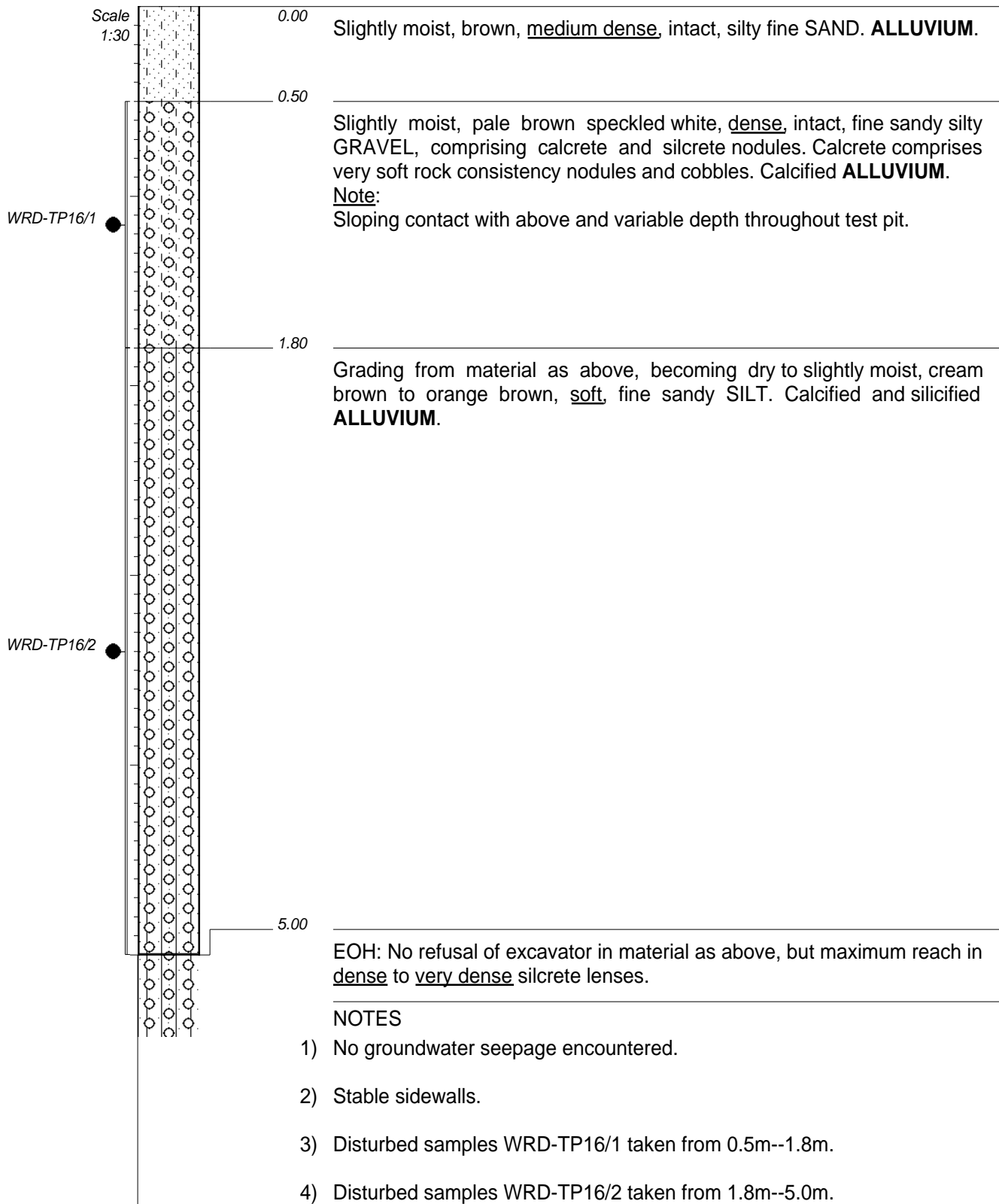
NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed samples WRD-TP15/1 taken from 0.0m--0.7m.
- 4) Disturbed sample WRD-TP15/2 taken from 0.7m--1.7m.
- 5) Disturbed samples WRD-TP15/3 taken from 1.7m--3.6m.

CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

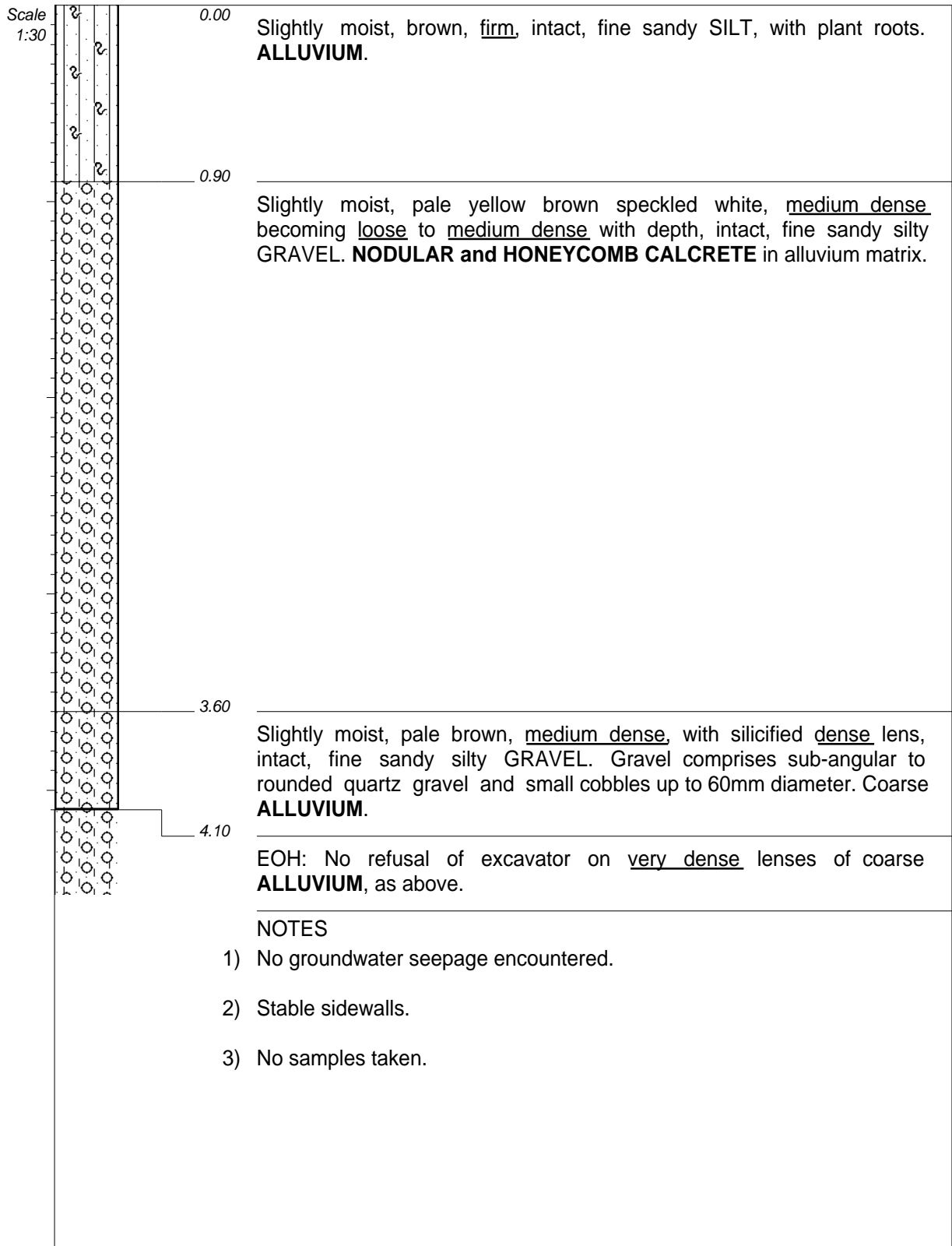
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803953.75
Y-COORD : 7582815.05



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

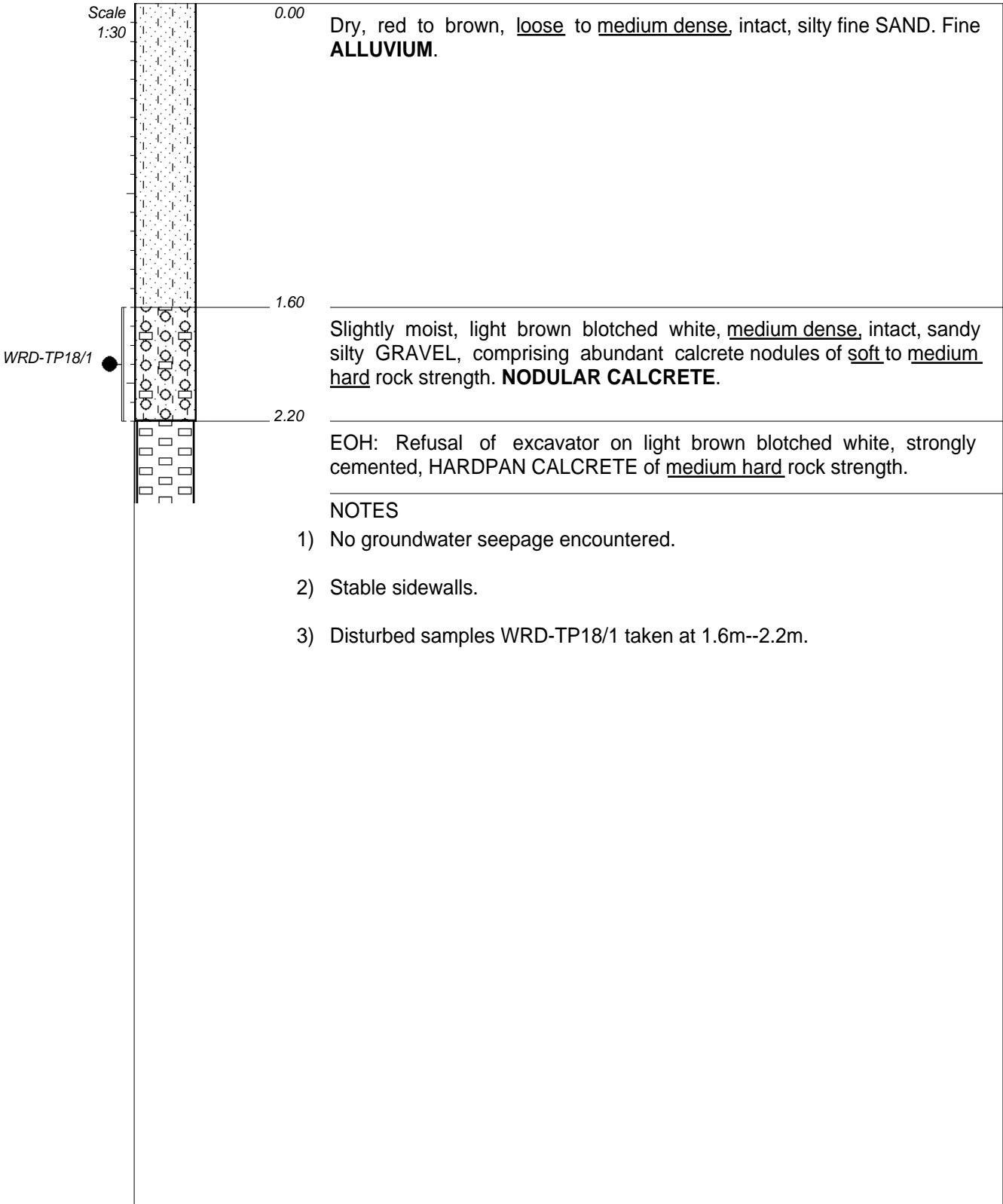
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803563.99
Y-COORD : 7582976.05



CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 11 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

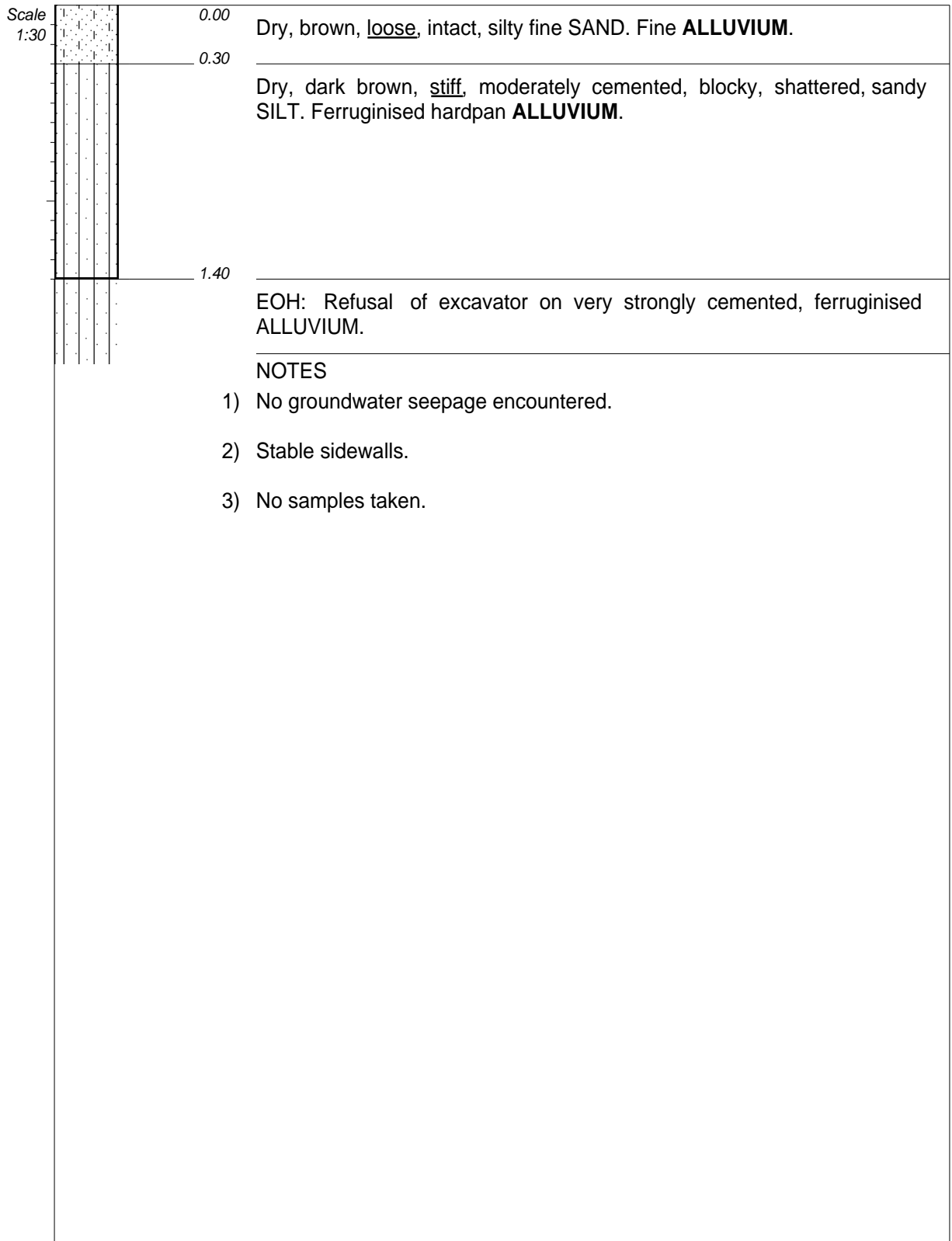
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804031.00
Y-COORD : 7583179.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 13 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

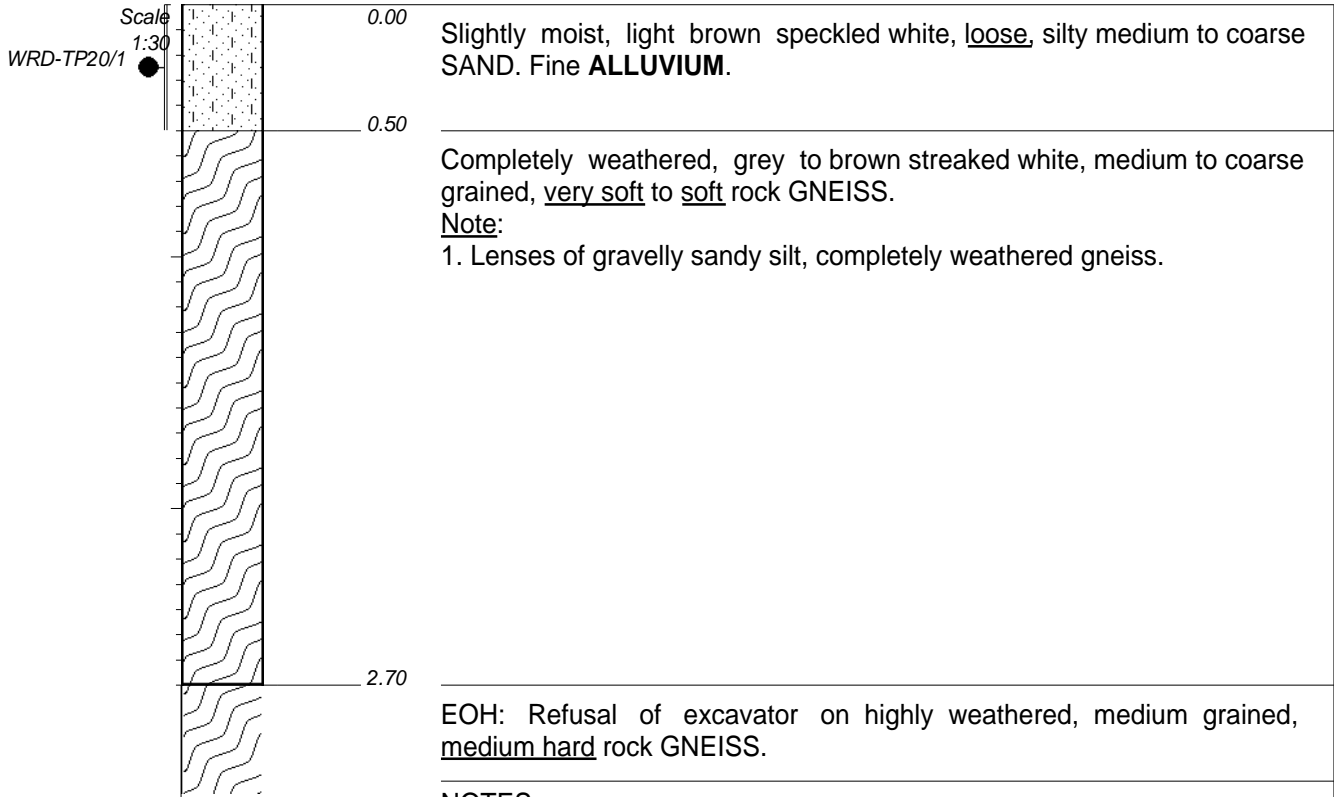
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804076.74
Y-COORD : 7583671.36



CONTRACTOR : Sandworx
 MACHINE : Excavator FR220D
 DRILLED BY : Joseph
 PROFILED BY : B Klaas
 TYPE SET BY : EM
 SETUP FILE : KPTP8.SET

INCLINATION : Vertical
 DIAM :
 DATE : May 2023
 DATE : 13 May 2023
 DATE : 07/07/2023 14:24
 TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
 X-COORD : 804095.95
 Y-COORD : 7583420.70



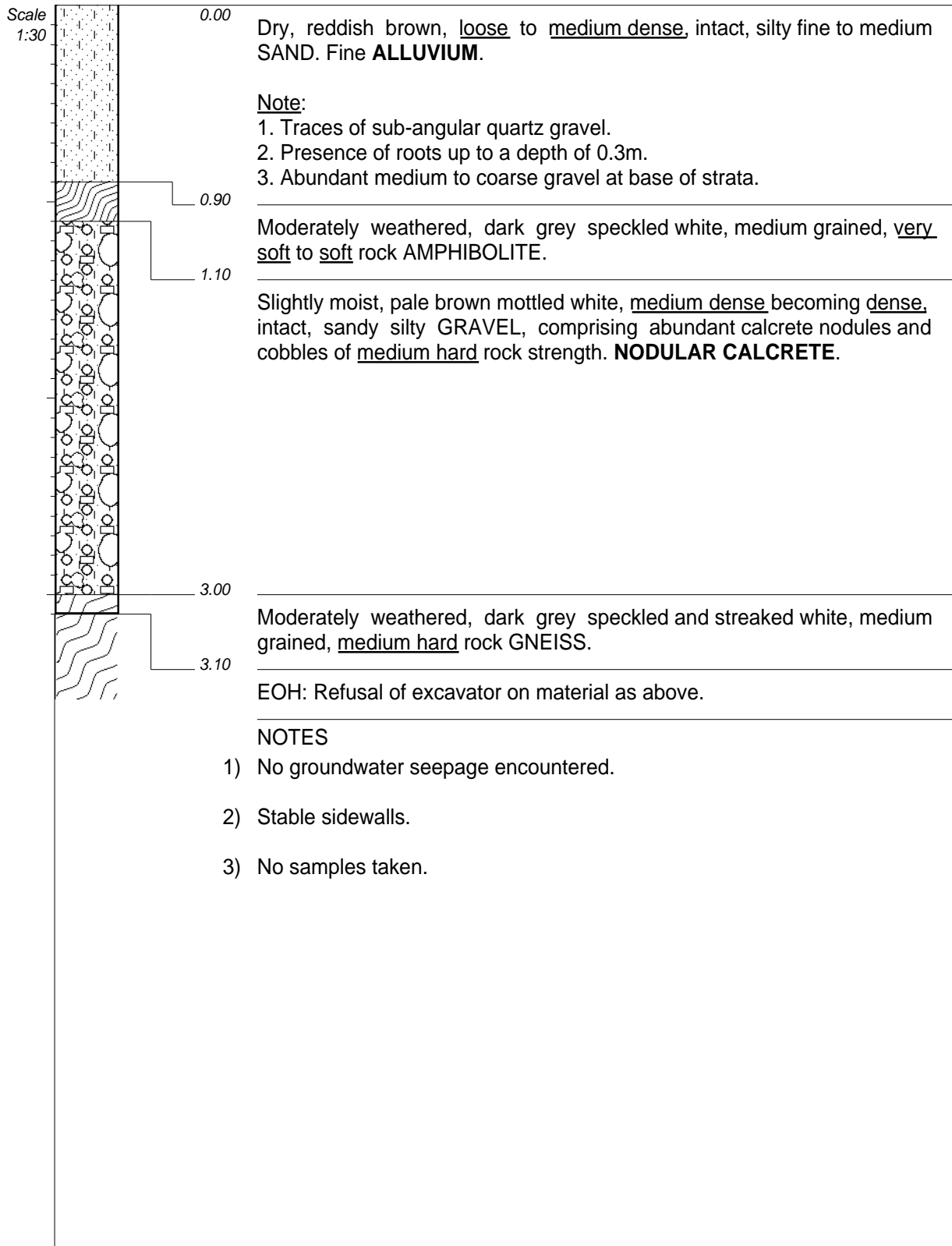
NOTES

- 1) No groundwater seepage encountered.
- 2) Collapse of sidewalls at 0.0m to 0.5m.
- 3) Disturbed samples WRD-TP20/1 taken at 0.0m--0.5m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

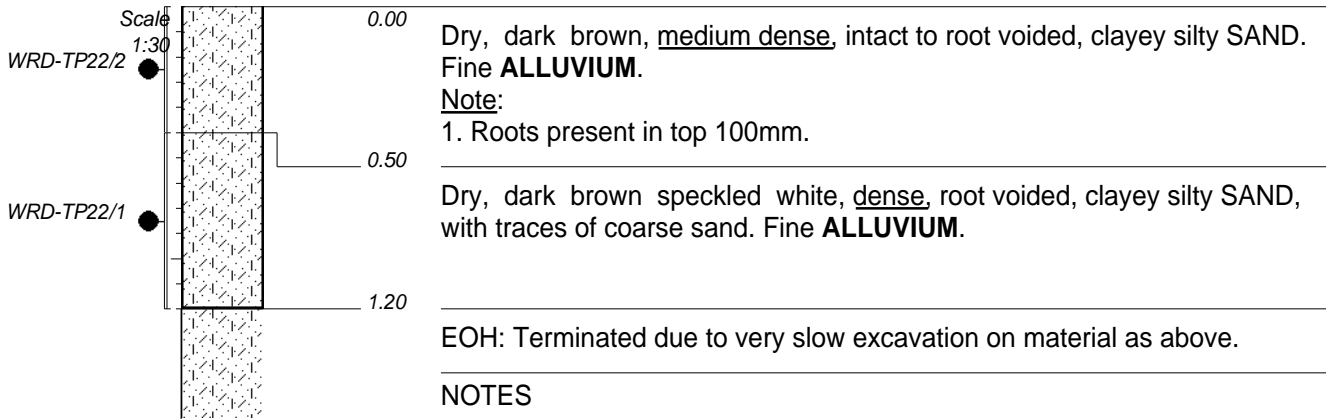
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804927.00
Y-COORD : 7583457.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804187.03
Y-COORD : 7584237.04



NOTES

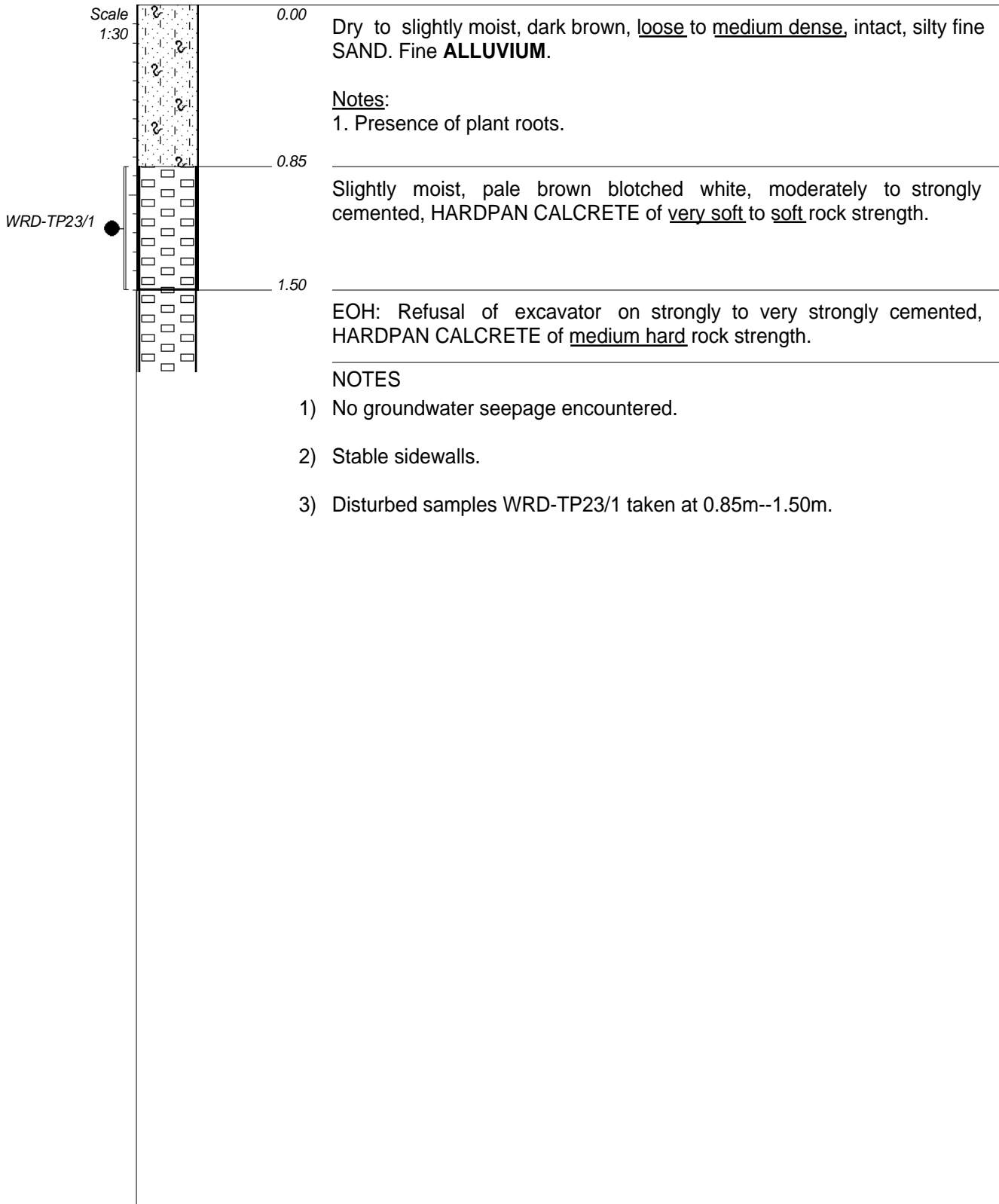
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP22/1 taken at 0.5m--1.2m.
- 4) Disturbed sample WRD-TP22/2 taken at 0.0m--0.5m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0805033
Y-COORD : 75824029

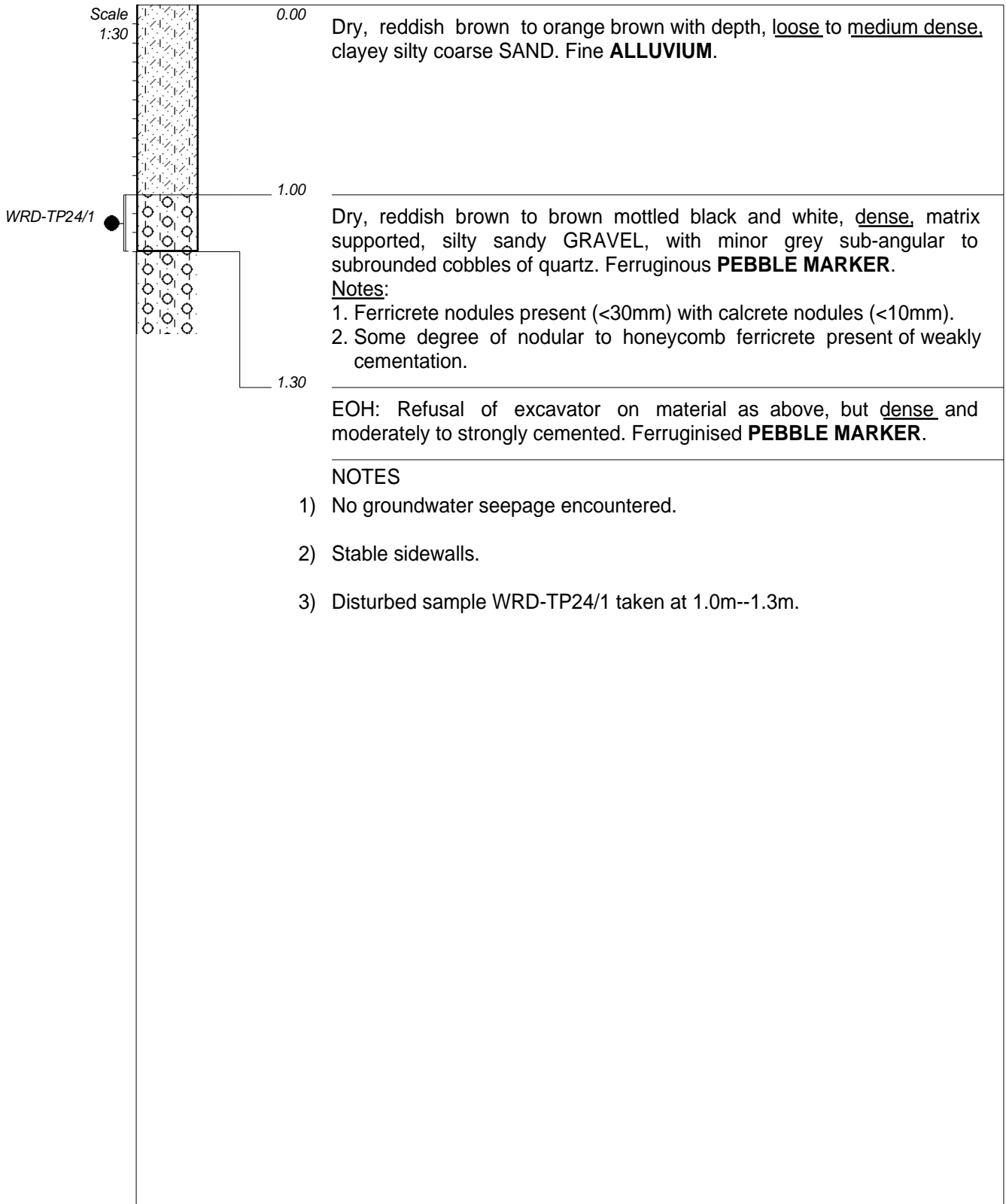
HOLE No: WRD-TP22



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804153.99
Y-COORD : 7584615.84

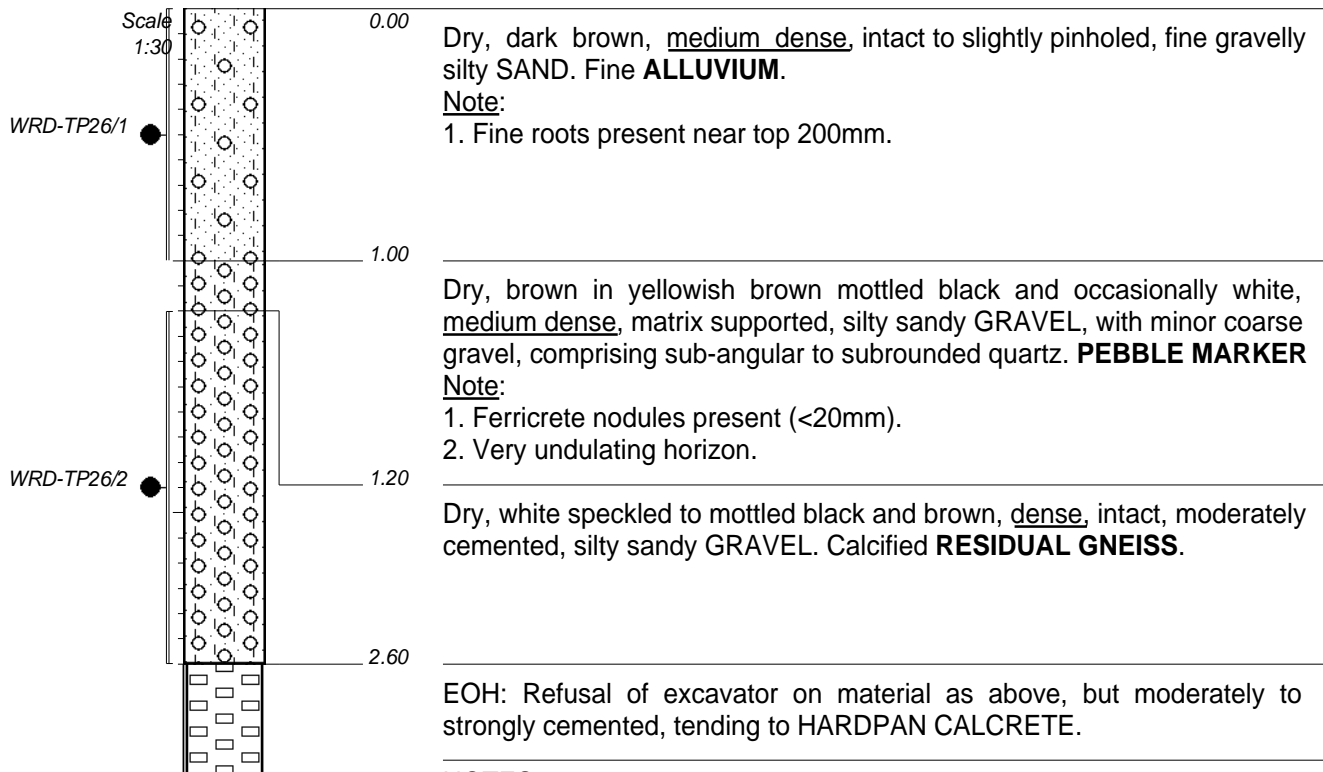


CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 25 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0805039
Y-COORD : 7584921

HOLE No: WRD-TP24



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP26/1 taken at 0.0m--1.0m.
- 4) Disturbed sample WRD-TP26/2 taken at 1.2m--2.6m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan

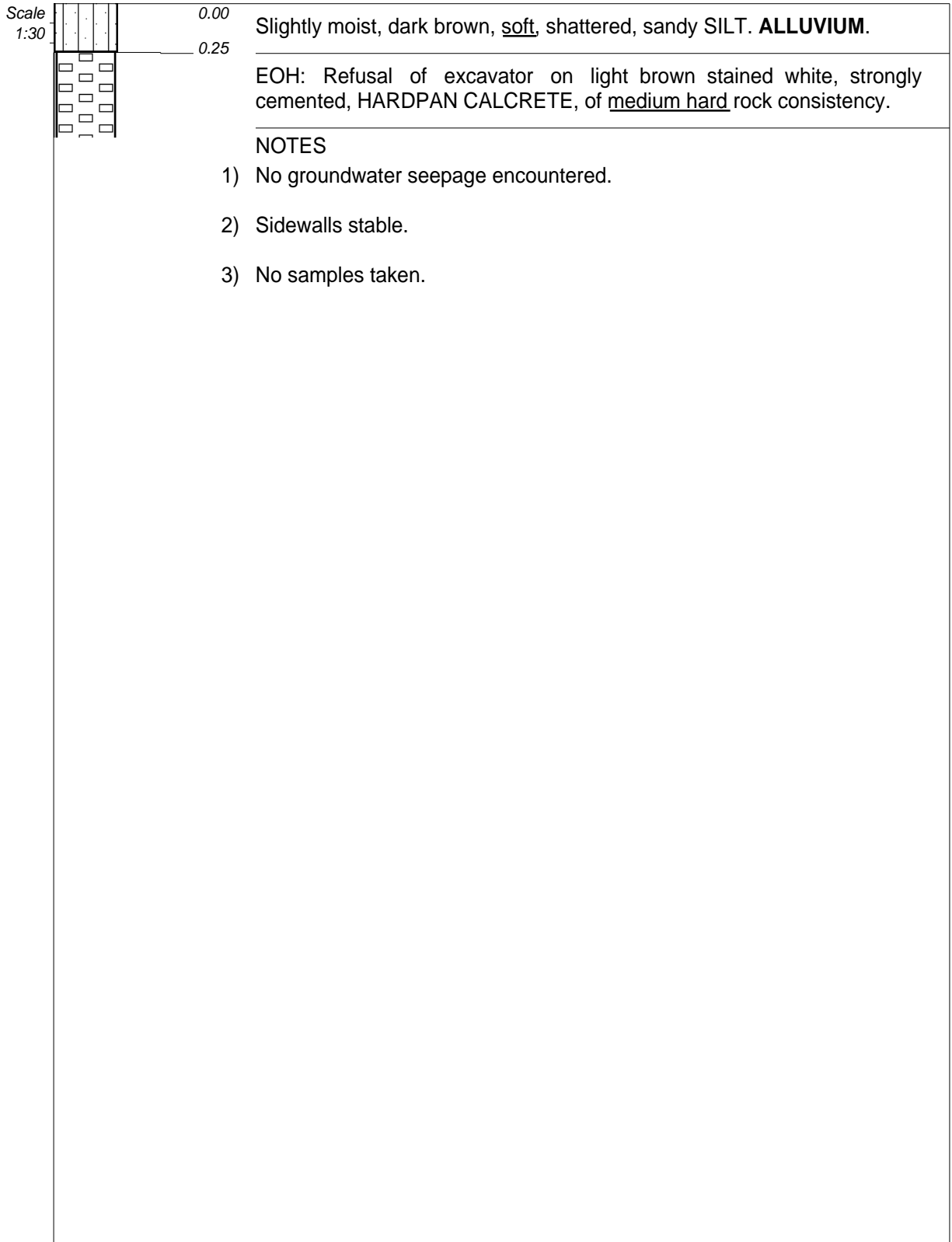
TYPE SET BY : EM
SETUP FILE : KPPT8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 25 May 2023

DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0803030
Y-COORD : 7584941

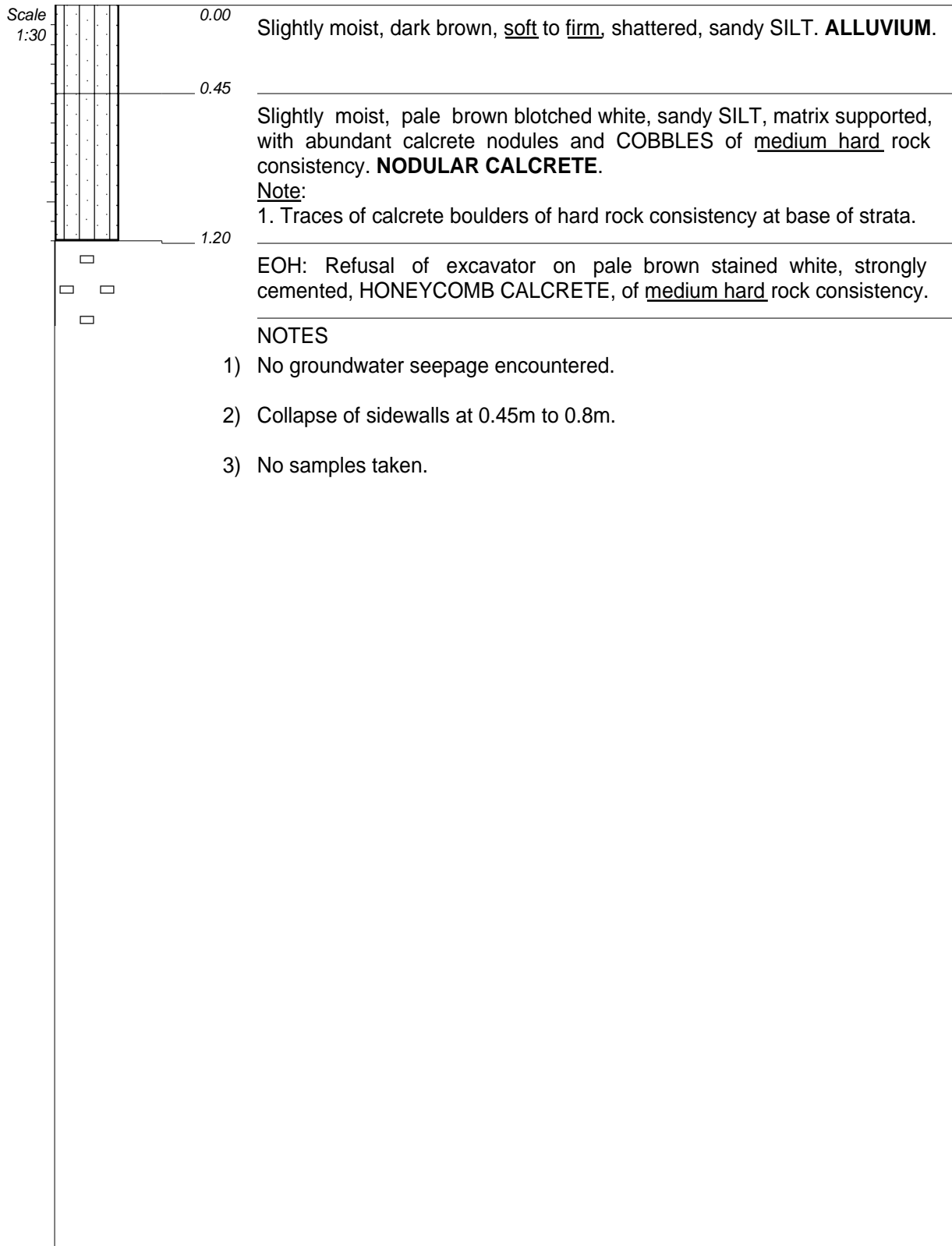
HOLE No: WRD-TP26



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

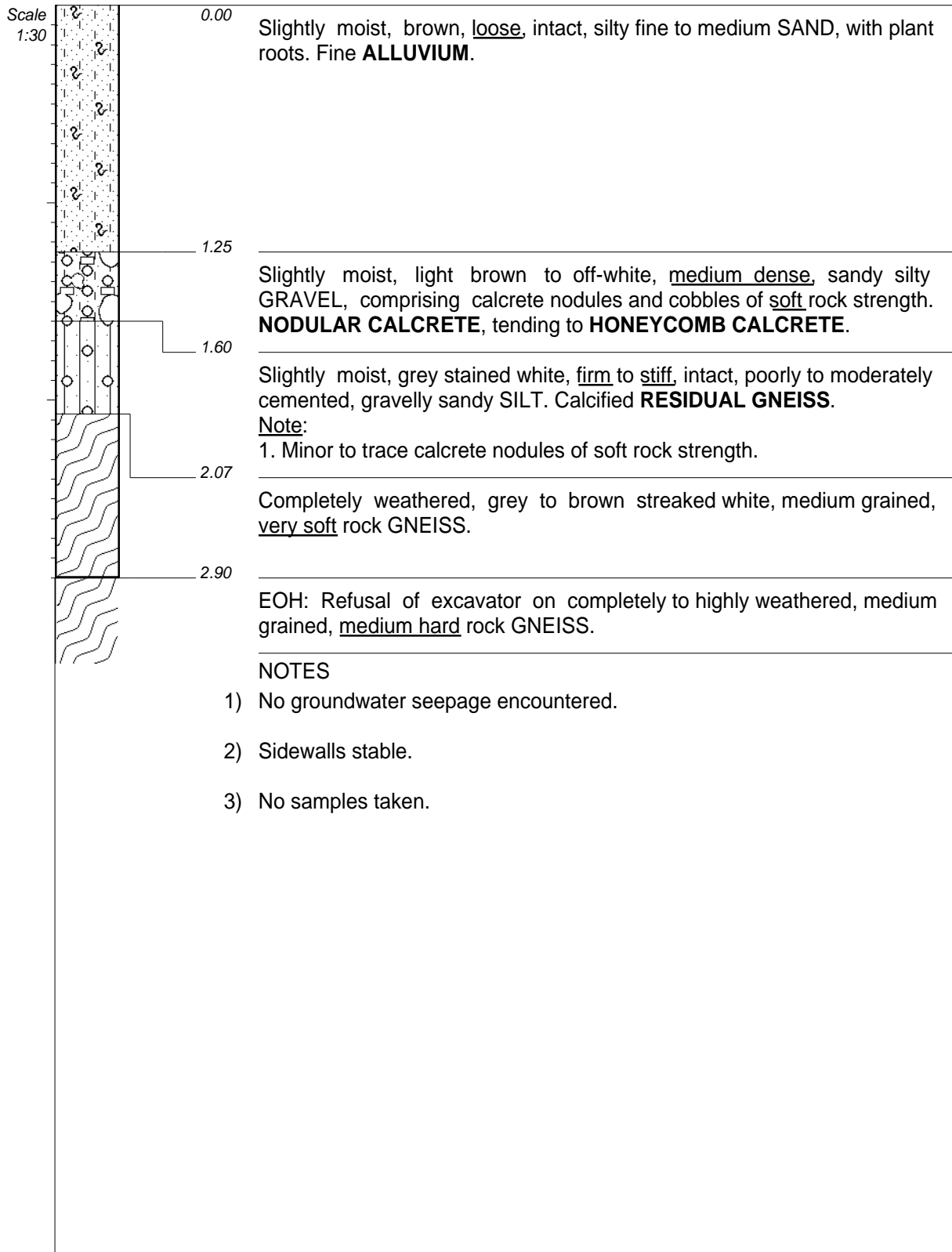
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802994.92
Y-COORD : 7585563.53



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

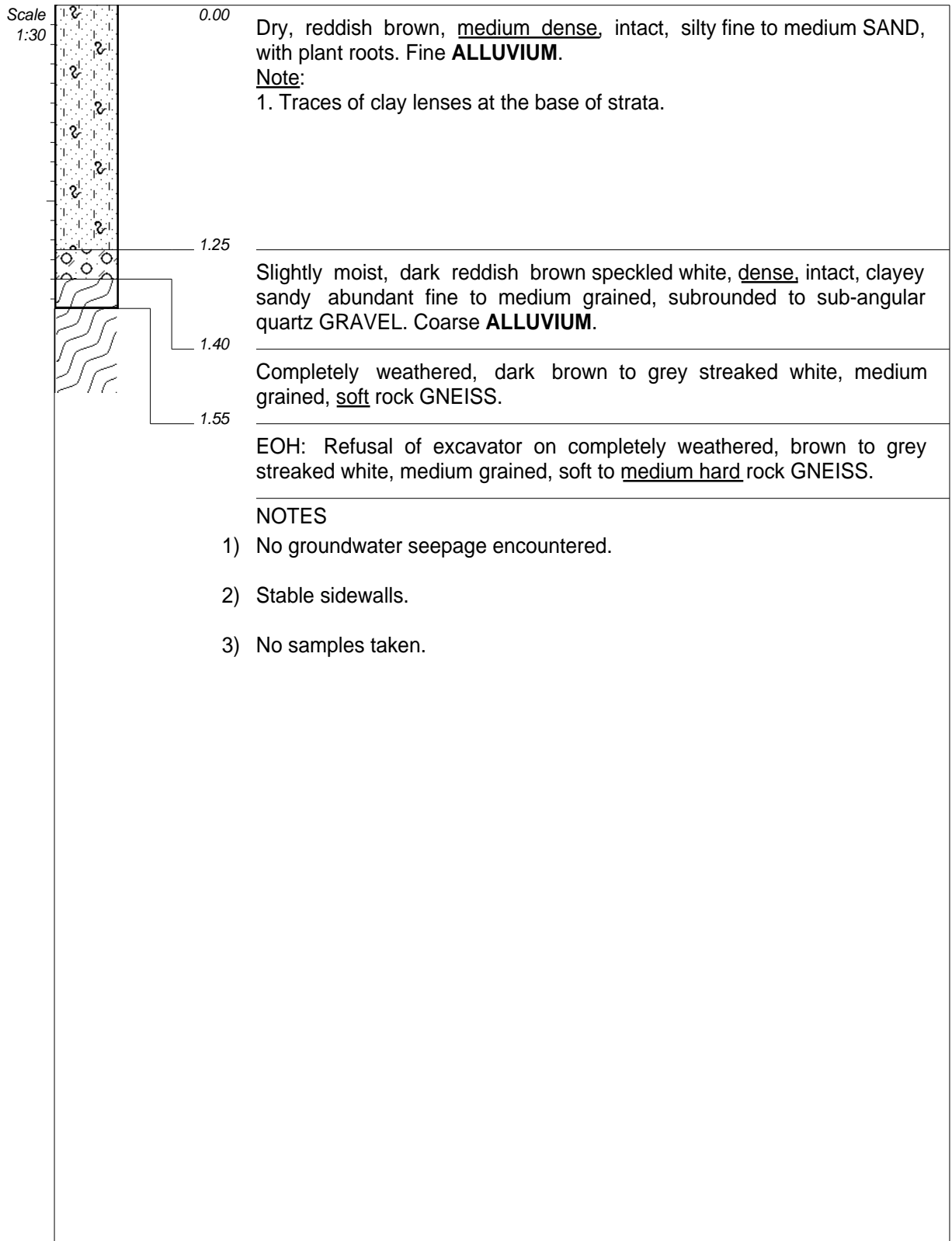
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 802706.77
Y-COORD : 7586159.95



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

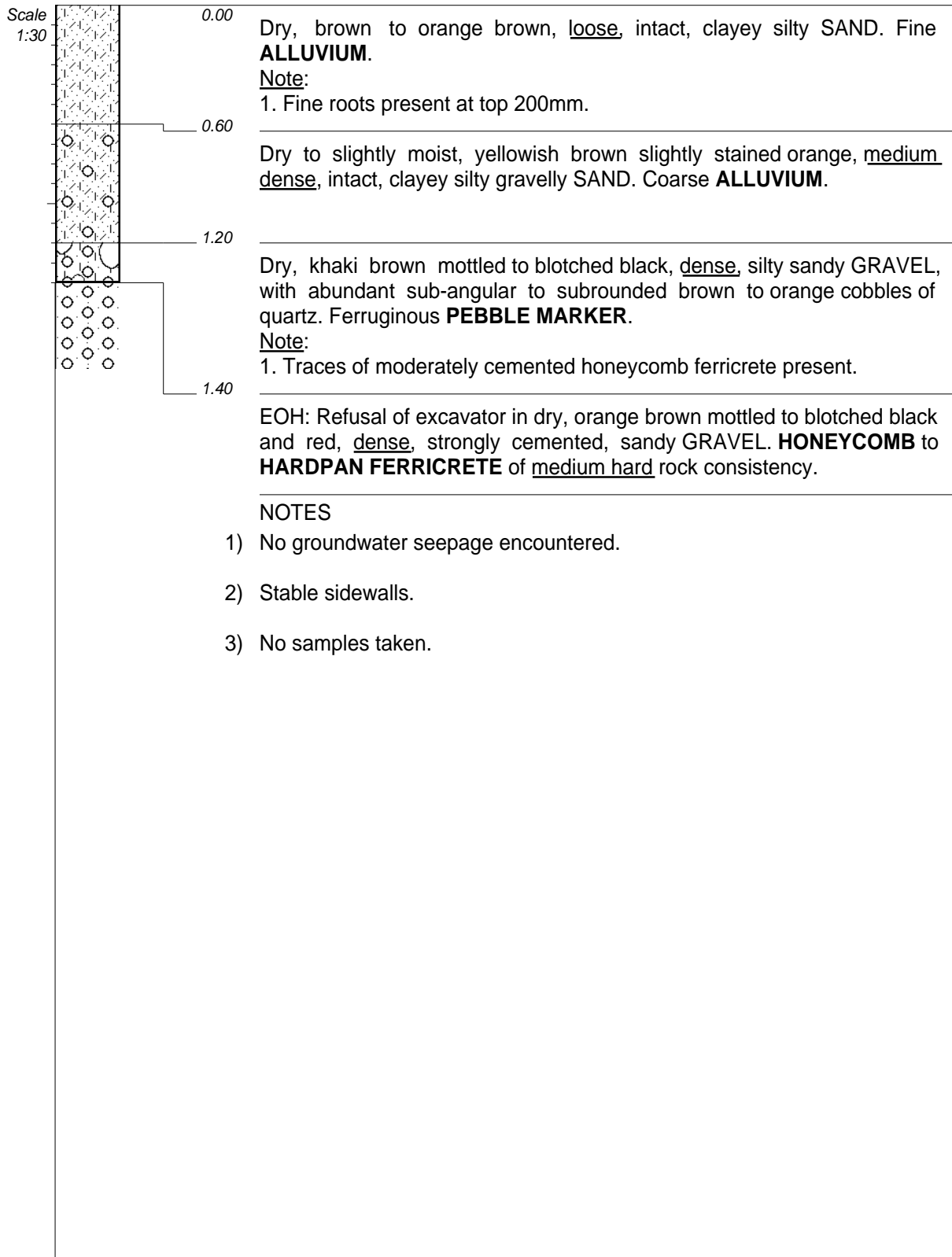
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803562.23
Y-COORD : 7586522.25



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

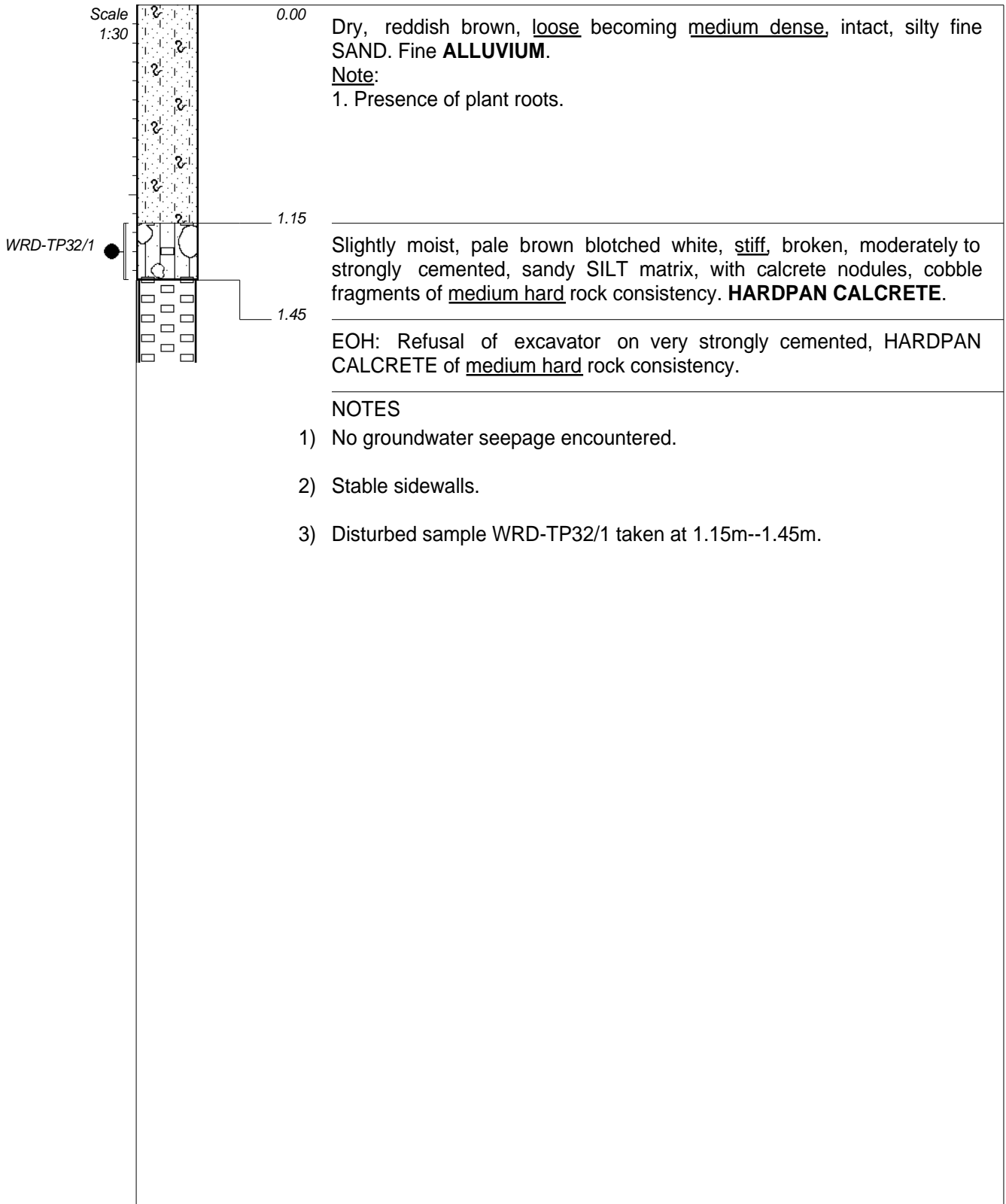
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803710.59
Y-COORD : 7585929.08



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 25 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

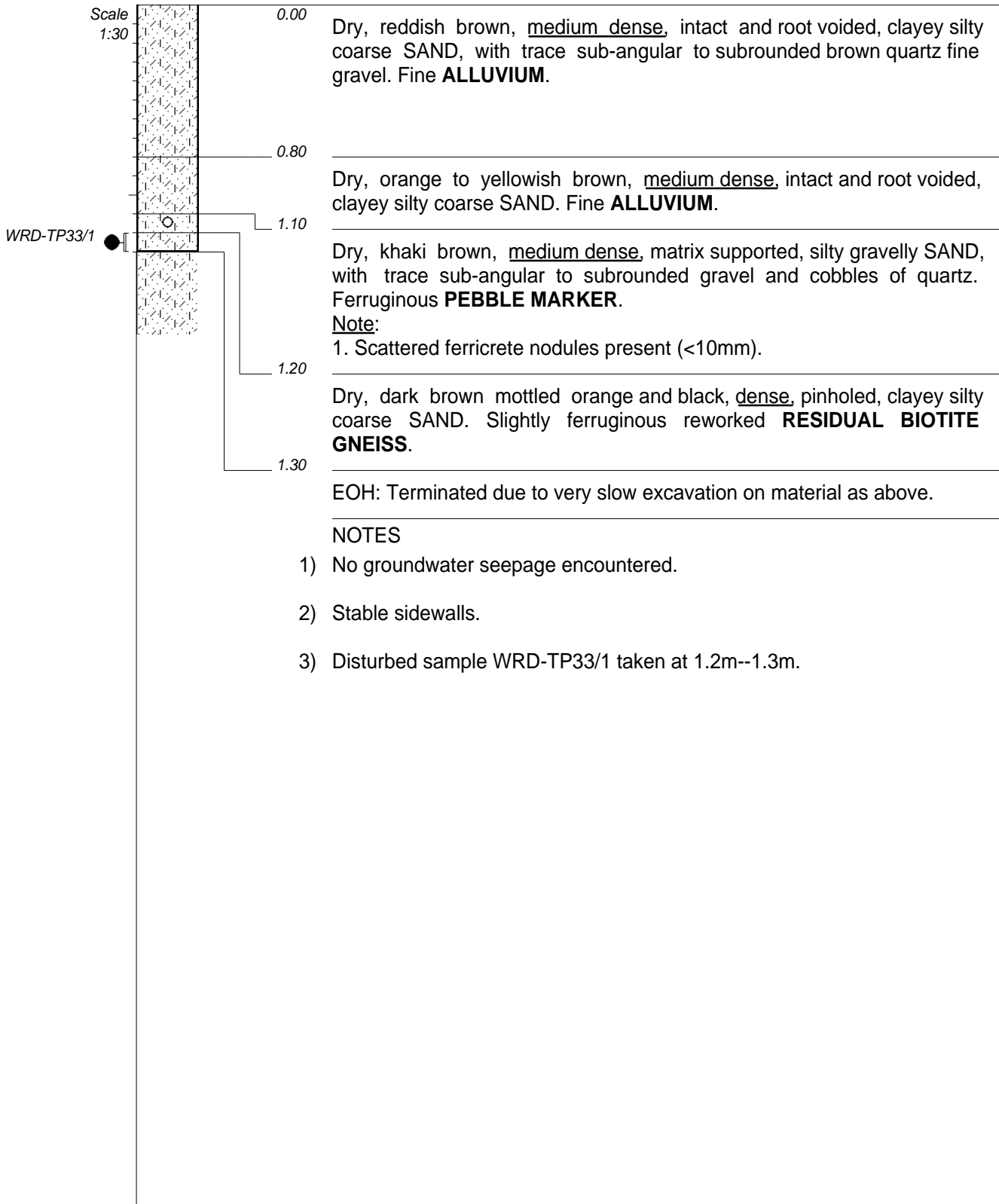
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0804445
Y-COORD : 7586187



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 17 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

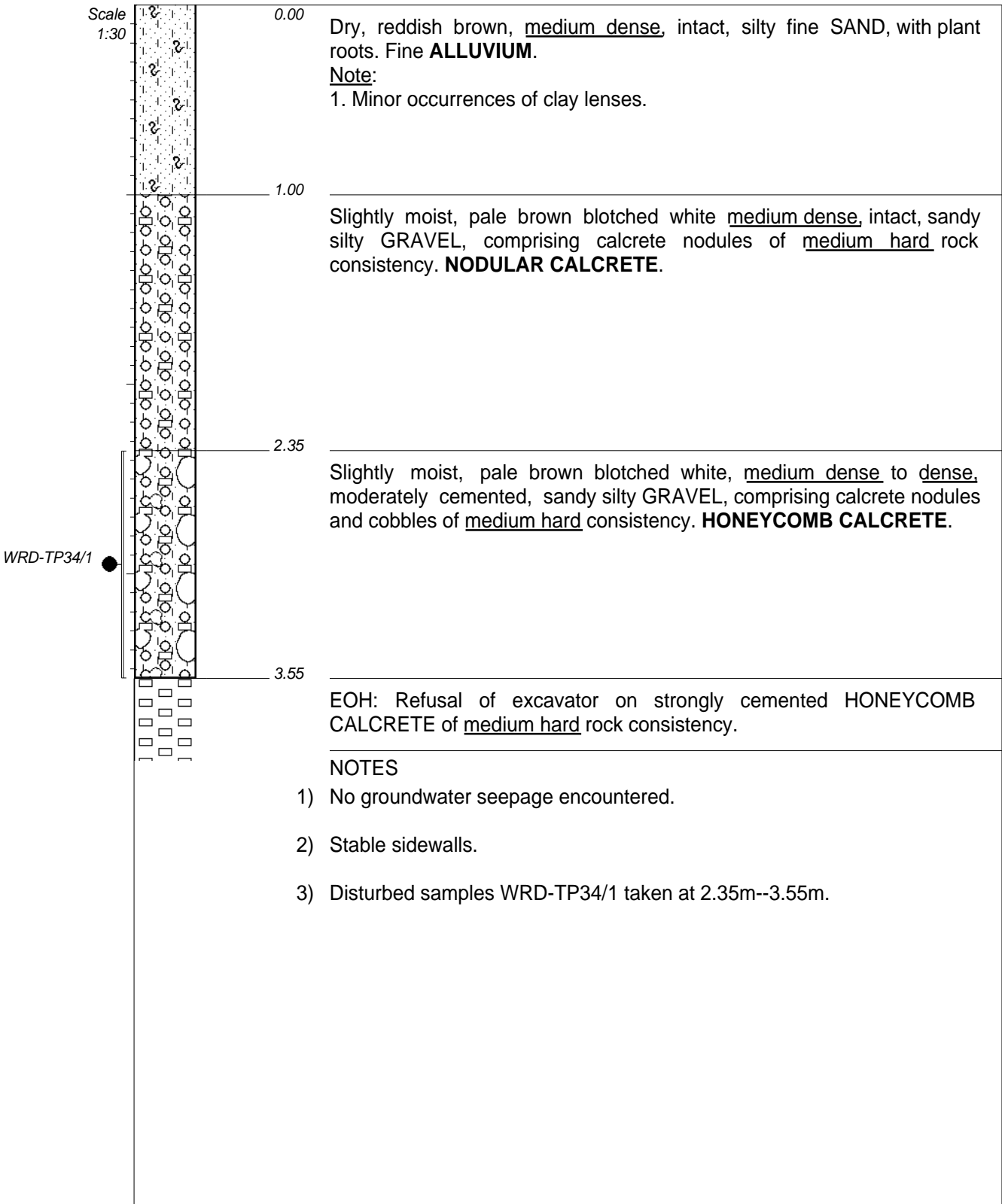
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803973.00
Y-COORD : 7585217.00



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 25 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

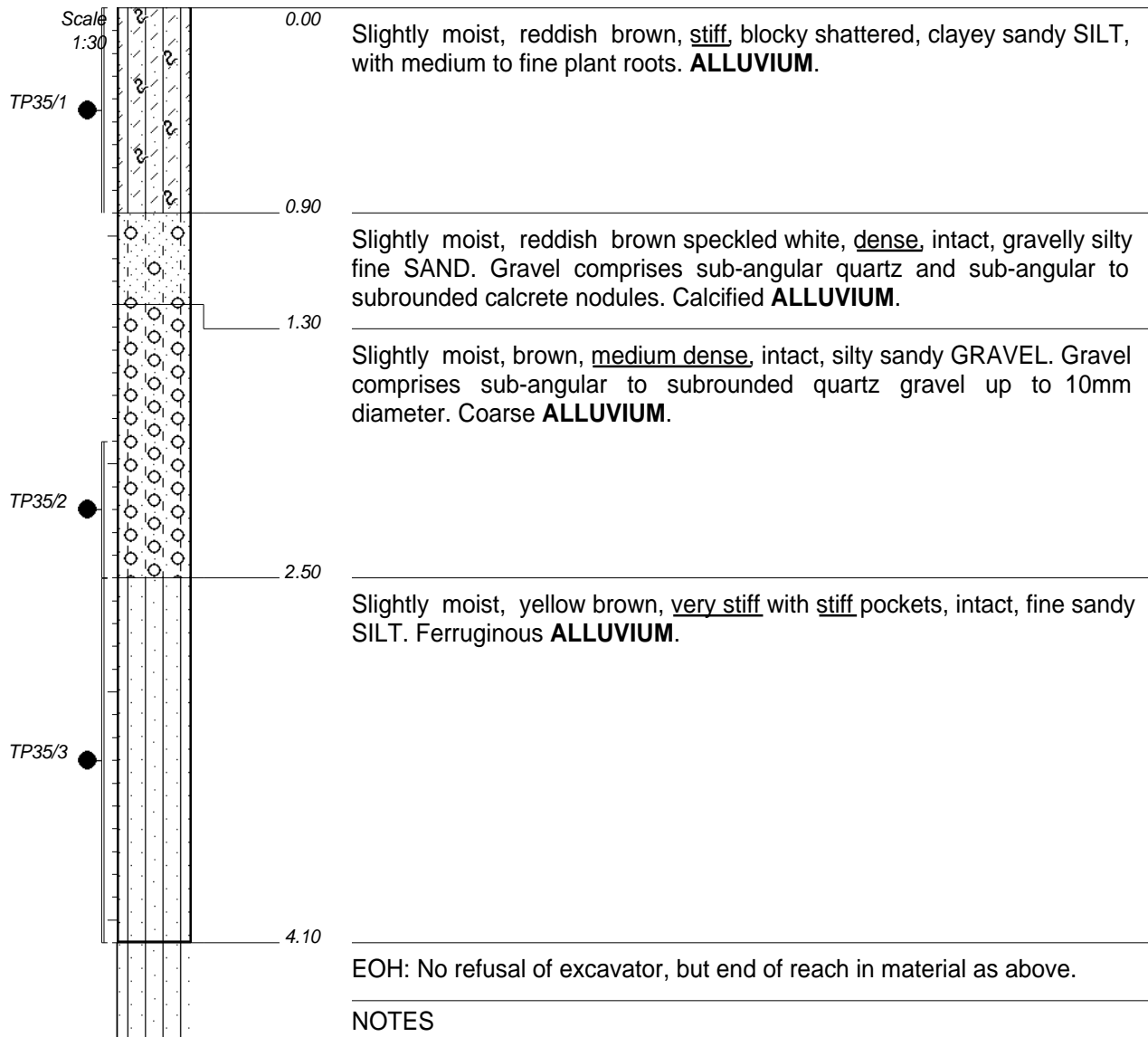
COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0804832
Y-COORD : 7585645



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 16 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 804786.59
Y-COORD : 7583725.60



NOTES

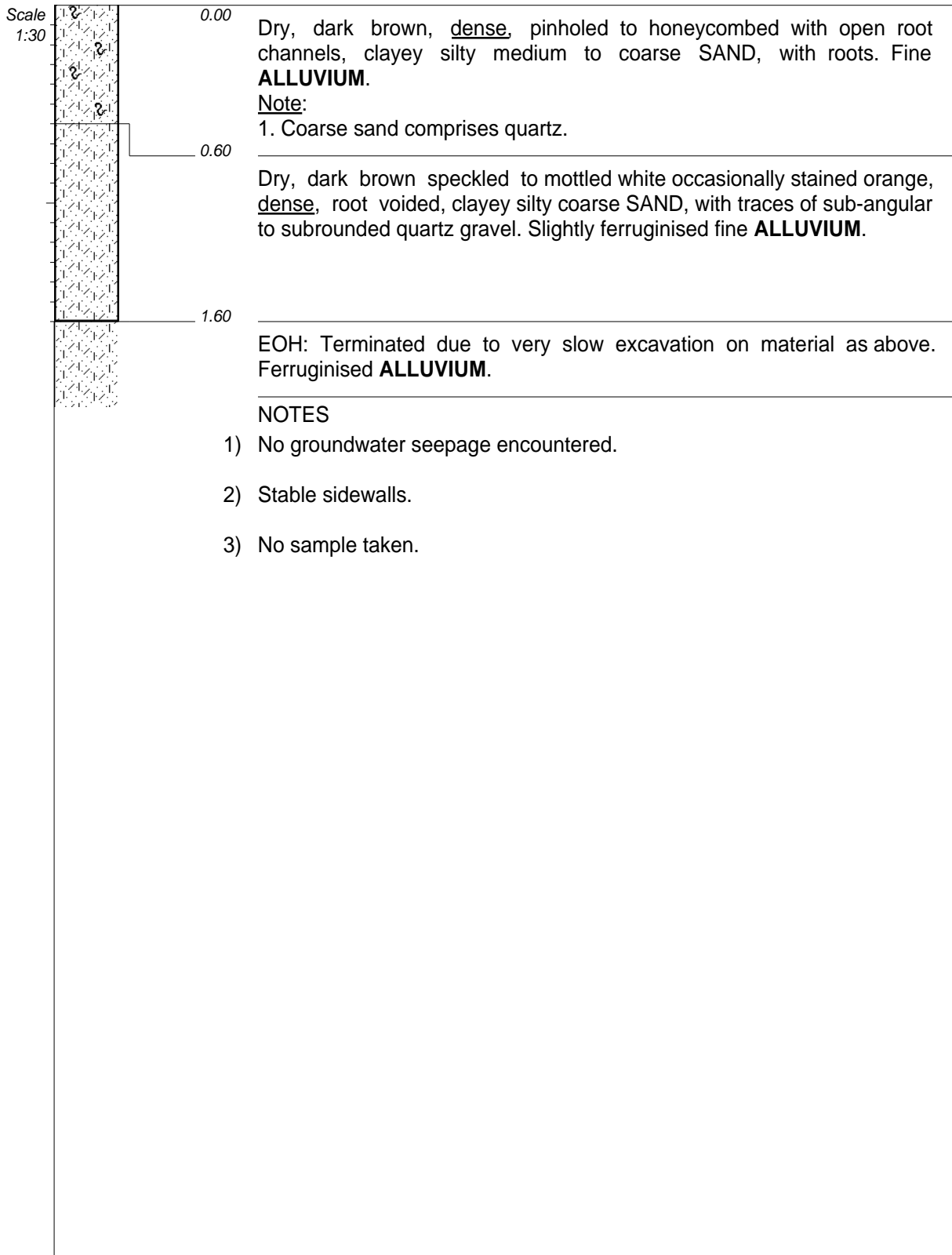
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample TP35/1 taken from 0.0m--0.9m.
- 4) Disturbed sample TP35/2 taken from 1.9m--2.5m.
- 5) Disturbed sample TP35/3 taken from 2.5m--4.1m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : B Klaas
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 10 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 803149.00
Y-COORD : 7582266.00

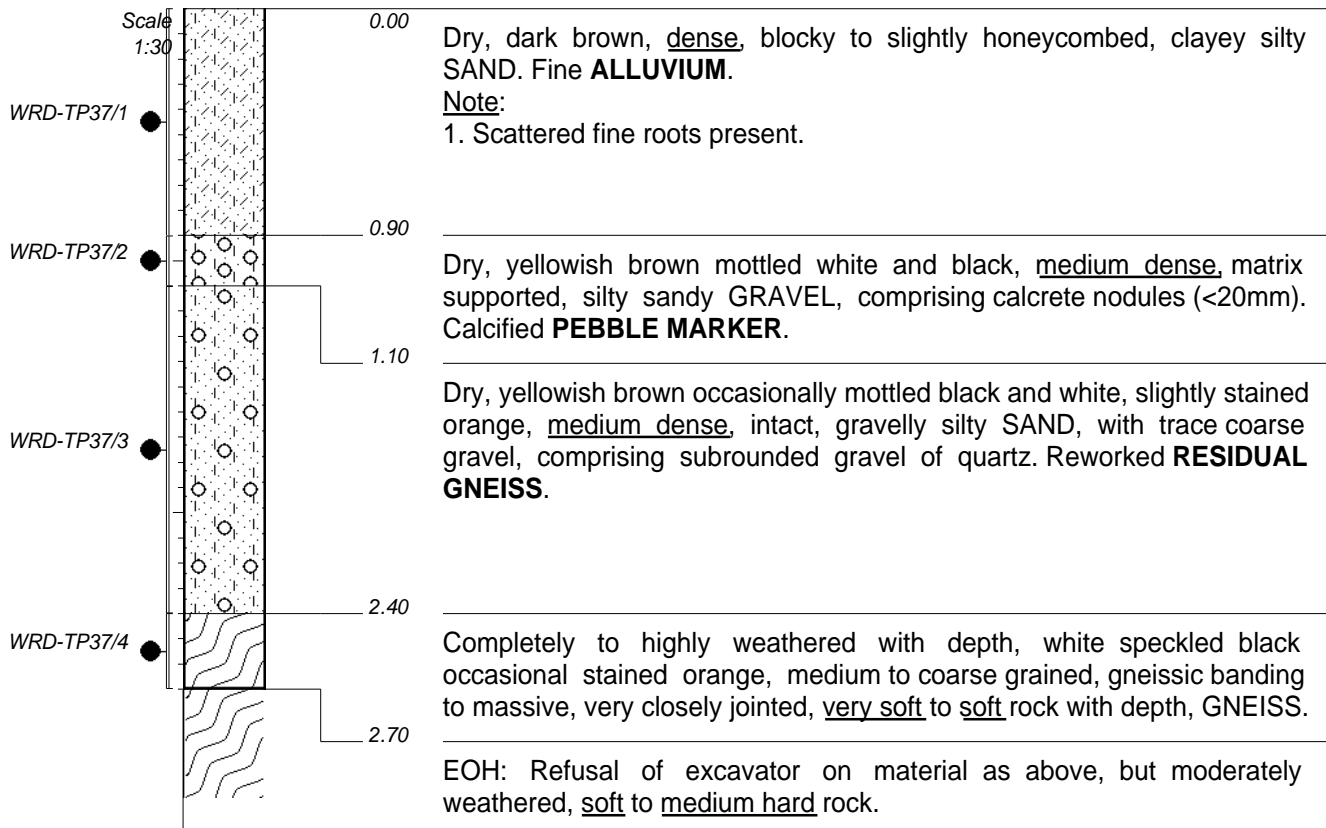
HOLE No: WRD-TP35



CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801345
Y-COORD : 7582051



NOTES

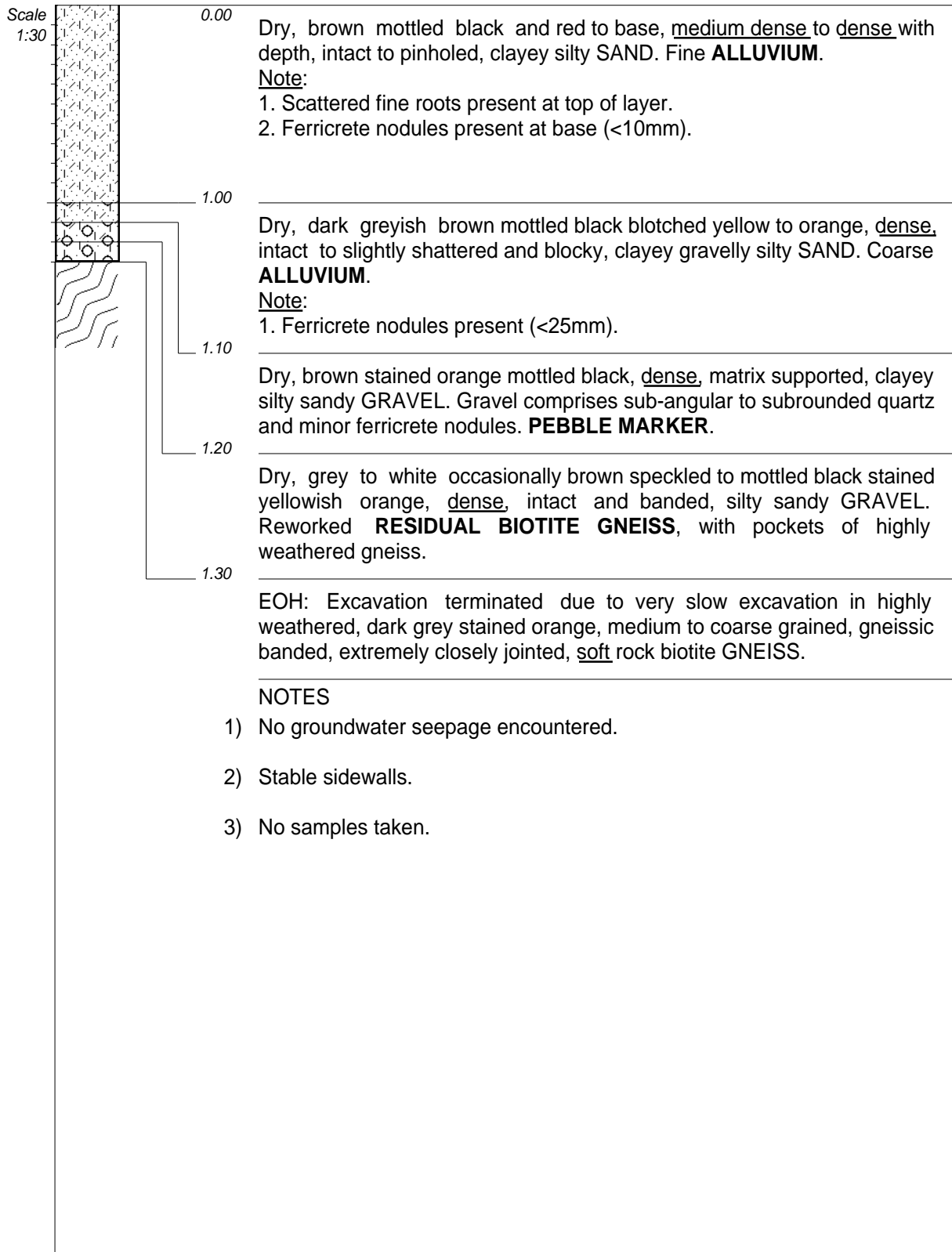
- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP37/1 taken at 0.0m--0.9m.
- 4) Disturbed sample WRD-TP37/2 taken at 0.9m--1.1m.
- 5) Disturbed sample WRD-TP37/3 taken at 1.1m--2.4m.
- 6) Disturbed sample WRD-TP37/4 taken at 2.4m--2.7m.

CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801515
Y-COORD : 7582442

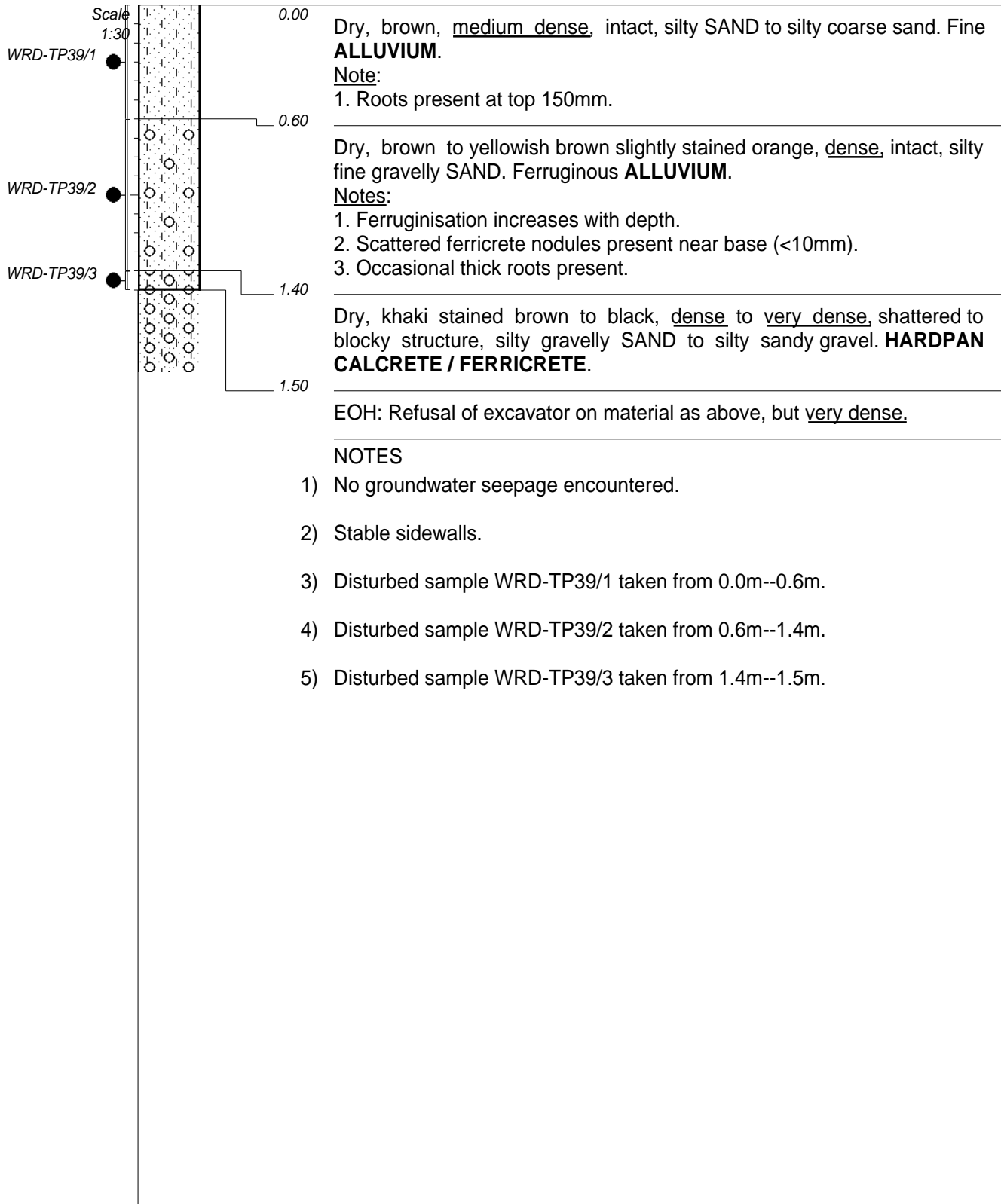
HOLE No: WRD-TP37



CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801515
Y-COORD : 7582442

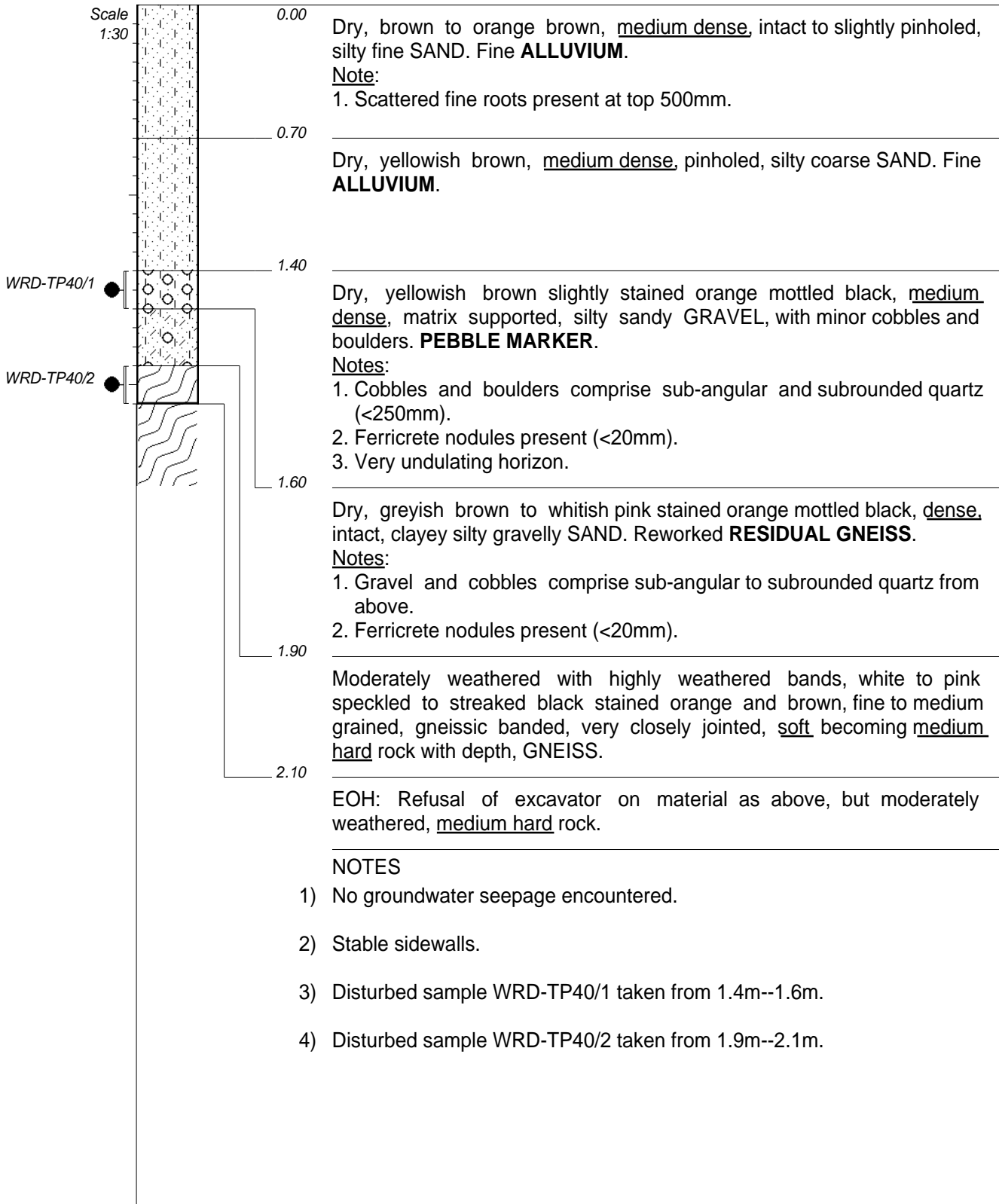


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801254
Y-COORD : 7582470

HOLE No: WRD-TP39

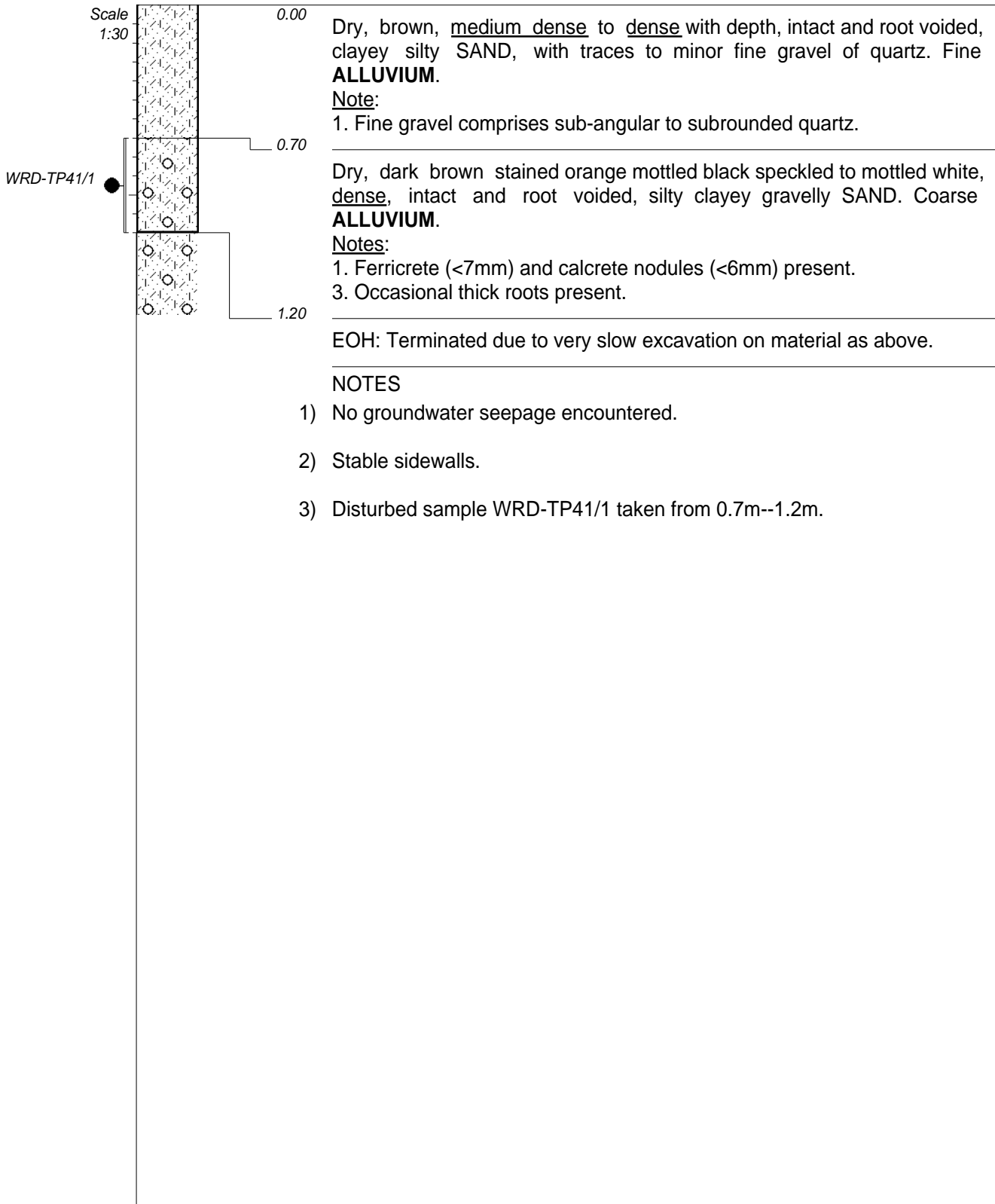


CONTRACTOR : Sandworx
MACHINE : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801382
Y-COORD : 7582856

HOLE No: **WRD-TP40**



CONTRACTOR : Sandworx
MACHINE : : Excavator FR220D
DRILLED BY : Joseph
PROFILED BY : C Homan
TYPE SET BY : EM
SETUP FILE : KPTP8.SET

INCLINATION : Vertical
DIAM :
DATE : May 2023
DATE : 24 May 2023
DATE : 07/07/2023 14:24
TEXT : ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM : UTM, WGS84 (33K)
X-COORD : 0801860
Y-COORD : 7582706

HOLE No: WRD-TP41

APPENDIX C2

Borehole Logs

HOLE No: HLP-BH01
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

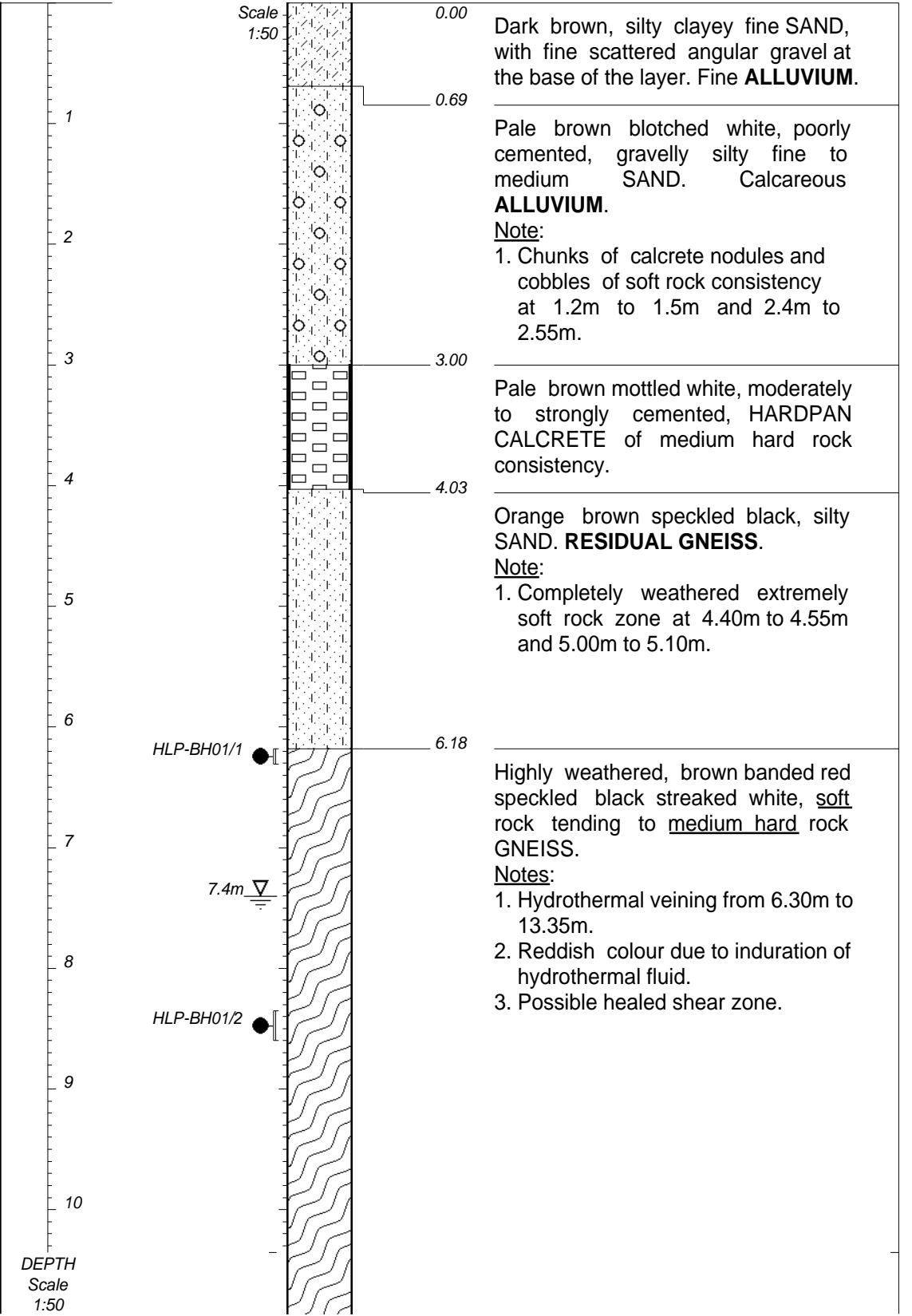


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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH01
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.71	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA
										2.00	100	0	0	NA
										3.00	90	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	4.13	92	0	0	NA	
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.68	89	0	0	NA
										6.18	97	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	6.89	100	100	61	10
										7.68	100	100	33	14
										9.18	100	100	71	5
										10.66	100	100	49	11



HOLE No: HLP-BH01
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

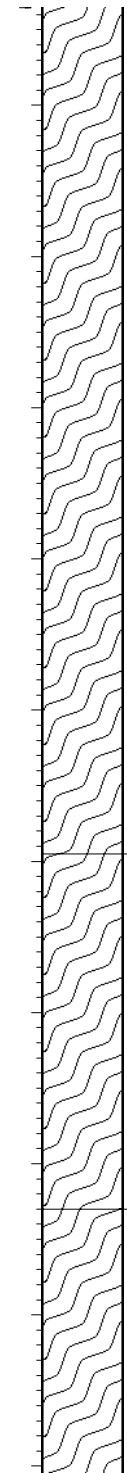


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH01
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF	10-20	1 2 3	0-10 20-30 40	VCJ CJ-MJ WJ	RJ RJ RJ	Silt Silt Silt	1 1-2 <1	12.18	94	94	62	9	11
										13.68	100	100	71	8	12
										15.18	100	97	18	17	14
										16.68	100	100	29	15	15
MG	GF	VIF	10-20	1 2 3	0-10 30 50-60	VCJ NM CJ-MJ	RJ RJ RJ	Silt Silty sand FeOx	1 1-2 <1	17.36	100	88	51	10	16
										18.18	93	93	37	11	17
										19.68	100	100	75	10	18
										20.07	100	100	64	7	19
															20



15.95
Moderately weathered, grey to brown speckled black banded white, soft rock GNEISS.
Notes:
1. Weak/very soft rock zone at 16.45m to 16.51m.
2. 40mm thick quartz veins intruded along fabric.
3. From 17.00m to 18.30m brecciated shear zone.

18.30
Slightly weathered, dark grey speckled black banded white, medium hard rock GNEISS becoming coarse grained and altered with depth.

20.07

NOTES
1) Perched water table encountered at 7.4m.

HOLE No: HLP-BH02
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

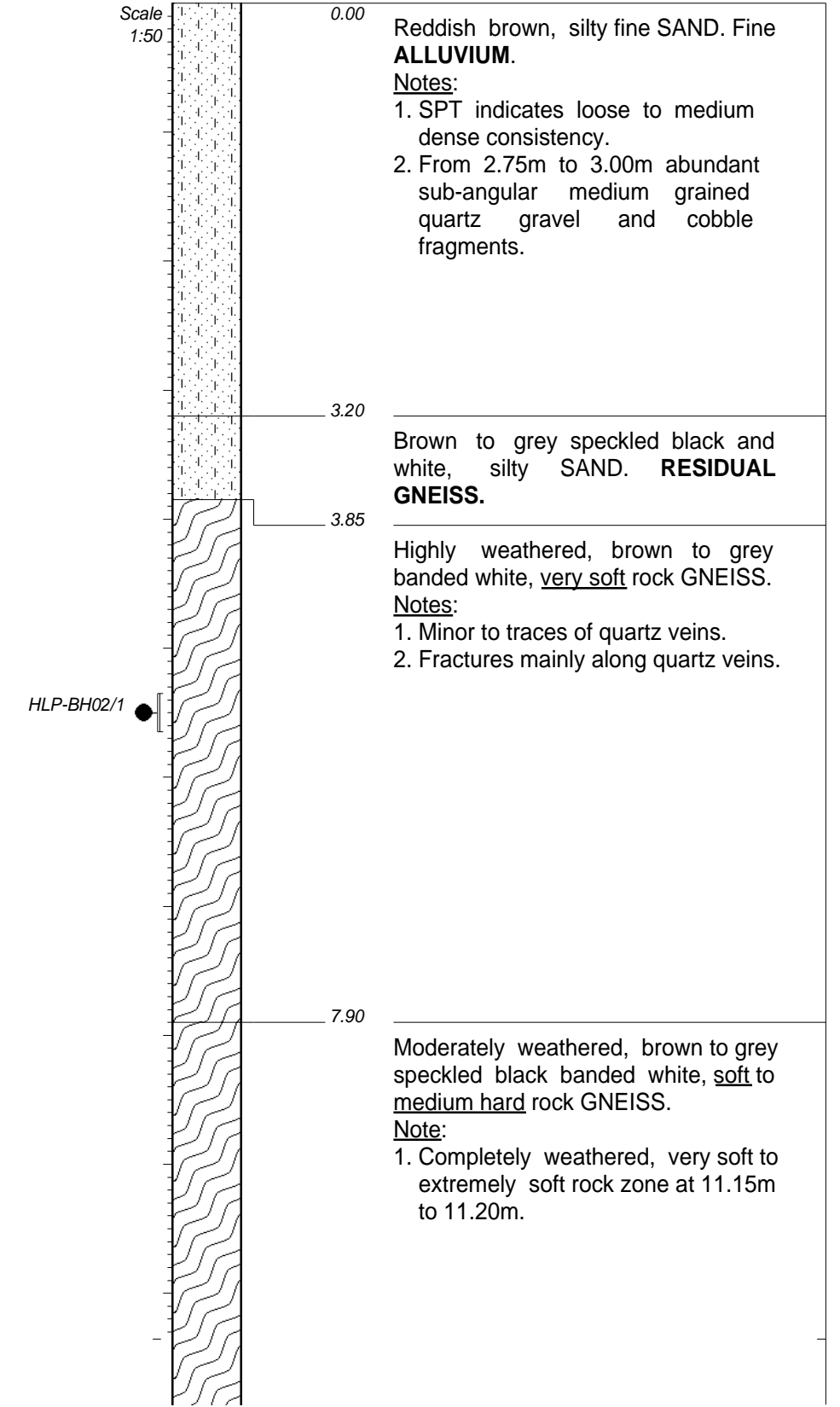


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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH02
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT	N=7		
										0.84	92	0	0	NA
										1.50	100	0	0	NA
										1.95	SPT	N=10		
										2.00	100	0	0	NA
										3.00	95	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.44	100	37	18	5
MG	GF	IF	10	1	0-10	CJ	RJ	Silt	<1	6.03	100	97	75	7
										7.58	100	97	74	9
										9.13	100	95	85	8
MG-CG	GF	VIF	10	1	0-10	CJ	RJ	Silt	<1	10.67	100	100	90	4



HOLE No: HLP-BH02
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

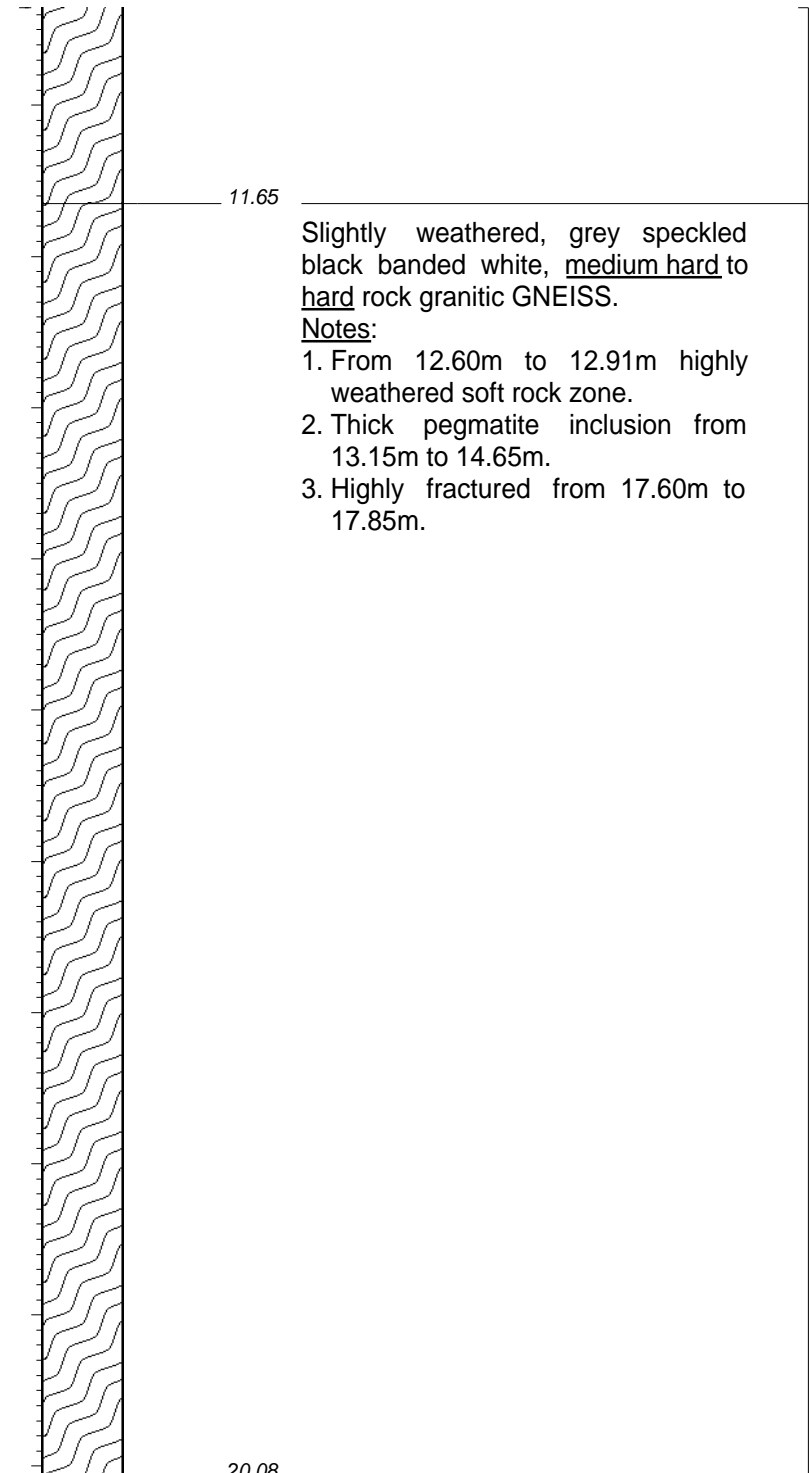


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH02
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
										12.17	100	100	95	4	11
										13.67	100	100	91	3	12
										15.17	99	99	85	5	13
MG-CG	MF-GF	NM	NM	1 2	0-10 40	CJ-MJ VCJ	RJ RJ	Silt Silt	1-2 1-2	16.67	100	100	86	5	14
										18.17	86	86	63	8	15
										19.43	100	100	93	2	16
										20.08	100	98	98	0	17
															18
															19
															20



11.65

Slightly weathered, grey speckled black banded white, medium hard to hard rock granitic GNEISS.

Notes:

1. From 12.60m to 12.91m highly weathered soft rock zone.
2. Thick pegmatite inclusion from 13.15m to 14.65m.
3. Highly fractured from 17.60m to 17.85m.

NOTES
1) NA: Not applicable.

HOLE No: HLP-BH03
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

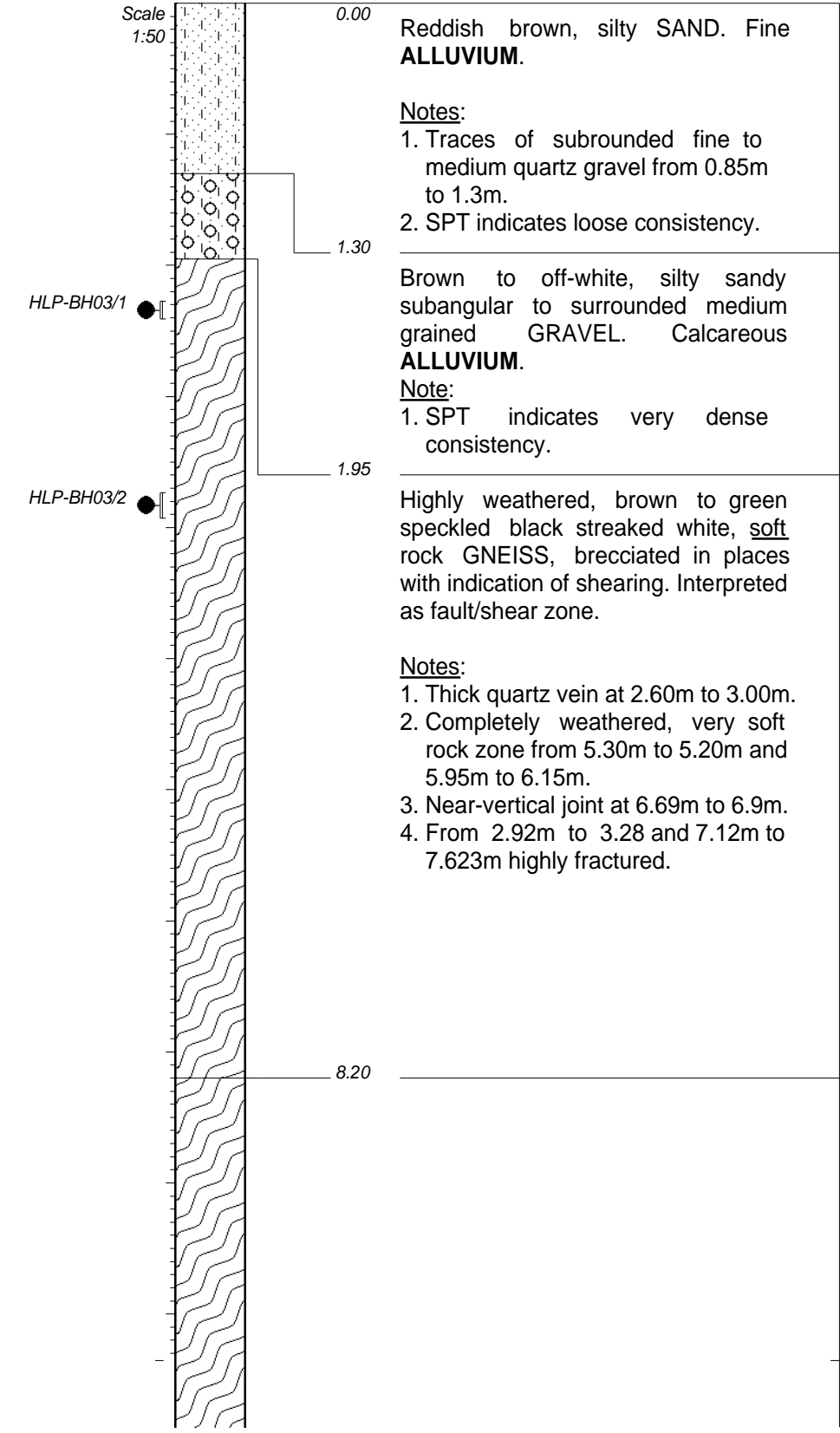


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH03
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT	N=4			
										0.84	100	0	0	NA	
										1.50	100	0	0	NA	
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.65	SPT	N=Ref			
										2.00	100	0	0	NA	
										3.28	100	79	9	13	
										4.68	96	89	33	9	
MG	GF	IF	70-80	1 2 3	0-10 20 80-90	VCJ CJ VWJ	RJ RJ RJ	Clayey silt Silt Calcite	1-2 1 2	6.18	100	97	47	9	
										7.62	100	97	28	17	
										9.16	99	94	58	8	
										10.68	64	54	24	5	



HOLE No: HLP-BH03
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

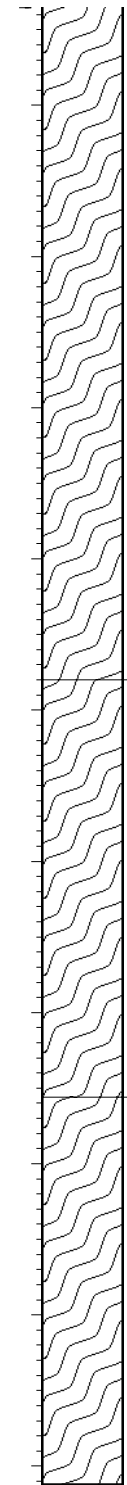


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH03
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF	80	1 2 3	0-10 70 80-90	CJ WJ MJ	RJ RJ RJ	FeOx & Silt Clean Silt	<1 - <1	11.40	100	100	38	14	11
										12.73	68	45	44	3	12
										13.68	100	83	33	15	13
										15.18	100	98	60	9	14
MG	GF	IF	10	1 2 3 4	0-10 45 60 30	VCJ-CJ CJ NM VCJ	RJ RJ RJ RJ	FeOx & Silt Silt Silty sand FeOx	<1 <1 1 <1	16.66	100	99	57	10	15
										18.16	100	93	63	7	17
MG	GF	IF	80	1 2	0-10 70	VCJ WJ	RJ RJ	Silt Silt	<1 <1	19.24	100	87	19	18	18
										20.13	100	96	17	12	19
															20



Moderately weathered, brown to grey streaked white, medium hard rock GNEISS.

Notes:

1. Core loss of 0.64m from 9.16m to 10.68m. Presumed to be due to washing out of fines.
2. Completely weathered, friable, pitted, extremely soft rock zone at 10.40m to 10.65m.
3. From 11.40m to 11.75m; 12.96m to 13.05m; and 13.10m to 13.24m highly broken zone.
4. Near vertical joint at 12.44m to 12.51m.

Slightly weathered, grey speckled black streaked and banded white, medium hard rock GNEISS, with brecciated rock in places, interpreted as shear zone.

Highly weathered, grey speckled black streaked white, medium hard rock GNEISS.

Note:

1. Breccia development at 17.56m to 17.65m and 19.40m to 19.52m.
2. Highly to completely weathered, very soft rock zone at 17.97m to 18.16m; 19.10m to 19.24m and 19.95m to 20.13m.

NOTES

- 1) NA: Not applicable.

HOLE No: HLP-BH04
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

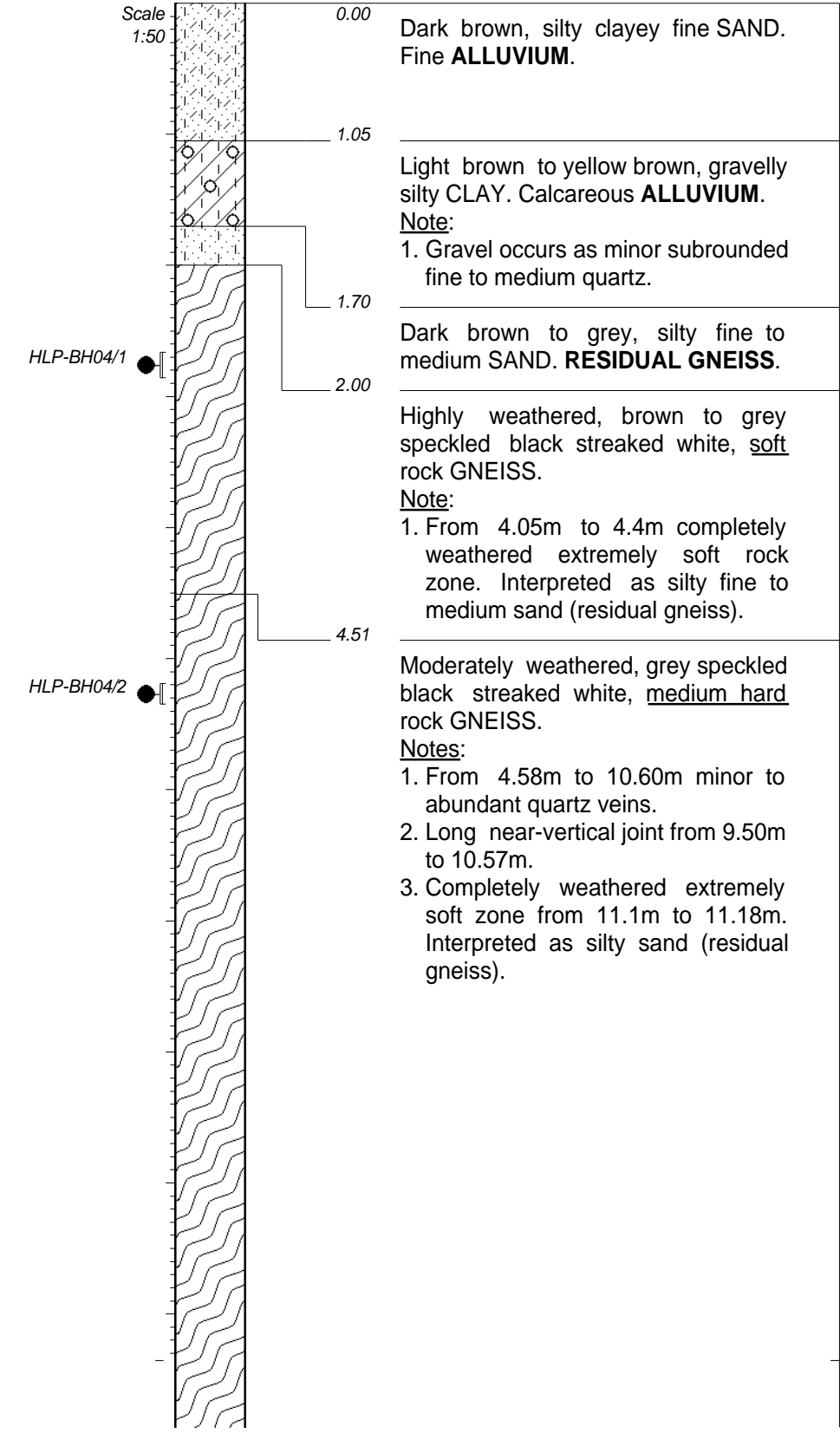


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH04
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT	N=20			0.00
										0.84	100	0	0	NA	1.05
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	1.70
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.88	SPT	N=Ref			2.00
										2.00	100	0	0	NA	2.00
MG	GF	VIF	0-10	1	0-10	VCJ	RJ	Sand	1-2	3.00	97	93	21	19	2.00
										4.58	75	54	0	15	4.51
MG	GF	VIF	5	1 2	0-10 90	VCJ-CJ NM	RJ RJ	Silt Silty SAND	1-2 2-3	6.15	100	100	41	12	6.15
										7.67	100	100	45	13	7.67
										9.17	100	100	66	8	9.17
										10.67	100	100	29	14	10.67



HOLE No: HLP-BH04
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

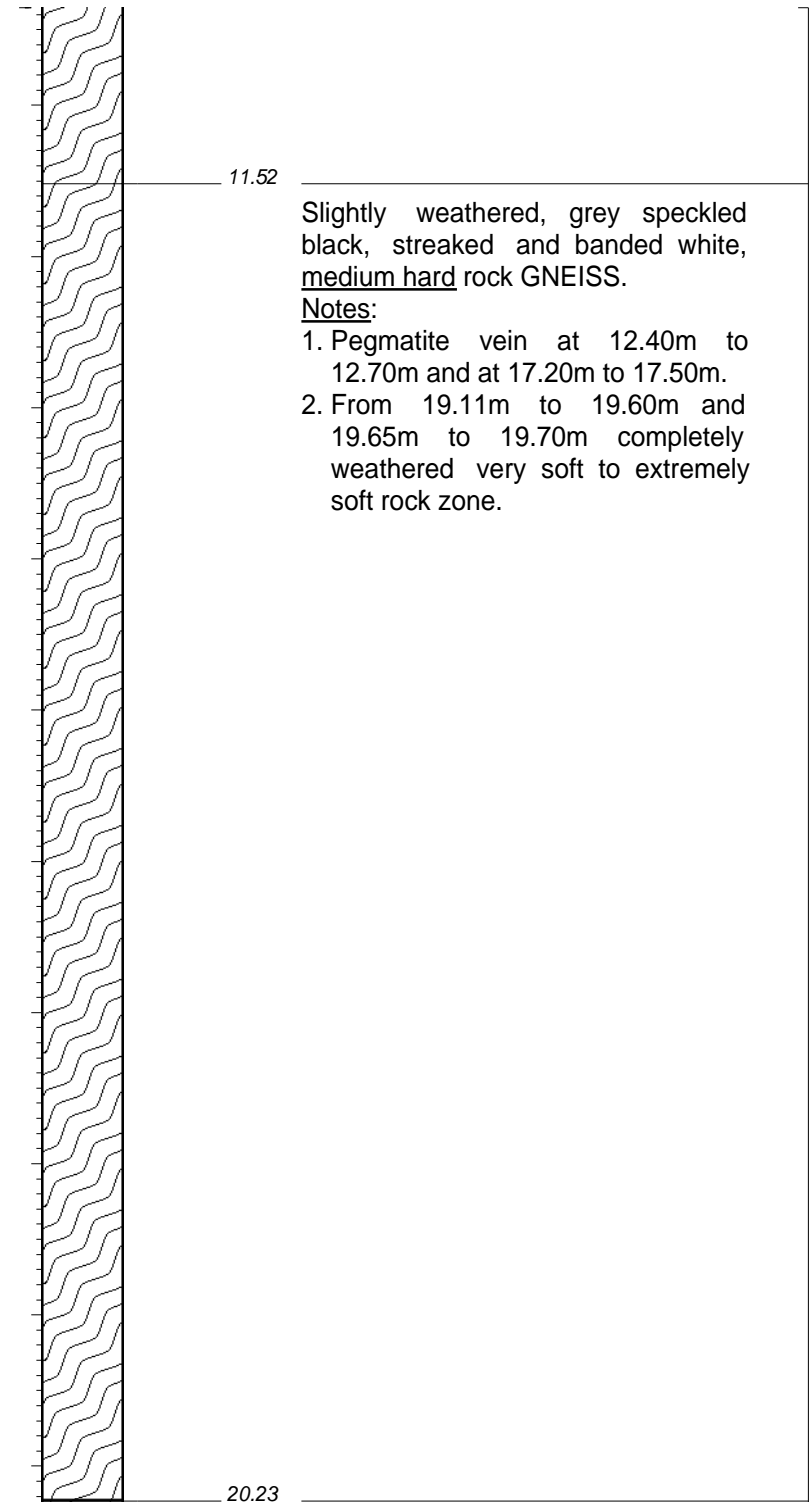


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH04
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF-VIF	10-20	1 2 3	0-10 20 40	VCJ-CJ CJ VWJ	RJ RJ RJ	Silt Silt FeOx	1-3 1 <1	12.17	100	95	60	9	11
										13.67	100	100	55	9	12
										15.20	100	98	49	10	13
										16.74	100	100	66	9	14
										18.23	97	95	70	10	15
										19.73	93	79	35	11	16
										20.23	100	100	74	16	17
															18
															19
															20



Slightly weathered, grey speckled black, streaked and banded white, medium hard rock GNEISS.
Notes:
1. Pegmatite vein at 12.40m to 12.70m and at 17.20m to 17.50m.
2. From 19.11m to 19.60m and 19.65m to 19.70m completely weathered very soft to extremely soft rock zone.

NOTES
1) NA: Not applicable.

HOLE No: Plant-BH01
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing PLA-planar VSR-very soft rock
 LF -laminated MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular



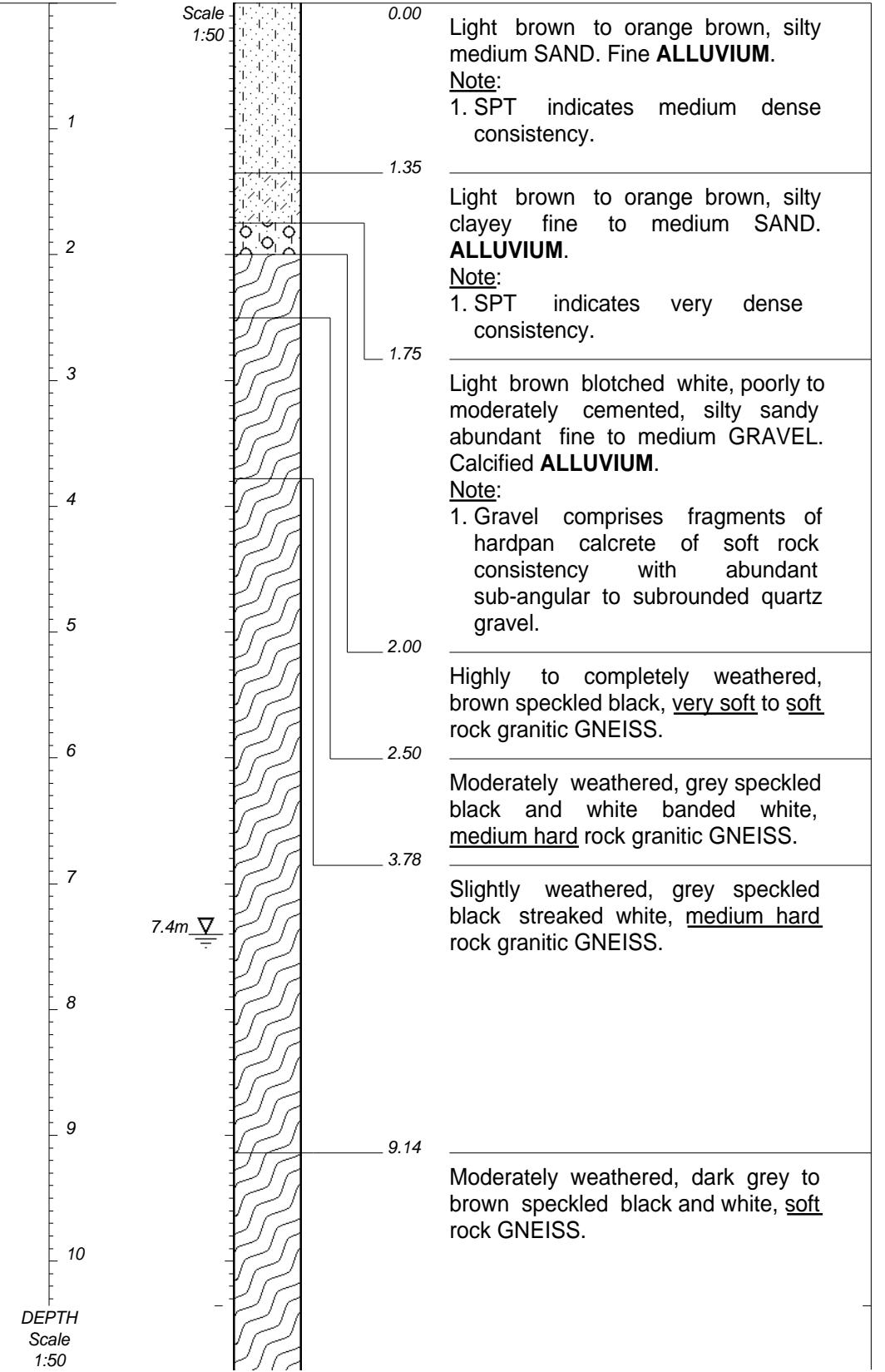
OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH01
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT	N=14		
										0.84	90	0	0	NA
										1.50	97	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.75	SPT	N=Ref		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	88	0	0	NA
MG	GF	NM	NM	1	0-10	VCJ	RJ	Silty sand	2-3					
MG-CG	GF-MF	NM	NM	1 2	0-10 20	VCJ CJ	RJ RJ	Silty sand Silty sand	1-2 2	3.50	91	91	31	15
MG-CG	GF-MF	NM	NM	1 2	0-10 30	CJ-MJ NM	RJ RJ	FeOx & Silt Silt	1 <1	4.65	97	97	63	7
										6.15	100	99	90	4
										7.65	97	97	95	3
										9.16	100	100	98	3
MG	GF	IF	10-20	1 2	0-10 20	VCJ VCJ	RJ RJ	Silty sand Sand	1 1	10.65	100	99	19	15



HOLE No: Plant-BH01
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH01
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG-CG	GF-MF	NM	NM	1 2	0-10 20-30	VCJ-MJ CJ-MJ	RJ RJ	Silt Silt	1 1	12.15	100	100	81	7	11
										13.65	100	100	76	6	12
										15.15	100	100	84	7	13
										16.65	98	98	71	8	14
										18.15	100	91	72	5	15
										19.65	100	97	73	7	16
										21.15	67	67	17	13	17
															18
															19
															20
															21

10.97

Slightly weathered, grey speckled black and white streaked white, medium hard rock granitic GNEISS.

Notes:

1. Pegmatite veins from 13.00m to 13.56m; 16.66m to 17.01m and 19.00m to 19.19m.
2. Highly broken zone at 17.05m to 17.20m.
3. Core loss due to pulverizing spinning of loose core at end of run (19.65m to 21.15m.)

20.67

HOLE No: Plant-BH01
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing PLA-planar VSR-very soft rock
 LF -laminated MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular



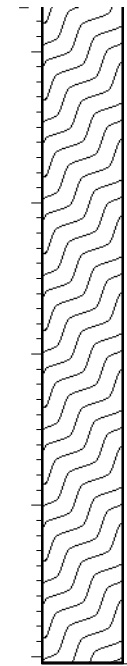
OMITIOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH01
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
MG	GF	VHF	70-80	1 2 3	0-10 10-20 45	VCJ-CJ CJ MJ	RJ RJ RJ	Silty sand Silt Silt	<1 <1 1	22.58	100	100	48	11
										24.08	99	99	79	7
										25.05	100	100	68	7



Slightly weathered with moderately weathered zones associated with closely jointed zones, grey speckled black banded white, soft rock becoming medium hard rock GNEISS.
 Note:
 1. Core loss of 0.67m at 20.39m to 21.06m due to washing out of fines.

- NOTES
- Perched water table encountered at 7.4m.
 - NA: Not applicable.
 - NM: Not measurable.

CONTRACTOR : RA Longstaff
 MACHINE : Sullivan HD
 DRILLED BY :
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
 DIAM :
 DATE : 2 May 2023
 DATE : 3 May 2023
 DATE : 07/07/2023 14:19
 TEXT : ..\PROFILES\PKFPLANTBH.TXT

COORDINATE SYSTEM :
 X-COORD : 802350
 Y-COORD : 7583844

HOLE No: Plant-BH01

HOLE No: Plant-BH02
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular



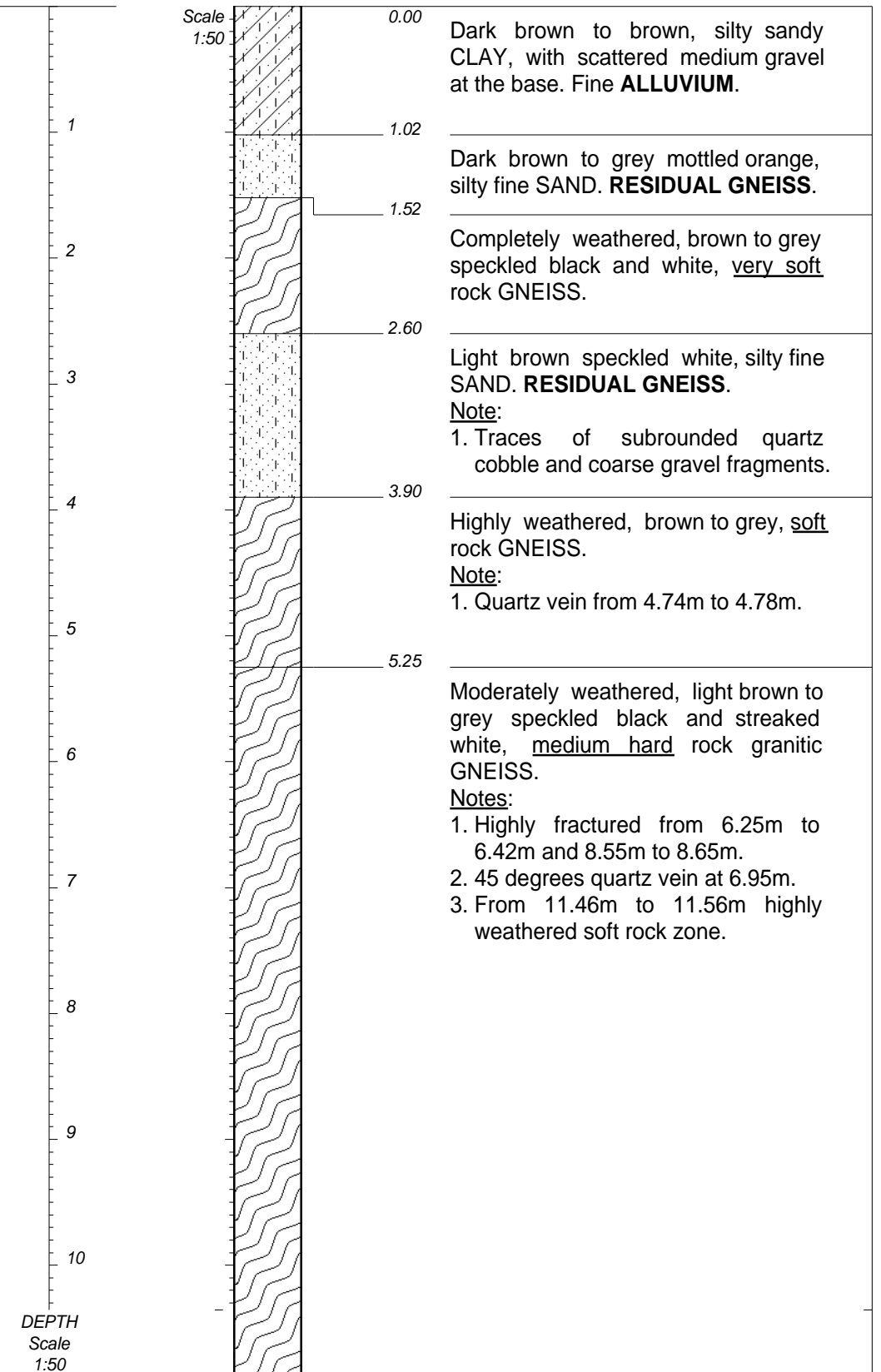
OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH02
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.54	93	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.02	100	8	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	42	42	8
MG	GF	NM	NM	1 2	0-10 30	VCJ VCJ	RJ RJ	Silt Silty sand	<1 1	1.52	SPT	N=Ref		
										2.12	92	72	23	13
										2.47	86	86	37	10
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.01	100	18	6	4
MG	GF	NM	NM	1	0-10	CJ	RJ	Silt	<1	4.43	76	76	0	19
										5.93	100	100	73	8
MG-CG	GF-MF	NM	NM	1	0-10	VCJ-CJ	RJ	Silty sand	1-2	7.50	99	97	49	12
										9.03	97	93	75	8
										10.40	100	100	64	11



HOLE No: Plant-BH02
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



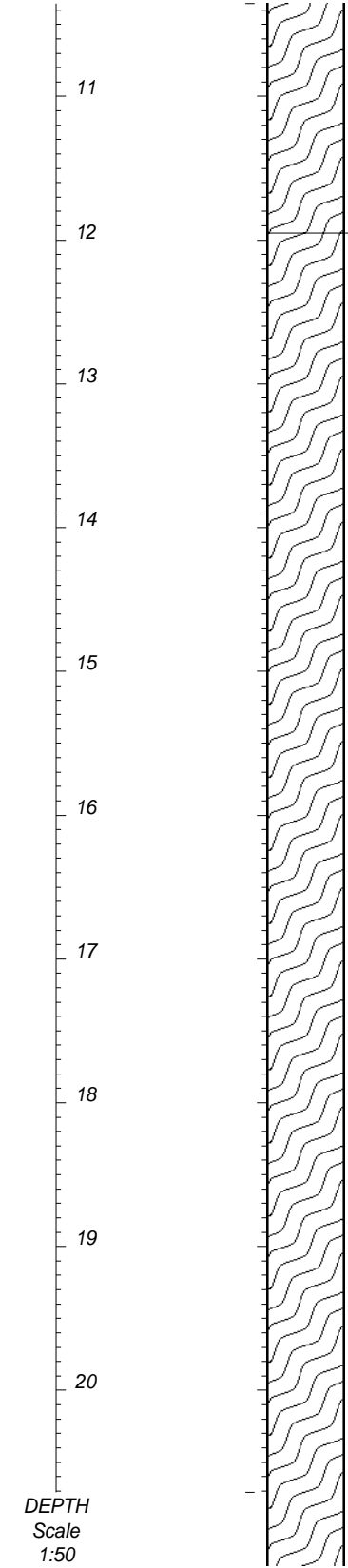
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PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH02
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
CG	MF-GF	NM	NM	1	0-10	CJ	RJ	Silty & clean	<1	11.98	97	97	41	12
				2	30-40	MJ	RJ	Calcite	1-2	13.50	100	100	80	6
				3	50-60	CJ-MJ	RJ	Calcite	1-2	14.60	97	97	75	9
				4	70	NM	RJ	FeOx	<1	15.27	100	100	90	6
										16.49	100	91	53	6
										17.99	100	100	81	5
										19.49	100	100	97	1
										20.73	99	99	78	4



11.95

Slightly weathered to unweathered, light grey to light brown speckled white and pink streaked black, hard to very hard rock granitic GNEISS, interpreted as a partially healed shear zone.

Notes:

- Highly broken zone at 16.31m to 16.49m.
- Rock mass improves with depth.

HOLE No: Plant-BH02
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



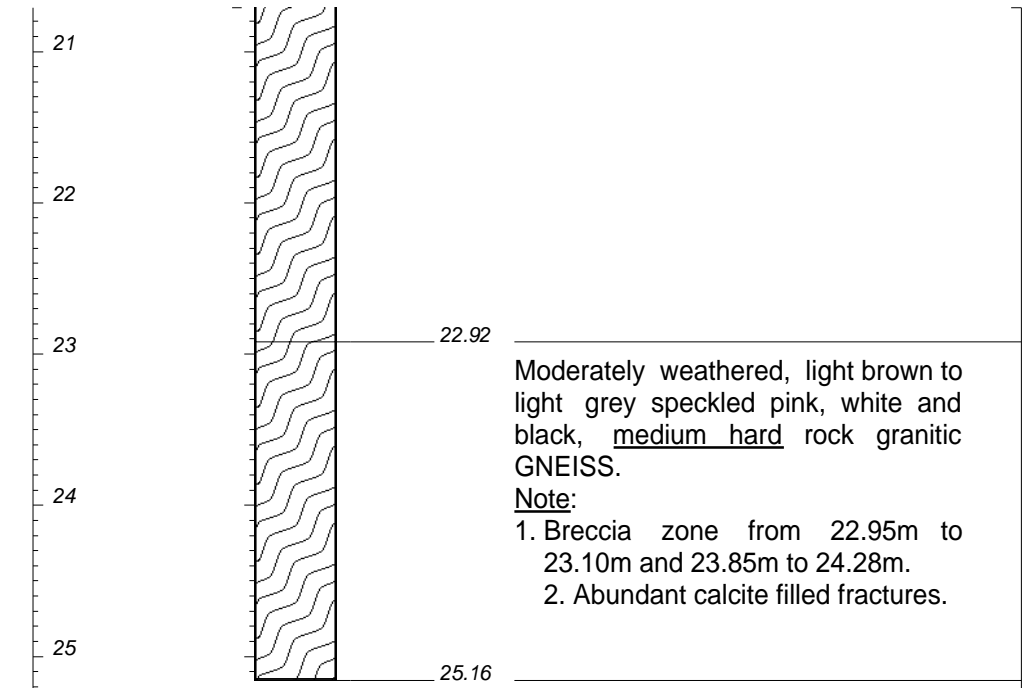
OMITIOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH02
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
										20.93	100	100	100	0
										22.27	100	100	93	2
										23.10	100	78	78	2
CG	MF-GF	NM	NM	1 2	0-10 30	VCJ VCJ	RJ RJ	Calcite & FeOx Calcite & FeOx	1 1	24.59	85	62	27	5
										25.16	100	100	56	9



Moderately weathered, light brown to light grey speckled pink, white and black, medium hard rock granitic GNEISS.
Note:
1. Breccia zone from 22.95m to 23.10m and 23.85m to 24.28m.
2. Abundant calcite filled fractures.

- NOTES
- 1) No groundwater seepage encountered.
 - 2) NA: Not applicable.
 - 3) NM: Not measurable.

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
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DEPTH Scale 1:50

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 4-6 May 2023
DATE : 5-6 May 2023
DATE : 07/07/2023 14:19
TEXT : ..\PROFILES\PKFPLANTBH.TXT

COORDINATE SYSTEM : WGS 84 UTM 33S
X-COORD : 801481.43
Y-COORD : 7583235.41

HOLE No: Plant-BH02

HOLE No: Plant-BH03
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 LF -laminated WJ -wide spacing VWJ-very wide spacng IRR-irregular



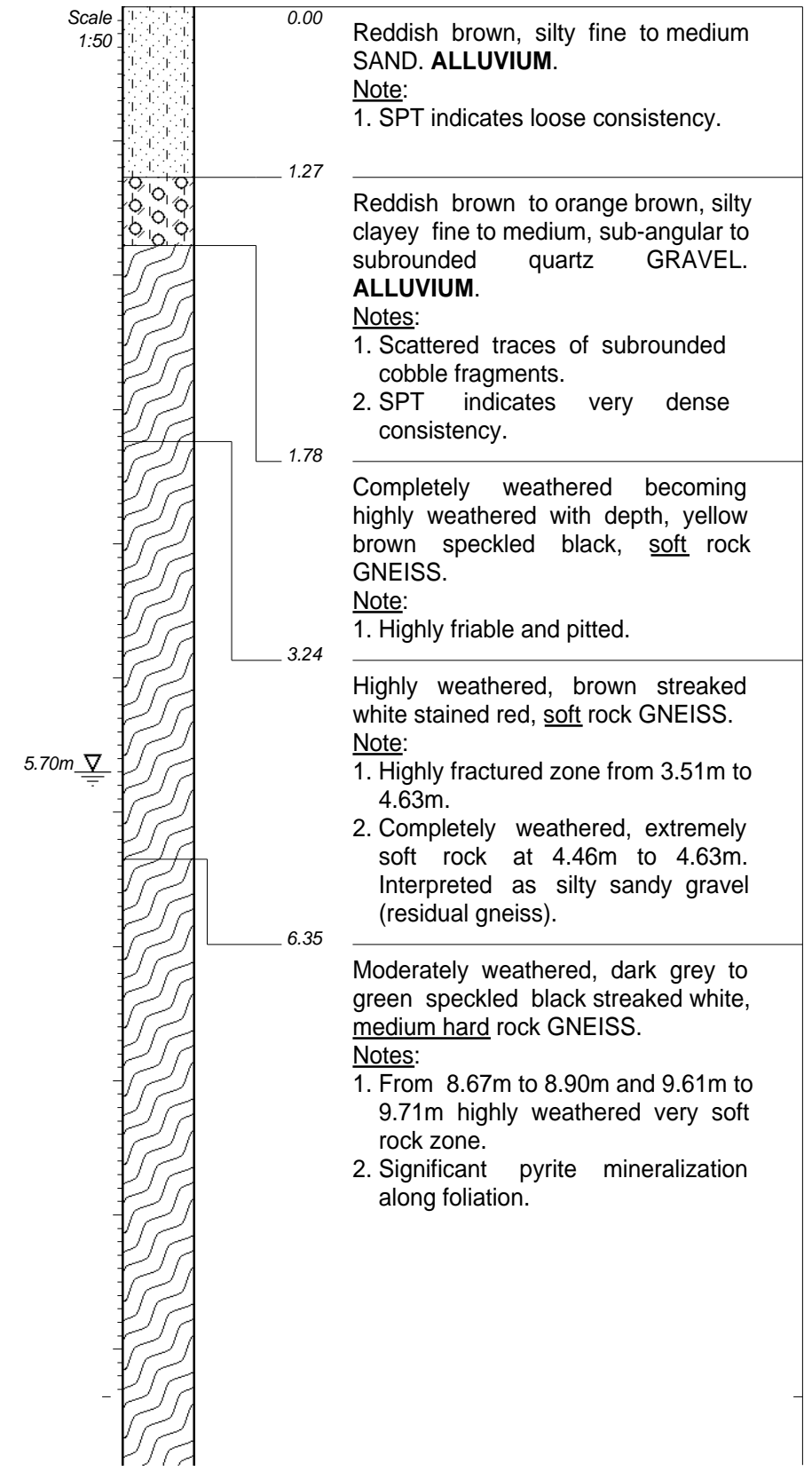
OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH03
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT	N=8			0.00
										0.84	100	0	0	NA	1
										1.50	100	0	0	NA	2
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.78	SPT	N=Ref			1.27
										2.00	100	100	68	5	2
MG-CG	GF	VIF	10-20	1 2	0-10 20	CJ CJ	RJ RJ	Silty sand Silty sand	1-2 2	3.51	99	95	62	7	3
										4.63	100	80	0	22	4
MG	GF	VIF	10-20	1 2	0-10 20-30	VCJ MJ	RJ RJ	FeOx & Silt Silty sand	1-2 <1	6.13	71	68	11	13	5
										7.61	100	100	75	9	6
										9.13	100	97	39	15	7
MG	GF	VIF-IF	10-20	1 2 3	0-10 40 70-80	VCJ-CJ VWJ VWJ	RJ RJ RJ	FeOx & silt Silt Clean	1-2 <1 -	10.63	100	99	45	12	8
															9
															10



HOLE No: Plant-BH03
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear SR -soft rock
 LF -laminated CJ -close spacing PLA-planar VSR-very soft rock
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular



OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH03
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
								Clean FeOx	- <1	12.13	100	99	35	16	
MG	GF	VIF	10	1 2	0-10 70	VCJ-MJ VCJ	RJ RJ	Clean FeOx	- <1	13.63	97	97	31	14	12.60 Slightly weathered to unweathered, dark grey speckled black streaked white, <u>hard</u> rock GNEISS.
								Silt Silt Silty sand	1 1-2 1	15.13	100	98	71	8	
MG	GF	VIF	10	1 2 3	0-10 20 70-80	VCJ CJ VWJ	RJ RJ RJ	Silt Silt Silty sand	1 1-2 1	16.63	100	87	15	17	14.90 Highly to completely weathered, grey to brown banded white, <u>very soft</u> to <u>soft</u> rock GNEISS. <u>Notes:</u> 1. Highly fractured from 15.37m to 16.21m. 2. Silt zone at 15.30m to 15.37m. 2. Highly fractured from 15.37m to 16.21m. 3. Higher mica content.
								Clean Calcite	- 1	18.13	100	100	61	7	16.33 Slightly weathered to unweathered, dark grey speckled black banded white, <u>hard</u> rock GNEISS. <u>Notes:</u> 1. Near vertical joint from 16.36m to 16.80m; 17.65m to 17.90m; 17.96m to 18.80m and 22.38m to 22.98m. 2. Moderately to highly weathered, interlayered medium hard rock zone at 20.32m to 20.55m. 3. Pegmatite vein from 21.62m to 21.75m and 23.87m to 23.99m. 4. Pyrite mineralization at 20.15m to 20.50m.
				1 2	0-10 90	CJ-MJ WJ	RJ RJ	Clean Calcite	- 1	19.63	100	100	56	2	
								Clean Calcite	- 1	21.13	96	95	72	8	

HOLE No: Plant-BH03
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose LF -laminated CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular



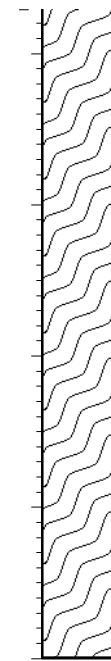
OMITIOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH03
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF	80	3	20	CJ	RJ	Silt	<1						21
				4	70	NM	RJ	Calcite	1-2	22.63	100	100	75	5	22
										24.13	100	100	70	5	23
										25.00	97	97	97	2	24
															25



- 25.00
- NOTES
- 1) Perched groundwater table encountered at 5.70m.
 - 2) NA: Not applicable.
 - 3) NM: Not measurable.

CONTRACTOR : RA Longstaff
 MACHINE : Sullivan HD
 DRILLED BY :
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
 DIAM :
 DATE : 4-6 May 2023
 DATE : 5-6 May 2023
 DATE : 07/07/2023 14:19
 TEXT : ..\PROFILES\PKFPLANTBH.TXT

COORDINATE SYSTEM : WGS 84 UTM 33S
 X-COORD : 802365.16
 Y-COORD : 7583468.49

HOLE No: Plant-BH03

HOLE No: Plant-BH04
Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



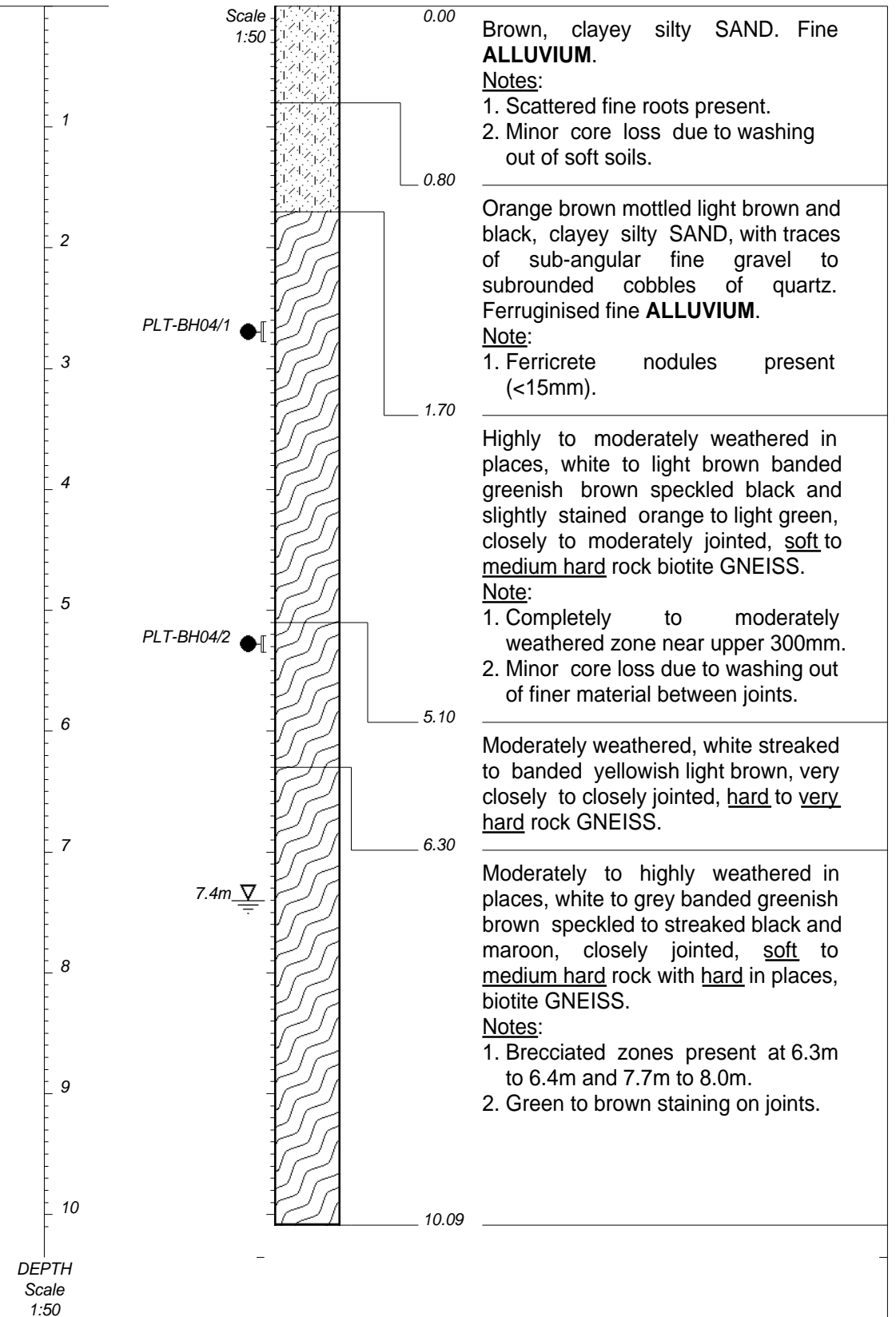
OMITOMIRE COPPER PROJECT
PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH04
Sheet 1 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	92.00	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100.00	0	-	NA
										1.70	SPT1	N=Ref		
										2.00	100.00	100	0	23
MG-CG	GF	IF	20-40	1 2	10-20 30-40	CJ MJ	RJ RJ	Chl + FeOx FeOx	<1 <1	3.54	88.00	88	55	10
										4.78	98.00	98	63	8
MG-CG	MF-GF	IF-VIF	10-20	1 2	10-20 40-60	VCJ-CJ MJ	SJ RJ	Cl.Silt & FeOx FeOx	<2 <1	6.28	100.00	100	59	22
MG-CG	GF	IF-VIF	10-30	1 2	20-30 50-60	CJ MJ-WJ	RJ RJ	Chl + FeOx FeOx	<1 <1	7.78	100	100	67	17
										9.28	87	87	64	9
										10.09	94	94	36	19



HOLE No: SWD-BH01
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 LF -laminated WJ -wide spacing VWJ-very wide spacng IRR-irregular

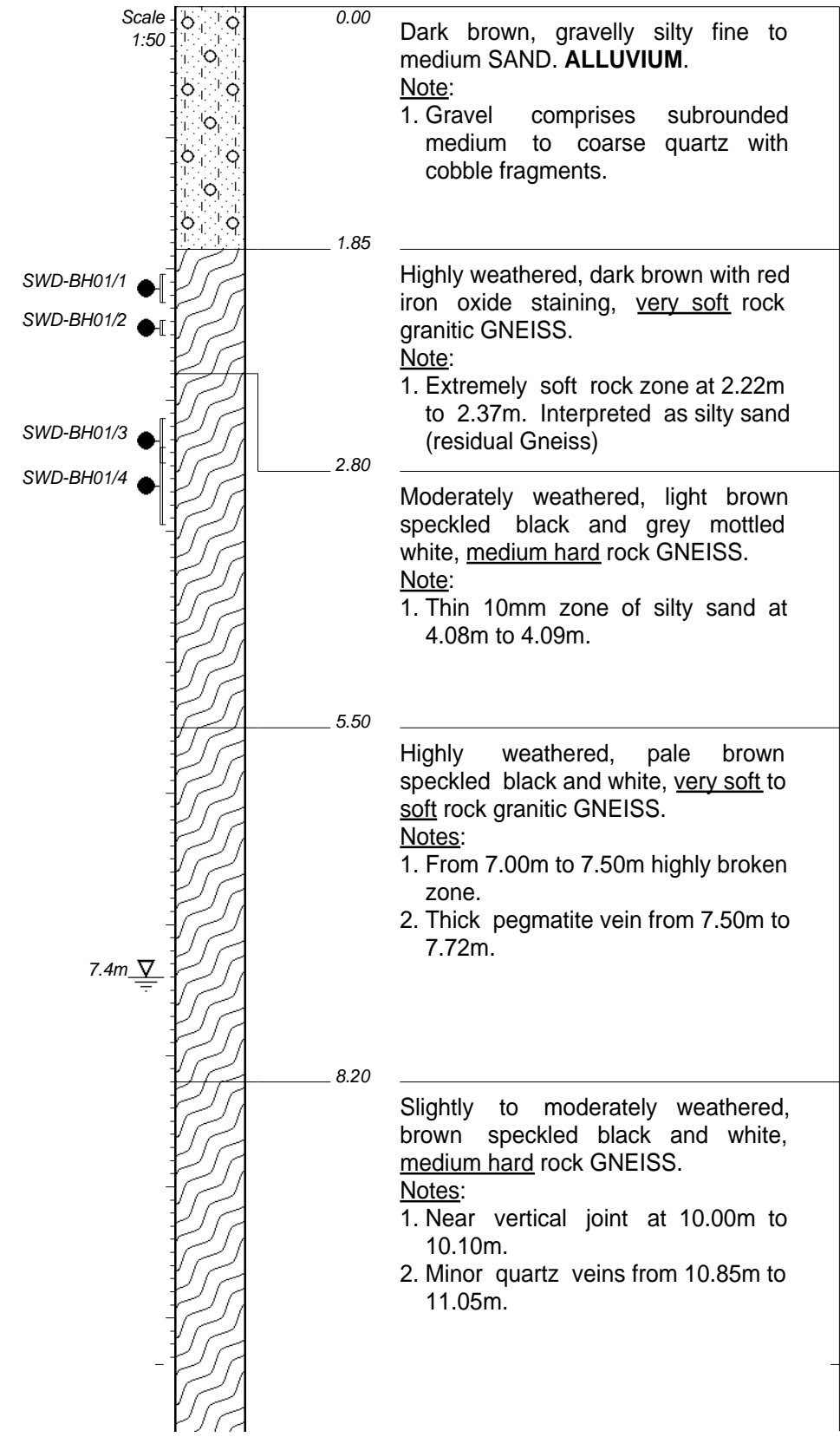


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH01
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.72	97	0	0	NA	0.00
										1.50	92	0	0	NA	1
										1.80	SPT	N=Ref			1.85
MG-CG	MF	NM	NM	1 2	0-10 20	VCJ-CJ CJ	RJ RJ	Silt Silty sand	1-2 1-2	3.00	90	75	75	11	2
										3.50	100	75	40	10	3
MG-CG	MF	NM	NM	1 2 3	0-10 20-30 40-50	CJ WJ WJ	RJ RJ RJ	Silty sand Silty sand Silt	2-5 1-2 2	4.32	100	100	78	7	4
										5.11	96	96	86	4	5
										5.60	87	87	51	9	5.50
MG	GF	IF	80	1 2	0-10 40-50	VCJ-CJ WJ	RJ RJ	Sand Silt	1-2 2	6.88	100	100	80	10	6
										8.09	99	99	44	11	7
										9.57	100	100	74	9	8
MG	GF	IF	85	1 2	0-10 80-90	VCJ-CJ NM	RJ RJ	Sand Silt	2 1-2						9
															10



HOLE No: SWD-BH01
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH01
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
										11.06	100	100	52	11	
MG	GF	IF	10-30	1 2 3 4	0-10 20-30 40-50 70	VCJ-CJ VCJ-CJ CJ NM	RJ RJ RJ RJ	Silty sand Silt Stained & Silt Stained	1 2 1-3 1	12.50	100	100	61	8	11.25 Highly weathered, brown speckled black banded white, <u>soft</u> rock GNEISS.
										14.03	99	99	54	9	
										15.50	95	95	71	8	14.85 Slightly weathered to unweathered, grey speckled black mottled white, <u>hard</u> to <u>very hard</u> rock granitic GNEISS.
										16.76	100	100	83	6	<u>Note:</u> 1. Moderately fractured from 23.00m to 23.70m. 2. Completely weathered zone at 22.85m to 22.92m.
MG-CG	MF-GF	NM	NM	1 2 3 4	0-10 30-40 40-50 80-90	CJ-MJ VWJ WJ WJ	RJ RJ RJ RJ	Clean Silt Silt & stained Silt	- 2-3 1 1	18.60	100	100	84	4	
										20.09	100	100	89	3	

HOLE No: SWD-BH01
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

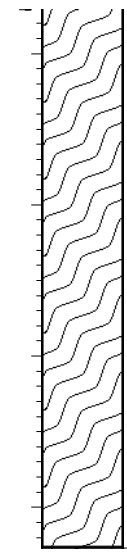


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH01
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
										21.39	100	100	100	2	
										22.82	100	100	100	0	
										24.27	88	88.00	55	8	



- 24.27
- NOTES**
- 1) Perched water table encountered at 7.4m.
 - 2) NA: Not applicable.
 - 3) NM: Not measurable.
 - 4) Sample SWD-BH01/1 taken at 2.04m--2.26m.
 - 5) Sample SWD-BH01/2 taken at 2.39m--2.51m.
 - 6) Sample SWD-BH01/3 taken at 3.14m--3.48m.
 - 7) Sample SWD-BH01/4 taken at 3.36m--3.95m.

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 13-15 May 2023
DATE : 14-15 May 2023
DATE : 07/07/2023 14:21
TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 801384
Y-COORD : 7581911

HOLE No: SWD-BH01

HOLE No: SWD-BH02
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 LF -laminated WJ -wide spacing IRR-irregular

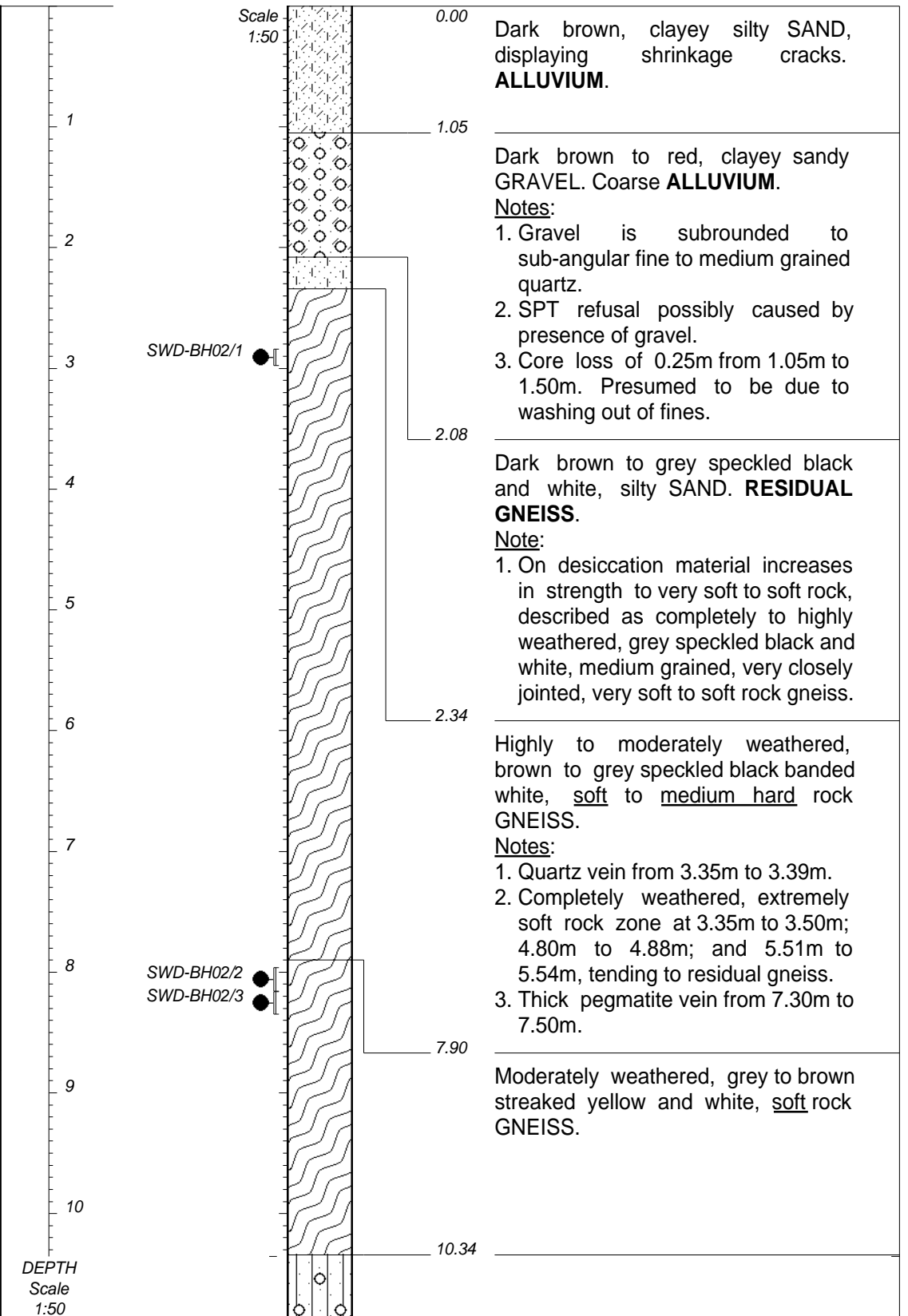


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.74	64	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	67	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.87	SPT	SPT=Ref		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	0	NA
MG	GF	IF	10-20	1 2	5-10 40-50	VCJ-CJ MJ	RJ RJ	Silt Silt	1 1-2	3.00	92	84	27	10
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-3	4.54	99	84	50	11
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-3	6.07	98	98	59	10
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-3	7.60	100	97	63	9
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-3	8.81	94	92	52	14
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-3	10.34	100	100	36	12



HOLE No: SWD-BH02
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 LF -laminated WJ -wide spacing IRR-irregular

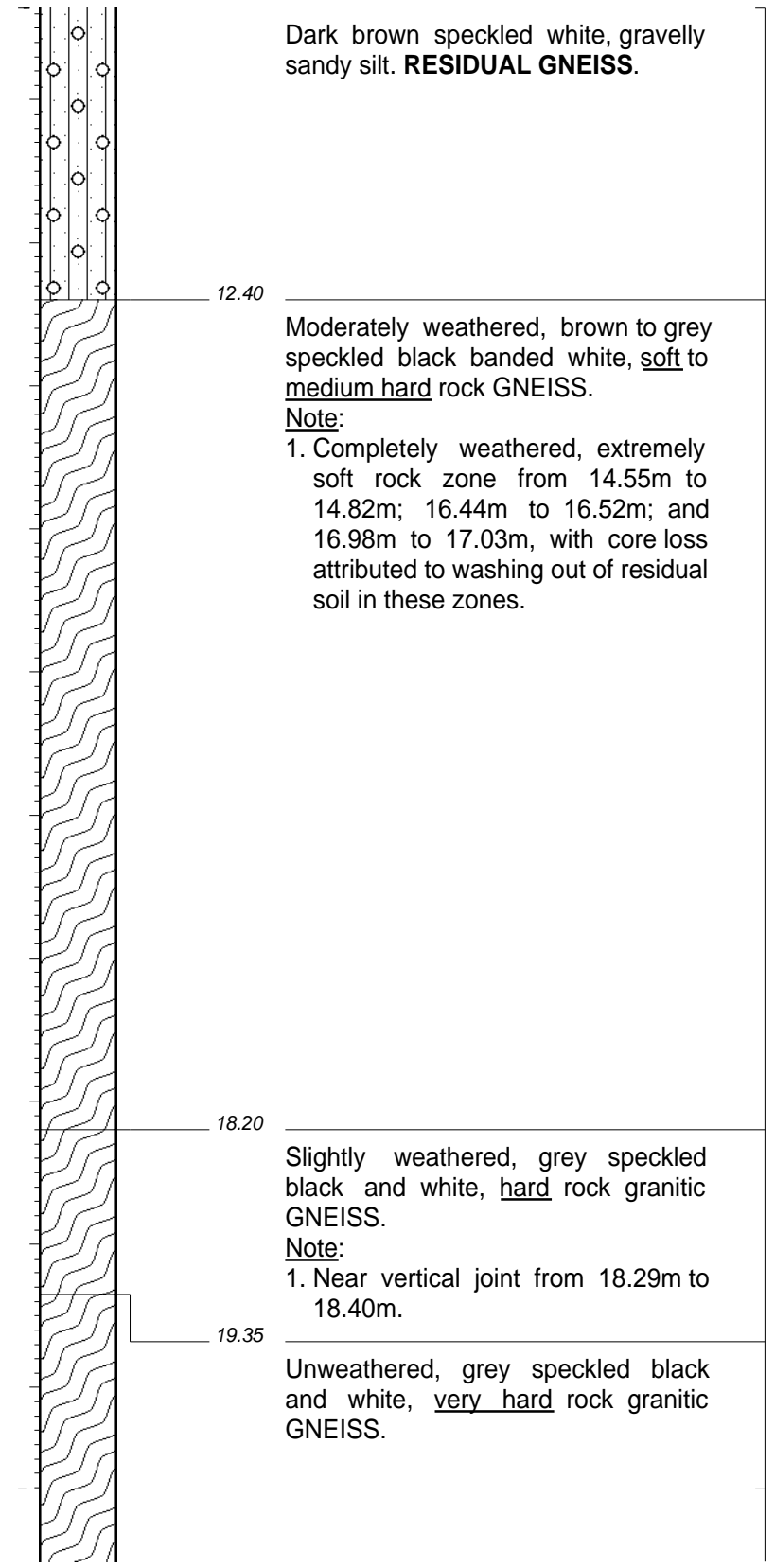


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	11.88	27	6	0	12	11
										12.64	100	100	26	8	12
										13.61	96	93	43	12	13
										15.11	87	83	15	17	14
MG	GF	IF	20-30	1	5-10	VCJ	RJ	Silt	<1	16.61	100	100	43	11	15
										18.11	96	93	40	15	16
										18.54	100	100	26	19	17
MG	MF-GF	NM	NM	1 2 3	0-10 20-30 80-90	VCJ CJ NM	RJ RJ RJ	Stained Stained Clean	<1 <1 -	19.61	100	98	87	6	18
										21.11	100	97	97	2	19
															20



HOLE No: SWD-BH02
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

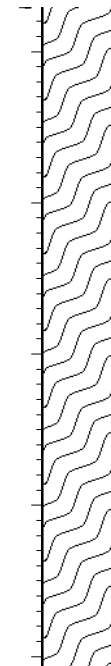
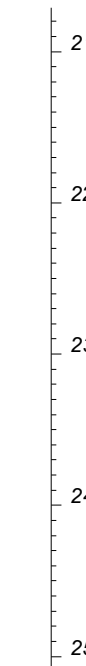


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
MG-CG	MF-GF	NM	NM	1	0-10	MJ-WJ	RJ	Stained	<1	22.53	100	100	98	1
										24.11	100	100	100	1
										25.11	100	100	100	0



25.11

NOTES

- 1) NA: Not applicable.
- 2) NM: Not measurable.
- 3) Sample SWD-BH02/1 taken at 2.84m--2.98m.
- 4) Sample SWD-BH02/2 taken at 7.96m--8.16m.
- 5) Sample SWD-BH02/3 taken at 8.16m--8.35m.

CONTRACTOR : RA Longstaff
 MACHINE : Sullivan HD
 DRILLED BY :
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
 DIAM :
 DATE : 17 April 2023
 DATE : 18 April 2023
 DATE : 07/07/2023 14:21
 TEXT : ..51\PROFILES\PKF\SWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
 X-COORD : 801681
 Y-COORD : 7581852

HOLE No: SWD-BH02

HOLE No: SWD-BH03
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

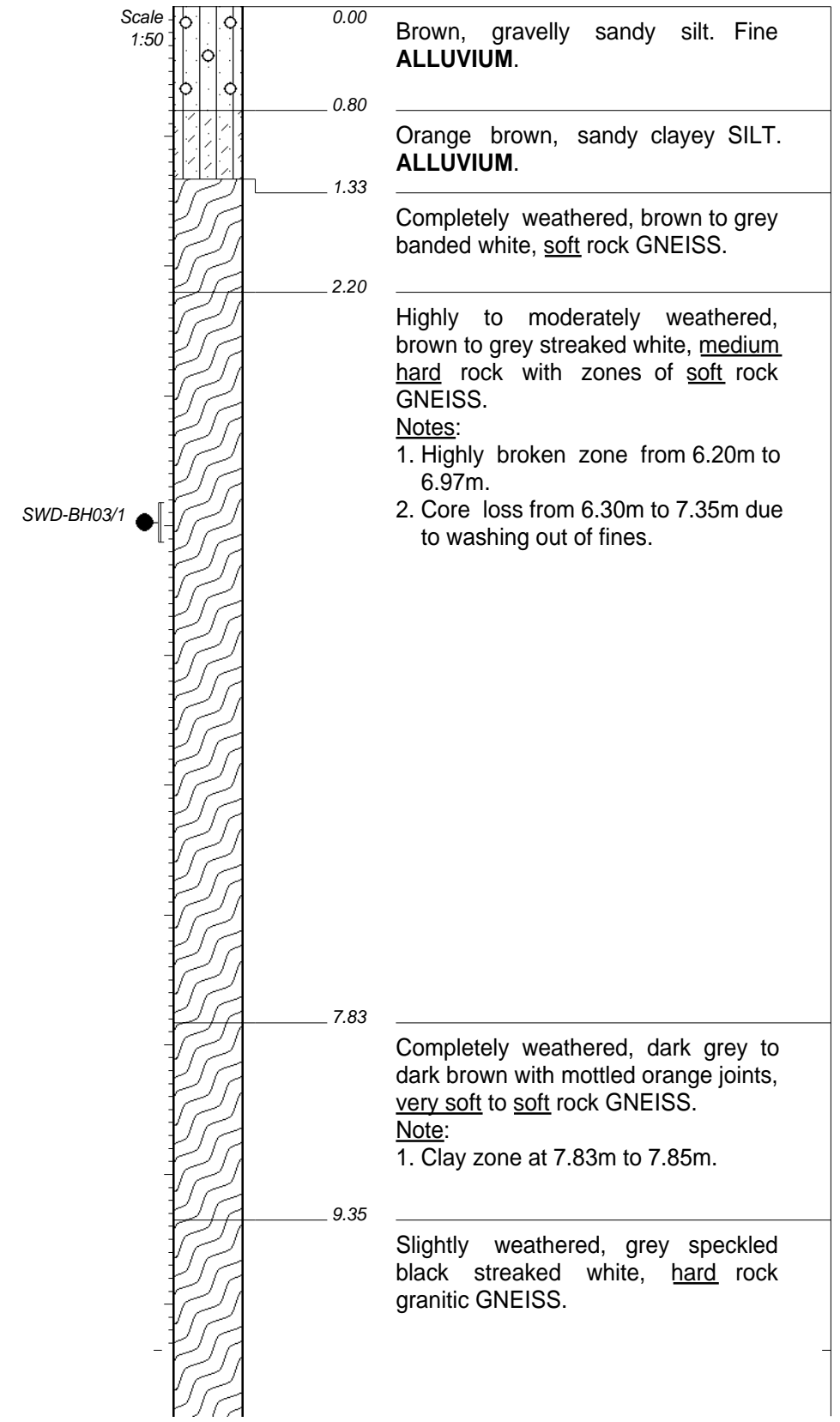


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH03
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.74	100	0	0	NA	0.00
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	0.80
MG	GF	-	-	1	0-10	VCJ-CJ	RJ	Silt	1	2.05	100	100	55	9	1.33
MG	GF	VIF	10-20	1 2	0-10 70	VCJ-CJ NM	RJ RJ	Silt + Sand FeOx & Calcite	1 2	3.54	100	97	67	11	2.20
										4.79	93	93	70	8	4.00
										6.30	96	92	58	11	5.00
MG	GF	VIF	10-20	1 2	0-10 60	CJ NM	RJ RJ	Silt FeOx	1 <1	7.35	84	70	10	13	7.00
										8.85	100	98	41	13	8.00
										10.40	98	98	45	14	9.35



HOLE No: SWD-BH03
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing PLA-planar VSR-very soft rock
 LF -laminated MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

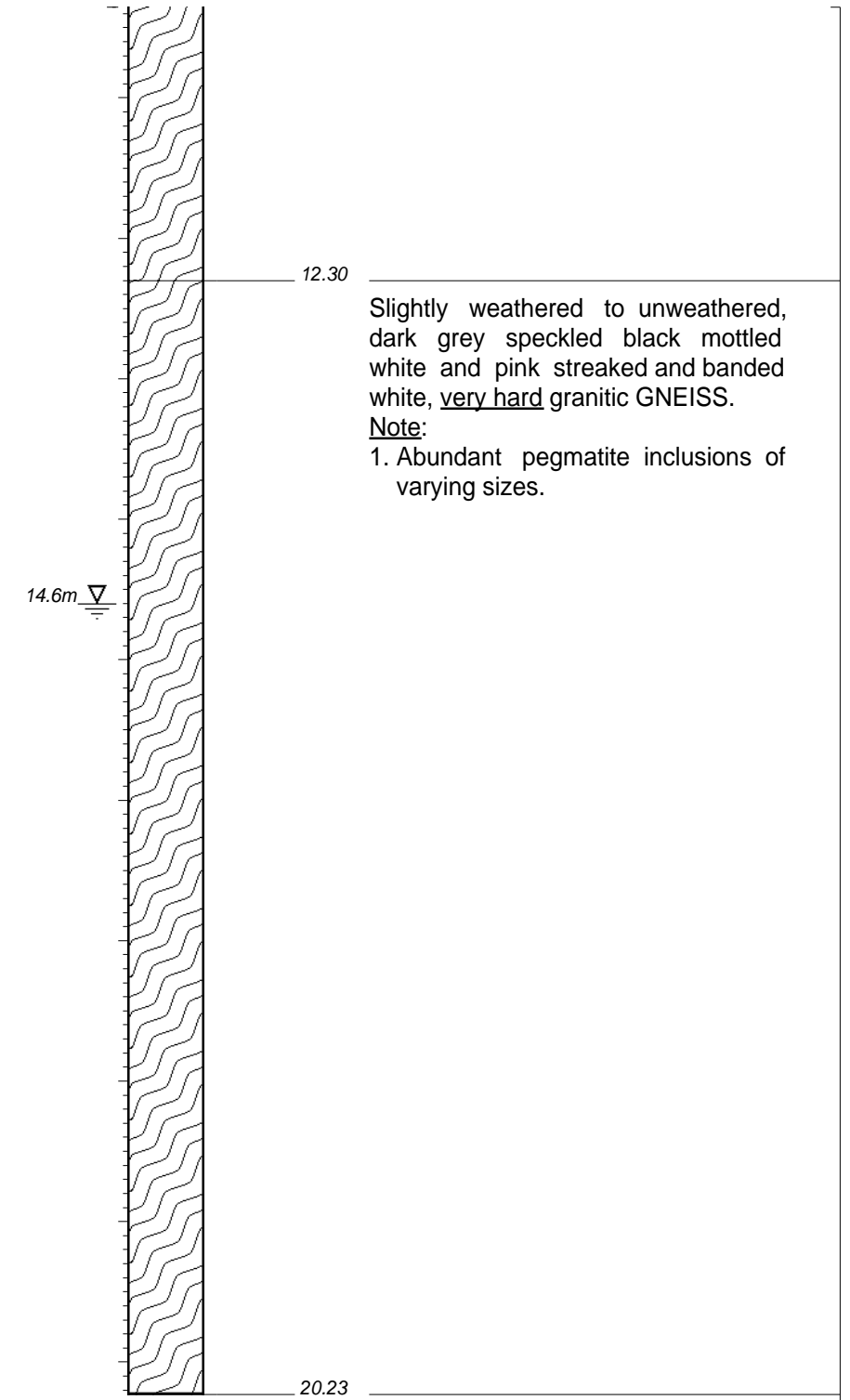


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH03
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	
MG	GF	VIF-IF	10-20	1 2	0-10 40	VCJ-CJ NM	RJ RJ	Pyrite FeOx & Calcite	<1 <1	11.95	100	100	41	14	11	
MG-CG	GF	IF	10-20	1 2 3	0-10 30 65	CJ-MJ VCJ CJ	RJ RJ RJ	Silt & Clean FeOx Clean	<1 <1 -	13.50	100	100	83	8	12	
										15.05	100	99	85	6	13	
										16.55	100	100	99	5	14	
										18.10	99	99	85	5	15	
										18.40	100	100	33	7	16	
										19.75	100	100	73	7	17	
										20.23	100	100	85	4	18	
															19	
																20



HOLE No: SWD-BH04
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

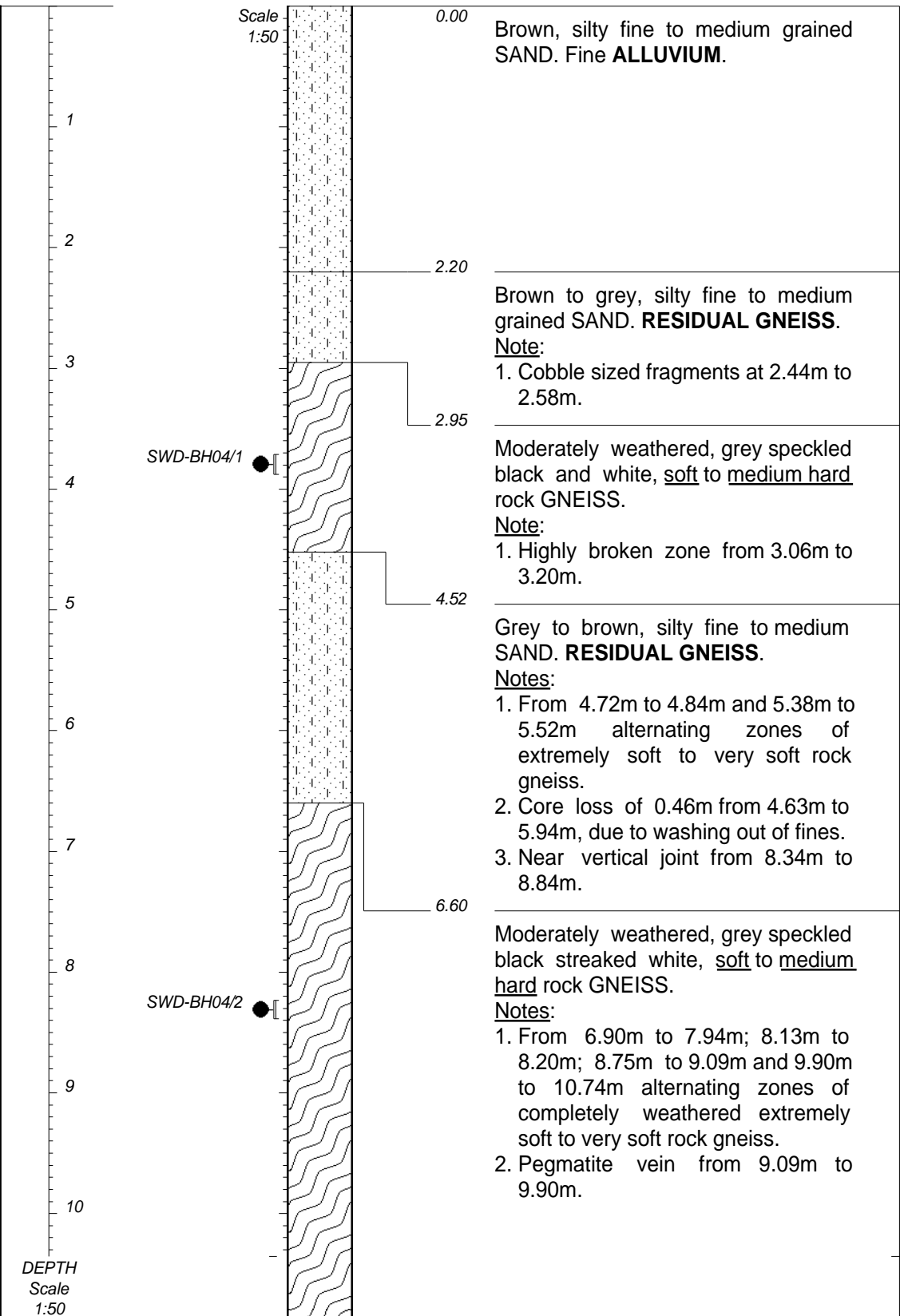


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH04
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	41	0	0	NA
										1.95	SPT	N=9		
										2.00	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.68	96	0	0	NA
										3.06	100	29	29	0
MG	GF	IF	10-30	1 2 3	5-10 40 50-60	VCJ WJ CJ	RJ RJ RJ	FeOx Silty sand FeOx	<1 1-2 <1	4.63	100	83	27	13
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	5.94	65	0	0	0
										6.63	100	0	0	0
MG	GF	IF	0-20	1 2	0-10 80-90	CJ CJ	RJ RJ	Sand FeOx & Silt	1 2-3	7.63	100	100	31	11
										9.03	100	100	23	19
										10.54	100	100	40	12



HOLE No: SWD-BH04
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

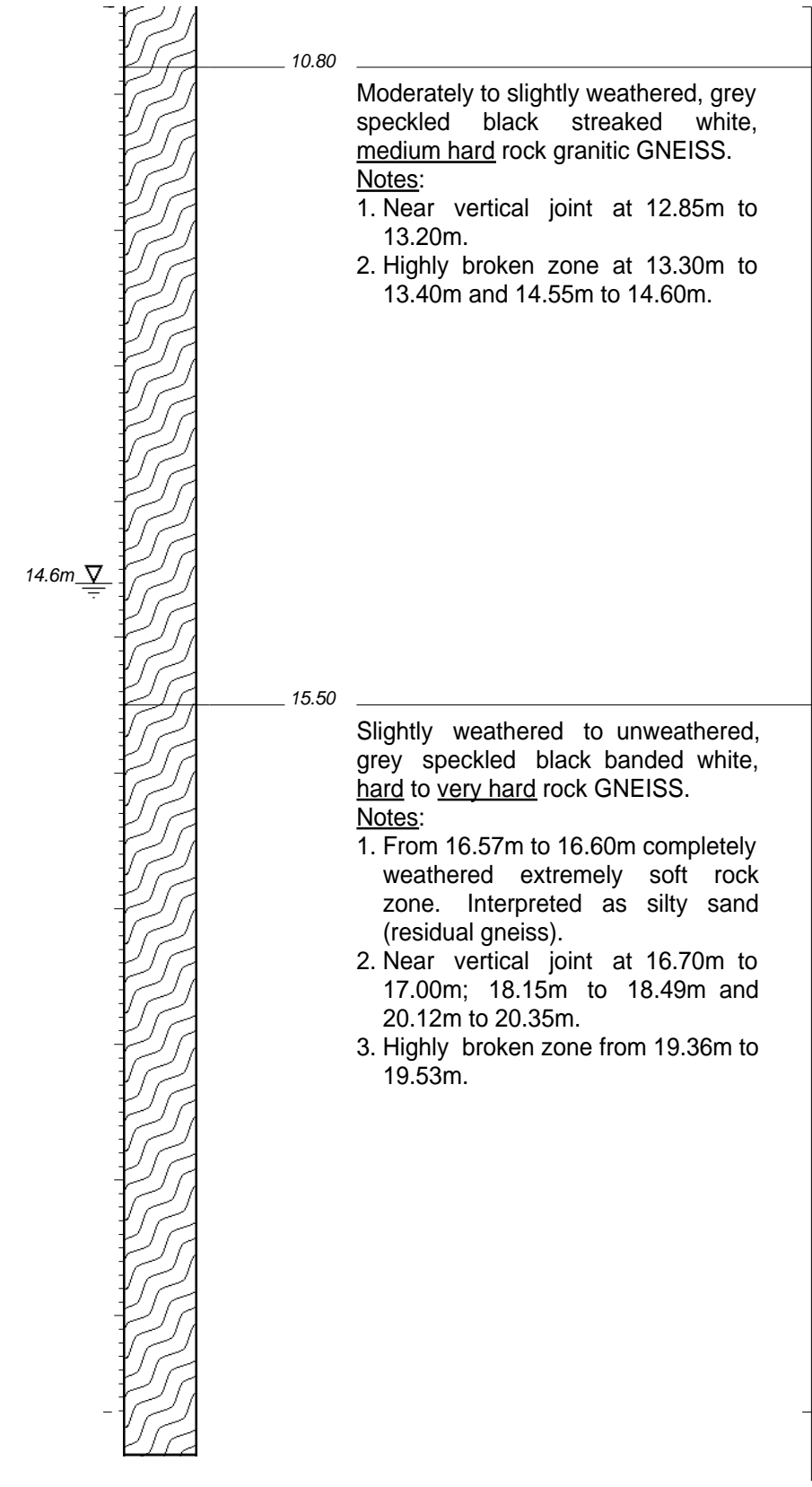


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH04
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF	20-30	1 2 3	0-10 70 80-90	VCJ-CJ NM VWJ	RJ SJ RJ	Silt Clayey silt FeOx & Silt	1 1 2-3	12.01	100	100	78	8	11
										13.56	96	96	30	15	12
										15.05	100	100	56	10	14
										15.56	100	100	54	10	15
										16.57	100	100	100	1	16
										17.98	97	95	67	9	17
										19.53	100	100	34	13	18
										21.03	100	100	68	7	19
															20



HOLE No: SWD-BH04
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

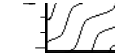


OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH04
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
															21



21.03

NOTES

- 1) Perched water table encountered at 14.6m.
- 2) NA: Not applicable.
- 3) NM: Not measurable.
- 4) Sample SWD-BH04/1 taken at 3.71m--3.88m.
- 5) Sample SWD-BH04/2 taken at 8.23m--8.39m.

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 12 May 2023
DATE : 12 May 2023
DATE : 07/07/2023 14:21
TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 801678
Y-COORD : 7581770

HOLE No: SWD-BH04

HOLE No: SWD-BH05
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

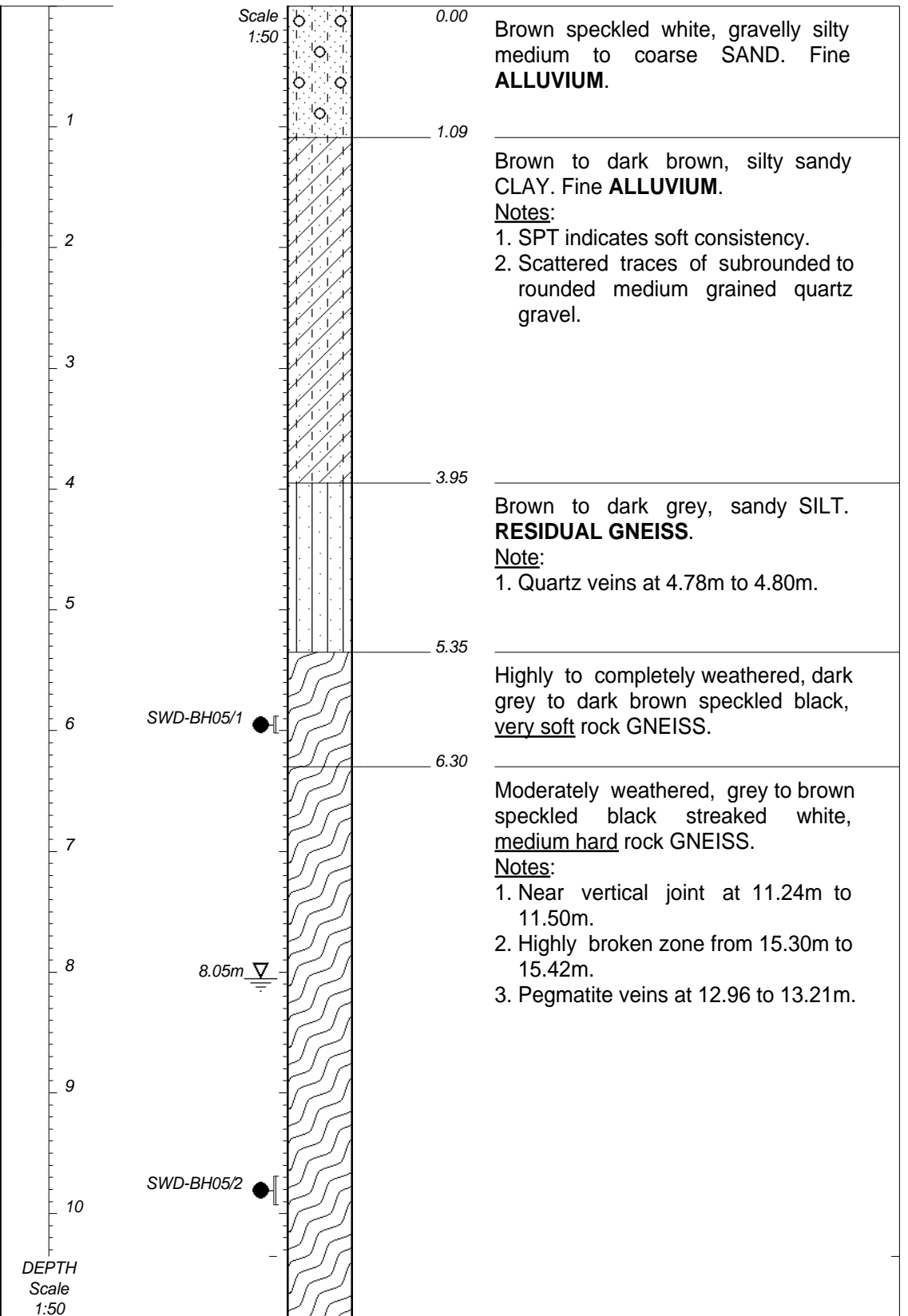


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH05
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	98	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.95	SPT	N=7		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	84	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.45	SPT	N=8		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.65	SPT	N=Ref		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.14	96	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.64	98	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	5.41	100	0	0	NA
FG-MG	GF	IF	0-10	1	0-10	CJ	RJ	Silt	<1	6.14	100	88	82	5
										7.44	100	100	48	12
										9.03	94	91	31	11
										10.53	100	100	55	5



HOLE No: SWD-BH05
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

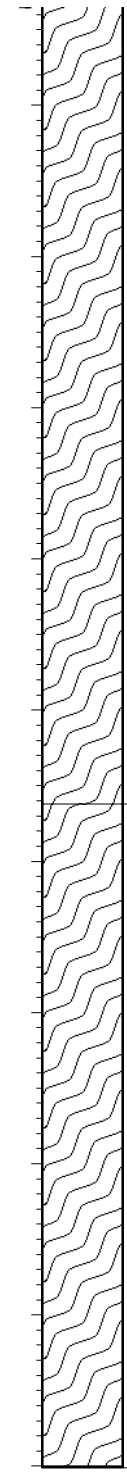


OMITOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH05
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	VIF	10-20	1 2 3	0-10 60 80-90	CJ WJ VWJ	RJ RJ RJ	Silty sand FeOx & Calcrete FeOx	1 <1 <1	12.08	100	100	50	10	11
										13.61	100	100	29	13	12
										15.14	92	92	65	10	13
										16.64	100	88	56	11	14
										18.14	95	85	60	10	15
										19.64	100	92	83	5	16
										20.01	100	100	100	5	17
															18
															19
															20



15.62
Slightly weathered, grey to brown speckled black streaked white, medium hard rock GNEISS.
Notes:
1. Completely weathered extremely soft rock zones at 16.64m to 16.70m; 17.00m to 17.08m and 1.89m to 18.93m.
2. Interpreted as residual gneiss, comprising dark brown silty sand.

20.01
NOTES
1) Perched water table encountered at 8.05m.

HOLE No: SWD-BH05
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



OMITIOMIRE COPPER PROJECT
GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH05
Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50

- 2) NA: Not applicable.
- 3) NM: Not measurable.
- 4) Sample SWD-BH05/1 taken at 5.88m--6.02m.
- 5) Sample SWD-BH05/2 taken at 9.69m--9.93m.

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 20-22 May 2023
DATE : 21-22 May 2023
DATE : 07/07/2023 14:21
TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 801740
Y-COORD : 7581876

HOLE No: SWD-BH05

HOLE No: SWD-BH06
Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

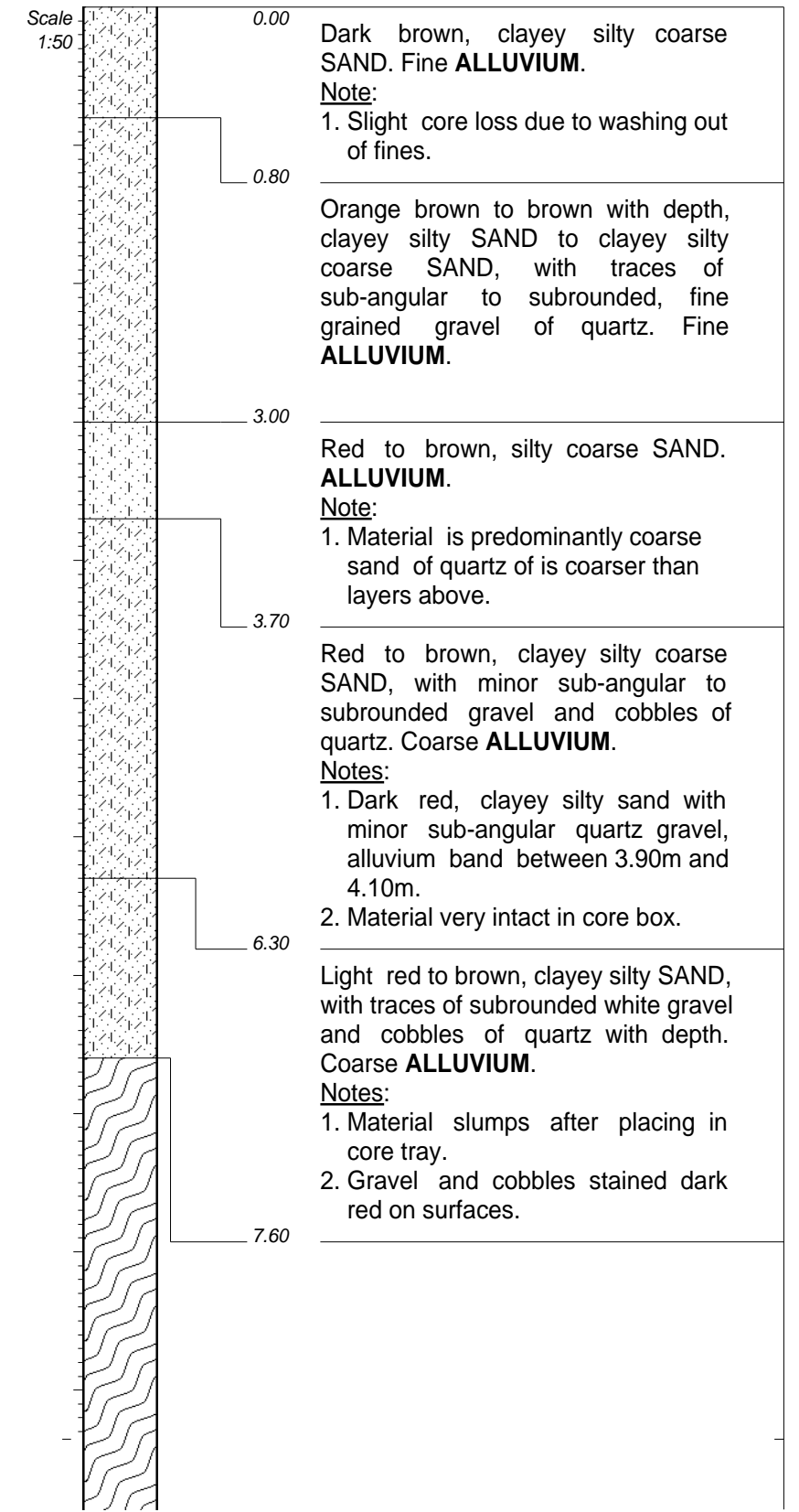


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH06
Sheet 1 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	86	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.95	SPT1	N=37		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	64	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.19	SPT2	N=Ref		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.75	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.05	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.76	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	6.26	100	11	0	NM
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	7.76	75	17	0	NM
										8.29	100	100	28	28
										9.26	100	100	15	19
										10.76	100	100	70	13



HOLE No: SWD-BH06
Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

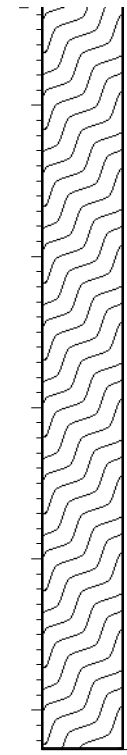
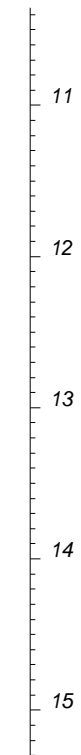


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH06
Sheet 2 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
MG-CG	GF	IF	30-50	1 2	20-30 40-50	CJ MJ	RJ RJ	FeOx & Chl FeOx	<1 <1	12.26	100	100	80	9
										13.76	100	100	56	17
										15.27	99	99	85	10



Highly to moderately weathered with depth, grey banded greenish brown to dark grey speckled black to green and stained orange on joints, closely to moderately jointed, soft to medium hard rock with depth, with pockets of very soft rock biotite GNEISS.

Notes:

1. Completely weathered zone present throughout zone at 8.10m to 8.30m; 12.80m to 13.00m; 14.10m to 14.20m; 14.57m to 14.67m.
2. Tends to migmatite in places, from below 13.20m with distinct ptygmatic folding.

NOTES

- 1) NA: Not applicable.
- 2) NM: Not measurable.

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
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CONTRACTOR : RA Longstaff
 MACHINE : Sullivan HD
 DRILLED BY :
 PROFILED BY : S YaFrance
 TYPE SET BY : EM
 SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
 DIAM :
 DATE : May 2023
 DATE : 1 June 2023
 DATE : 07/07/2023 14:21
 TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
 X-COORD : 803037.82
 Y-COORD : 7581516.63

HOLE No: SWD-BH06

HOLE No: SWD-BH07
Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

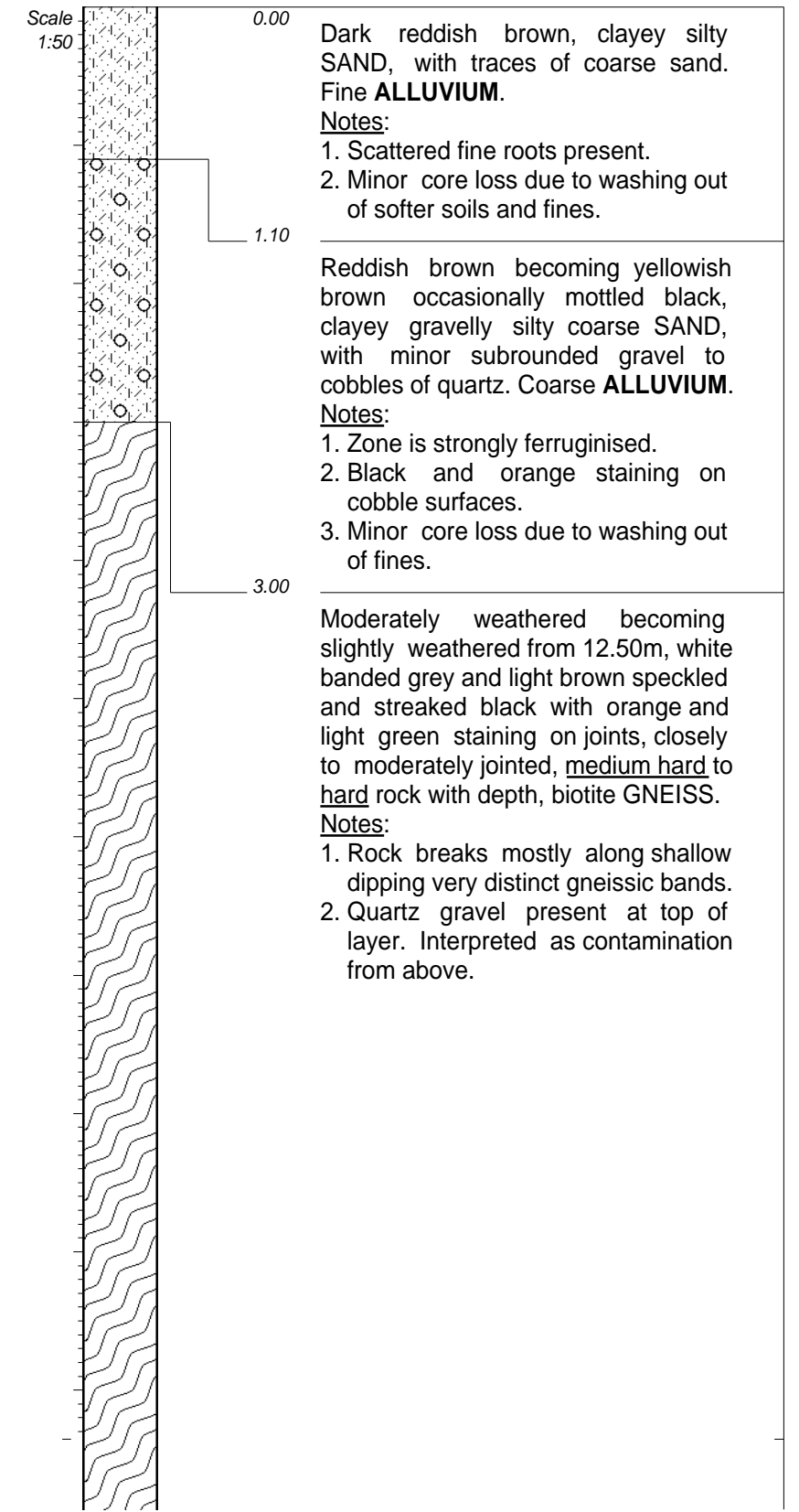


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH07
Sheet 1 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	81	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	97	0	-	NA
MG-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	VCJ-CJ MJ	SJ RJ	FeOx & Chl & mica FeOx & Chl & mica	<1 <1	4.50	83	69	14	13
MG-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	VCJ-CJ MJ	SJ RJ	FeOx & Chl & mica FeOx & Chl & mica	<1 <1	6.05	100	100	54	14
MG-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	VCJ-CJ MJ	SJ RJ	FeOx & Chl & mica FeOx & Chl & mica	<1 <1	7.55	100	100	71	15
MG-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	VCJ-CJ MJ	SJ RJ	FeOx & Chl & mica FeOx & Chl & mica	<1 <1	9.05	100	100	75	13
MG-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	VCJ-CJ MJ	SJ RJ	FeOx & Chl & mica FeOx & Chl & mica	<1 <1	10.60	100	100	81	12



HOLE No: SWD-BH07
Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

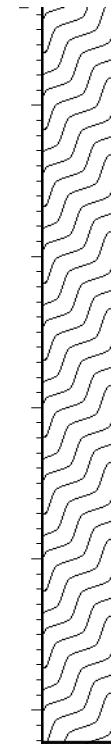


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH07
Sheet 2 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
										12.15	100	100	70	13	11
										13.75	98	98	62	9	12
										15.22	100	100	83	8	13
															14
															15



15.22
NOTES
1) NA: Not applicable.
2) NM: Not measurable.

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 2023
DATE : 1 June 2023
DATE : 07/07/2023 14:21
TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 804077
Y-COORD : 7582410

HOLE No: SWD-BH07

HOLE No: SWD-BH08
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

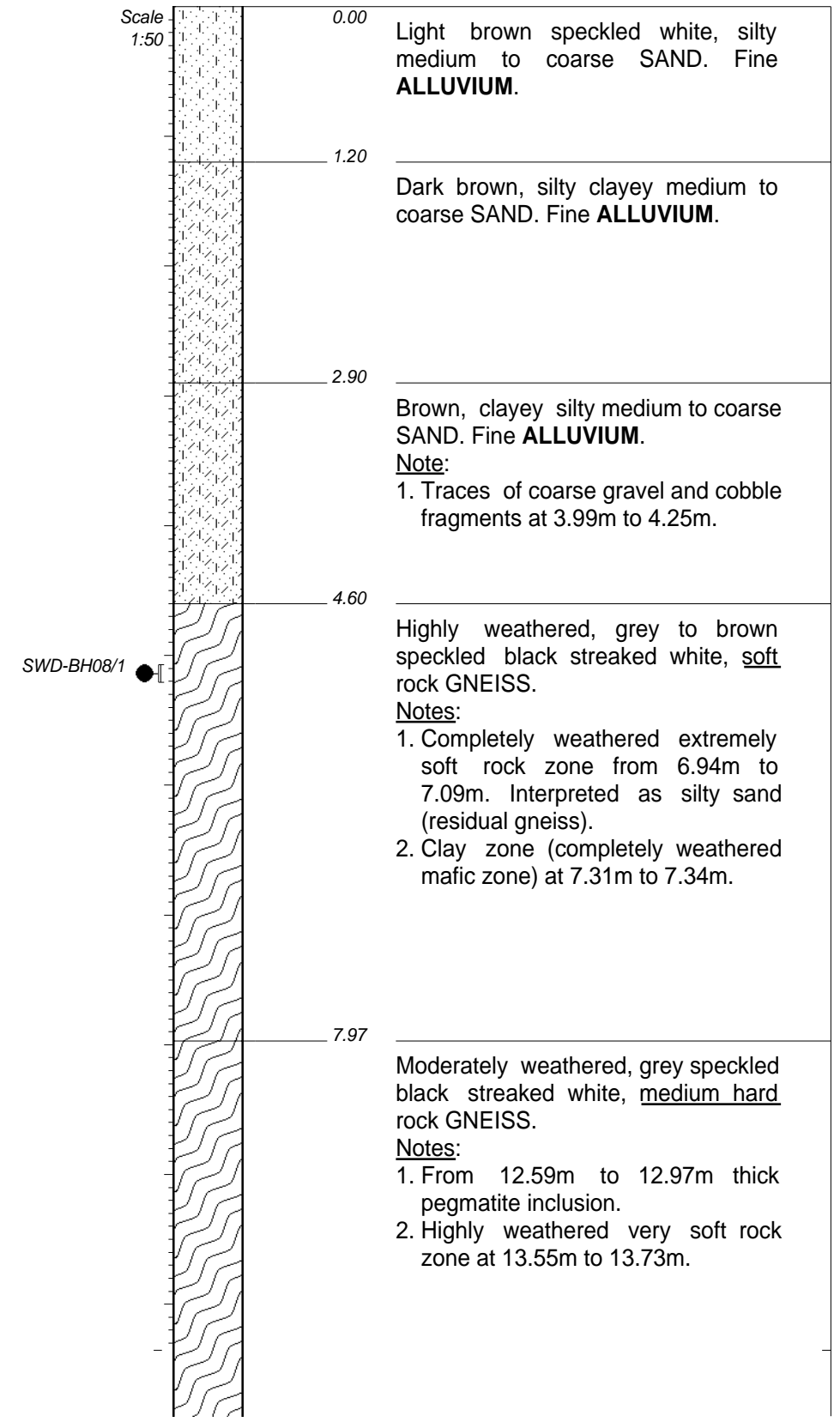


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH08
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	71	0	0	NA	0.00
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	1.20
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.95	SPT	N=13			2.00
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	0	NA	2.90
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	100	0	0	NA	3.90
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.45	SPT	N=10			4.60
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.19	91	0	0	NA	4.60
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.78	100	0	0	NA	4.60
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	5.50	65	65	27	14	4.60
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	6.24	99	99	19	23	4.60
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	7.77	92	76	39	10	7.97
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	9.30	100	100	59	12	7.97
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	10.80	96	93	59	11	7.97



HOLE No: SWD-BH08
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

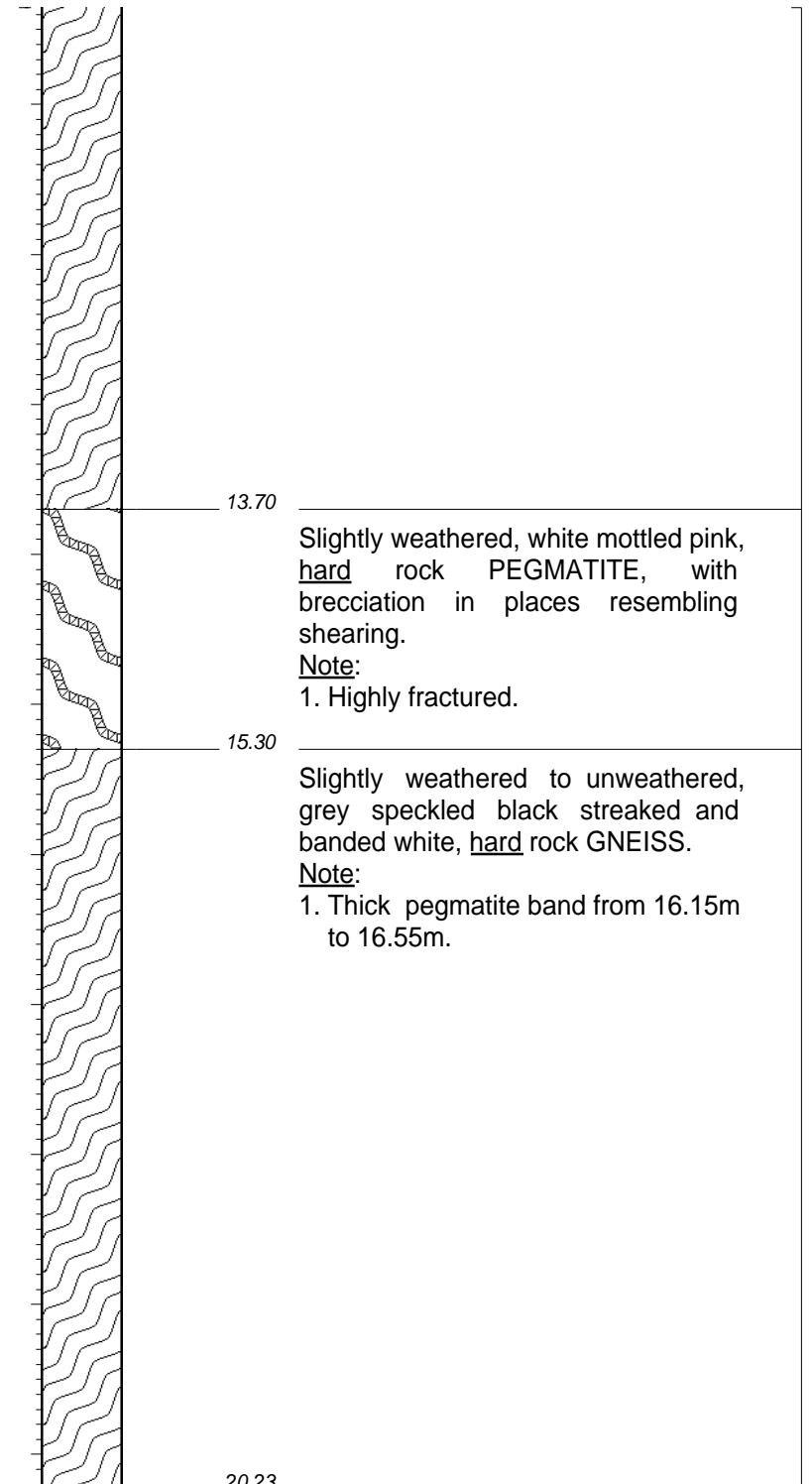


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH08
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	IF	80	1 2 3	0-10 20 60-70	CJ CJ WJ	RJ RJ RJ	Sand Silt FeOx	1 <1 <1	12.30	97	97	51	11	
										13.80	97	93	47	11	
CG-VCG	MF	NM	NM	1	30-40	CJ	RJ	Silt	3	14.55	100	85	13	20	
										15.30	95	51	0	NM	
MG	GF	IF	10-20	1 2 3	0-10 20 80	CJ CJ CJ	RJ RJ RJ	Clean Silt FeOx	- <1 <1	16.80	97	97	68	8	
										18.30	100	99	64	10	
										19.80	100	100	65	8	
										20.23	100	100	28	16	



NOTES
1) NA: Not applicable.

HOLE No: SWD-BH09
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved SF -schistose JOINT SPACING JOINT SHAPE MHR-medium hard rock
 SF -schistose VCJ-very close spacg CUR-curvilinear SR -soft rock
 GF -gneissose CJ -close spacing MJ -medium spacing UND-undulating VSR-very soft rock
 LF -laminated WJ -wide spacing VWJ-very wide spacng IRR-irregular

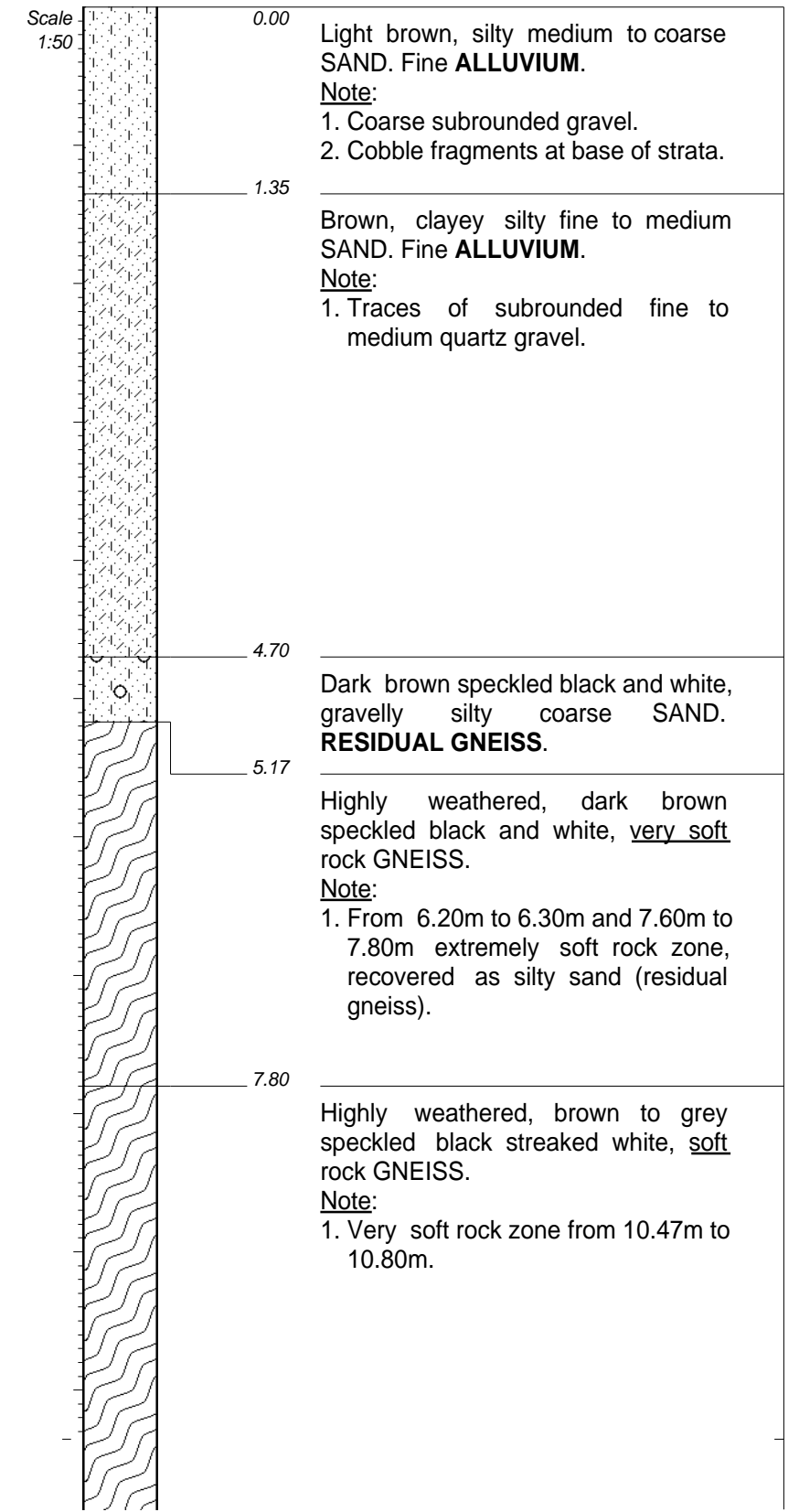


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH09
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.95	SPT	N=8		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.09	71	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.56	96	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.34	68	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.79	SPT	N=13		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.30	90	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.84	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	6.30	88	73	1	4
MG	GF	NM	NM	1 2	0-10 80	CJ NM	RJ RJ	Silt Silt	1 1	7.80	83	62	15	8
MG	GF	IF	0-20	1 2	10 30	VCJ-CJ CJ	RJ RJ	Silt Silt	1 1-2	9.24	99	99	77	2
MG	GF	IF	0-20	1 2	10 30	VCJ-CJ CJ	RJ RJ	Silt Silt	1 1-2	10.80	100	100	62	10



HOLE No: SWD-BH09
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	

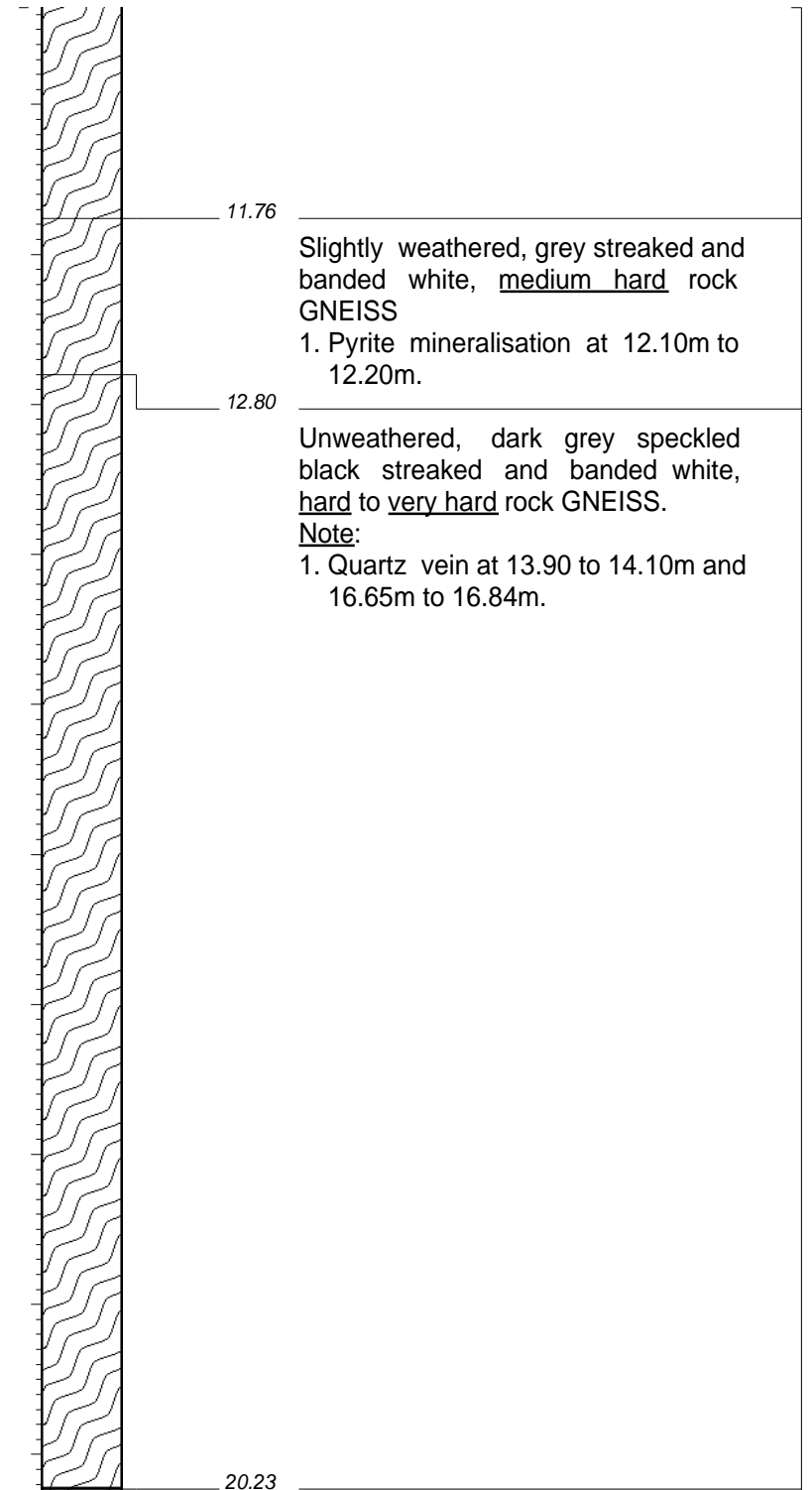


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH09
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
										12.30	100	98	73	10	
MG	GF	IF	0-20	1 2	10 30	VCJ-CJ CJ	RJ RJ	FeOx Clean	<1 -	13.03	100	100	59	14	
										13.80	99	99	99	4	
										15.30	99	99	99	3	
MG-CG	GF	IF	10-20	1 2	10 20	CJ-MJ CJ-MJ	RJ RJ	Clean Clean	- -	16.80	100	100	95	7	
										18.30	100	100	93	5	
										19.80	100	100	97	4	
										20.23	100	100	100	1	



NOTES
1) NA: Not applicable.

HOLE No: SWD-BH09
Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
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CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
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LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH09
Sheet 3 of 3

JOB: 3010047806

2) NM: Not measurable.

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : 13-15 May 2023
DATE : 15 May 2023
DATE : 07/07/2023 14:21
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COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 804688
Y-COORD : 7583254

HOLE No: SWD-BH09

HOLE No: SWD-BH10
Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS
 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock
 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock
 FF -foliated CG -coarse grain RJ -rough HR -hard rock
 CF -cleaved MHR-medium hard rock
 SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock
 GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock
 LF -laminated CJ -close spacing PLA-planar
 MJ -medium spacing UND-undulating
 WJ -wide spacing STE-stepped
 VWJ-very wide spacng IRR-irregular

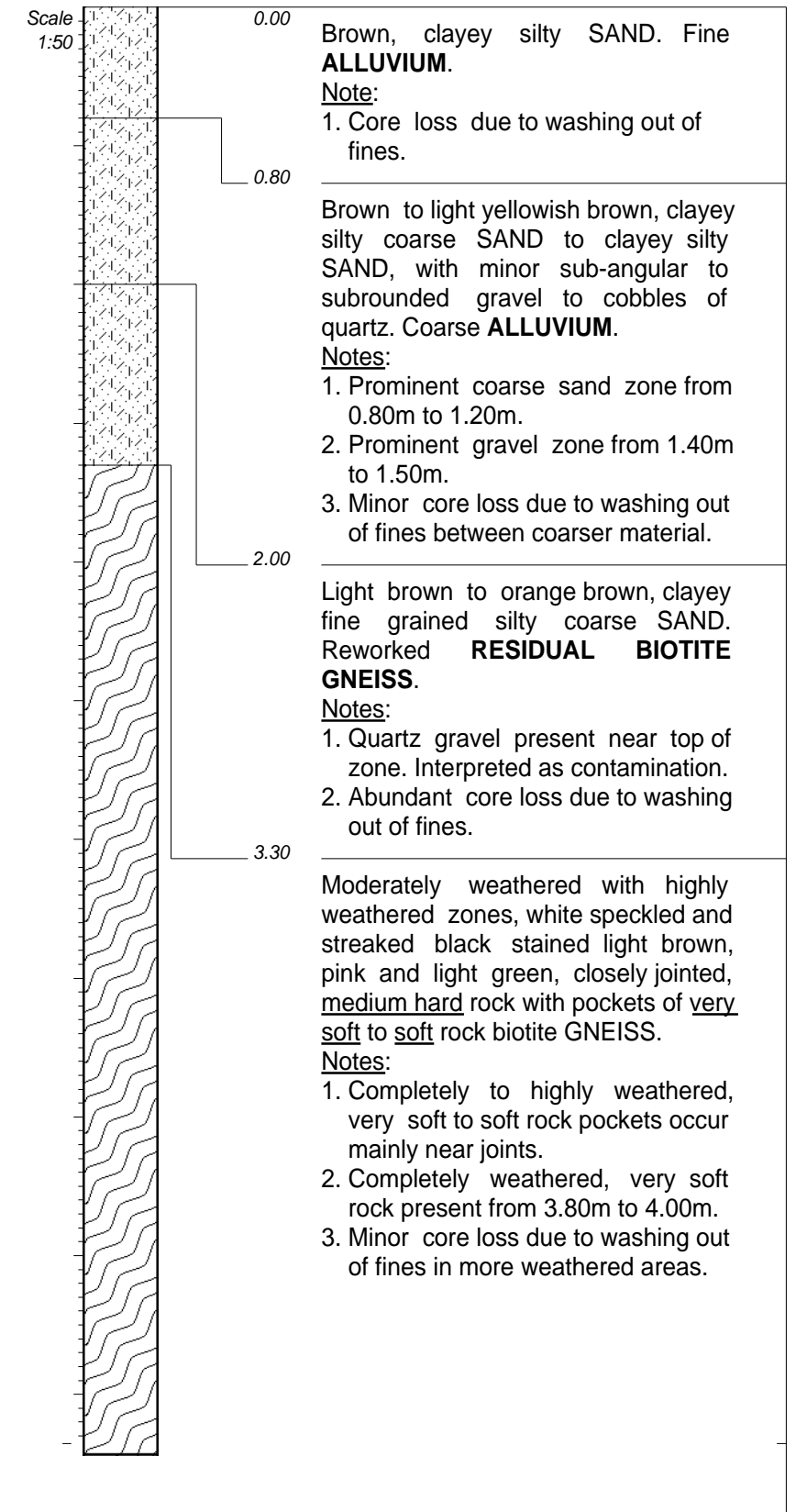


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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH10
Sheet 1 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	71	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	71	12	0	NM
										1.95	SPT1	N=Ref		
										2.00	100	0	-	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	59	0	-	NA
										3.15	SPT2	N=Ref	-	NA
										4.34	92	67	0	24
										5.88	94	94	27	19
MG-CG	GF	IF-VIF	0-10	1 2 3	10-30 40-50 80-90	CJ-MJ MJ WJ	SJ RJ RJ	Silty sand + mica FeOx & Chl & mica FeOx & mica & Chl	<2 <1 <1	7.34	100	100	45	14
										8.89	99	99	19	22
										10.44	99	99	29	23



HOLE No: SWD-BH10
Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH10
Sheet 2 of 2

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50

10.44

- NOTES
- 1) NA: Not applicable.
 - 2) NM: Not measurable.

CONTRACTOR : RA Longstaff
MACHINE : Sullivan HD
DRILLED BY :
PROFILED BY : S YaFrance
TYPE SET BY : EM
SETUP FILE : KPBHCO08.SET

INCLINATION : Vertical
DIAM :
DATE : June 2023
DATE : 1 June 2023
DATE : 07/07/2023 14:21
TEXT : ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM : WGS84 UTM 33S
X-COORD : 801538
Y-COORD : 7581868

HOLE No: SWD-BH10

HOLE No: WRD-BH01
Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



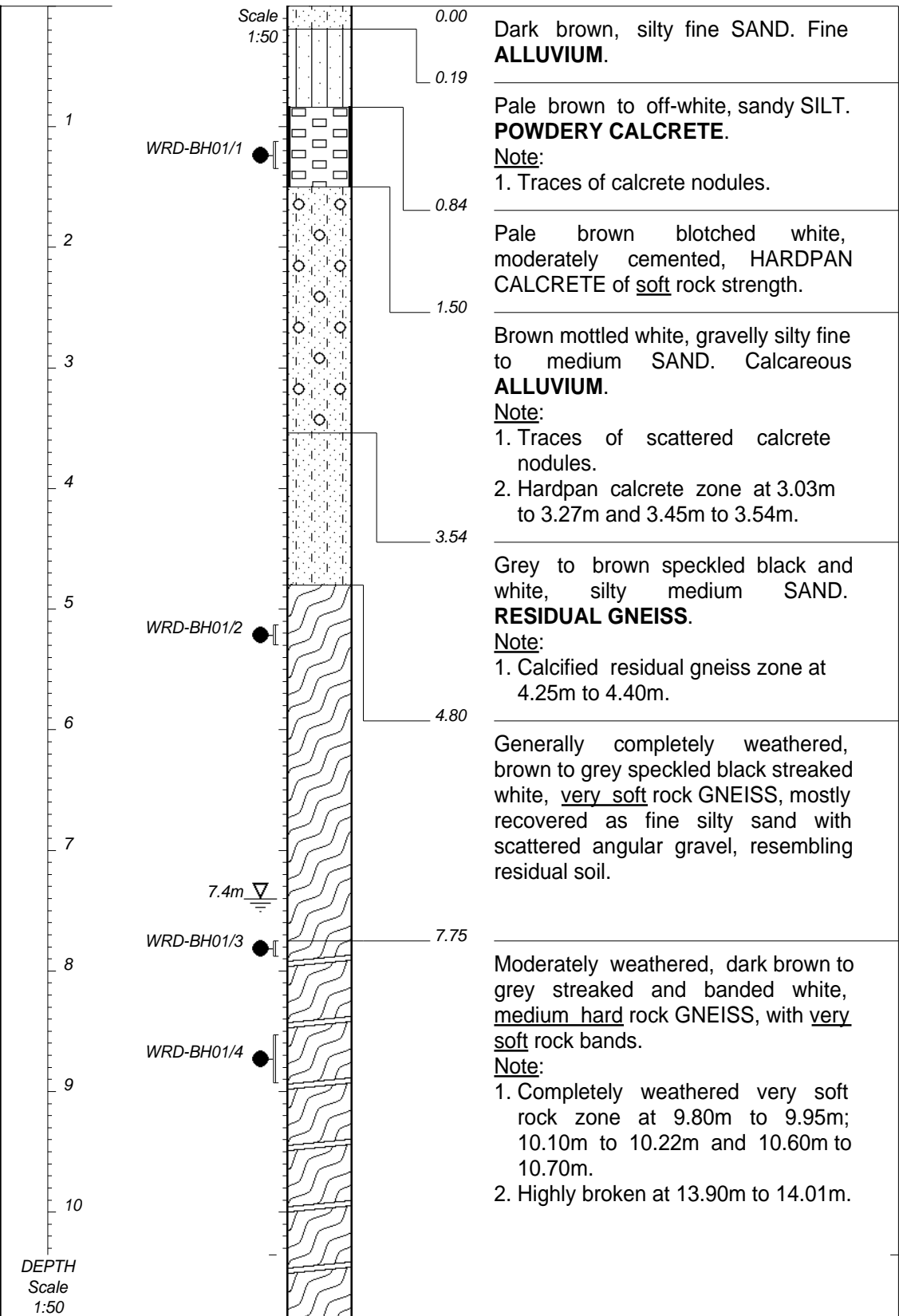
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WASTE ROCK DUMP

GEOTECHNICAL INVESTIGATION

HOLE No: WRD-BH01
Sheet 1 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	98	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	98	0	0	NA
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.49	77	0	0	NA
MG	GF	IF	20	1	0-10	CJ	RJ	Silty sand	<1	4.75	100	0	0	NA
MG	NM	NM	NM	NM	NM	NM	NM	NM	NM	5.47	100	76	49	7
										6.25	100	0	0	NA
										6.97	99	28	19	3
										7.75	99	0	0	NA
										8.23	100	100	27	10
										9.25	100	100	71	9
										10.75	100	91	48	9



HOLE No: WRD-BH01
Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC	GRAIN SIZE	JOINT ROUGHNESS	ROCK HARDNESS
MF -massive	FG -fine grained	SLJ-slickensided	EHR-extremely hard rock
BF -bedded	MG -medium grain	SJ -smooth	VHR-very hard rock
FF -foliated	CG -coarse grain	RJ -rough	HR -hard rock
CF -cleaved			MHR-medium hard rock
SF -schistose	JOINT SPACING	JOINT SHAPE	SR -soft rock
GF -gneissose	VCJ-very close spacg	CUR-curvilinear	VSR-very soft rock
LF -laminated	CJ -close spacing	PLA-planar	
	MJ -medium spacing	UND-undulating	
	WJ -wide spacing	STE-stepped	
	VWJ-very wide spacng	IRR-irregular	



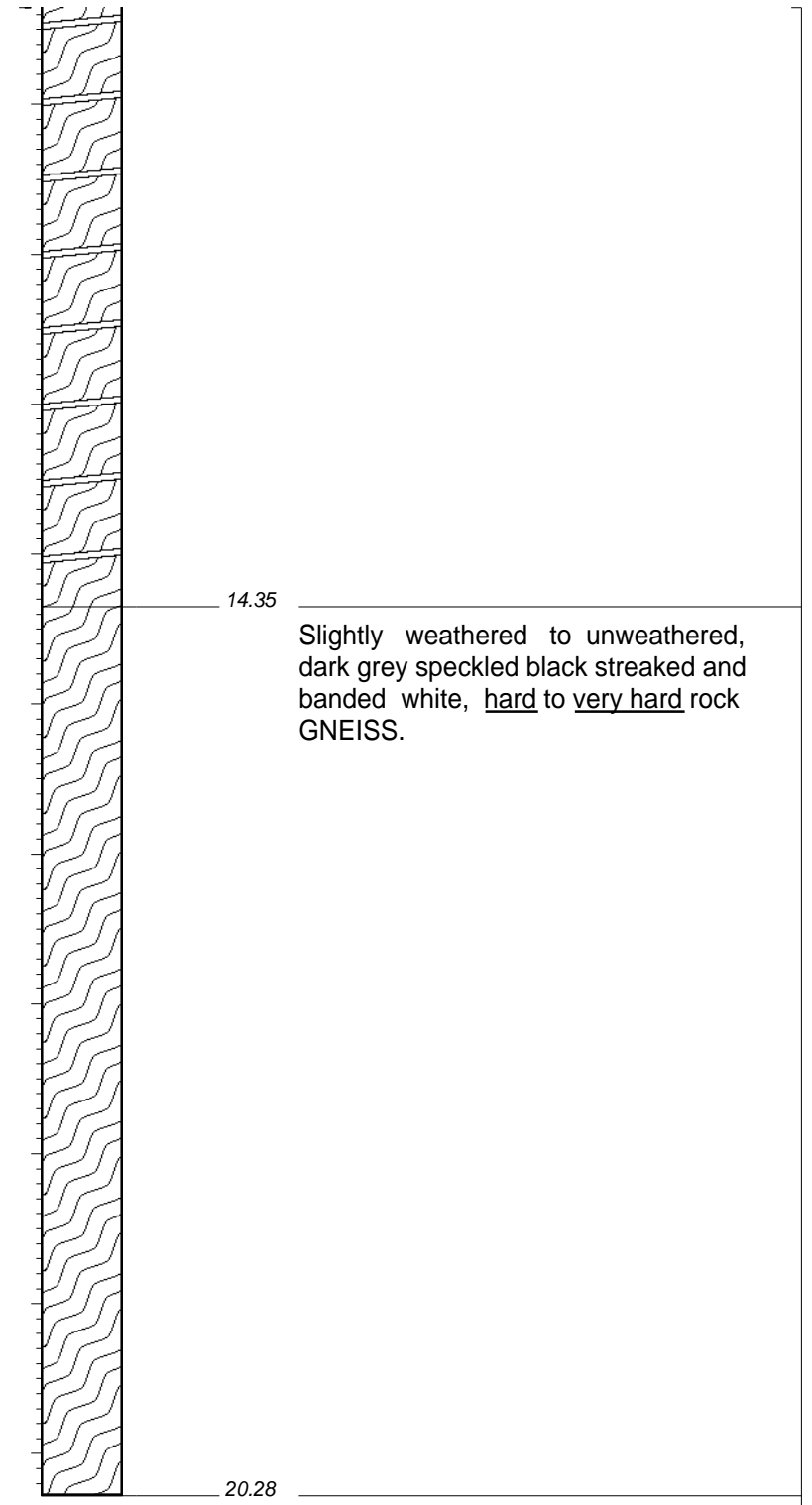
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WASTE ROCK DUMP

GEOTECHNICAL INVESTIGATION

HOLE No: WRD-BH01
Sheet 2 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Roughness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50
MG	GF	VIF-IF	10-20 10-20	1 2	0-10 20	VCJ-CJ VCJ-CJ	RJ RJ	Sand Sand	1-2 2-3	12.25	99	99	80	7	11
										13.75	94	94	63	11	12
										15.25	100	92	53	9	13
										16.75	97	97	84	5	14
MG	GF	VIF-IF	10-20 10-20	1 2	0-10 30	CJ-MJ CJ-MJ	RJ RJ	Clean Stained	- <1	18.25	100	100	97	5	15
										19.75	100	100	85	6	16
										20.28	91	91	91	0	17
															18
															19
															20



APPENDIX D

Photographs of Test Pits and Borehole Cores

Appendix D1

Test Pit Photographs

Appendix D2

Boreholes Core Photographs

APPENDIX D1

Test Pit Photographs

STORMWATER DIVERSION CHANNEL



Plate 1: Typical soil profile at SWD-TP04.



Plate 2: Shallow excavator refusal at SWD-TP08 on ferruginized alluvium of soft rock strength.



Plate 3: Northerly view of excavations underway at SWD-TP11.



Plate 4: Zoomed in view of ferruginized soils at SWD-TP11.



Plate 5: Shallow refusals encountered on soft rock gneiss at SWD-TP25.



Plate 6: Rehabilitation of SWD-TP26 within low-lying clay pans.

HEAP LEACH PAD



Plate 7: Refusal at HLP-TP06 on very dense residual gneiss.



Plate 8: Matrix supported pebble marker with ferricrete nodules at HLP-TP09.

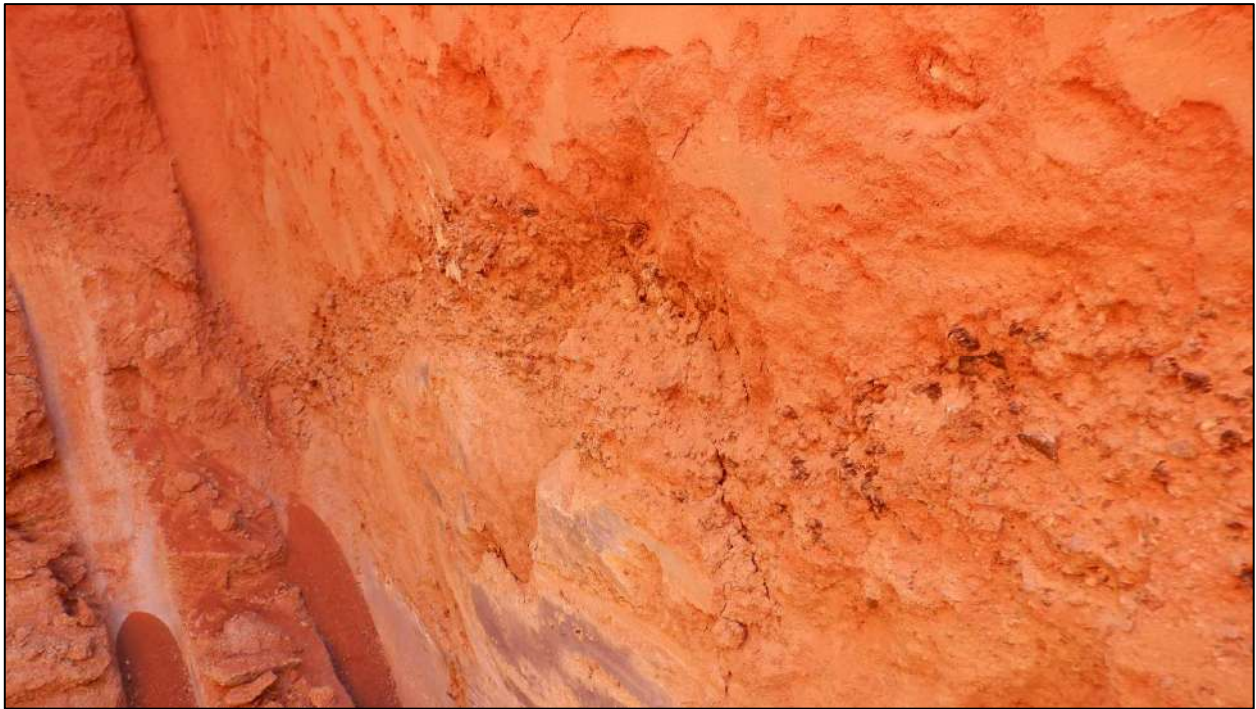


Plate 9: Thin undulating pebble marker observed at HLP-TP13.



Plate 10: Typical soil profile at HLP-TP24.



Plate 11: Typical soil profile at HLP-TP26.



Plate 12: Shallow excavator refusal on ferruginized alluvium at HLP-TP28.

WASTE ROCK DUMP (WRD)



Plate 13: View of soil profile and shallow water table at WRD-TP01.



Plate 14: Macro-instability of test pit sidewalls at WRD-TP01.



Plate 15: Typical pedogenic profile with refusal on medium hard rock gneiss.



Plate 16: Shallow refusal on soft rock gneiss WRD-TP20.



Plate 17: Presence of folding at WRD-TP20.



Plate 18: Shallow refusal on hardpan calcrete of soft rock strength at WRD-TP23.



Plate 19: Typical soil profile at WRD-TP29.



Plate 20: WRD-TP34 exposing well developed nodular calcrete horizon.



Plate 21: Zoomed in view of well-developed calcrete nodules of soft rock strength.



Plate 22: Ongoing sampling activities at WRD-TP34.

NEW ROAD



Plate 23: Existing farm roads in the vicinity of RD-TP01.



Plate 24: Fine alluvium at RD-TP01.



Plate 25: Rehabilitated RD-TP02.

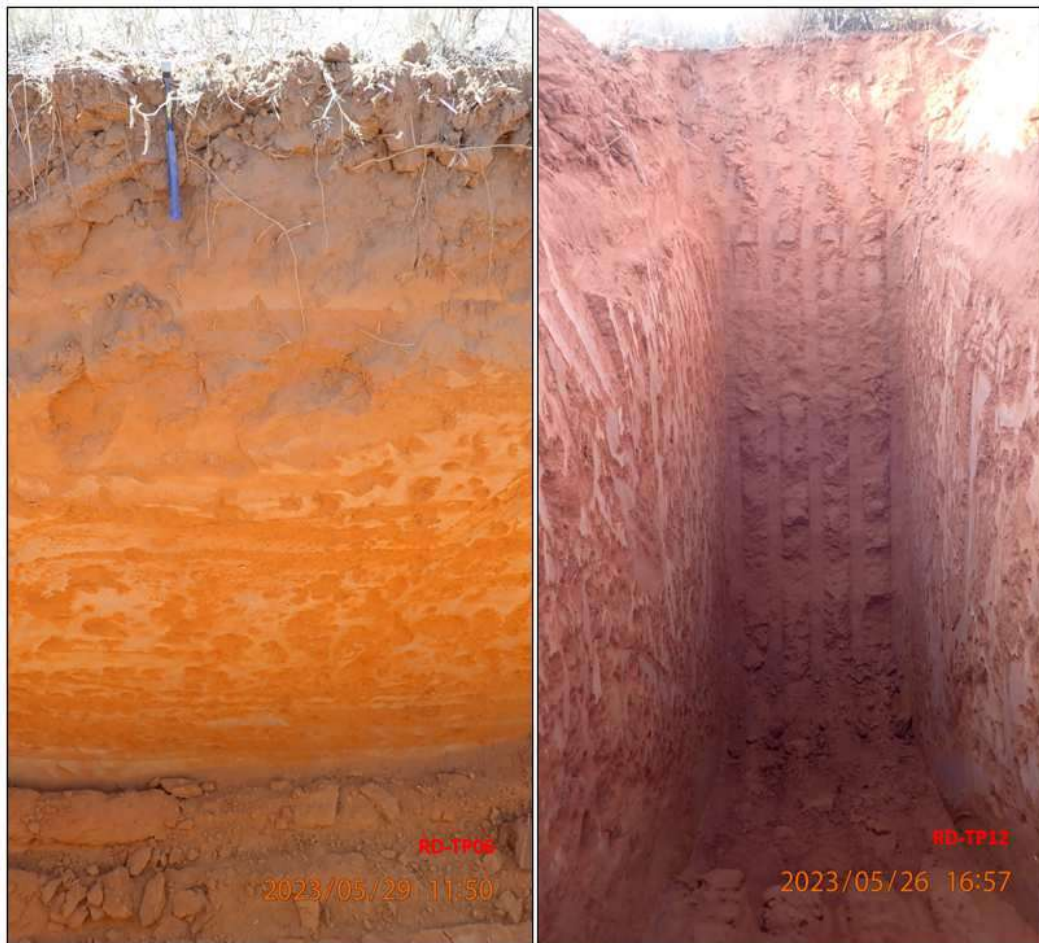


Plate 26: Typical soil profile at RD-TP06 and RD-TP12.



Plate 27: Typical soil profile at RD-TP16 and RD-TP20.

POTENTIAL BORROW SOURCES



Plate 28: Excavations underway at AM-TP01.



Plate 29: Steeply dipping gneiss outcrops.



Plate 30: Access roads observed within the vicinity of the identified borrow area.



Plate 30: Typical profile west of the potential borrow area.



Plate 31: Clayey silty sand material recovered from AM-TP06B.



Plate 32: Typical fine alluvial soils overlying ferruginous alluvium.

APPENDIX D2

Borehole Core Photographs

STORMWATER DIVERSION CHANNEL

BH01-SWD

0 – 24.27m



Omitomire Copper Project
Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Geotechnical Investigation Photographic Record



BH02-SWD

0 – 25.11m



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BH03-SWD

0 – 20.23m





BH04-SWD

0 – 21.03m



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BH05-SWD

0 – 20.01m



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Geotechnical Investigation Photographic Record



BH06-SWD

0 – 15.26m

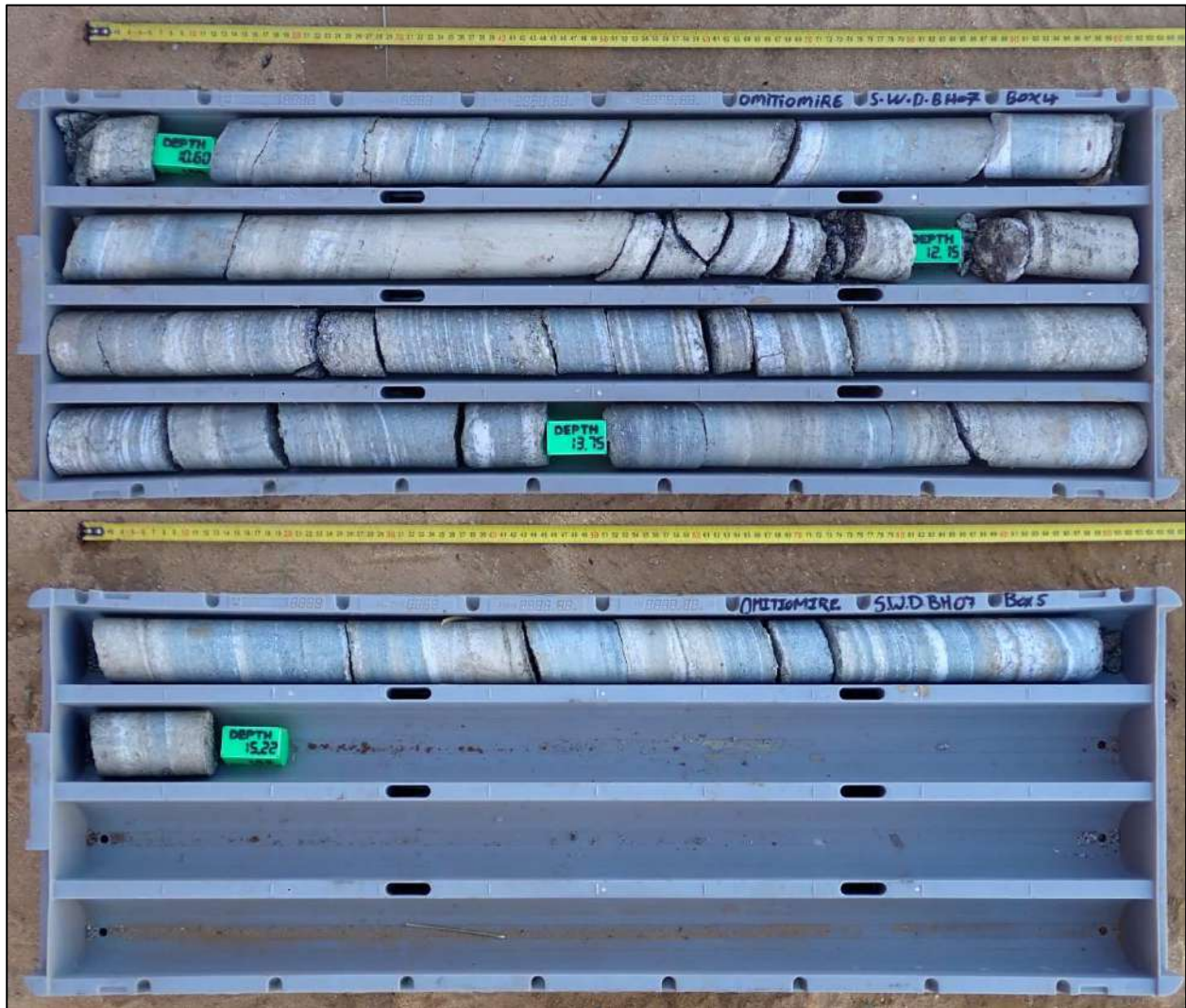




BH07-SWD

0m – 15.22m





BH08-SWD

0m – 20.01m





BH09-SWD

0 – 20.23m





BH10-SWD

0 – 20.09m





HEAP LEACH PAD

BH01-HLP

0 – 20.07m



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Geotechnical Investigation Photographic Record



BH02-HLP

0 – 20.08m





BH03-HLP

0 – 20.13m





BH04-HLP

0.- 20.23m





PROCESSING PLANT

BH01-Plant

0 – 25.05m





BH02-Plant

0 – 25.16m







BH03-Plant

0– 25.00m







BH04 – Plant

0 – 10.09m

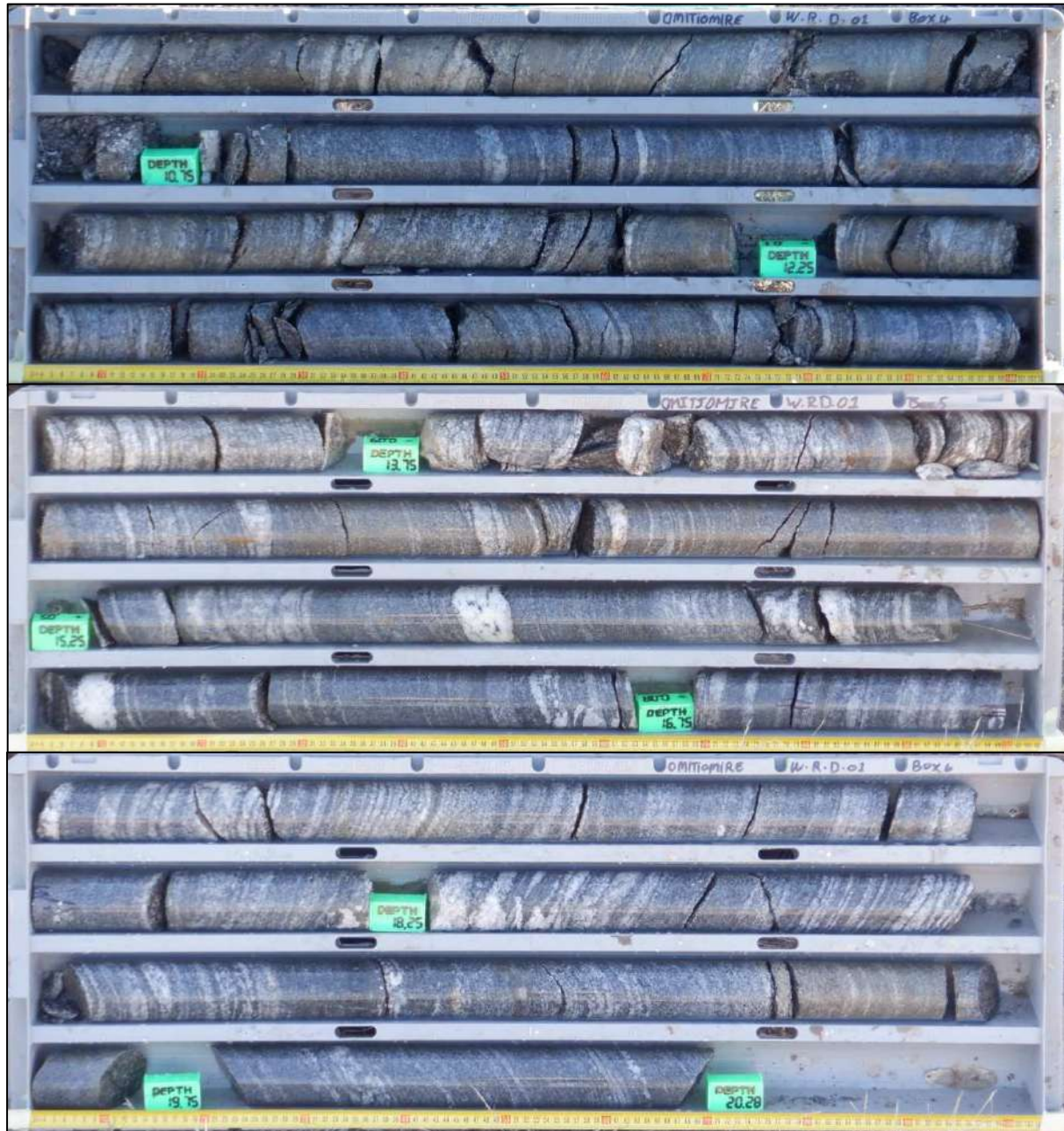


WASTE ROCK DUMP (WRD)

BH01-WRD

0 – 20.28m





APPENDIX E

Terminologies and Conventions used in Soil and Rock Logging

LIST OF BASIC SOIL DESCRIPTORS

CHARACTERISTIC COLOUR PATTERN / FEATURE

Term	Description
Speckled	Very small patches of colour, < 2mm.
Mottled	Irregular patches of colour, 2 - 6mm.
Blotched	Large irregular patches of colour, 6 - 20mm.
Banded	Approximately parallel bands of varying colour, describe thickness / spacing.
Streaked	Randomly oriented streaks of colour, describe thickness / spacing.
Stained	Local colour variations associated with discontinuity surfaces.

CONSISTENCY OF GRANULAR SOILS

Consistency	Description	Typical Dry Density (kg/m ³)	Saturated SPT Blow Counts, N
Very loose	Crumbles easily when scraped with geological pick.	< 1450	< 4
Loose	Small resistance to penetration by sharp end of geological pick.	1450 - 1600	4 - 10
Medium dense	Considerable resistance to penetration by sharp end of geological pick.	1600 - 1750	10 - 30
Dense	Very high resistance to penetration of sharp end of geological pick, requires many blows of pick for excavation.	1750 - 1925	30 - 50
Very dense	High resistance to repeated blows of geological pick, requires power tools for excavation.	> 1925	> 50

CONSISTENCY OF COHESIVE SOILS

Consistency	Description	UCS	Saturated SPT Blow Counts, N	
			Sensitive soils	Insensitive soils
Very soft	Pick head can easily be pushed in to the shaft of handle, easily moulded by fingers.	< 50 kPa	< 2	< 5
Soft	Easily penetrated by thumb, sharp end of pick can be pushed in 30 to 40mm, moulded with some pressure.	50 - 125 kPa	2 - 4	5 - 10
Firm	Indented by thumb with effort, sharp end of pick pushed in up to 10mm, very difficult to mould with fingers, can just be penetrated with ordinary hand spade.	125 - 250 kPa	4 - 8	10 - 25
Stiff	Penetrated by thumb nail, slight indentation produced by pushing pick point into soil, cannot be moulded by fingers, requires hand pick for excavation.	250 - 500 kPa	8 - 15	25 - 50
Very stiff	Indented by thumb nail with difficulty, slight indentation produced by blow of pick point, requires power tools for excavation.	0.5 - 1 MPa	15 - 20	50 - 80

STRUCTURE

Term	Description
Intact	Structureless, no structure identified.
Fissured	Soil contains discontinuities which may be open or closed, stained or unstained, and of variable origin.
Slickensided	This term qualifies other terms to describe discontinuity surfaces which are smooth or glossy and possible striated.
Shattered	Very closely to extremely closely spaced discontinuities resulting in gravel sized soil fragments which are usually stiff to very stiff and difficult to break down through moulding by fingers.
Micro-shattered	As above, but sand-sized fragments.
Stratified / laminated / foliated / etc	These and other accepted geological terms may be used to describe sedimentary structures in transposed soils as well as relict structures in residual soils.
Pinhole	Pinhole-sized voids or pores (up to say 2mm) which may require a hand lens to identify.
Honeycombed	Similar to pinhole but voids and pores > 2mm, pore size may be specified in mm.
Matrix-supported	Clasts supported by matrix.
Clast-supported	Clasts touching, matrix may or may not be present.

PARTICLE SIZE CLASSES (MIT CLASSIFICATION)

Grain size (mm)	Classification	Field Identification
< 0.002	Clay	Feels sticky or soapy, soils hands, shiny when wet.
0.002 - 0.06	Silt	Chalky feel on teeth, when dry rubs off hands, dilatent.
0.06 - 0.2	Fine sand	Gritty feel on teeth.
0.2 - 0.6	Medium sand	Observed with naked eye.
0.6 - 2	Coarse sand	Observed with naked eye.
2 - 6	Fine gravel	Observed with naked eye.
6 - 20	Medium gravel	Observed with naked eye.
20 - 60	Coarse gravel	Observed with naked eye.
60 - 200	Cobbles	Observed with naked eye.
> 200	Boulders	Observed with naked eye.

ORIGINS OF TRANSPORTED SOILS IN SOUTHERN AFRICA

Origin	Agency of Transportation	Problems that can be Expected
Littoral and mobile dune sands	Waves, current and tides	Collapsible fabric; instability of dredged marine deposit excavations; high soluble salt content; variable carbonate cementation.
Estuarine and deltaic	Tidal rivers depositing into saline water	Compressibility; variability; sensitivity; quick sand; high soluble salt content.
Talus (coarse colluvium)	Gravity	Slope instability.
Silty or clayey hillwash (fine colluvium)	Sheetwash	Expansive characteristics; compressibility; dispersive characteristics.
Aeolian deposits	Wind	Collapsible fabric; mobile (dunes); poor compaction characteristics.
Sandy soils of mixed origin	Sheetwash, wind, termites	Collapsible fabric; dispersive characteristics; compressibility, subject to flooding.
Alluvium	Streams	Expansive characteristics; dispersive characteristics; compressibility, subject to flooding.
Lacustrine	Streams depositing in lakes, pans or vleis	Compressibility; expansive characteristics; high soluble salt content.

LIST OF BASIC ROCK DESCRIPTORS

CHARACTERISTIC COLOUR PATTERN / FEATURE

Term	Description
Speckled	Very small patches of colour, < 2mm.
Mottled	Irregular patches of colour, 2 - 6mm.
Blotched	Large irregular patches of colour, 6 - 20mm.
Banded	Approximately parallel bands of varying colour, describe thickness / spacing.
Streaked	Randomly oriented streaks of colour, describe thickness / spacing.
Stained	Local colour variations associated with discontinuity surfaces.

GRAIN SIZE

Abbreviation	Description	Size (mm)	Explanatory notes
vFG	Very fine grained	< 0.2	Individual grains cannot be seen with a hand lens (x10 magnification).
FG	Fine grained	0.2 - 0.6	Grains not visible to the naked eye, visible as individual grains under hand lens.
MG	Medium grained	0.6 - 2.0	Grains clearly visible under hand lens, just visible to the naked eye.
CG	Coarse grained	2.0 - 6.0	Grains clearly visible to the naked eye.
vCG	Very coarse grained	> 6.0	Grains measurable.

FABRIC TYPE

Abbreviation	Description
BAF	Banded
BF	Bedded
CF	Cleaved
FBF	Flow banded
FF	Foliated
GF	Gneissic
LF	Laminated
MF	Massive
SF	Schistose

FABRIC SPACING

Abbreviation	Description	Spacing
VTF	Very thickly	> 2m
TF	Thickly	0.6 - 2m
MEF	Medium	0.2 - 0.6m
HF	Thinly	60mm - 0.2m
VHF	Very thinly	20 - 60mm
IF	Intensely	6 - 20mm
VIF	Very intensely	< 6mm

DEGREE OF WEATHERING

Abbreviation	Description	Extent of Intact Rock Discoloration	Fracture Condition	Fracture Surface Characteristics	Original Texture	Grain Boundary Condition
UW	Unweathered / fresh	None	Closed or discoloured	Unchanged or may be stained	Preserved	Tight
SW	Slightly weathered	< 20% of fracture spacing on both sides of fracture	May contain thin filling of altered material	Stained or partial discoloration	Preserved	Tight
MW	Moderately weathered	> 20% of fracture spacing on both sides of fracture	May contain filling of altered material	Partial to complete discoloration, friable except in poorly cemented rocks	Preserved	Partial opening
HW	Highly weathered	Throughout	-	Friable and usually pitted	Mainly preserved	Partial separation
CW	Completely weathered	Throughout	-	Resembles a soil	Partly preserved	Complete separation

ROCK HARDNESS FIELD CLASSIFICATION

Abbreviation	Description	Range of UCS (MPa)	Explanatory notes
Soil	-	< 1	Soil description in terms of <u>M</u> oisture / <u>C</u> olour / <u>C</u> onsistency / <u>S</u> tructure / <u>S</u> oil Type / <u>O</u> ther Features / <u>W</u> ater.
ESR	Extremely soft	0.25 - 1.0	Indented by thumbnail (should also be described as a soil on log sheet).
VSR	Very soft	1 - 3	Crumbles under firm blows with sharp end of geological pick, can be pared/peeled with a knife.
SR	Soft	3 - 10	Shallow indentations of 2 to 4mm under firm blows with sharp end of geological pick, can just be scraped with a knife.
MHR	Medium hard	10 - 25	Handheld specimen breaks / fractures with firm blow of geological pick, specimen cannot be scraped or pared with a knife.
HR	Hard	25 - 70	Handheld specimen requires more than one blow with geological pick before breaking / fracturing.
VHR	Very hard	70 - 200	Specimen requires many blows with geological pick before breaking / fracturing.
EHR	Extremely hard	> 200	Specimen can only be chipped under firm blows of geological hammer.

JOINT SPACING

Abbreviation	Description for Joints, Faults or Other Fractures	Spacing
EWJ	Extremely widely	> 6m
VWJ	Very widely	> 2m / 2 - 6m
WJ	Widely	0.6 - 2m
MJ	Moderately	0.2 - 0.6m
CJ	Closely	60mm - 0.2m
VCJ	Very closely	20 - 60mm
ECJ	Extremely closely	< 20mm

JOINT MICRO-ROUGHNESS

Abbreviation	Description
SLJ	Slickensided
SJ	Smooth
RJ	Rough

JOINT MICRO-SHAPE

(i.t.o. surface expression over < 100mm length)

Abbreviation	Description
CUR	Curvilinear
PLA	Planar
UND	Undulating
STE	Stepped
IRR	Irregular

INFILL THICKNESS

Abbreviation	Description	Infill Thickness
CT	Closed	0mm
VT	Very tight	0 - 0.1mm
T	Tight	0.1 - 0.25mm
PO	Partly open	0.25 - 0.5mm
O	Open	0.5 - 2.5mm
MW	Moderately wide	2.5 - 10mm
W	Wide	10 - 25mm
VW	Very wide	25mm - 10cm
EW	Extremely wide	10cm - 1m
C	Cavernous	> 1m

INFILL TYPE

Abbreviation	Description	Explanatory notes
NF	None	
STF	Staining only	
<i>Non-softening sheared material, e.g. calcite</i>		
NFF	Fine	
NMF	Medium	
NCF	Coarse	
<i>Soft sheared material, e.g. talc</i>		
SFF	Fine	
SMF	Medium	
SCF	Coarse	
<i>Gouge description (if)</i>		
TG	Thick gouge	Gouge thickness < amplitude
VTG	Very thick gouge	Gouge thickness > amplitude of irregularities

Notes :

If more than 25mm, give actual thickness.

If more than 25mm, append photo(s).

Unified Soil Classification System Descriptors (Hazelton & Murphy, 2007)

USCS Group	Group Descriptor
GW	Well-graded gravels, gravel-sand mixtures, little or no fines.
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines.
GM	Silty gravels, poorly graded gravel-sand-silt mixtures.
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures.
SW	Well-graded sands, gravelly sands, little or no fines.
SP	Poorly graded sands, gravelly sands, little or no fines.
SM	Silty sands, poorly graded sand-silt mixtures.
SC	Clayey sands, poorly graded sand-clay mixtures.
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity.
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
OL	Organic silts and organic silt-clays of low plasticity.
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.
CH	Inorganic clays of high plasticity, fat clays.
OH	Organic clays of medium to high plasticity.
PT	Peat and other highly organic soils.

AASHTO Classification System Descriptors (ASTM D3282-15, 2019)

AASH TO Group	General Material Classification	% passing 2.00 mm	% passing 0.425 mm	% passing 0.075 mm	Liquid Limit (LL)	Plasticity Index (PI)	Significant Constituents	Rating as Subgrade
A-1-a	Granular	≤ 50%	≤ 30%	≤ 15%	-	≤ 6	Stone fragments, gravel and sand	Excellent to good
A-1-b	Granular	-	≤ 50%	≤ 25%	-	≤ 6	Stone fragments, gravel and sand	Excellent to good
A-3	Granular	-	≥ 51%	≤ 10%	-	N.P.	Fine sand	Excellent to good
A-2-4	Granular	-	-	≤ 35%	≤ 40	≤ 10	Silty or clayey gravel and sand	Excellent to good
A-2-5	Granular	-	-	≤ 35%	≥ 41	≤ 10	Silty or clayey gravel and sand	Excellent to good
A-2-6	Granular	-	-	≤ 35%	≤ 40	≥ 11	Silty or clayey gravel and sand	Excellent to good
A-2-7	Granular	-	-	≤ 35%	≥ 41	≥ 11	Silty or clayey gravel and sand	Excellent to good
A-4	Silt-Clay	-	-	≥ 36%	≤ 40	≤ 10	Silty soils	Fair to poor
A-5	Silt-Clay	-	-	≥ 36%	≥ 41	≤ 10	Silty soils	Fair to poor
A-6	Silt-Clay	-	-	≥ 36%	≤ 40	≥ 11	Clayey soils	Fair to poor
A-7-5	Silt-Clay	-	-	≥ 36%	≥ 41	≥ 11	Clayey soils	Fair to poor
A-7-6	Silt-Clay	-	-	≥ 36%	≥ 41	≥ 11	Clayey soils	Fair to poor

Notes:

1. PI of subgroup A-7-5 is less or equal than the LL – 30.
2. PI of subgroup A-7-6 is greater than the LL – 30.
3. N.P. – Non-plastic.

Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX F

Geophysical Testing Report

GREGORY SYMONS GEOPHYSICS

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Tel +264-(0)61-225292

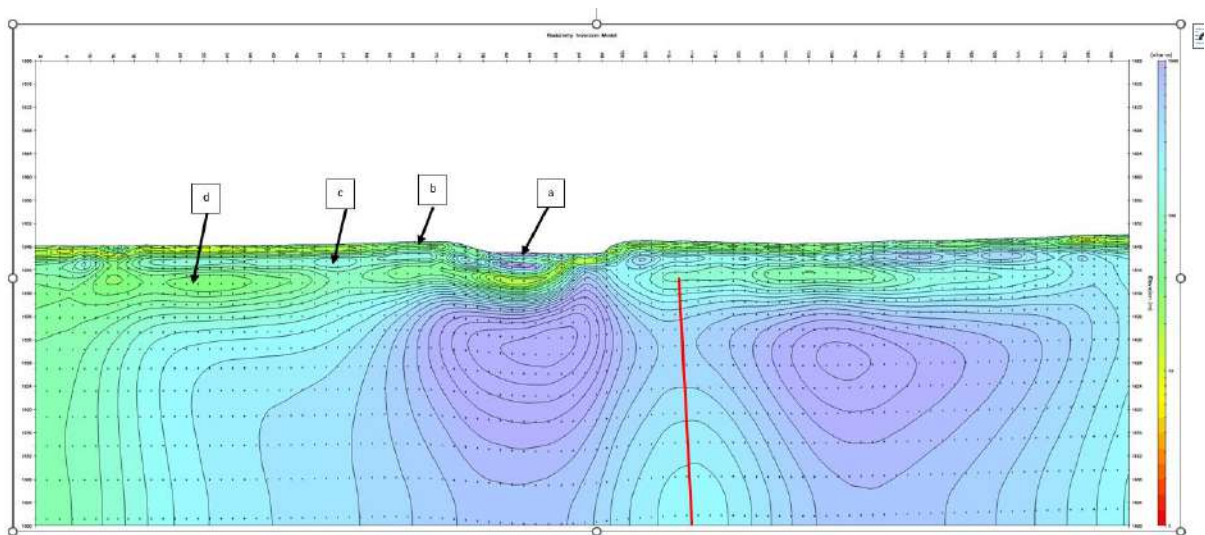
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Field and Interpretational Report: Electrical Resistivity Tomography Survey undertaken for Knight Piésold Consulting.



Electrical Resistivity Tomography (ERT) results from Line 9 at Omitiomire. The overburden can be divided into a, a sandy horizon in channel, b, a clayey horizon in floodplain and c, a sandy horizon in floodplain. Horizon d is interpreted as weathered basement. Vertical faulting is marked by the red line which appears to be underneath the horizontal Kalahari overburden layers within the basement.

Report Prepared for Knight Piésold Consulting.

Report Prepared by: D-J Adams and G Symons

Gregory Symons Geophysics

March 2023

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Appendix 1: Electrical Resistivity Tomography 2D Inversion Sections.

Appendix 2: Data Archive

1. Introduction

This report summarizes details of the Electrical Resistivity Tomography (ERT) survey conducted for Knight Piésold Consulting by Gregory Symons Geophysics (GSG) at Omitiomire, northwest of Gobabis, in Omaheke region, Namibia.

The client requested 13 lines for the survey, which we completed over 14 days. Using the above-mentioned geophysical method, we surveyed the 13 lines as indicated on the location map (Figure 1) below. The survey started on the 3rd of March 2023, and we completed it on the 14th of March, 2023. In the report folder with this report is a comprehensive time sheet in Excel.

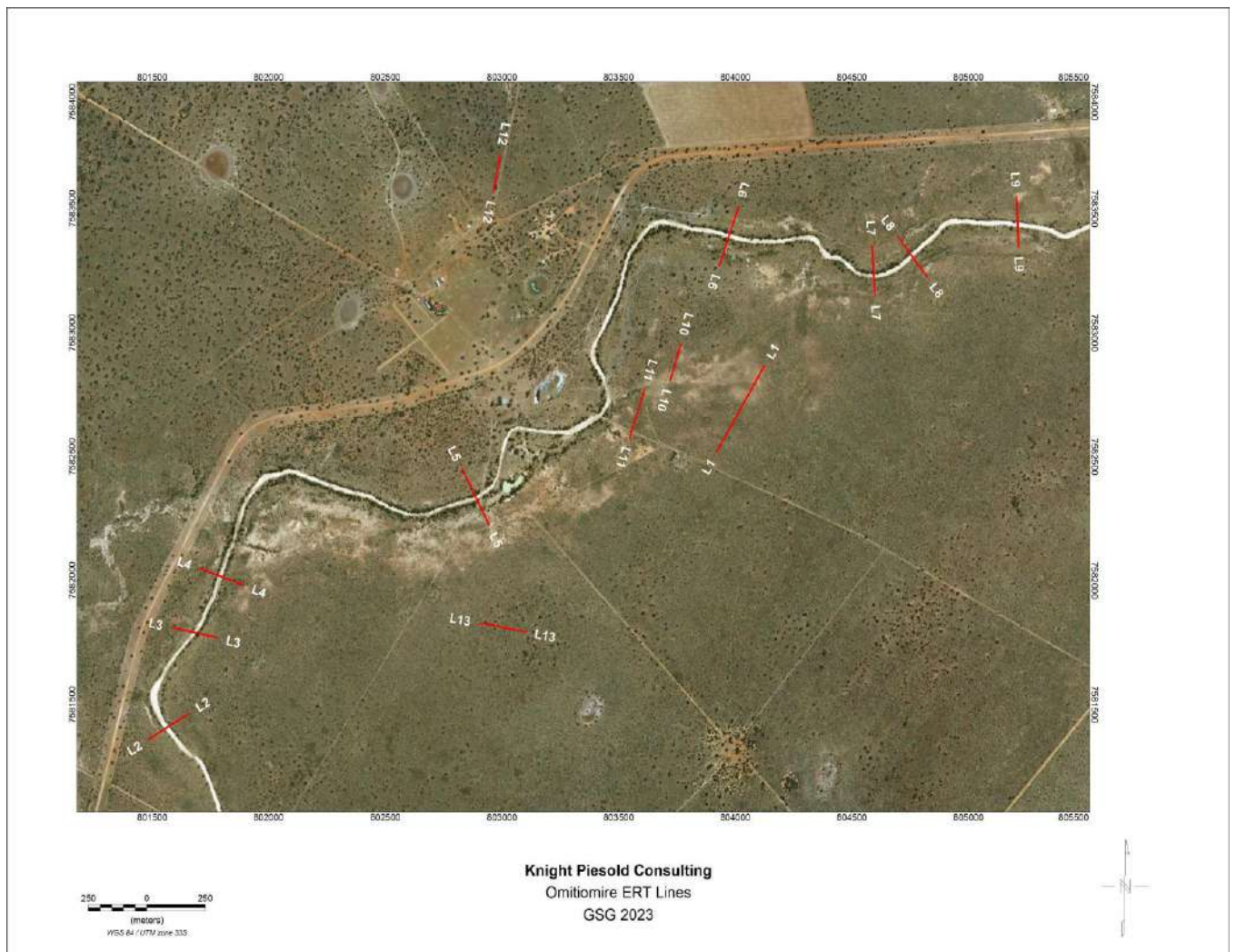


Figure 1: A map showing the thirteen ERT lines surveyed at Omitiomire.

2. Electrical Resistivity Tomography Method

In the Electrical Resistivity Tomography (ERT) method, we inject a current (AB) into the ground and measure a voltage (MN). From this, we can derive a resistivity at a certain point below the surface. In Figure 2 below we show the relationship between AB and MN and the plotting point for the resistivity below the surface.

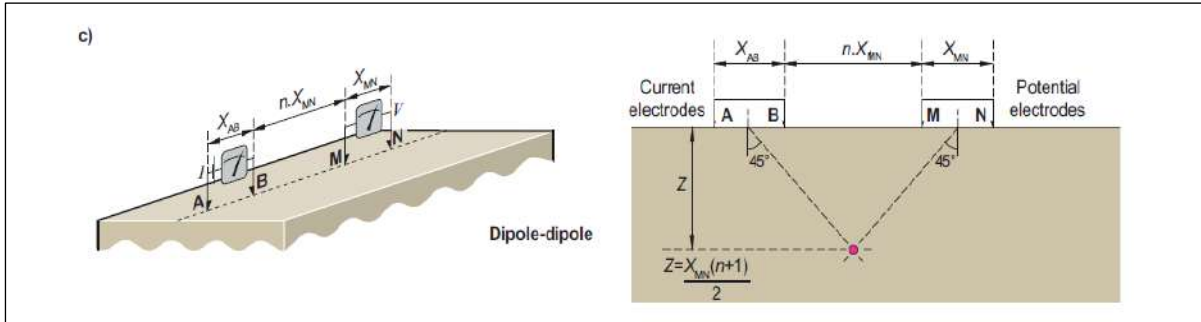


Figure 2: Diagram showing AB injection electrode, MN measurement electrodes and the plotting point.

By arranging 96 electrodes spaced regularly along a line, we can select different combinations of AB and MN to generate numerous subsurface plotting points, as indicated in Figure 3 below. In this example, we have the 96 electrodes spaced 5 m apart, generating 1031 subsurface resistivity measurements.

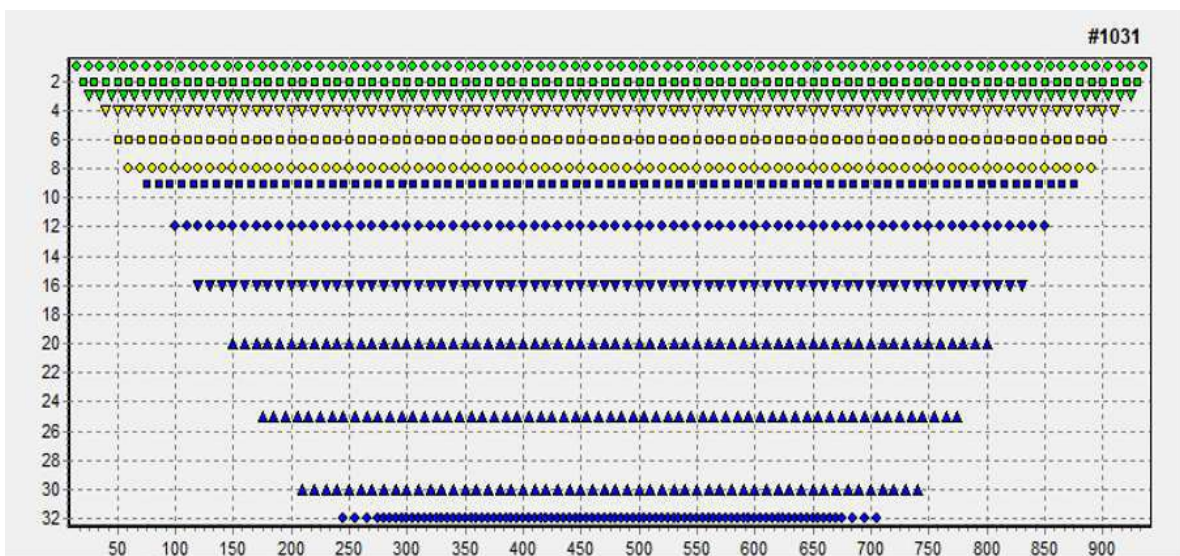


Figure 3: Diagram showing the multiplicity of ERT resistivity measurements below a 1km (960m) line section using 96 electrodes and 5 m station spacing.

Traditional dipole-dipole measurements would typically yield only about 200 subsurface measurements. With the ERT and the additional 5-fold increase in subsurface data, we can:

- Model overburden and shallow features well.
- Detect vertical features below overburden such as faults and fractures.

- Have a better depth of investigation than traditional dipole-dipole measurements due large AB/MN offsets.

3. Equipment and Personnel

3.1 Equipment

- DGPS Trimble R2 System,
- Syscal R2 resistivity system, comprising the R2 Syscal receiver, battery powered transmitter, multiplexer box and all related wires and electrodes.
- Two double cab bakkies and a land cruiser.

3.2 Personnel

- David-John Adams, Phillip Nauyoma, Hilton Stern, and Cecil Murangi with field assistants for data acquisition,
- Gregory Symons, Neville Brown, and David-John Adams (Geophysicists) for data processing, reporting, client liaison and QC.

4. Surveys Specifications, Data Reduction and Processing

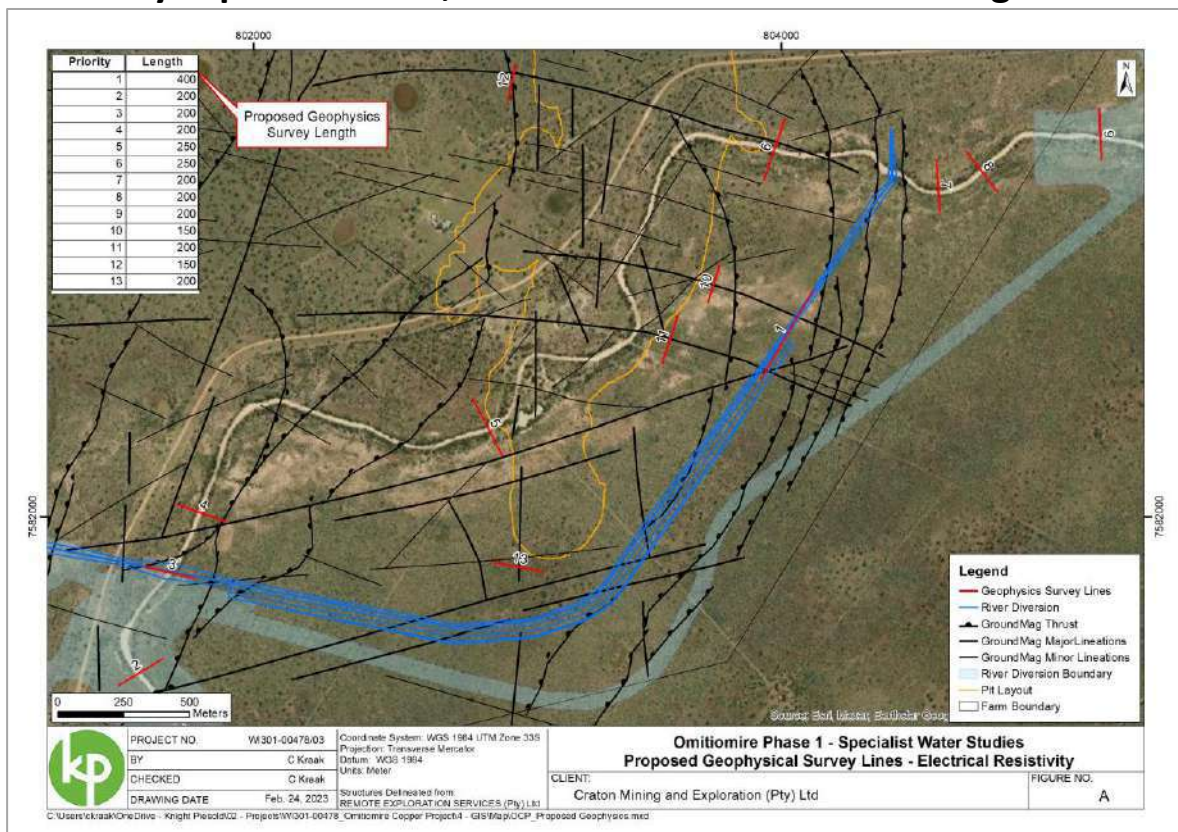


Figure 4: Map Showing position of Lines relative to pit outline, faulting derived from Aeromagnetic Interpretation, and position of current and possible future channel/s to the river.

The survey was collected using the following parameters:

- Station Spacings: 2m on all lines except for Lines 5 and 6 where 3 m was used. We also collected repeat data on Line 1 at a 4 m dipole spacing.
- Line lengths: 400, 250, 200, 150 m.
- Number of Electrodes: 96 electrodes.
- Number of Multi-node boxes: 6 of which each had 16 electrodes connected to it and one connector cable that connects each box to the resistivity meter.
- The data was processed with the Iris Software (ProsysII) to examine the pseudo section in the field and to convert the data in a suitable format (*.dat).
- The data was then imported to software provided by ZONGE where invalid data was pruned, DGPS data coordinates and elevations were merged with the resistivity data, and a resistivity depth inversion section produced. In Appendix 1, the resistivity depth inversions are presented with the measured and calculated pseudo-sections.

5. Interpretation

The inverted resistivity depth section shows the following features:

- A maximum of 3 overburden layers are observed. This stratigraphy is defined as (a) a top sandy resistive layer, followed by (b), a conductive clay bearing layer and (c) a deeper resistive sandy layer.
- Below this overburden we consistently appear to find weathered conductive basement which changes at depth to resistive fresh rock.
- In places we see vertical to sub-vertical conductive features in the weathered and fresh basement indicative of vertical faulting. The vertical faulting appears to not penetrate the overburden. Clear faulting is indicated in red while more tentative faulting is indicated in black.
- Figure 4 above shows a structural interpretation of the area based on an aeromagnetic interpretation. Thrust faults are purported to be bringing in the Cu mineralization and are typically “old” (late Damaran 480-500 My). We would not expect to see these faults in the resistivity data and in general this is the case with the current resistivity data set. Vertical faulting in a N/S and E/W direction is related to Cretaceous and late Karroo tectonics which appears to be detected by the resistivity as conductive water bearing faults and fractures. Overburden is Kalahari aged with no or little evidence of faulting. The resistive layers may be related to windblown Kalahari. The conductive overburden layer appears to be associated with the Nossob River and could be related to flood events where the Nossob River has overflowed its banks and deposited more clay like lithologies on the flood plain.

In Figures 5 to 17 below, the above interpretation scheme is implemented onto each of the resistivity lines.

Interpretation of Individual Lines

Resistivity Depth Sections are presented below. The following features are highlighted:

- Overburden stratigraphy as defined into units belonging to the Kalahari a) sandy, b) clayey, c) sandy.
- Weathered basement d).
- Faulting divided into clear faulting (red) and tentative faulting (black).

Line1, spacing = 4m, length = 400m

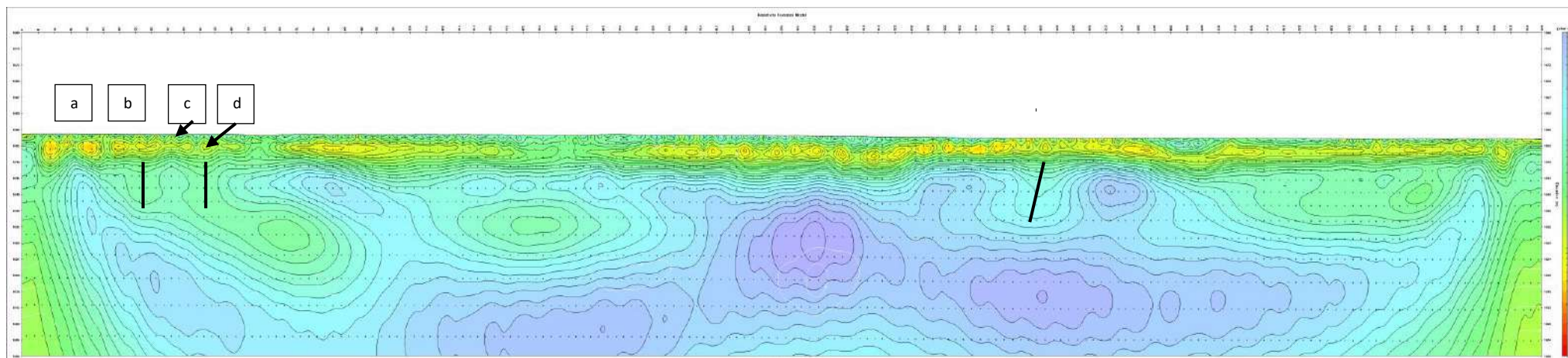


Figure 5: Showing

Faulting at stations: 32 (1), 48 (1), and 256 (1)

Weathered and Overburden units (d) and (c) present.

Line2, spacing = 2m, length = 200m

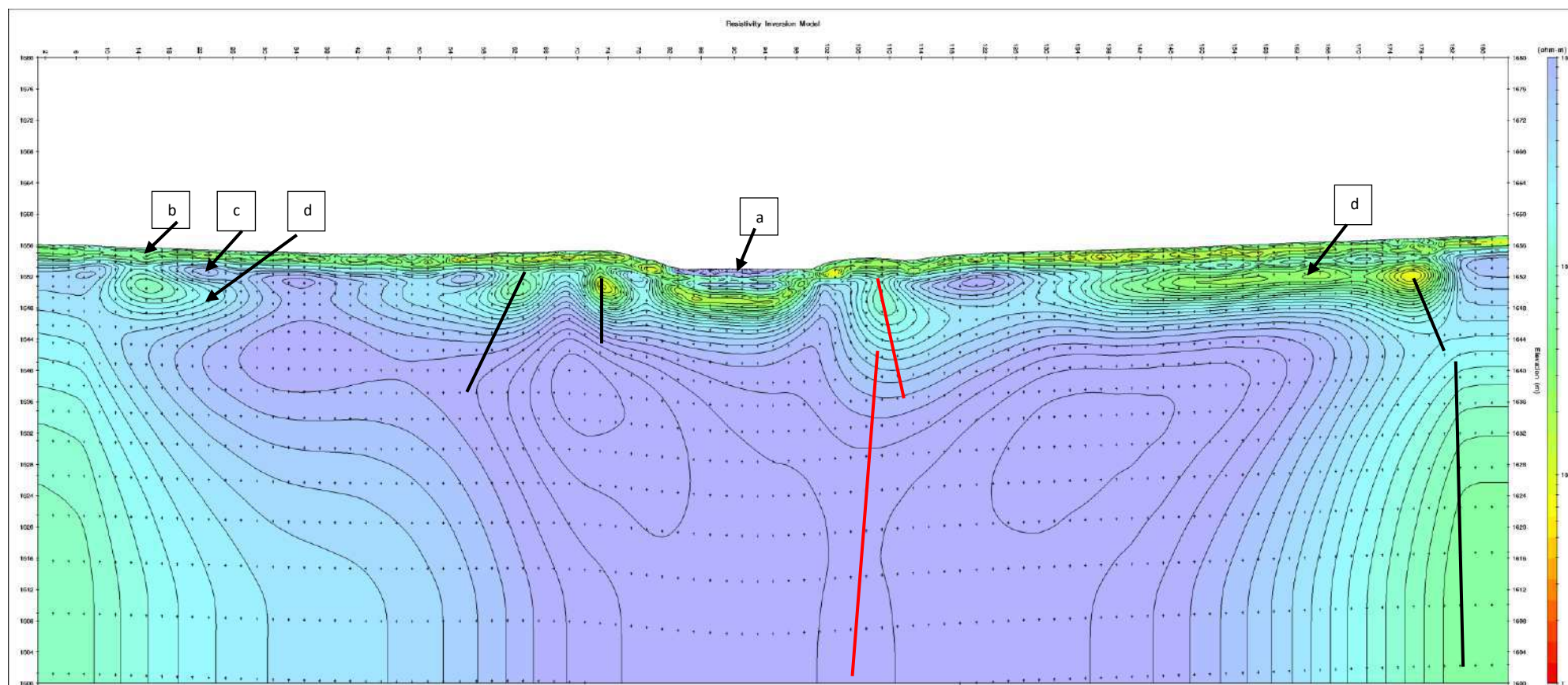


Figure 6: Showing

Faulting at stations: 62(1), 72(1), 110(2), 178(1)

Weathered and Overburden units (d), (c), (b) and (a) present.

Line3, spacing = 2m, length = 200m

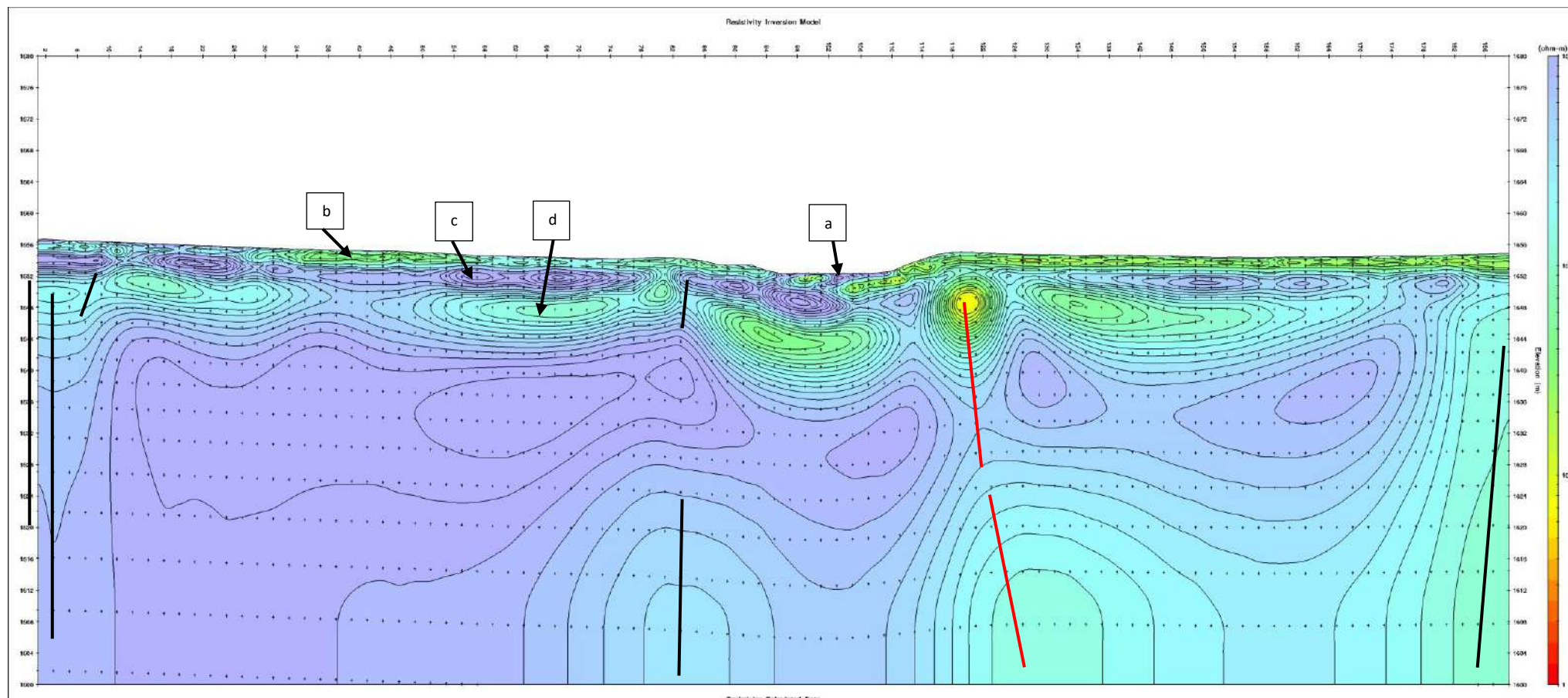


Figure 7: Showing
Faulting at stations: 6(1), 82(1), 120(2), 188(1)
Weathered and Overburden units (d), (c), (b) and (a) present.

Line4, spacing = 2m, length = 200m

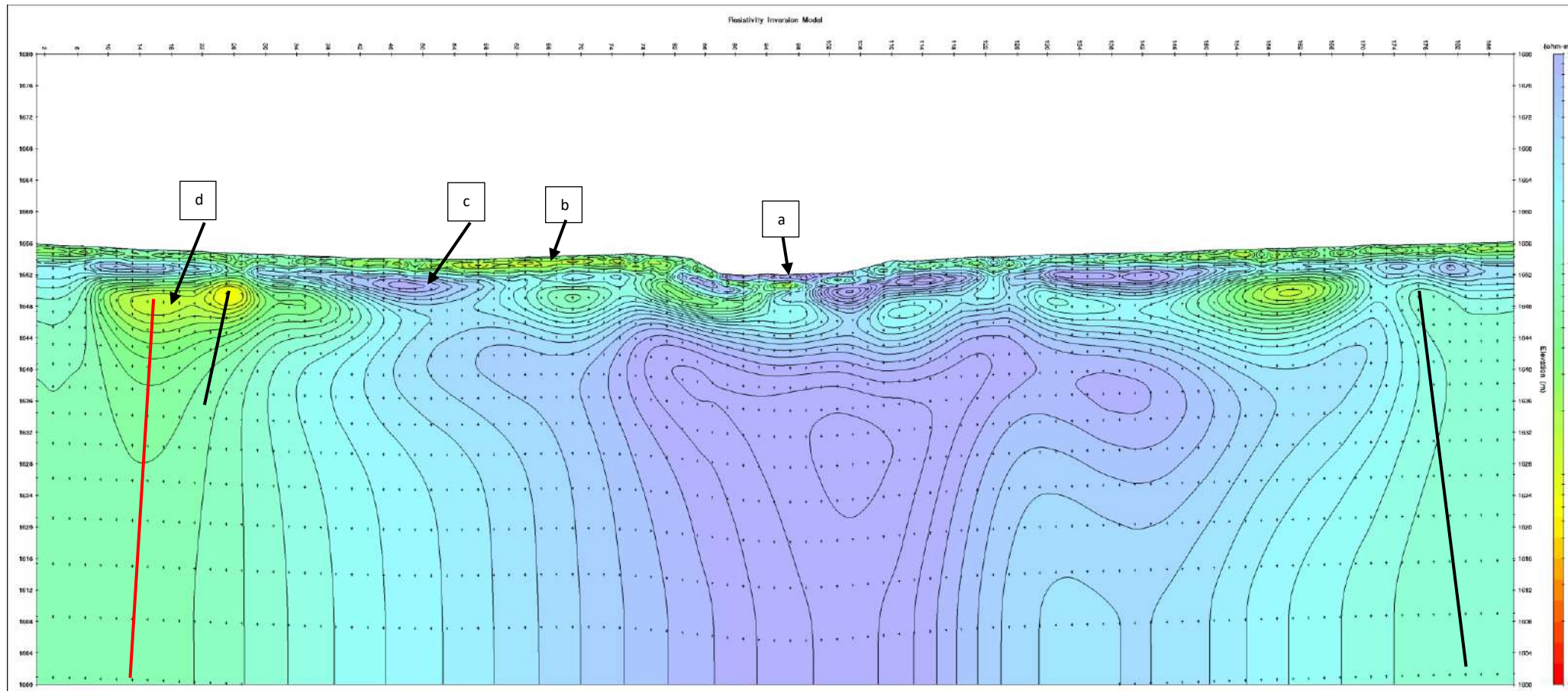


Figure 8: Showing

Faulting at stations: 14(2), 22(1), 178(1)

Weathered and Overburden units (d), (c), (b) and (a) present.

Line5, spacing = 3m, length = 250m

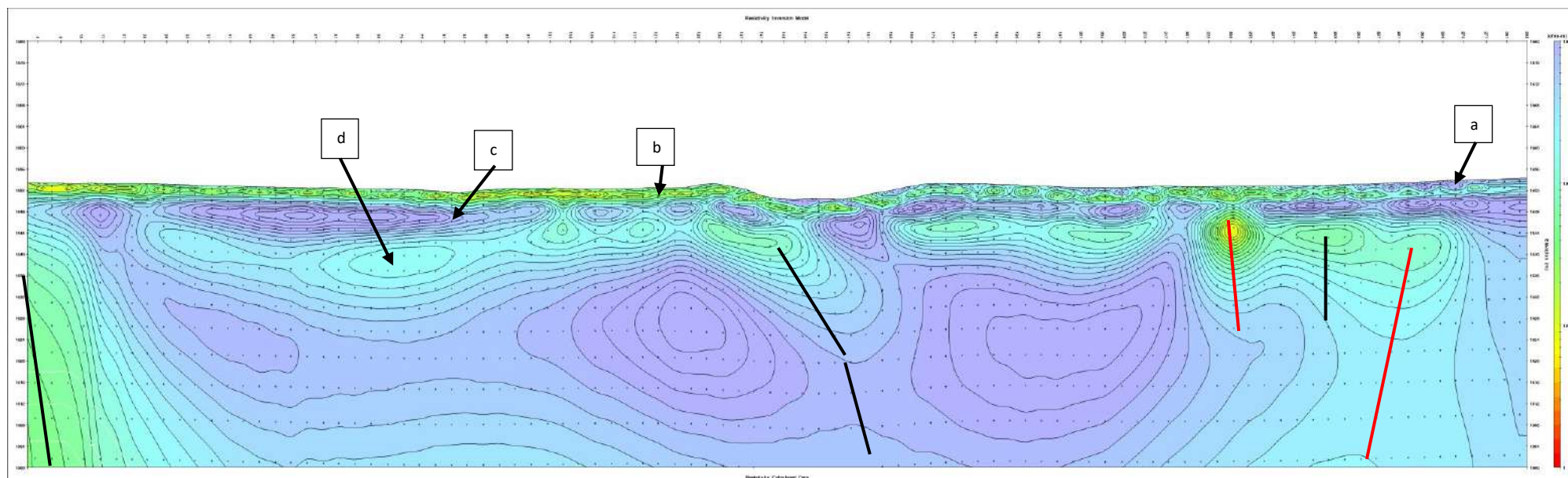


Figure 9: Showing

Faulting at stations: 5(1), 143(1), 229(2), 247(1), 262(2)

Weathered and Overburden units (d), (c), (b) and (a) present.

Line6, spacing = 3m, length = 250m

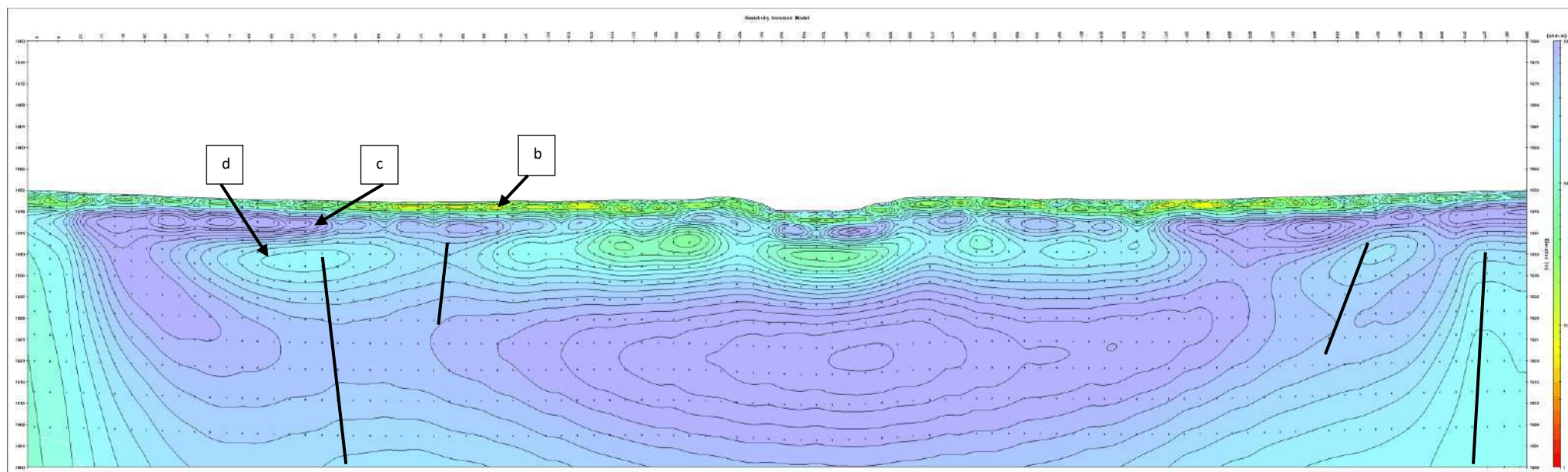


Figure 10: Showing

Faulting at stations: 57(1), 81(1), 253(1), 277(1)

Weathered and Overburden units (d), (c), and (b) present.

Line7, spacing = 2m, length = 200m

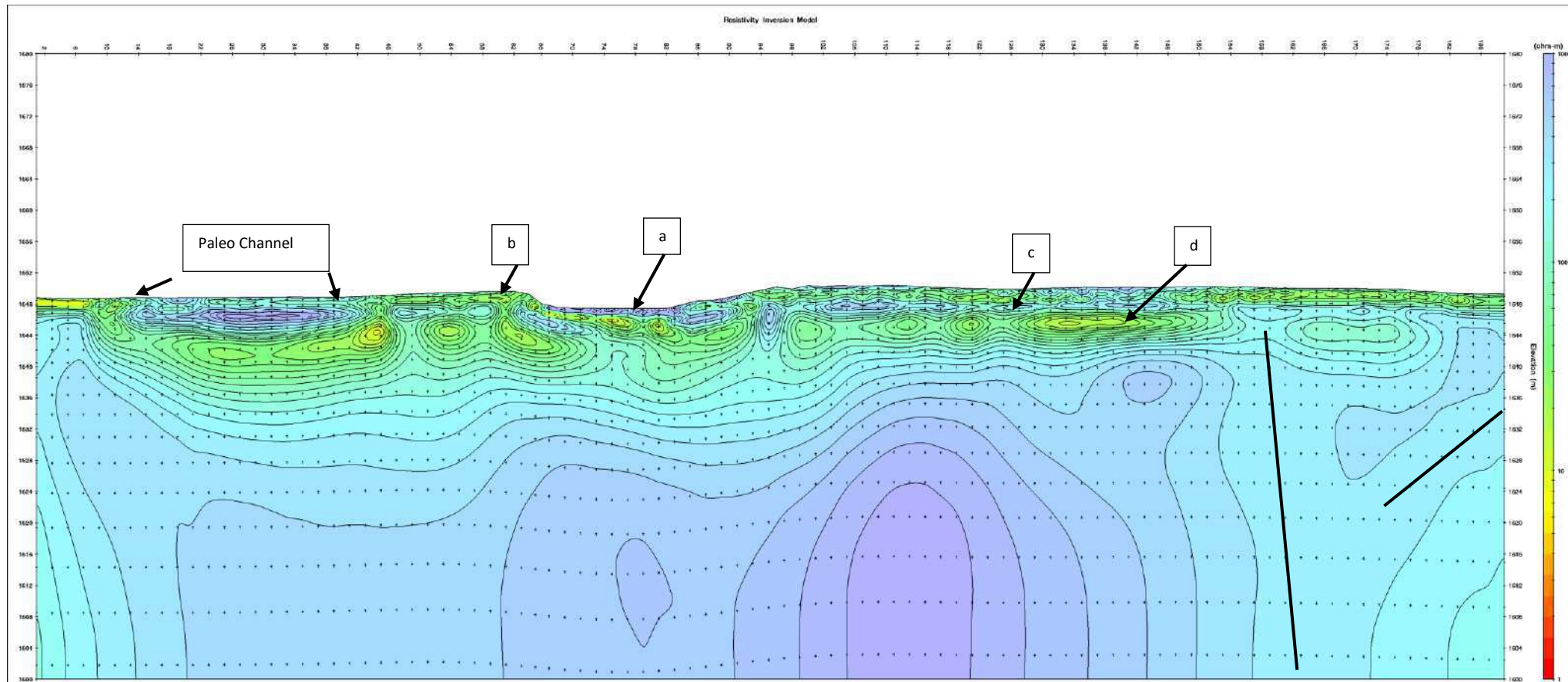


Figure 11: Showing
Faulting at stations: 157 (1)
Weathered and Overburden units (d), (c), (b) and (a) present.

Line8, spacing = 2m, length = 200m

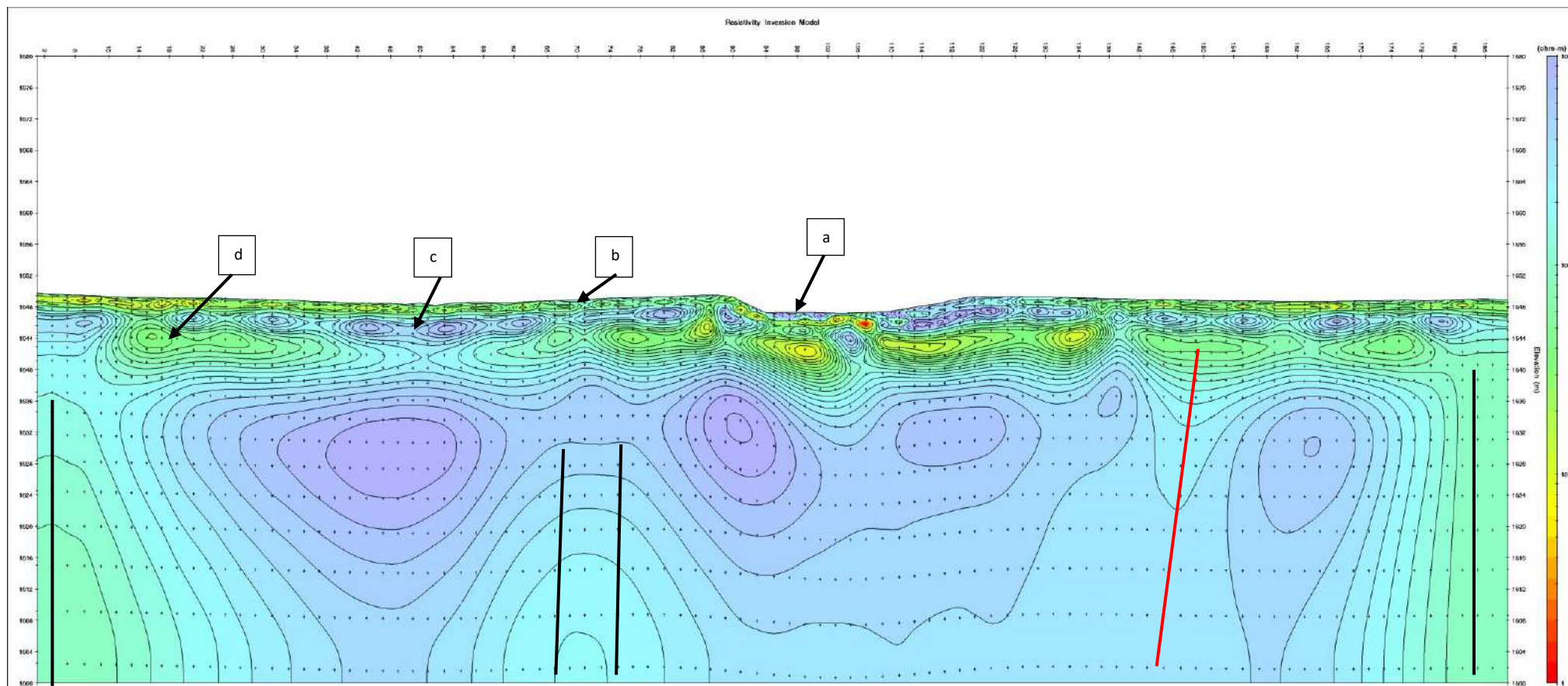


Figure 12: Showing

Faulting at stations: 3 (1), 68 (1), 74 (1), 148 (2), 184(1)

Weathered and Overburden units (d), (c), (b) and (a) present.

Line9, spacing = 2m, length = 200m

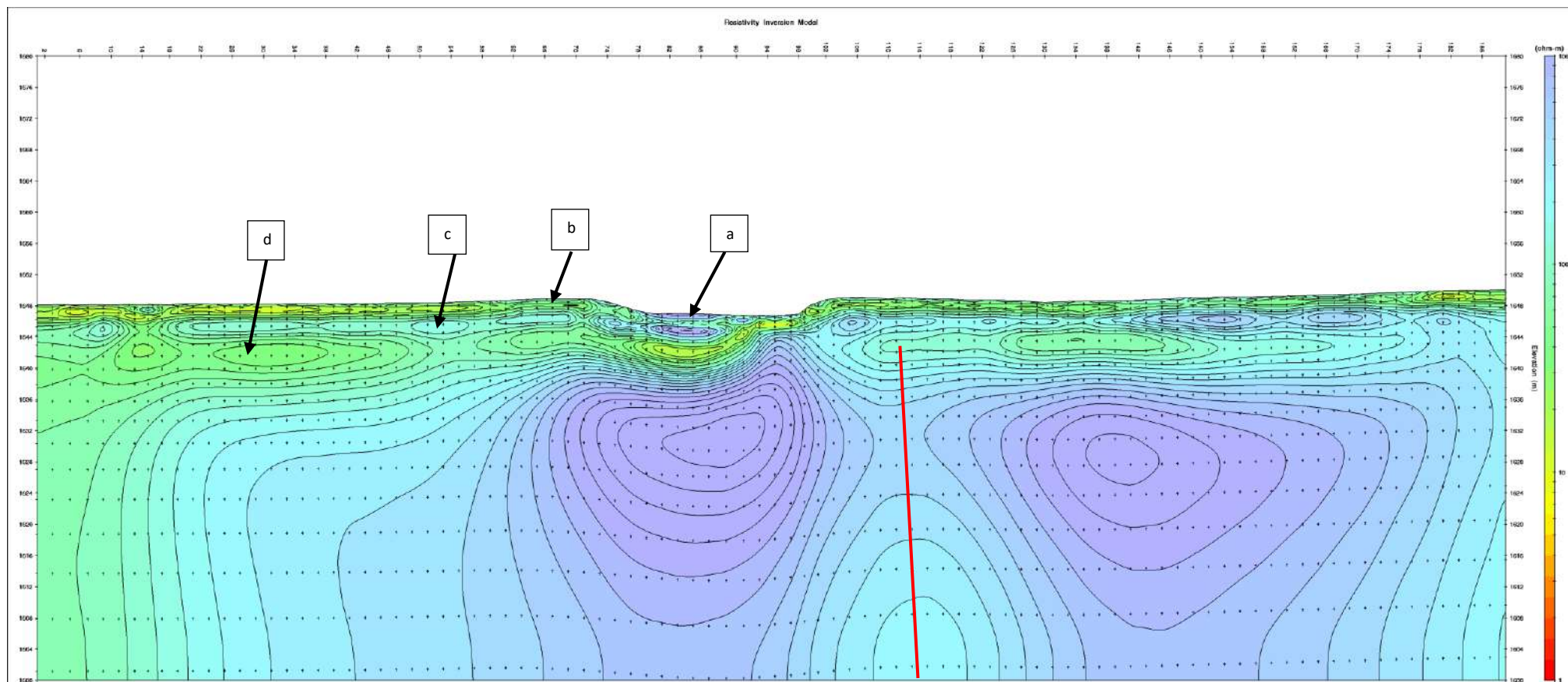


Figure 13: Showing
Faulting at stations: 110(2)
Weathered and Overburden units (d), (c), (b) and (a) present.

Line10, spacing = 2m, length = 150m

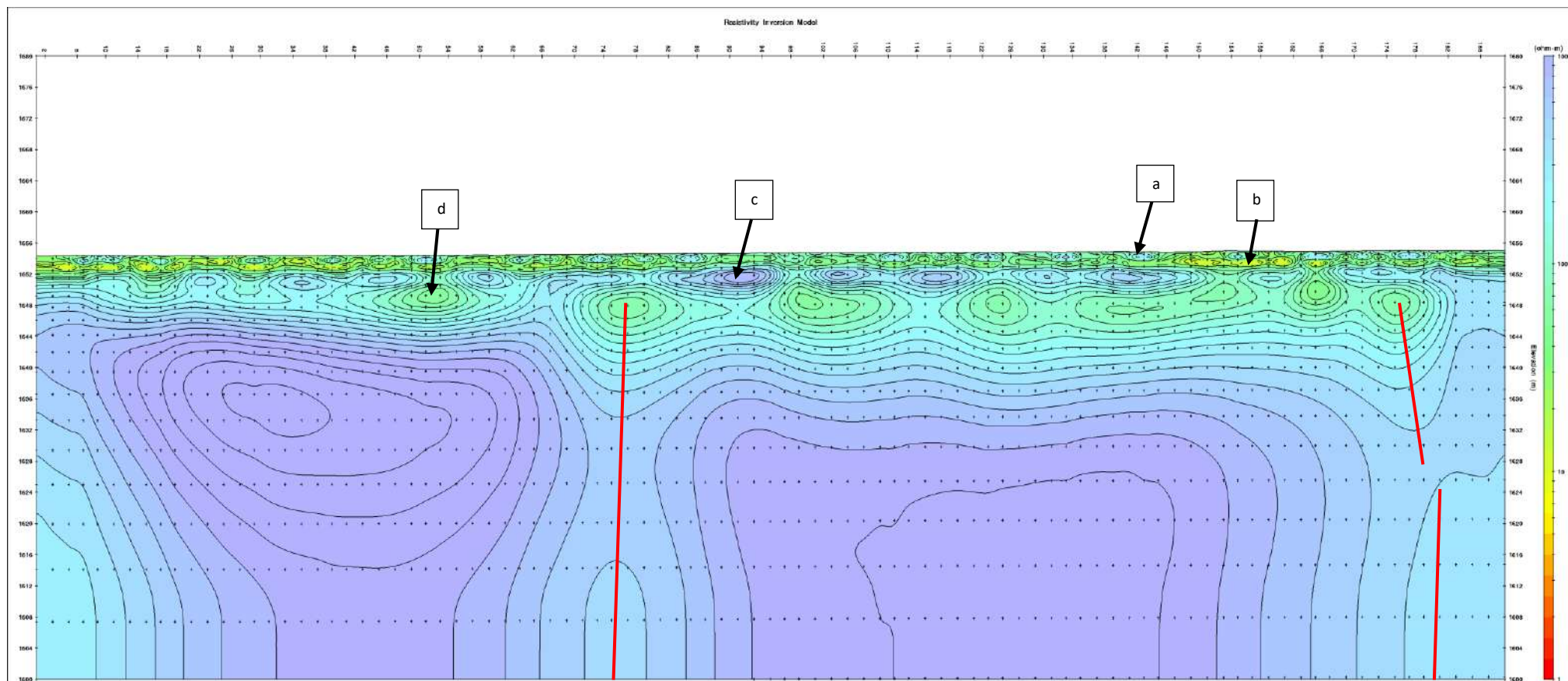


Figure 14: Showing

Faulting at stations: 77(2), 176(2)

Weathered and Overburden units (d), (c), and (a) present.

Line11, spacing = 2m, length = 200m

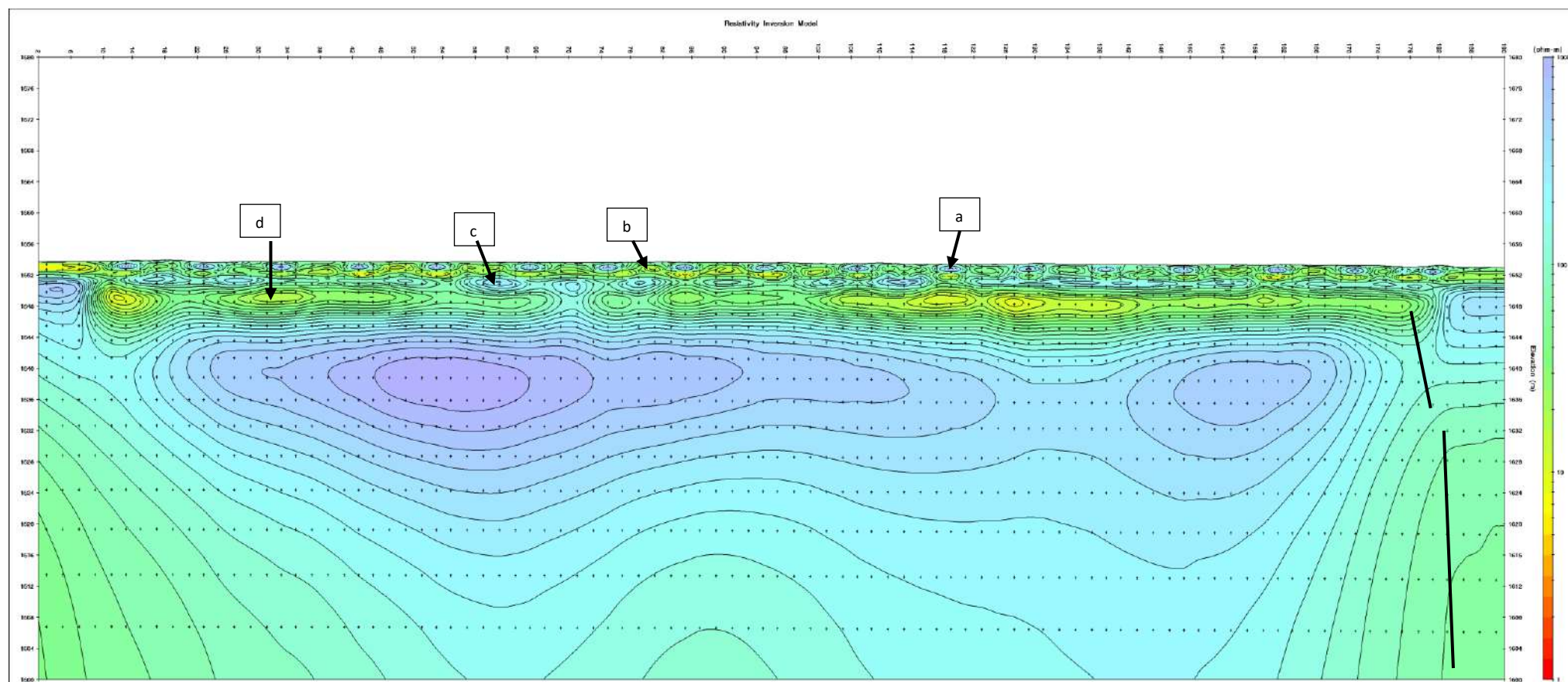


Figure 15: Showing
Faulting at stations: 178(1)
Weathered and Overburden units (d), (c), and (a) present.

Line12, spacing = 2m, length = 150m

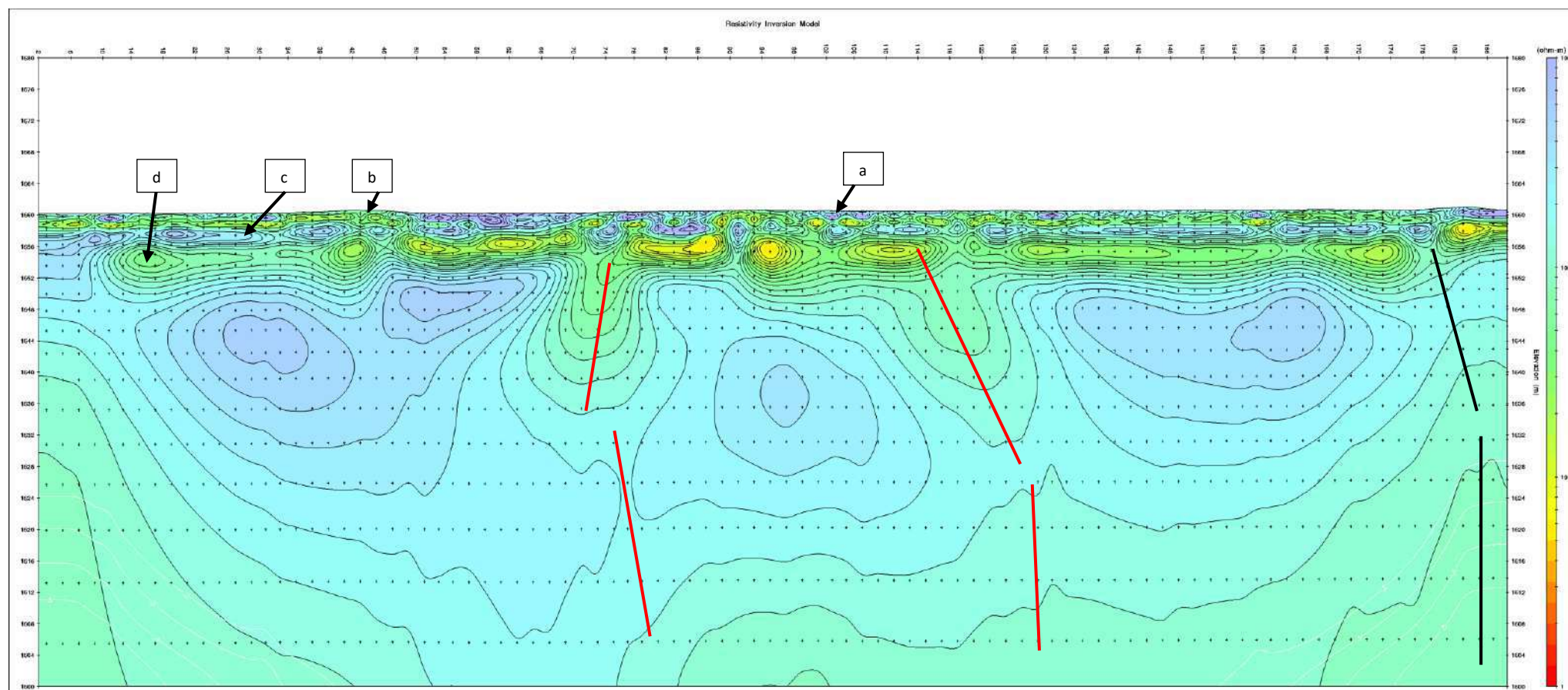


Figure 16: Showing

Faulting at stations: 74(2), 114(2), 178(1)

Weathered and Overburden units (d), (c), and (a) present.

Line13, spacing = 2m, length = 200m

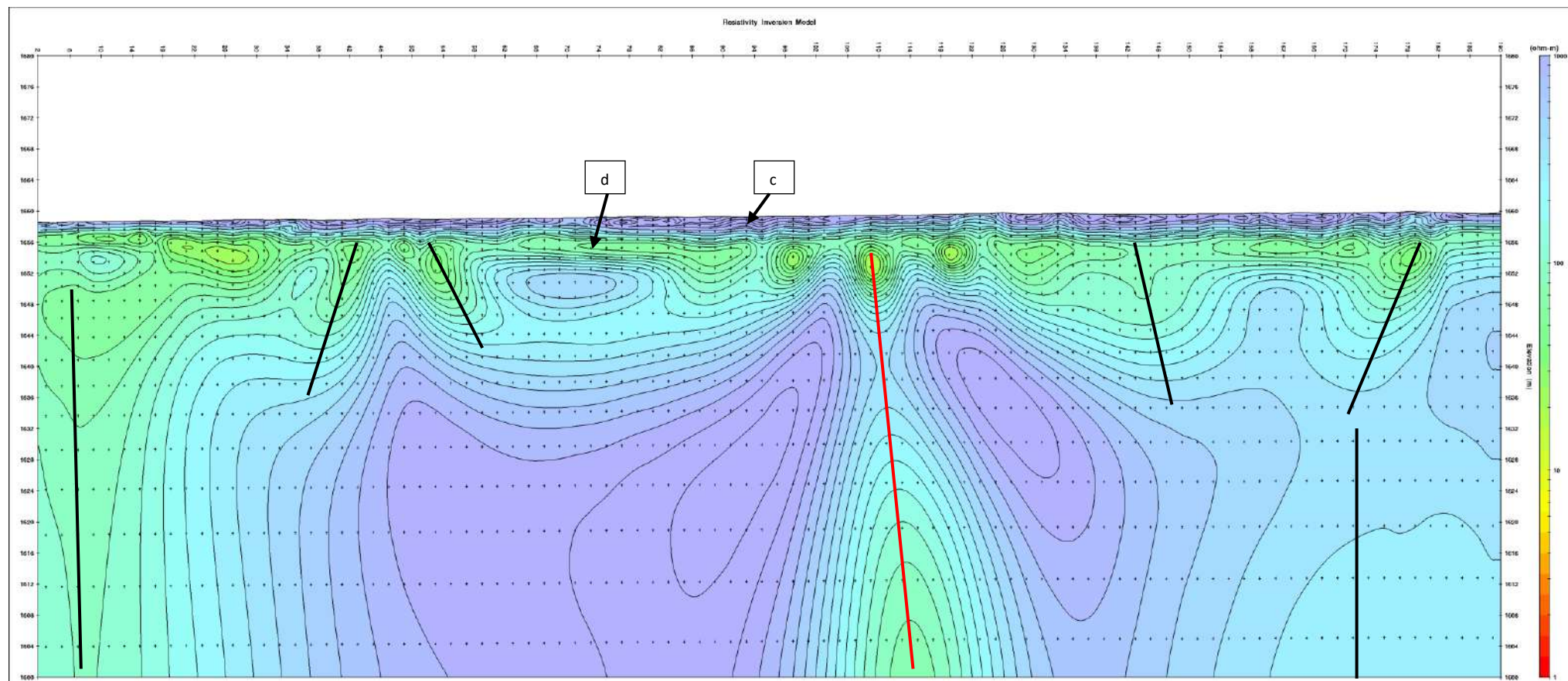


Figure 17: Showing

Faulting at stations: 6(1), 42(1), 52(1), 108(2), 142(1), 180(1)

Weathered and Overburden units (d) and (c) present.

In Figure 18 below we show the interpreted faults from the resistivity data overlain onto the aeromagnetic fault interpretation.

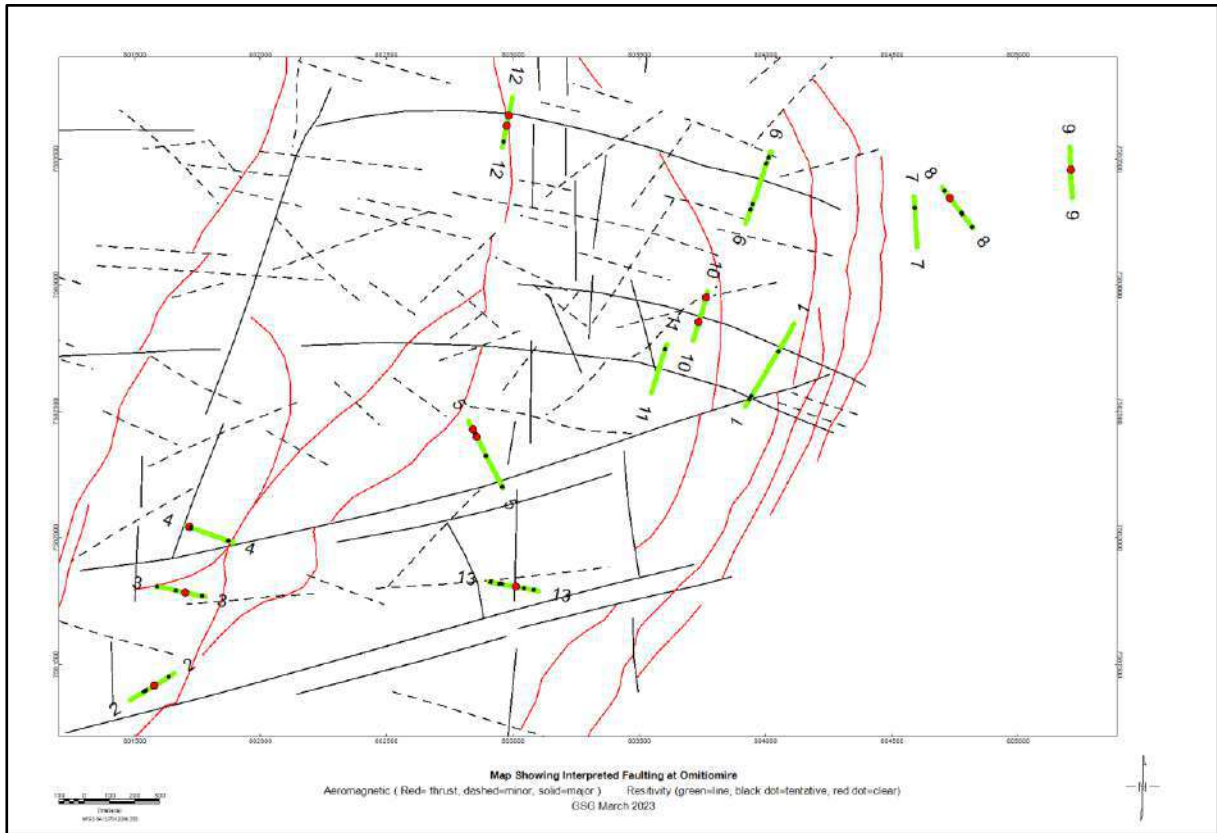


Figure 18: This Figure shows the resistivity lines in green with interpreted faults (red dot = clear fault, black dot = tentative fault) overlain onto aeromagnetic lineaments and faults.

Table 1 below details the top position of all faults interpreted above.

Table1 Giving Position and Classification of Faults (2= clear, 1= tentative)

Line	Line	Station	Easting	Northing	Elevation	Mark
L1	1	32	803935.714	7582548.374	1654.881	1
L1	1	48	803943.655	7582562.251	1654.811	1
L1	1	256	804051.586	7582740.021	1653.922	1
L2	2	62	801537.401	7581392.809	1655.018	1
L2	2	72	801545.994	7581398.061	1655.332	1
L2	2	110	801578.469	7581417.581	1654.217	2
L2	2	178	801636.920	7581452.683	1656.891	1
L3	3	6	801590.365	7581809.515	1656.473	1
L3	3	82	801664.492	7581793.448	1654.292	1
L3	3	120	801701.212	7581785.456	1655.011	2
L3	3	188	801767.768	7581771.843	1654.851	1
L4	4	14	801718.221	7582043.958	1655.256	2
L4	4	22	801725.668	7582041.312	1655.007	1
L4	4	178	801871.949	7581988.970	1655.831	1
L5	5	6	802956.323	7582203.160	1653.391	1

Line	Line	Station	Easting	Northing	Elevation	Mark
L5	5	144	802892.140	7582325.361	1650.425	1
L5	5	228	802855.418	7582400.767	1652.807	2
L5	5	246	802847.442	7582416.943	1652.960	1
L5	5	261	802840.451	7582430.191	1653.488	2
L7	7	156	804590.057	7583306.515	1650.193	1
L8	8	4	804817.748	7583230.576	1649.570	1
L8	8	68	804778.943	7583281.525	1648.801	1
L8	8	74	804775.455	7583286.338	1649.106	1
L8	8	148	804730.517	7583344.823	1648.917	2
L8	8	184	804708.645	7583373.431	1648.977	1
L9	9	110	805209.109	7583456.612	1648.842	2
L10	10	76	803735.145	7582856.130	1654.578	2
L10	10	176	803765.165	7582951.791	1655.019	2
L11	11	178	803601.863	7582746.873	1653.049	1
L13	13	6	802909.977	7581827.520	1658.585	1
L13	13	42	802945.444	7581821.024	1658.838	1
L13	13	52	802955.332	7581819.224	1658.942	1
L13	13	108	803010.498	7581809.196	1659.442	2
L13	13	142	803043.997	7581803.238	1659.598	1
L13	13	180	803081.411	7581796.278	1659.881	1

6. Conclusions and recommendations

Conclusions

- Faults interpreted from the ERT resistivity sections indicated in red are confidently interpreted as faults or fractures that may contain or transport water.
- In the top 0-8m we see a clear Kalahari stratigraphy consisting of windblown or reiver sediment. The stratigraphic succession is an upper resistive sandy layer, a middle conductive clayey layer, and a lower resistive sandy layer.
- Basement rocks below the overlying Kalahari appear to be weathered in places and more conductive.
- The Kalahari succession in this area appears to be influenced by episodic flooding of the Nossob River into its flood plain.

Recommendations

- Faults marked in red are recommended as possible candidates that may carry or conduct ground water.
- When planning a pit, shallow Kalahari conductive and more clayey sediments should be borne in mind as a potential mining hazard. As this is a very shallow unit, earth moving machinery may get bogged down during rainy periods where these sediments are present. Secondly this material although shallow may show a tendency to collapse into the pit during rainy periods. Care is advised in pit and pit access road planning.

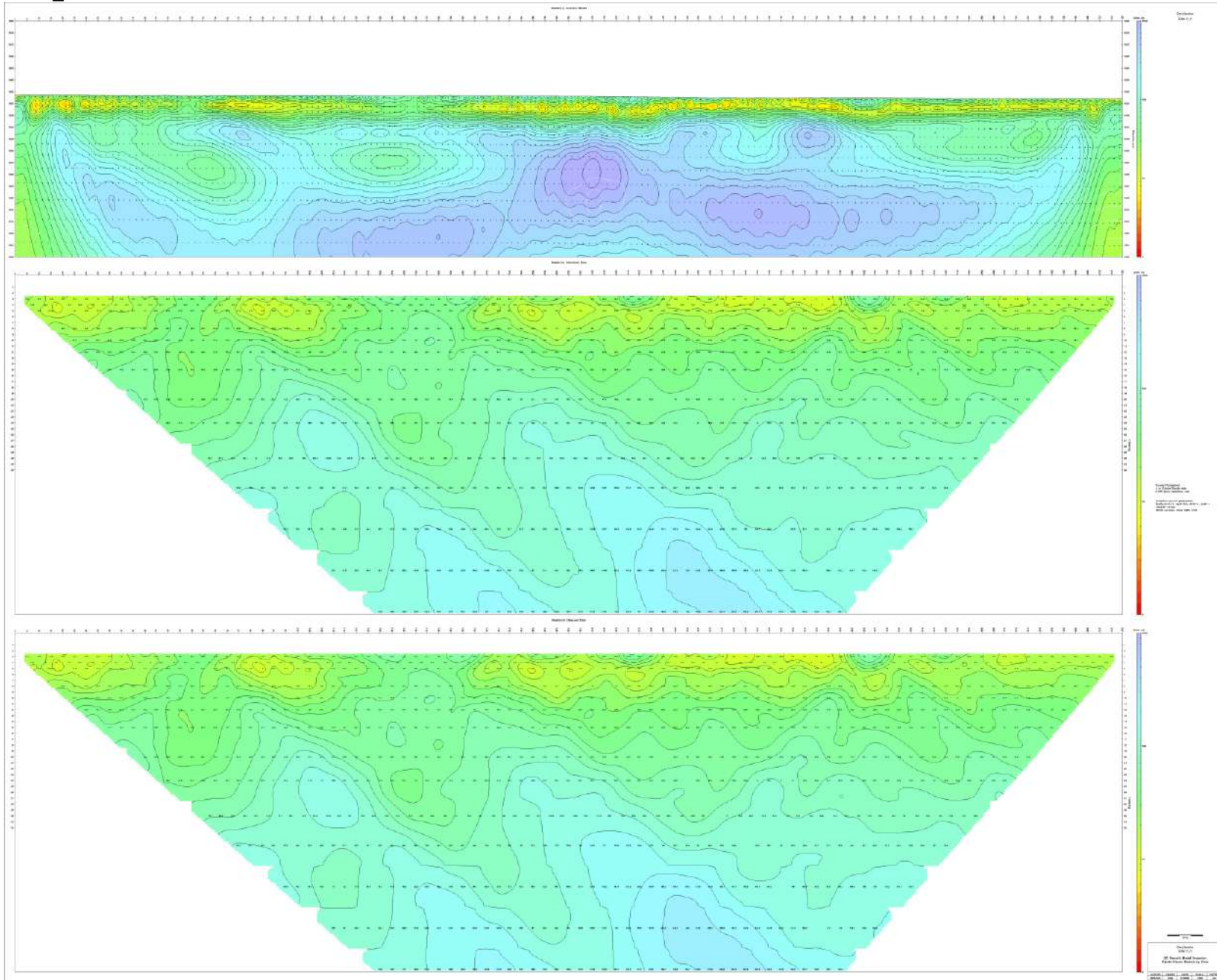
7. Acknowledgements

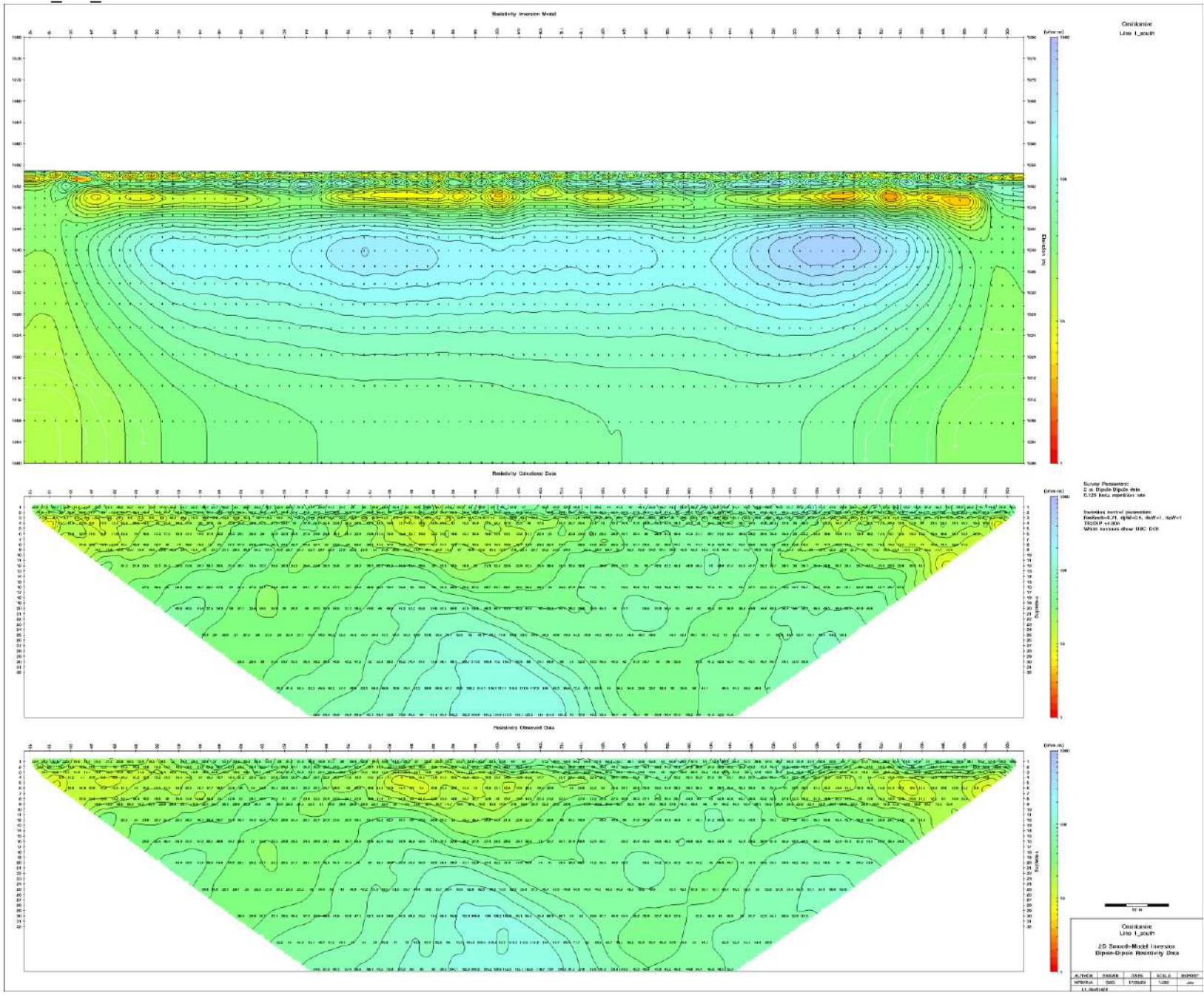
We thank Knight Piésold Consulting and all related parties for all the support rendered to us during this survey. Craton Mining and exploration is thanked for the camping accommodation.

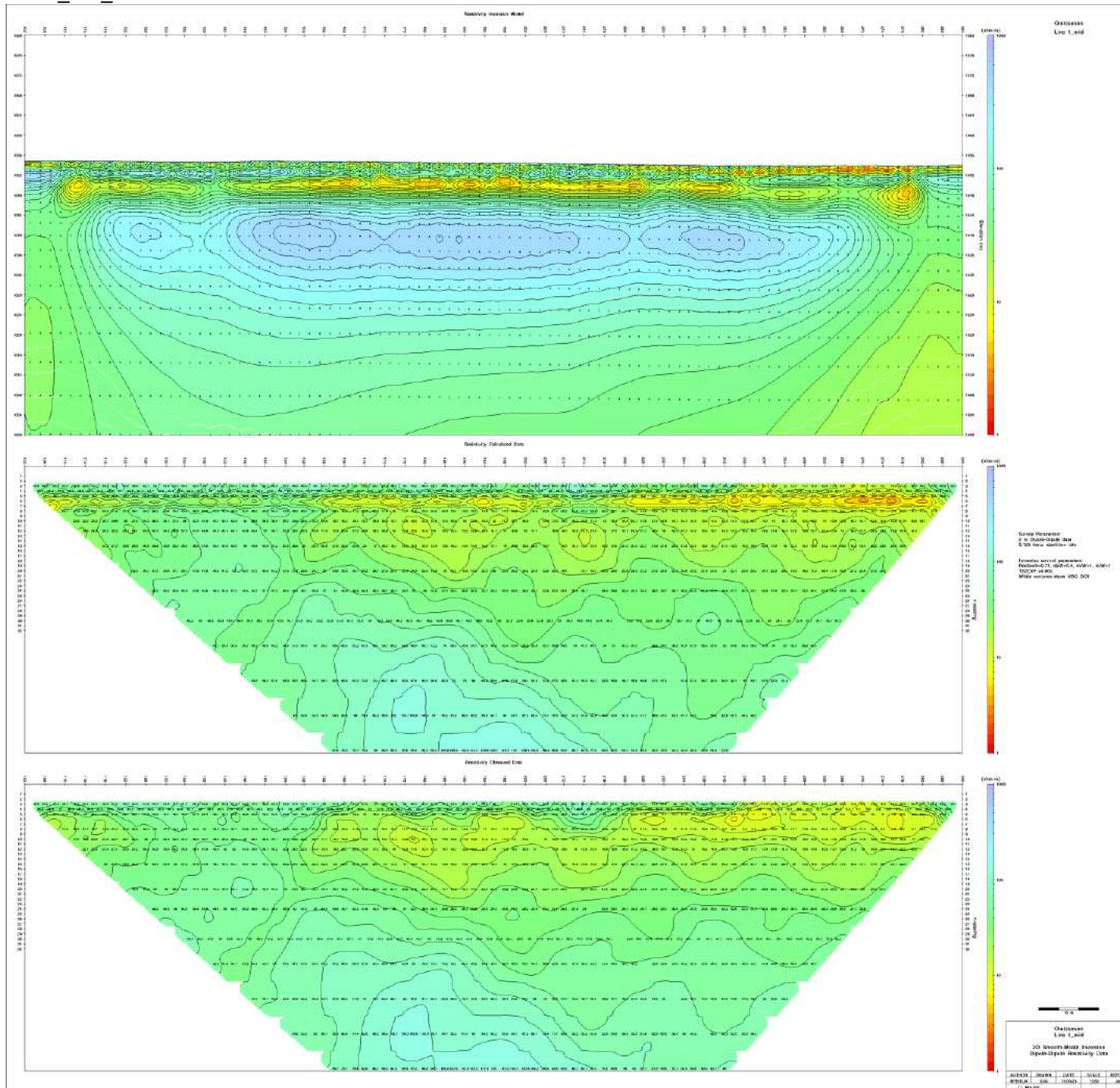
G Symons, D-J Adams
(Geophysicists)
March 2023.

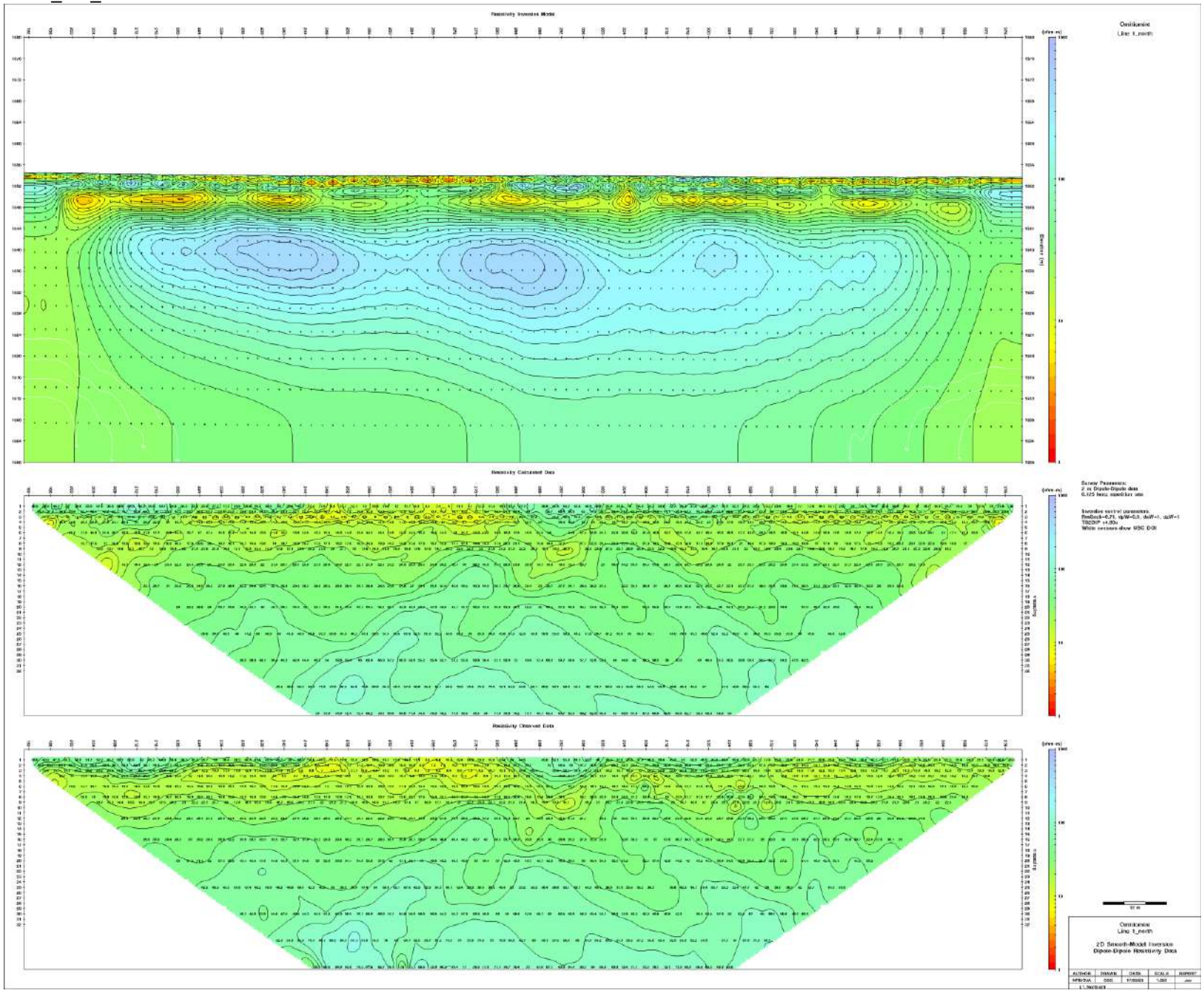
Appendix 1: Electrical Resistivity Tomography 2D Inversion Sections

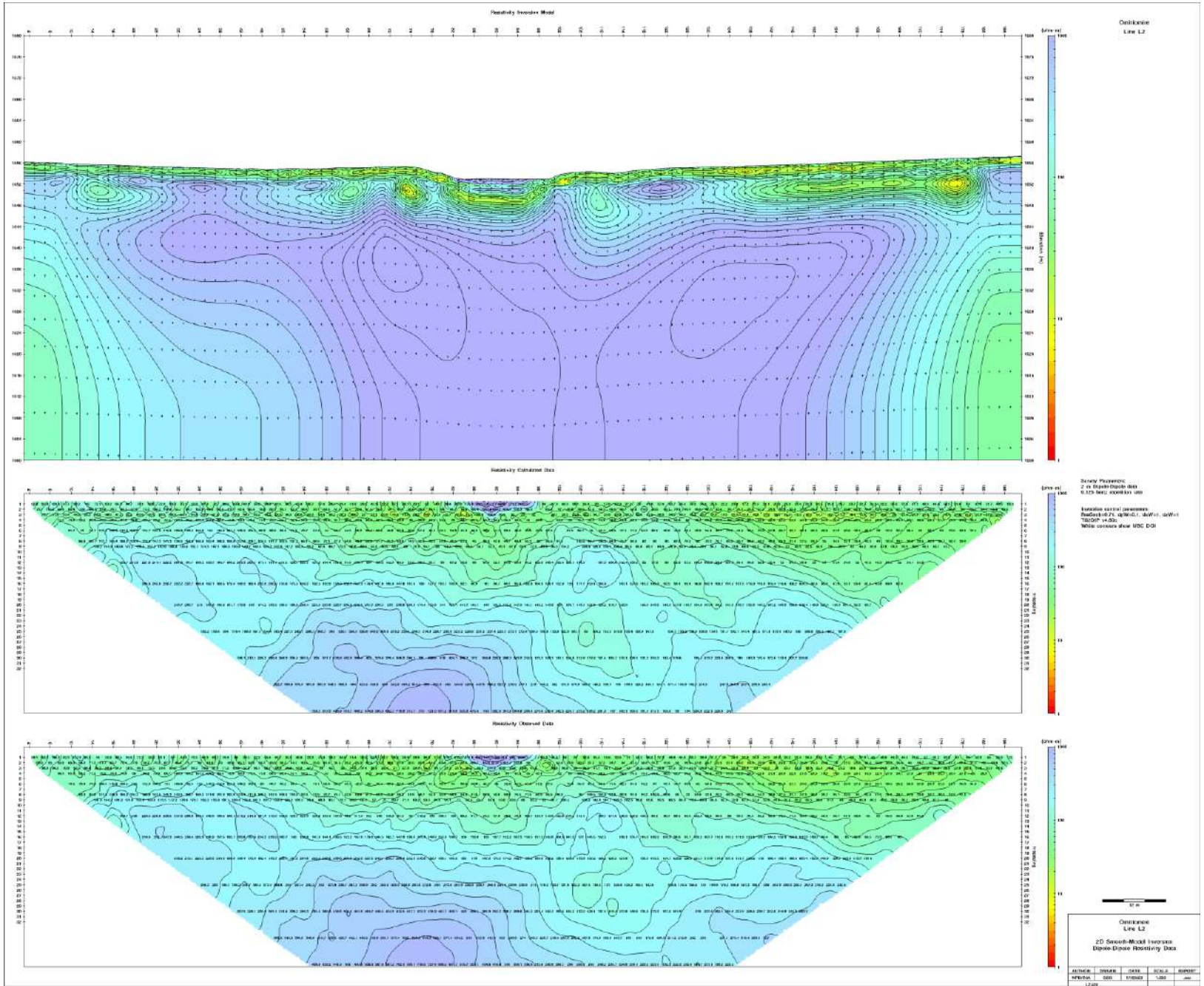
Line1_4m



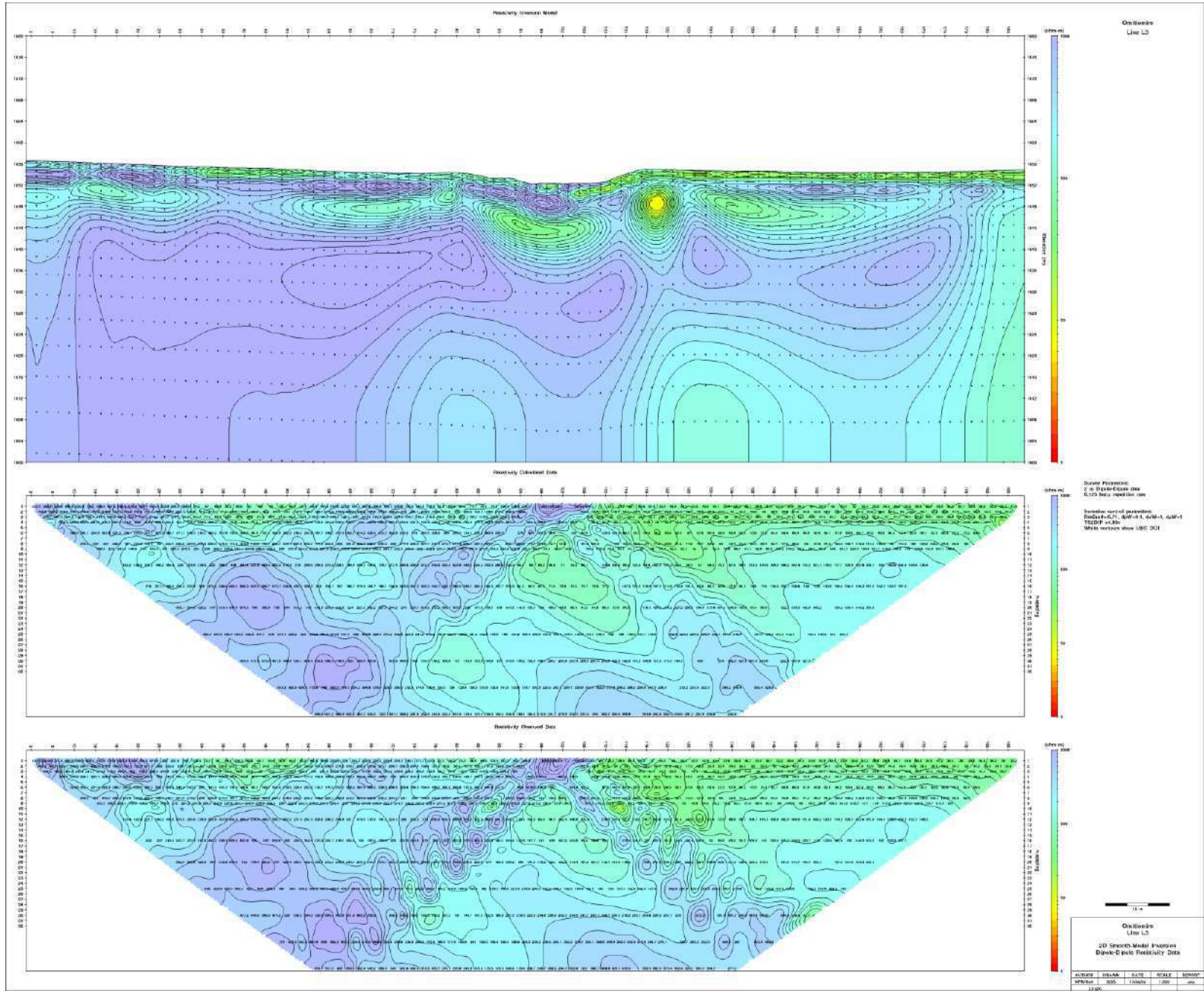




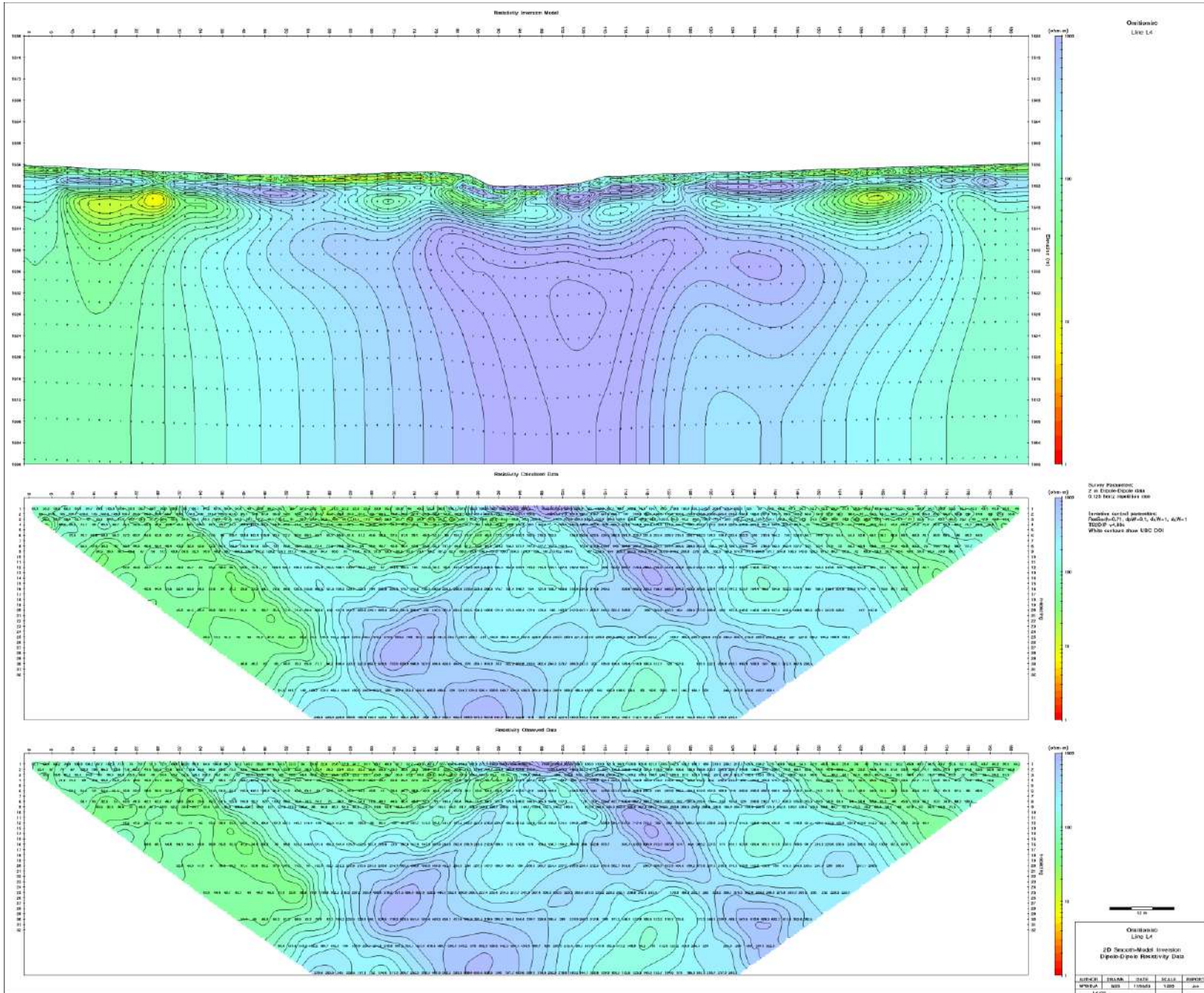




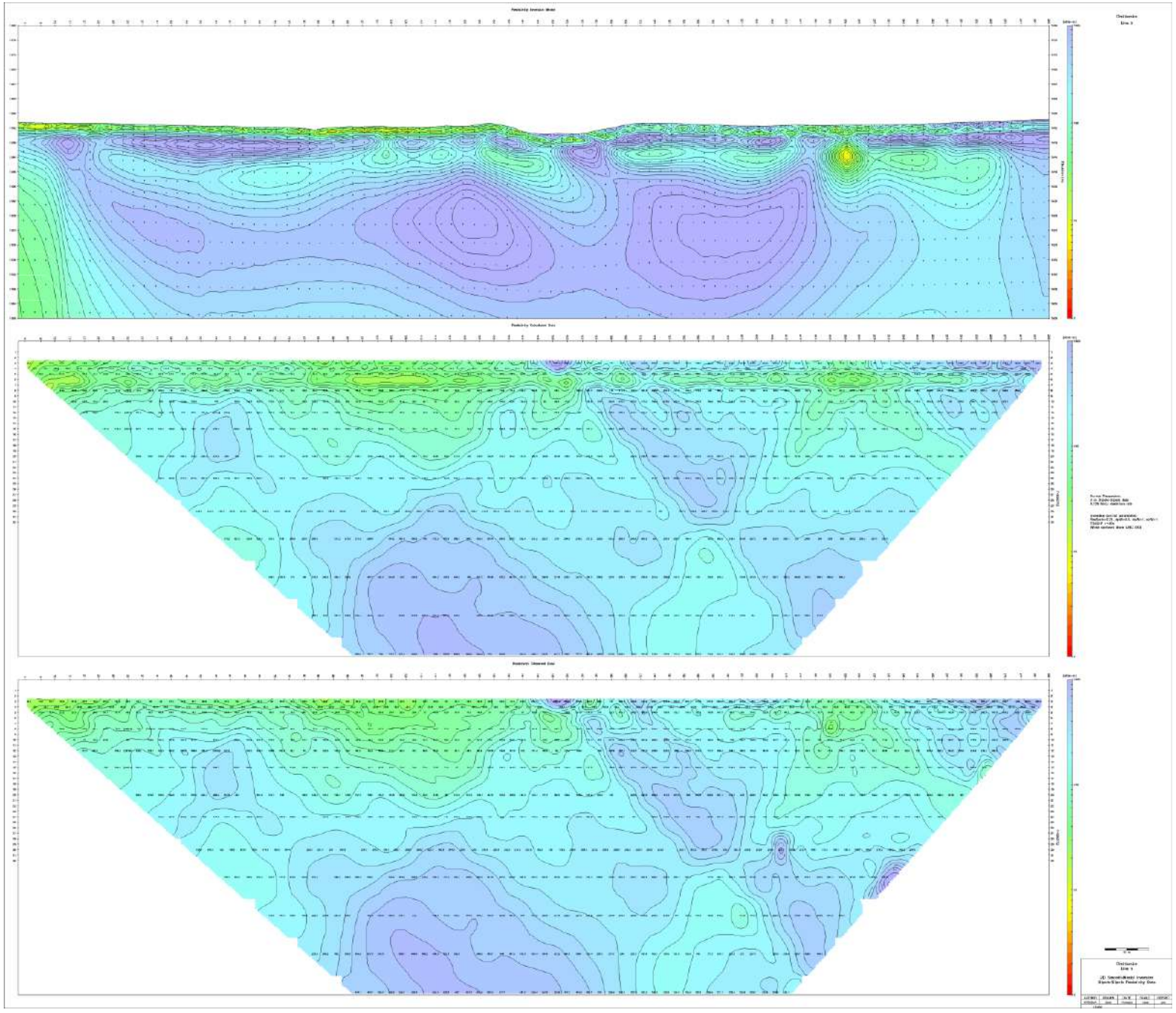
Line3



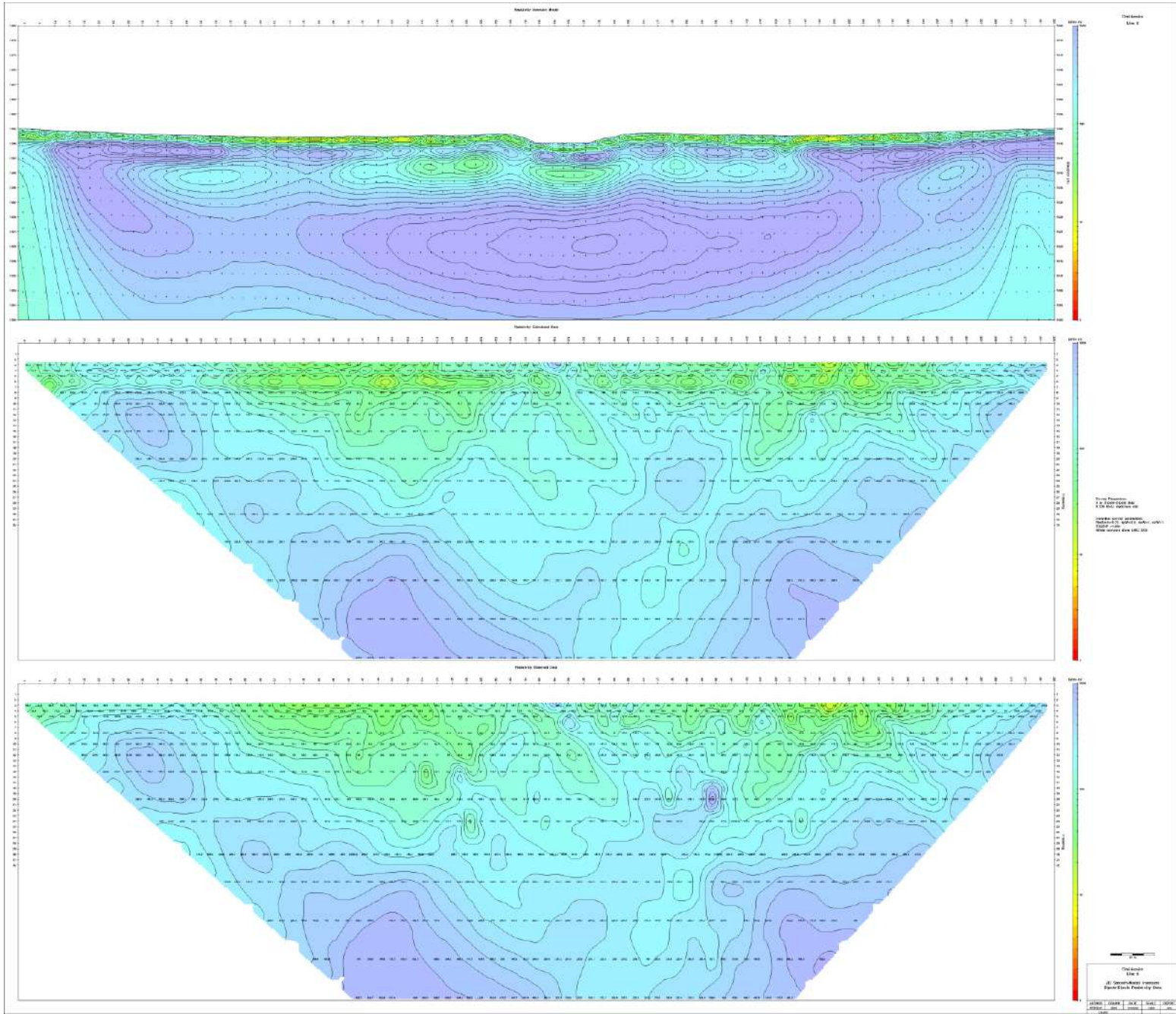
Line4

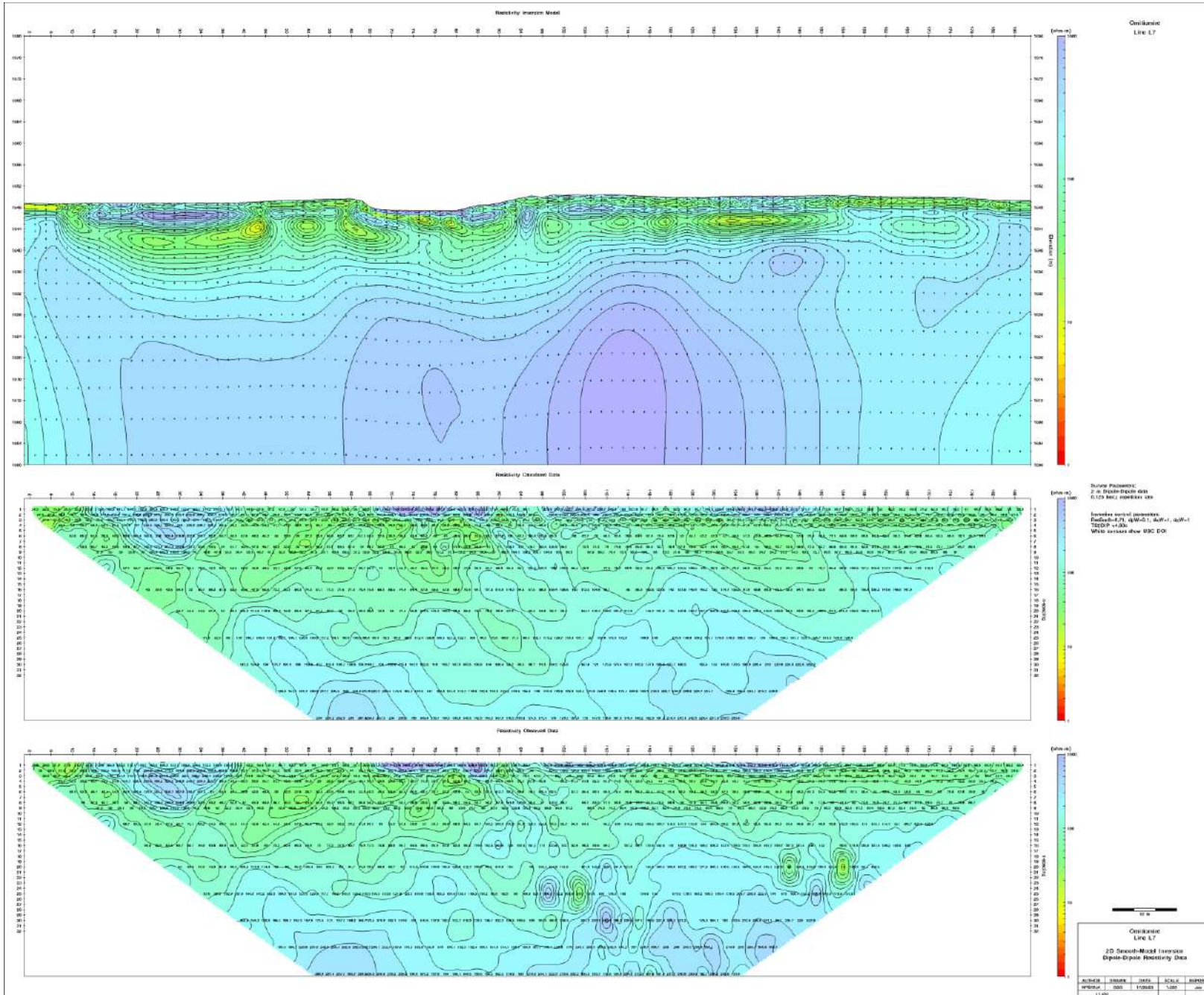


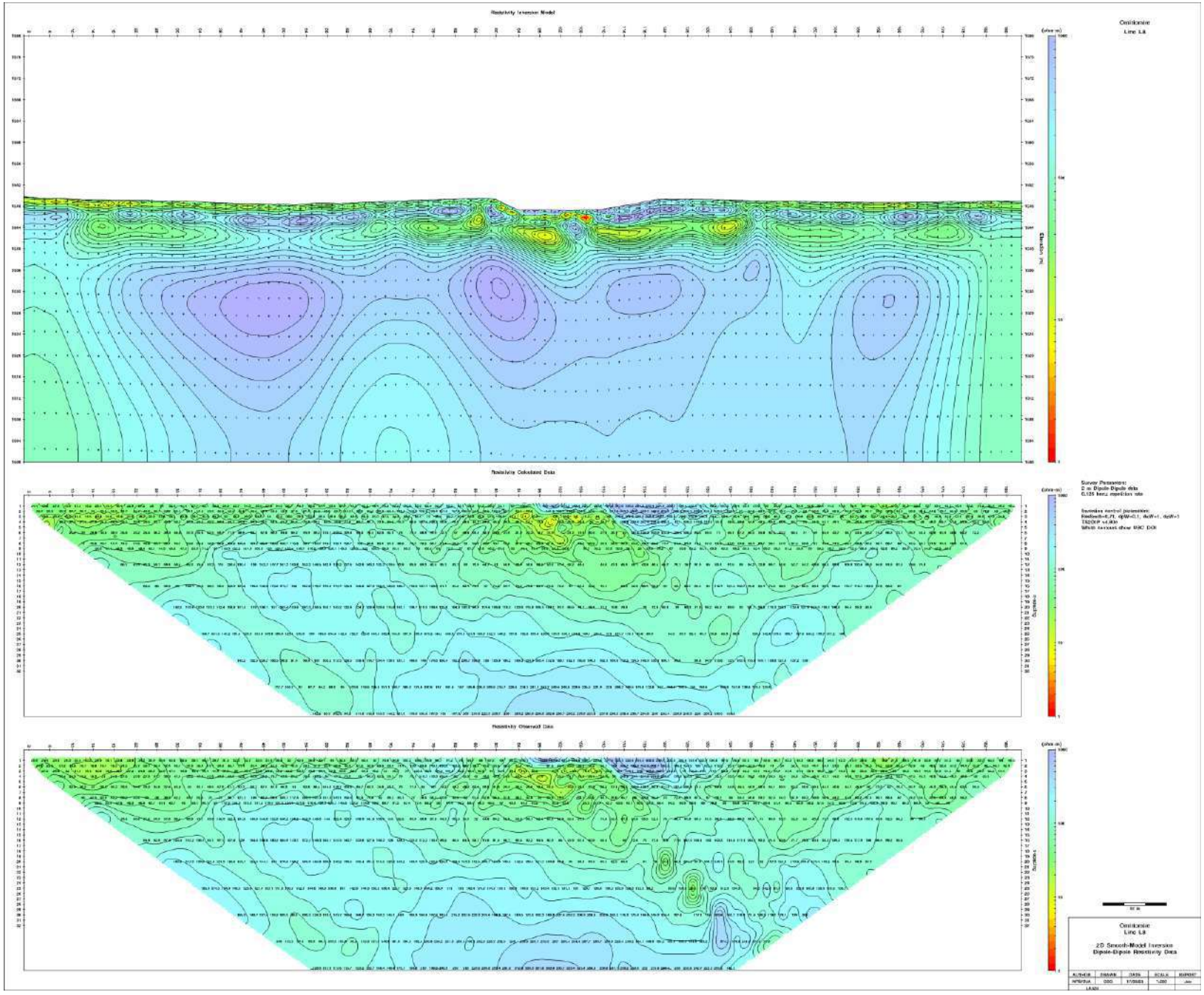
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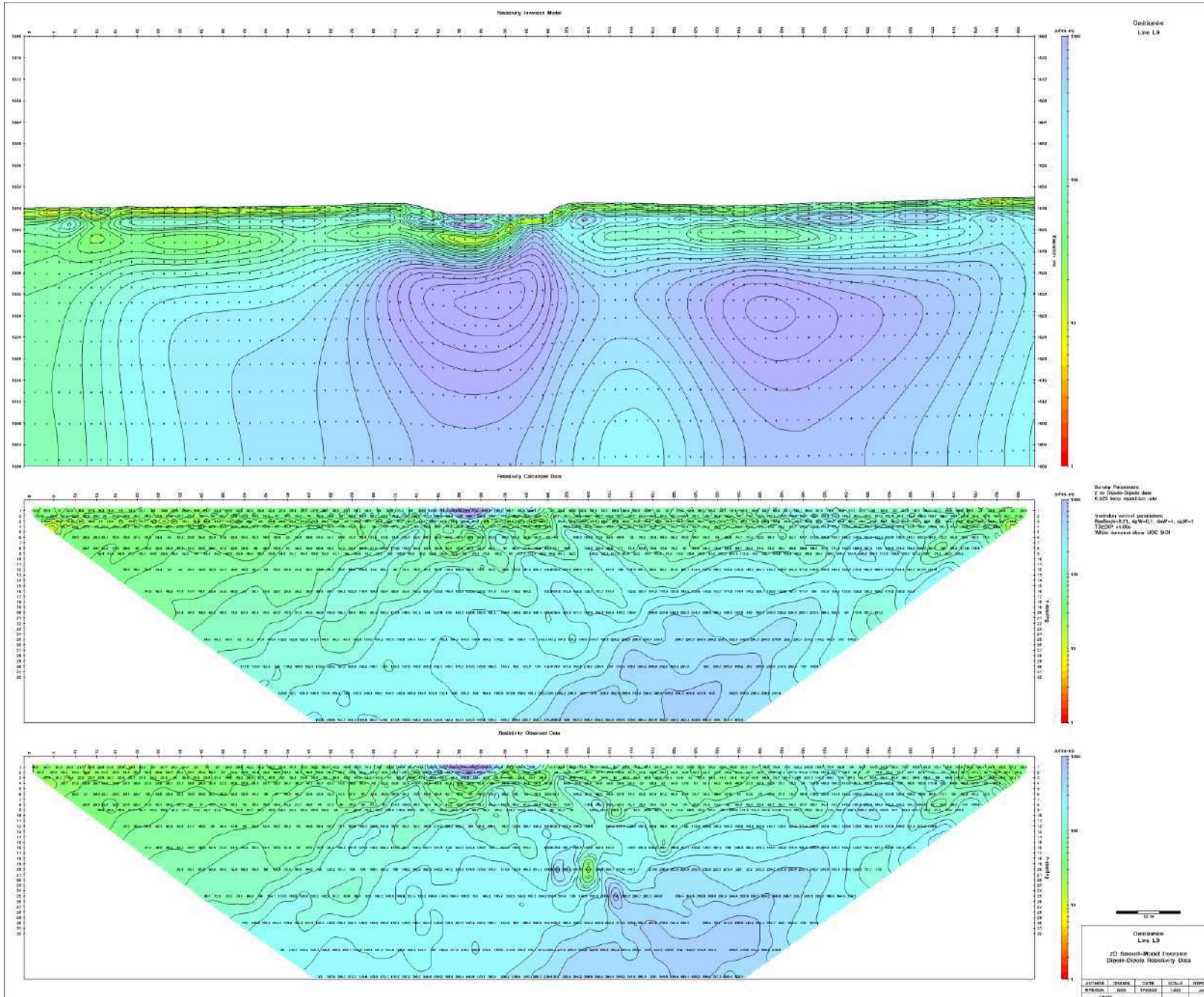


Line6

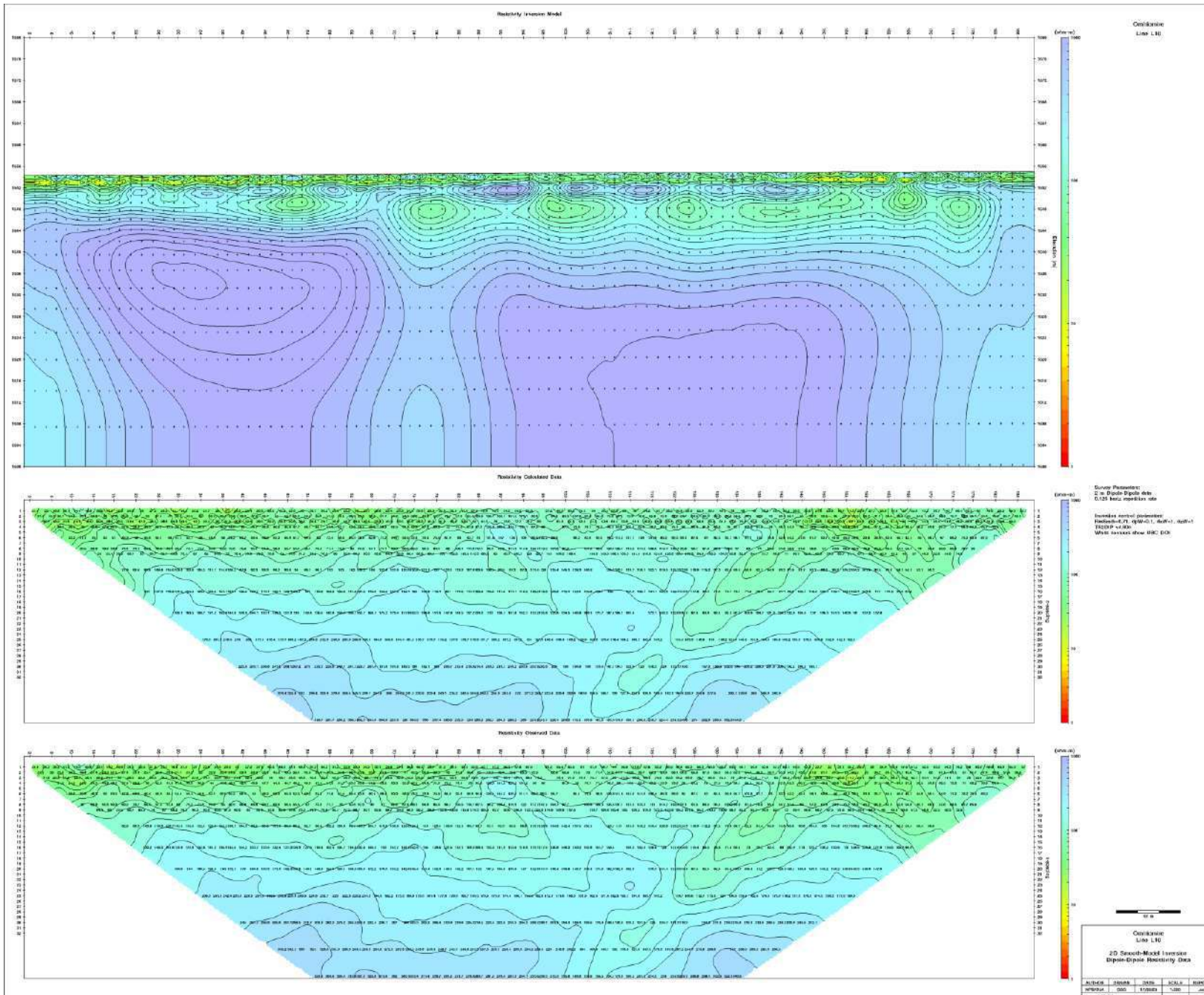




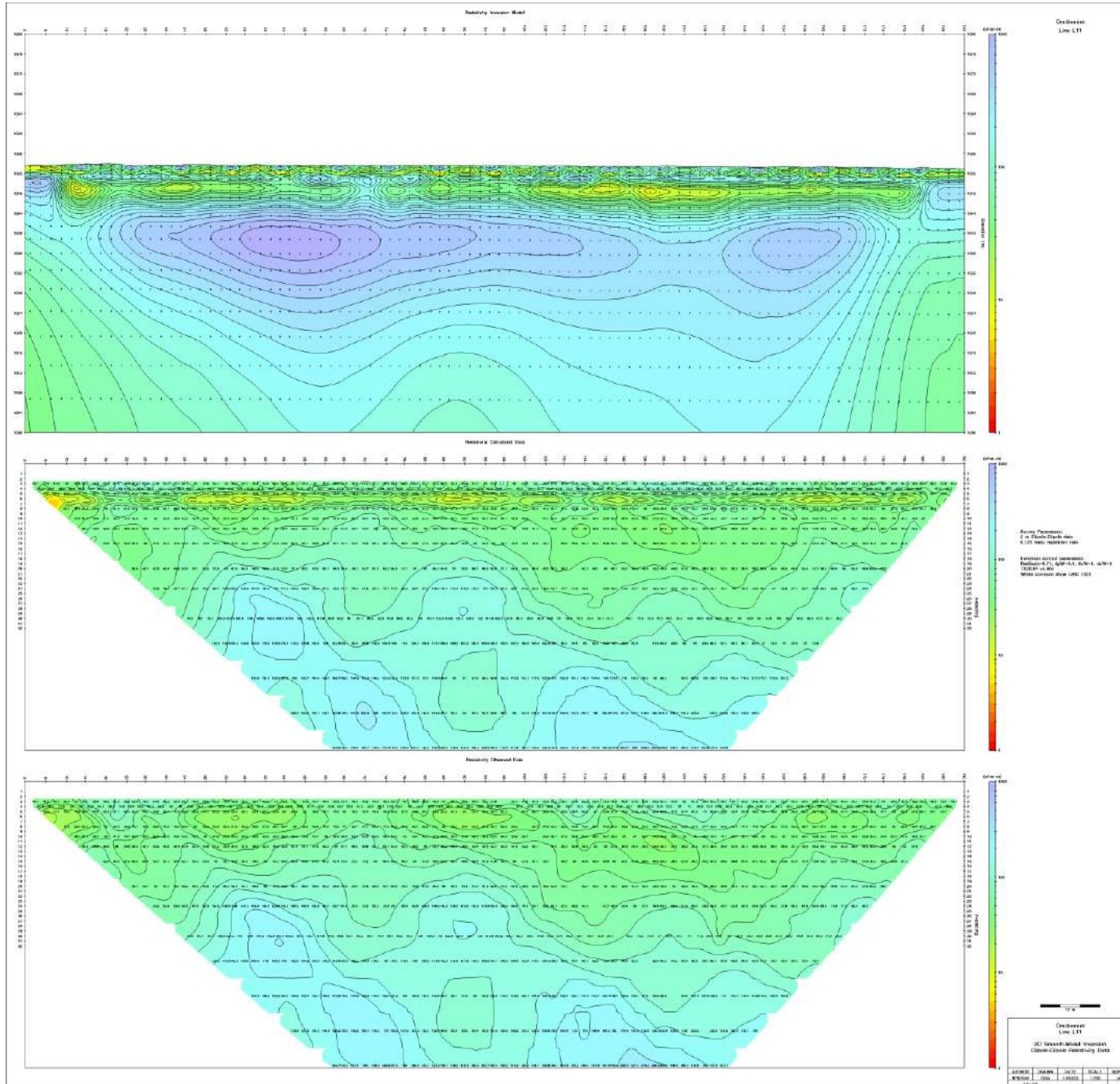




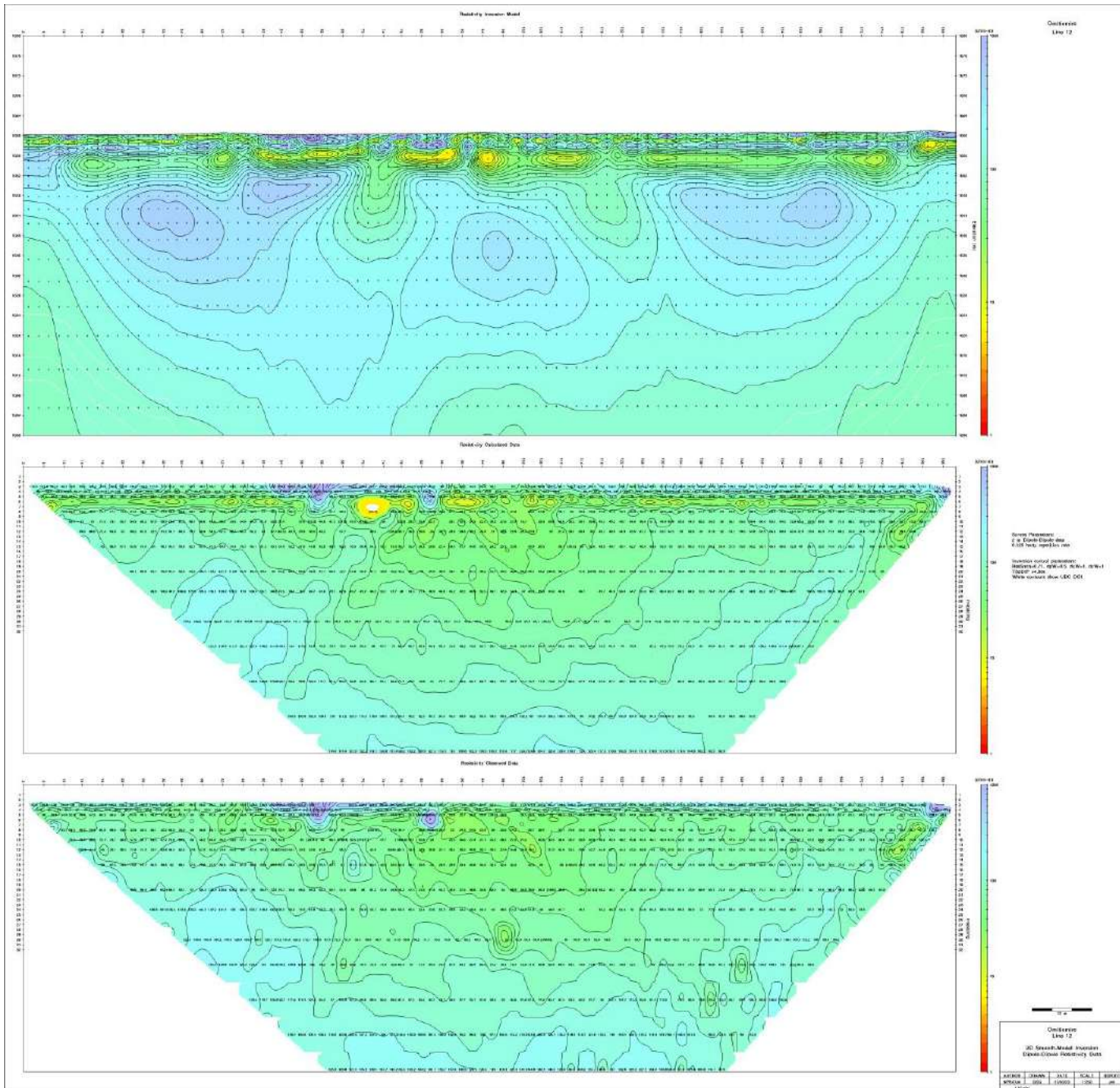
Line10



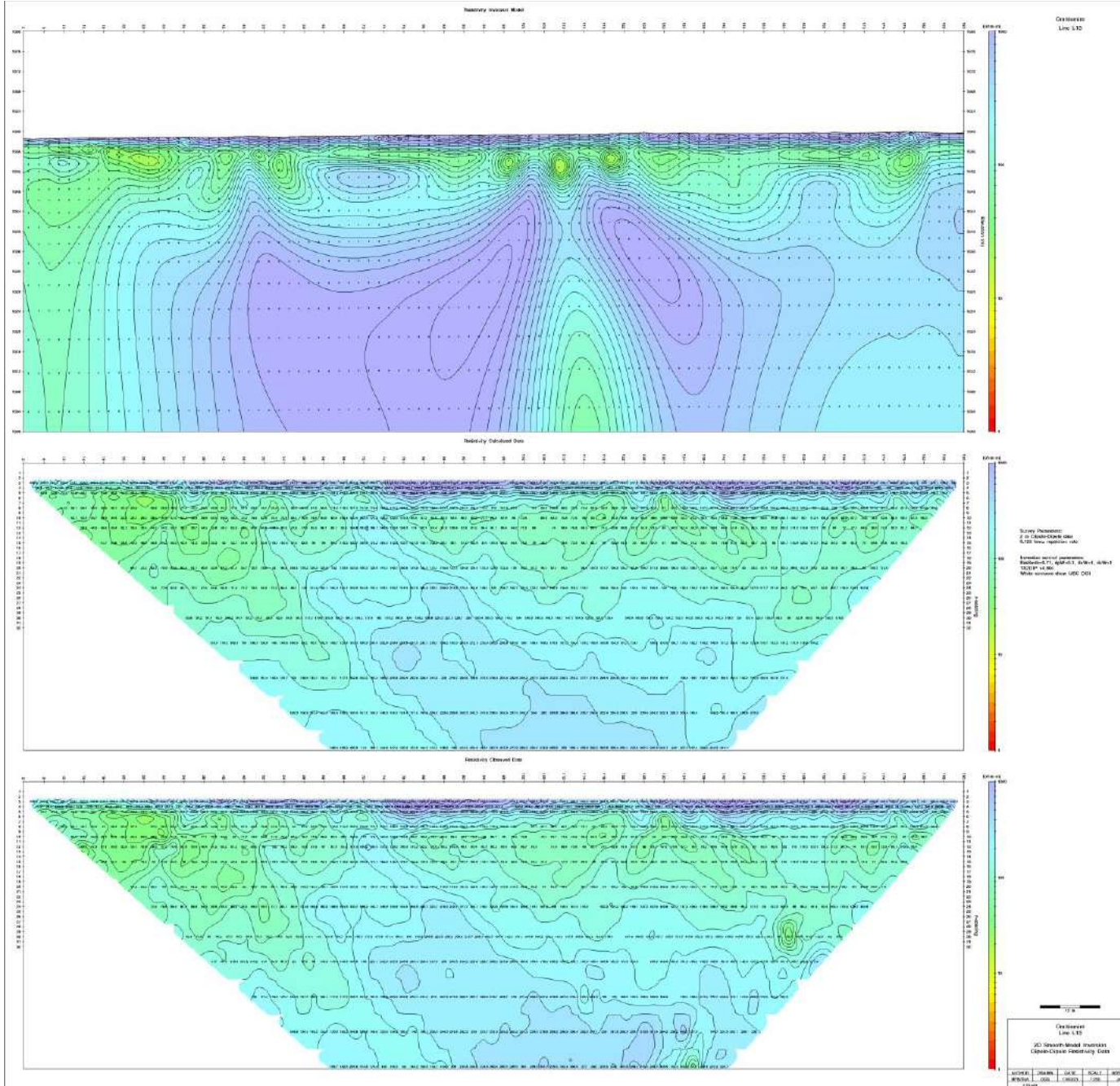
Line11



Line12



Line13



Appendix 2: Data Archive

GSG organized an archive of all data as follows:

- Report: A folder with this report as a word document, a timesheet Excel document, and a PNGs folder with all the Maps for the three Surveys,
- Resistivity: A folder with all raw and processed Electrical Resistivity Tomography Data. Each line is presented, the most important files being the PNG, map and Geosoft *.grd files which give the inverted resistivity results.

APPENDIX G

Laboratory Test Results

Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Potential Borrow Sources

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1612

Page no : 1

sample detail	Sample no.	S/10394	S/10395	S/10396	S/10399	S/10402	S/10405
	Chainage						
	Position (from LHS)	AM-TP01	AM-TP02	AM-TP05	AM-TP06	AM-TP08-B	AM-TP04
	Depth sampled (mm)	400-600	0-700	0-600	400-1000	500-700	300-1100
	Sample description	Brown stained orange brown mottled black, clayey silty sand, slightly ferruginised fine alluvium	Reddish brown, clayey silty sand with traces of subangular to subrounded fine gravel, fine alluvium	Brown, clayey silty sand, fine alluvium	Reddish brown, clayey silty sand with traces of subangular to subrounded gravel to fine gravel, fine alluvium	Dark brown with orange staining, clayey silty sand, fine alluvium	Reddish brown, clayey silty sandy gravel, calcareous alluvium
Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping	
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Compaction (%)							
sieve analysis (% passing)	63,0 mm						100
	53,0 mm						88
	37,5 mm						80
	26,5 mm						64
	19,0 mm		100				50
	13,2 mm		99			100	41
	4,75 mm	100	99	100	100	100	25
	2,00 mm	100	97	100	99	99	21
	0,425 mm	83	83	92	83	84	18
	0,075 mm	48	46	41	43	48	4
Grading Modulus (GM)	0.69	0.74	0.67	0.75	0.69	2.57	
Atterberg Limits	Liquid Limit (LL)	26	26	19	22	25	
	Plasticity Index (PI)	10	9	5	7	9	1
	Linear Shrinkage (LS)	4.0	3.0	2.5	2.5	3.5	
Mod AASHTO & CBR values	CBR @ 100 %				29		147
	CBR @ 98 %				18		129
	CBR @ 95 %				9		105
	CBR @ 93 %				5		92
	CBR @ 90 %				<3		75
	Swell @ 100 %				0.0		0.0
	Mod AASHTO (kg/m ³)				2079		2149
O.M.C. (%)				9.2		6.5	
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture				90.0% Mod AASHTO @ OMC		90.0% Mod AASHTO @ OMC
	Permeability cm/s				1.29 E-05		1.01 E-04
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

Namibia Technical Services cc

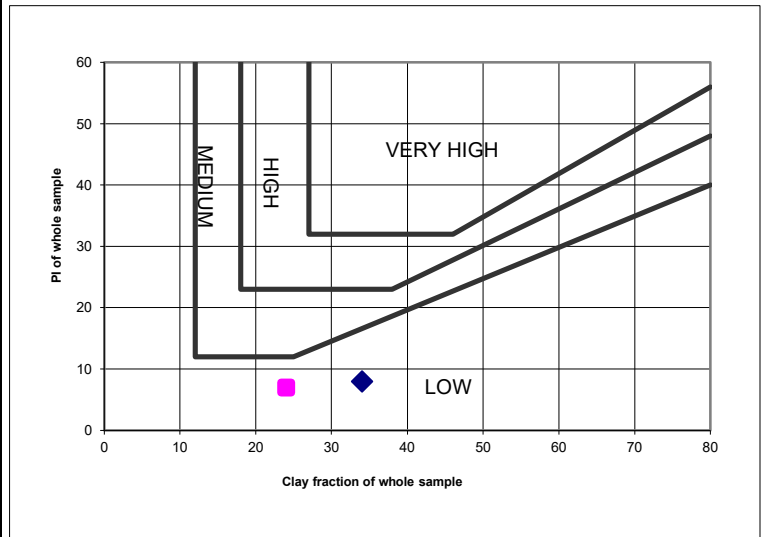
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

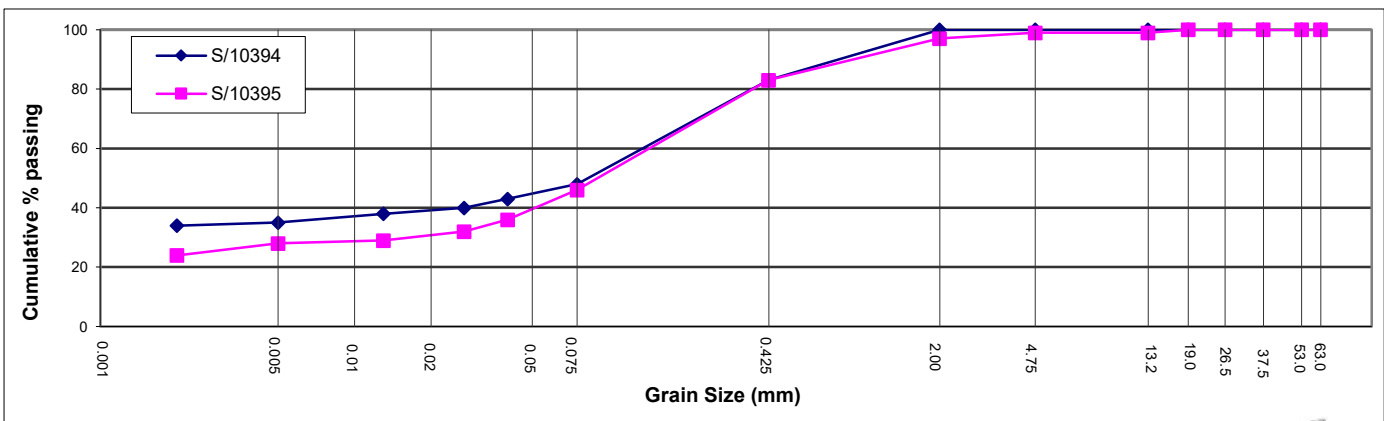
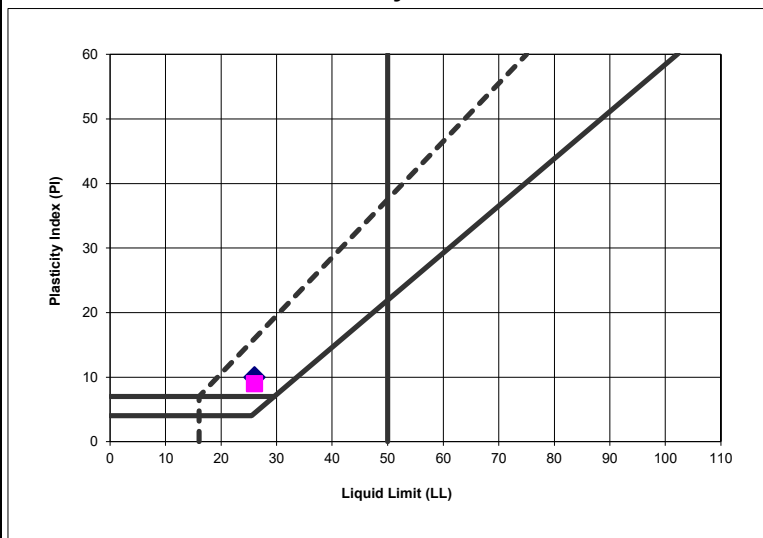
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10394	S/10395
Report No.	SG23/1612	
Depth (mm)	400-600	0-700
Position	AM-TP01	AM-TP02
Material description	Clayey sand	Clayey sand
Moisture (%)		
SG	2.528	2.697
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		100
13.2 mm		99
4.75 mm	100	99
2.00 mm	100	97
0.425 mm	83	83
0.075 mm	48	46
Hydrometer Analysis (% passing)		
0.040 mm	43	36
0.027 mm	40	32
0.013 mm	38	29
0.005 mm	35	28
0.002 mm	34	24
% Clay (<0.002 mm)	34	24
% Silt (<0.075, >0.002 mm)	14	22
% Sand (<4.75, >0.075 mm)	52	53
% Gravel (> 4.75 mm)	0	1
Atterberg Limits		
Liquid Limit (-0.425)	26	26
Plasticity Index (-0.425)	10	9
Linear Shrinkage (%)	4.0	3.0
PI of whole sample	8	7
Grading Modulus	0.69	0.74
TRB Classification	A-4 (0)	A-4 (0)
Unified Classification	SC	SC

Potential Expansiveness



Plasticity Chart




Namibia Technical Services cc

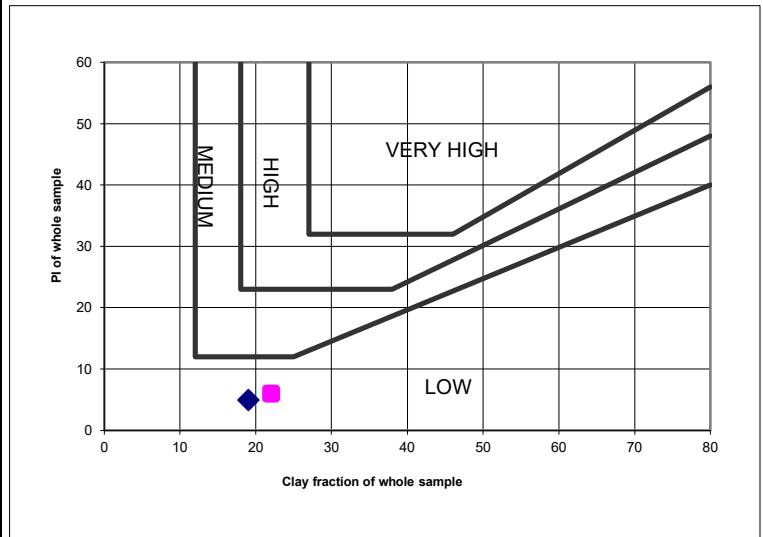
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

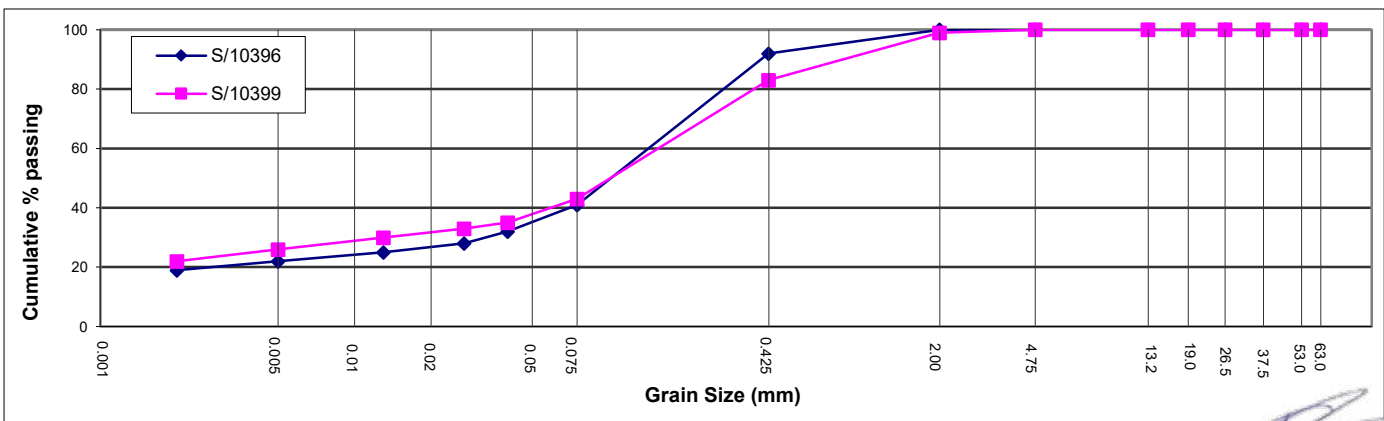
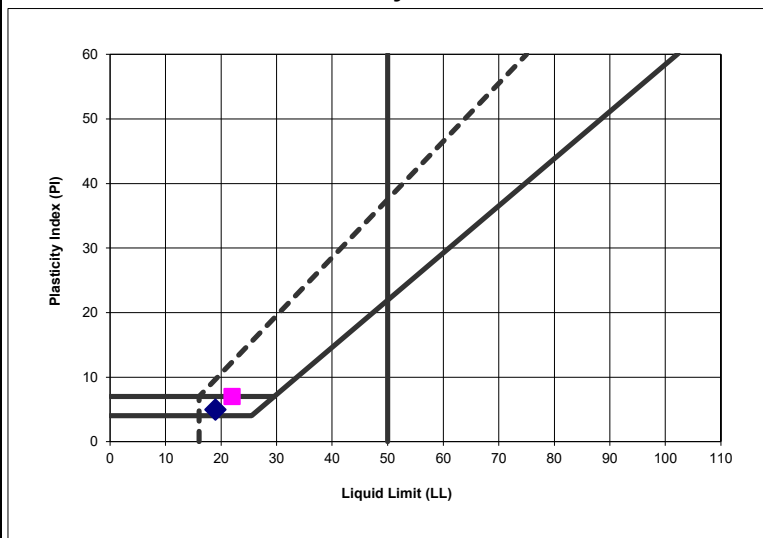
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10396	S/10399
Report No.	SG23/1612	
Depth (mm)	0-600	400-1000
Position	AM-TP05	AM-TP06
Material description	Silty, clayey sand	Clayey sand
Moisture (%)		
SG	2.741	2.652
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		
13.2 mm		
4.75 mm	100	100
2.00 mm	100	99
0.425 mm	92	83
0.075 mm	41	43
Hydrometer Analysis (% passing)		
0.040 mm	32	35
0.027 mm	28	33
0.013 mm	25	30
0.005 mm	22	26
0.002 mm	19	22
% Clay (<0.002 mm)	19	22
% Silt (<0.075, >0.002 mm)	22	21
% Sand (<4.75, >0.075 mm)	59	57
% Gravel (> 4.75 mm)	0	0
Atterberg Limits		
Liquid Limit (-0.425)	19	22
Plasticity Index (-0.425)	5	7
Linear Shrinkage (%)	2.5	2.5
PI of whole sample	5	6
Grading Modulus	0.67	0.75
TRB Classification	A-4 (0)	A-4 (0)
Unified Classification	SC-SM	SC

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

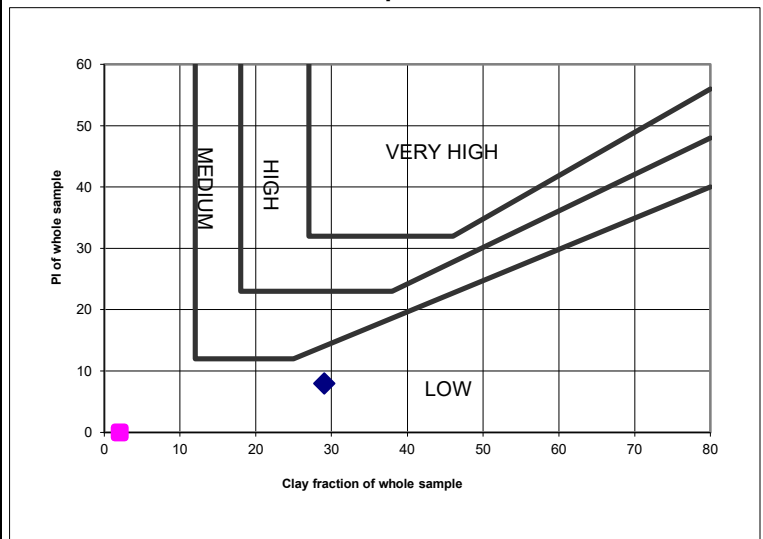
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

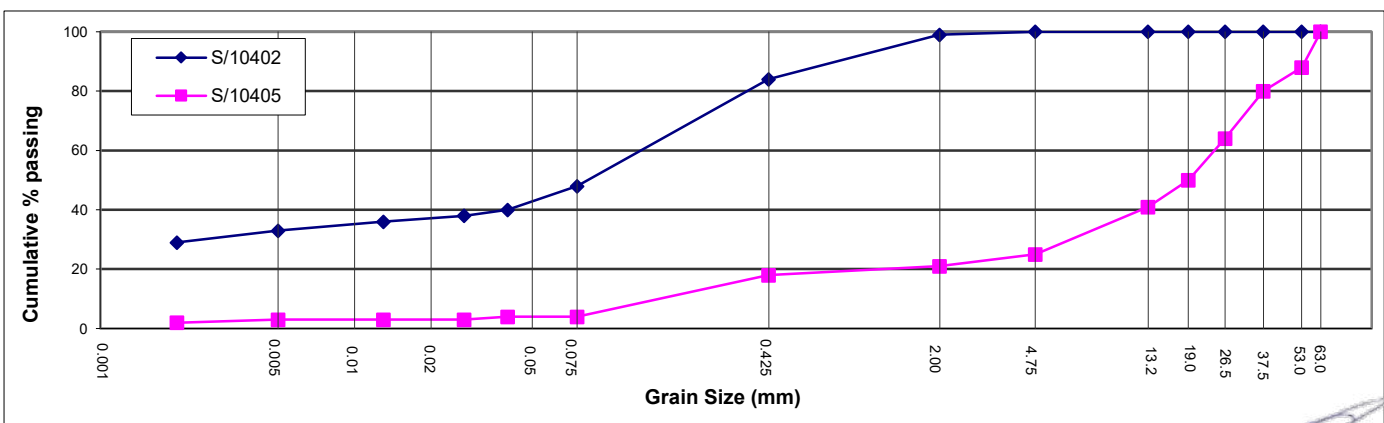
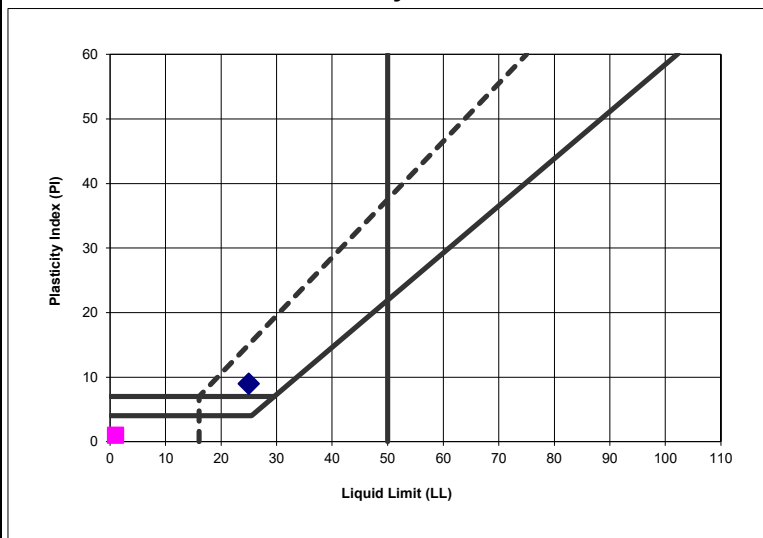
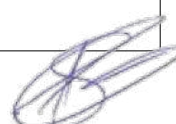
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10402	S/10405
Report No.	SG23/1612	
Depth (mm)	500-700	300-1100
Position	AM-TP08-B	AM-TP04
Material description	Clayey sand	Poorly graded gravel with sand
Moisture (%)		
SG	2.620	2.624
Sieve Analysis (% passing)		
63.0 mm		100
53.0 mm		88
37.5 mm		80
26.5 mm		64
19.0 mm		50
13.2 mm	100	41
4.75 mm	100	25
2.00 mm	99	21
0.425 mm	84	18
0.075 mm	48	4
Hydrometer Analysis (% passing)		
0.040 mm	40	4
0.027 mm	38	3
0.013 mm	36	3
0.005 mm	33	3
0.002 mm	29	2
% Clay (<0.002 mm)	29	2
% Silt (<0.075, >0.002 mm)	19	2
% Sand (<4.75, >0.075 mm)	52	21
% Gravel (> 4.75 mm)	0	75
Atterberg Limits		
Liquid Limit (-0.425)	25	
Plasticity Index (-0.425)	9	1
Linear Shrinkage (%)	3.5	0.0
PI of whole sample	8	0
Grading Modulus	0.69	2.57
TRB Classification	A-4 (0)	A-1-a (0)
Unified Classification	SC	GP

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Potential Borrow Sources

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1612

Page no : 5

sample detail	Sample no.	S/10406	S/10407	S/10408			
	Chainage						
	Position (from LHS)	AM-TP02	AM-TP04	AM-TP03			
	Depth sampled (mm)	900-1100	1200-1500	700-1000			
	Sample description	Completely weathered, dark grey streaked white to reddish brown, soft to	Completely to highly weathered, white speckled to streaked	Reddish brown, clayey silty sandy gravel with cobbles, ferruginised /			
	Method of Preparation used	Scalping	Scalping	Scalping			
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm						
	53,0 mm			100			
	37,5 mm	100		96			
	26,5 mm	95		87			
	19,0 mm	90		77			
	13,2 mm	84	100	71			
	4,75 mm	60	98	51			
	2,00 mm	45	87	34			
	0,425 mm	34	40	28			
	0,075 mm	20	21	15			
Grading Modulus (GM)	2.01	1.52	2.23				
Atterberg Limits	Liquid Limit (LL)	32		23			
	Plasticity Index (PI)	12	1	8			
	Linear Shrinkage (LS)	6.0		3.5			
Mod AASHTO & CBR values	CBR @ 100 %		76				
	CBR @ 98 %		51				
	CBR @ 95 %		28				
	CBR @ 93 %		18				
	CBR @ 90 %		10				
	Swell @ 100 %		0.0				
	Mod AASHTO (kg/m ³)		2020				
	O.M.C. (%)		8.8				
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.



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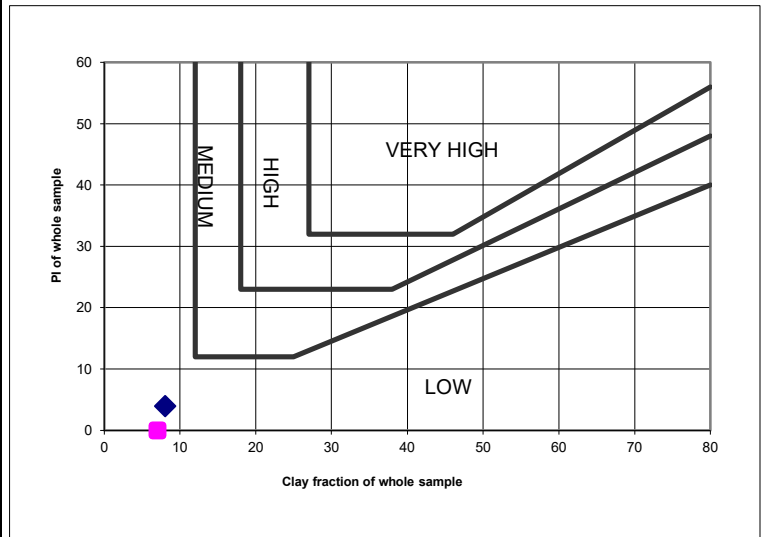
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

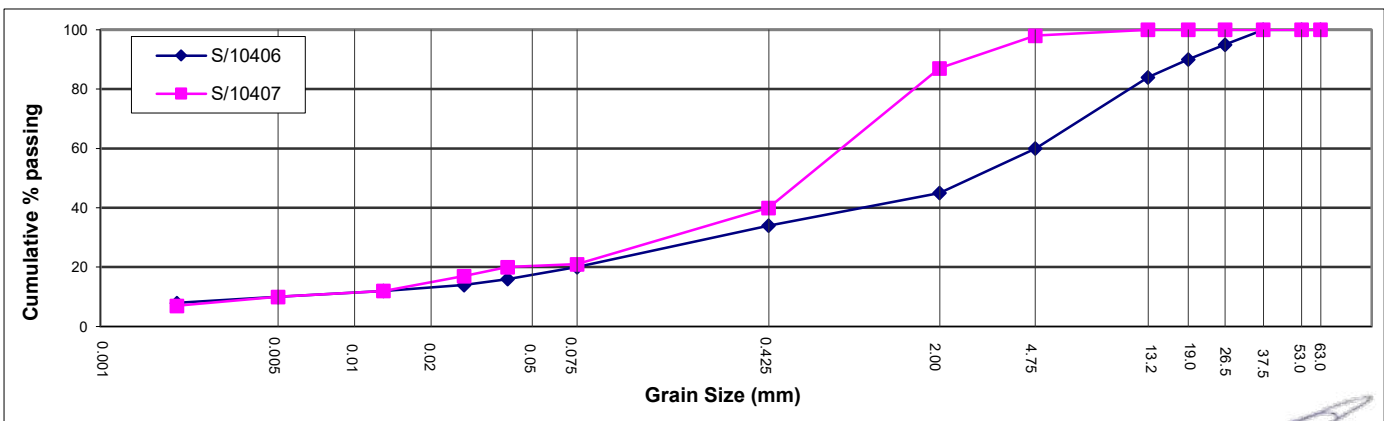
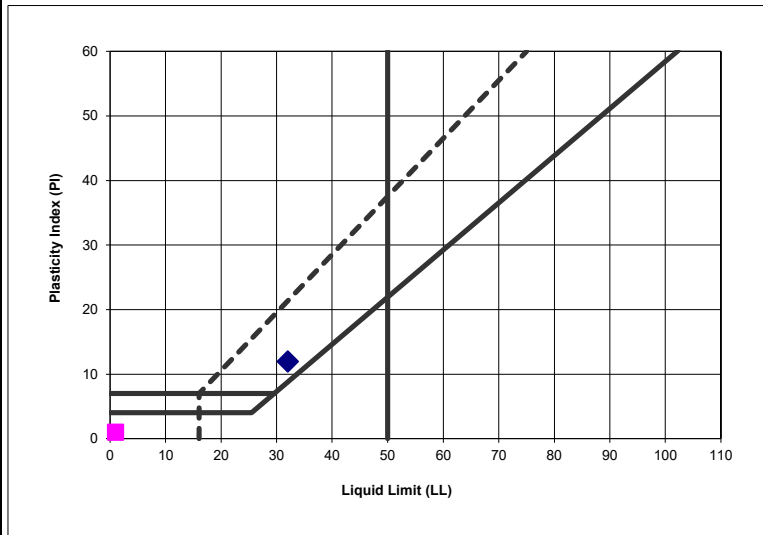
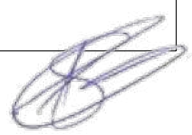
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10406	S/10407
Report No.	SG23/1612	
Depth (mm)	900-1100	1200-1500
Position	AM-TP02	AM-TP04
Material description	Clayey sand with gravel	Silty sand
Moisture (%)		
SG	2.629	2.571
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm	100	
26.5 mm	95	
19.0 mm	90	
13.2 mm	84	100
4.75 mm	60	98
2.00 mm	45	87
0.425 mm	34	40
0.075 mm	20	21
Hydrometer Analysis (% passing)		
0.040 mm	16	20
0.027 mm	14	17
0.013 mm	12	12
0.005 mm	10	10
0.002 mm	8	7
% Clay (<0.002 mm)	8	7
% Silt (<0.075, >0.002 mm)	12	14
% Sand (<4.75, >0.075 mm)	40	77
% Gravel (> 4.75 mm)	40	2
Atterberg Limits		
Liquid Limit (-0.425)	32	
Plasticity Index (-0.425)	12	1
Linear Shrinkage (%)	6.0	0.0
PI of whole sample	4	0
Grading Modulus	2.01	1.52
TRB Classification	A-4 (1)	A-1-b (0)
Unified Classification	SC	SM

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

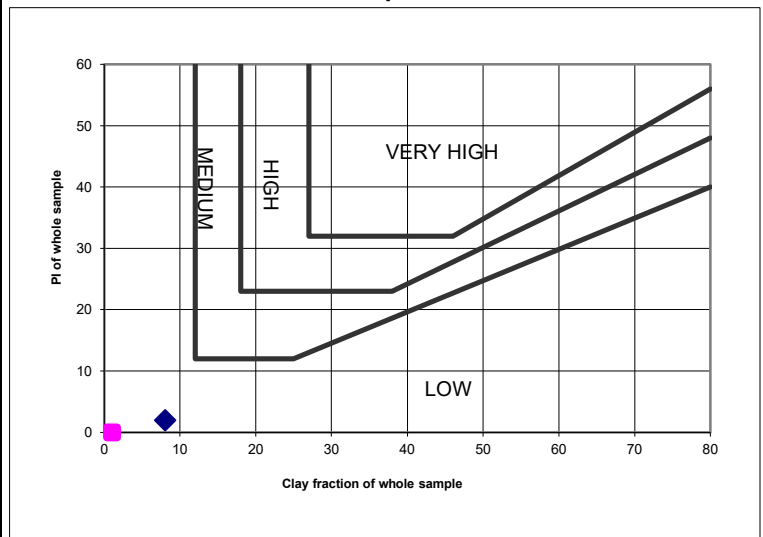
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

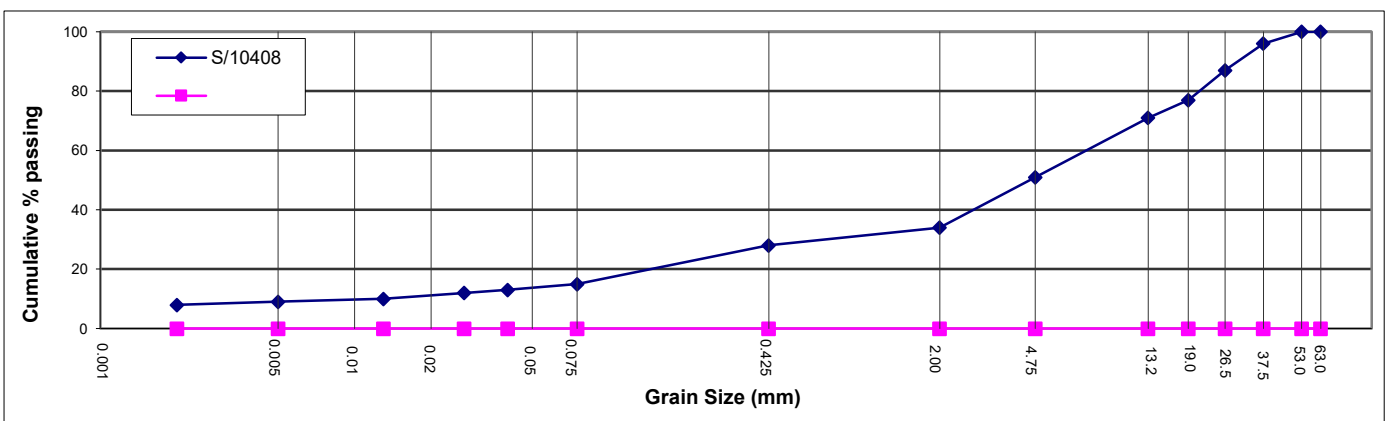
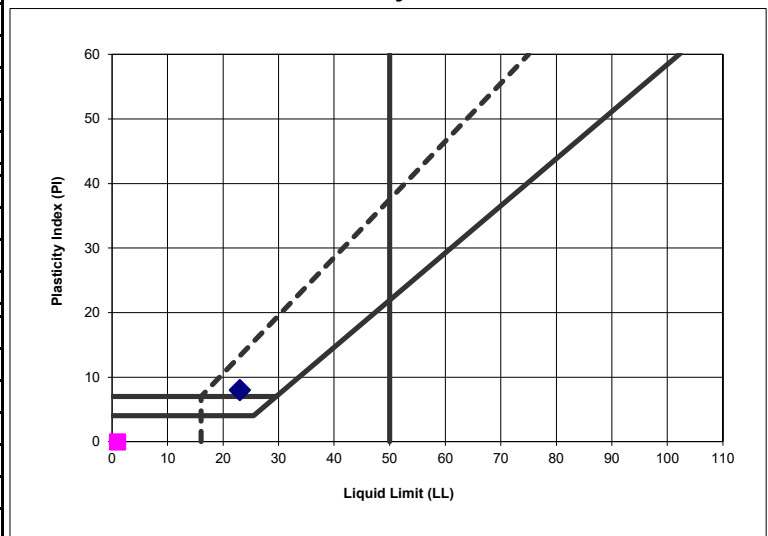
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10408
Report No.	SG23/1612
Depth (mm)	700-1000
Position	AM-TP03
Material description	Clayey gravel with sand
Moisture (%)	
SG	2.658
Sieve Analysis (% passing)	
63.0 mm	
53.0 mm	100
37.5 mm	96
26.5 mm	87
19.0 mm	77
13.2 mm	71
4.75 mm	51
2.00 mm	34
0.425 mm	28
0.075 mm	15
Hydrometer Analysis (% passing)	
0.040 mm	13
0.027 mm	12
0.013 mm	10
0.005 mm	9
0.002 mm	8
% Clay (<0.002 mm)	8
% Silt (<0.075, >0.002 mm)	7
% Sand (<4.75, >0.075 mm)	36
% Gravel (> 4.75 mm)	49
Atterberg Limits	
Liquid Limit (-0.425)	23
Plasticity Index (-0.425)	8
Linear Shrinkage (%)	3.5
PI of whole sample	2
Grading Modulus	2.23
TRB Classification	A-2-4 (0)
Unified Classification	GC

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Heap Leach Pad (HLP)

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1616

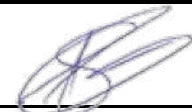
Page no : 1

sample detail	Sample no.	S/10409	S/10412	S/10414	S/10418	S/10419	S/10421
	Chainage						
	Position (from LHS)	HLP-TP03	HLP-TP08	HLP-TP24	HLP-TP13	HLP-TP24	HLP-TP20
	Depth sampled (mm)	800-2300	650-1350	1200-1750	300-2250	800-1200	1600-1800
	Sample description	Yellowish brown to khaki, silty sandy gravel, calcified coarse alluvium	Brown, clayey sandy silt. Fine Alluvium	Brown, sandy silty gravel. Coarse Alluvium.	Light brown to brown, gravelly silty fine sand. Calcareous Alluvium.	Brown, gravelly sandy silt. Calcareous Alluvium.	Completely weathered, white speckled to streaked black stained khaki to orange, very soft rock, gneiss
Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping	
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Compaction (%)							
sieve analysis (% passing)	63,0 mm	89				100	
	53,0 mm	66		100		88	
	37,5 mm	66		83		85	
	26,5 mm	57		73		80	
	19,0 mm	48	100	68		79	100
	13,2 mm	45	98	63		79	97
	4,75 mm	38	98	50	100	76	77
	2,00 mm	33	95	43	99	74	60
	0,425 mm	23	77	28	74	53	25
	0,075 mm	13	60	9	27	19	11
Grading Modulus (GM)	2.31	0.68	2.20	1.00	1.54	2.04	
Atterberg Limits	Liquid Limit (LL)	27	56				28
	Plasticity Index (PI)	12	22	1	1	1	8
	Linear Shrinkage (LS)	6.0	10.0				2.5
Mod AASHTO & CBR values	CBR @ 100 %	55	4	191	81		
	CBR @ 98 %	45	<3	107	60		
	CBR @ 95 %	33	<3	45	38		
	CBR @ 93 %	27	<3	25	28		
	CBR @ 90 %	20	<3	11	18		
	Swell @ 100 %	0.0	0.1	0.1	0.0		
	Mod AASHTO (kg/m ³)	1858	1517	2138	2054		
	O.M.C. (%)	13.7	23.3	6.5	7.3		
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture		90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC		93.0% Proctor @ OMC	
	Permeability cm/s		9.26 E-07	2.19 E-04		1.51 E-04	
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Heap Leach Pad (HLP)

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1616

Page no : 2

sample detail	Sample no.	S/10419				
	Chainage					
	Position (from LHS)	HLP-TP24				
	Depth sampled (mm)	800-1200				
	Sample description	Brown, gravelly sandy silt. Calcareous Alluvium.				
Method of Preparation used						
field density	Testing depth (mm)					
	Dry density (kg/m ³)					
	Moisture (%)					
	Mod AASHTO (kg/m ³)					
	O.M.C. (%)					
Compaction (%)						
sieve analysis (% passing)	63,0 mm	100				
	53,0 mm	88				
	37,5 mm	85				
	26,5 mm	80				
	19,0 mm	79				
	13,2 mm	79				
	4,75 mm	76				
	2,00 mm	74				
	0,425 mm	53				
	0,075 mm	19				
Grading Modulus (GM)	1.54					
Atterberg Limits	Liquid Limit (LL)					
	Plasticity Index (PI)	1				
	Linear Shrinkage (LS)	0.0				
Mod AASHTO & CBR values	CBR @ 100 %					
	CBR @ 98 %					
	CBR @ 95 %					
	CBR @ 93 %					
	CBR @ 90 %					
	Swell @ 100 %					
	Mod AASHTO (kg/m ³)					
O.M.C. (%)						
Proctor	Proctor MDD (kg/m ³)	2085				
	O.M.C. (%)	6.7				
Chem	pH					
	Conductivity (Sm ⁻¹)					

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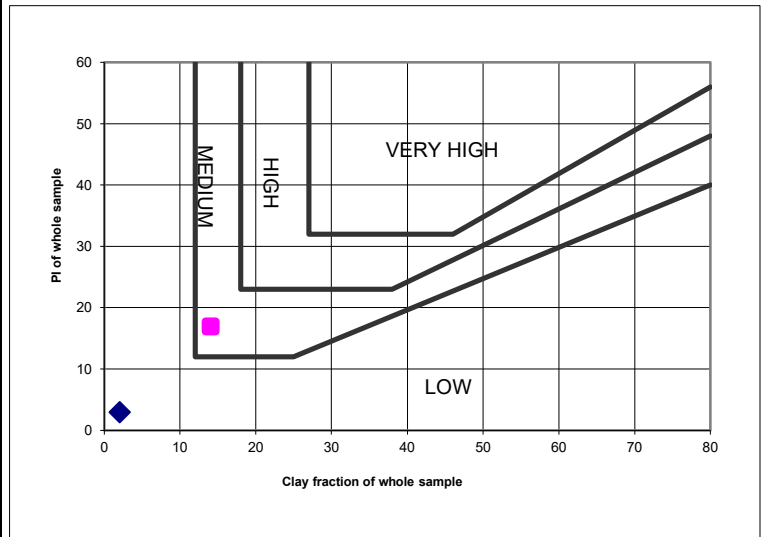
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

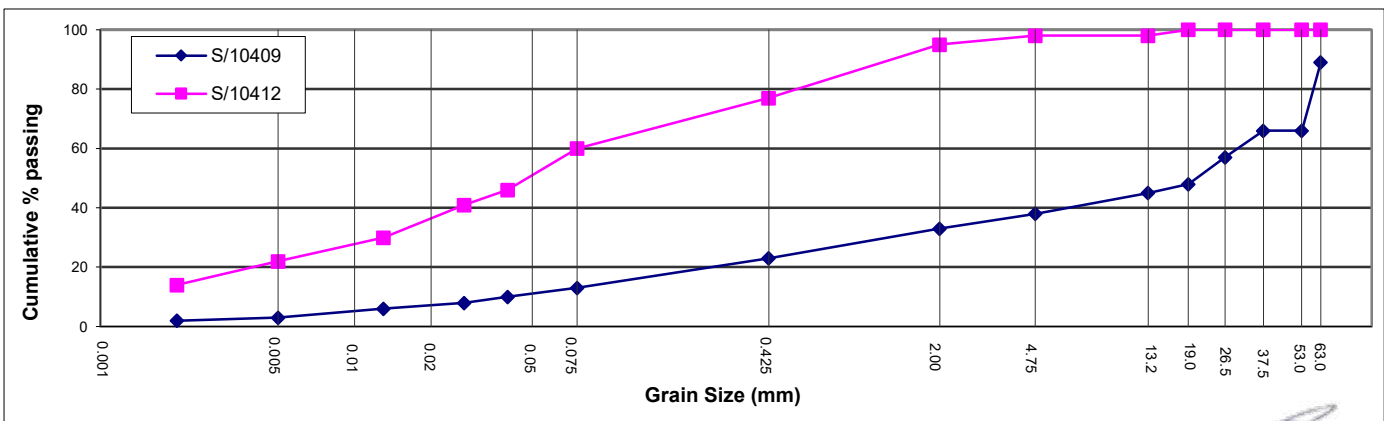
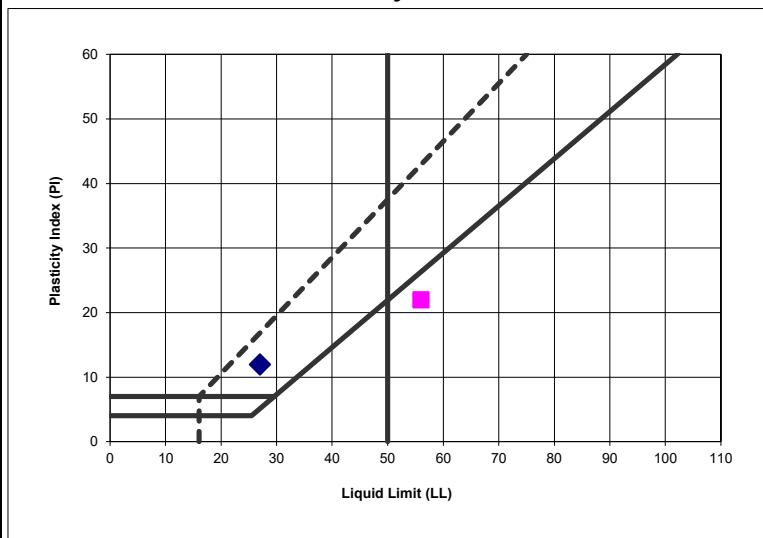
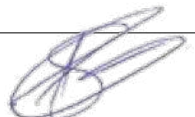
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10409	S/10412
Report No.	SG23/1616	
Depth (mm)	800-2300	650-1350
Position	HLP-TP03	HLP-TP08
Material description	Clayey gravel with sand	Sandy elastic silt
Moisture (%)		
SG	2.608	2.492
Sieve Analysis (% passing)		
63.0 mm	89	
53.0 mm	66	
37.5 mm	66	
26.5 mm	57	
19.0 mm	48	100
13.2 mm	45	98
4.75 mm	38	98
2.00 mm	33	95
0.425 mm	23	77
0.075 mm	13	60
Hydrometer Analysis (% passing)		
0.040 mm	10	46
0.027 mm	8	41
0.013 mm	6	30
0.005 mm	3	22
0.002 mm	2	14
% Clay (<0.002 mm)	2	14
% Silt (<0.075, >0.002 mm)	11	46
% Sand (<4.75, >0.075 mm)	25	38
% Gravel (> 4.75 mm)	62	2
Atterberg Limits		
Liquid Limit (-0.425)	27	56
Plasticity Index (-0.425)	12	22
Linear Shrinkage (%)	6.0	10.0
PI of whole sample	3	17
Grading Modulus	2.31	0.68
TRB Classification	A-2-6 (0)	A-7-5 (12)
Unified Classification	GC	MH

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

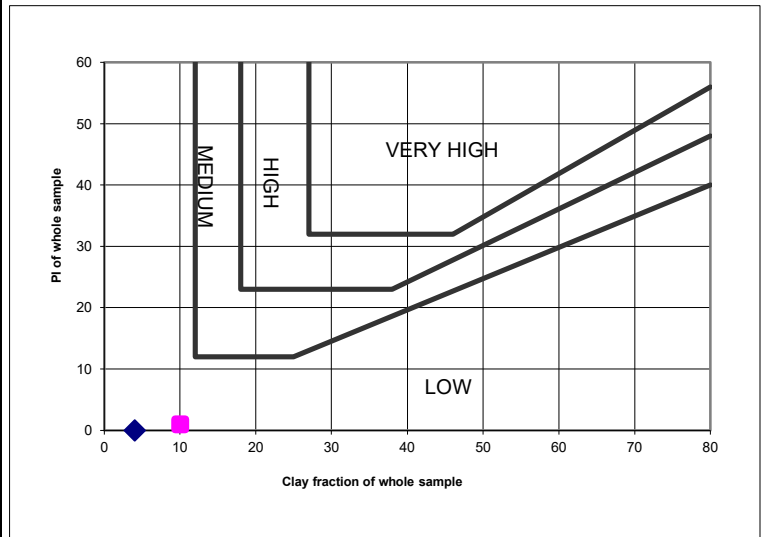
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

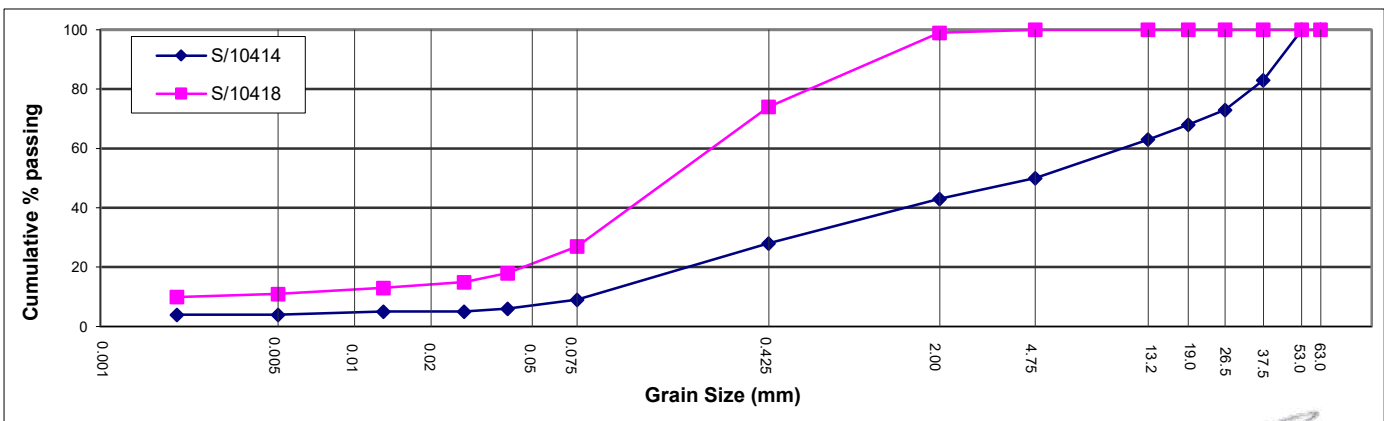
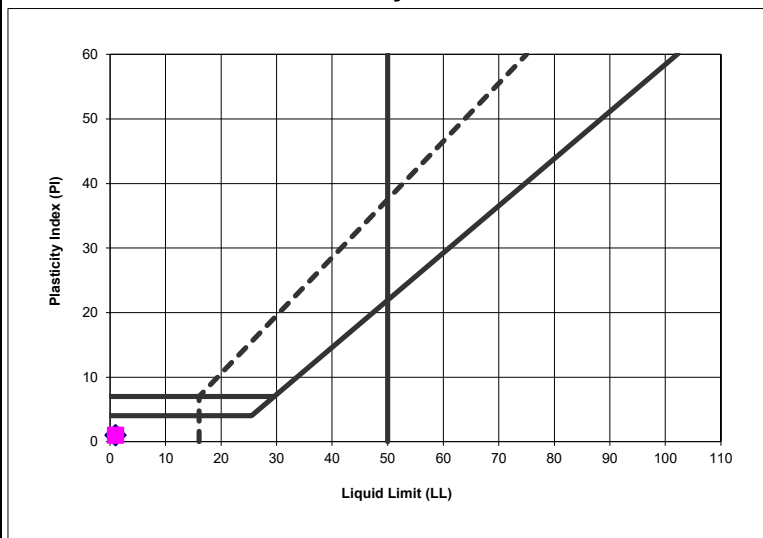
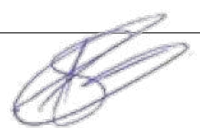
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10414	S/10418
Report No.	SG23/1616	
Depth (mm)	1200-1750	300-2250
Position	HLP-TP24	HLP-TP13
Material description	Poorly graded gravel with silt and sand	Silty sand
Moisture (%)		
SG	2.564	2.620
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm	100	
37.5 mm	83	
26.5 mm	73	
19.0 mm	68	
13.2 mm	63	
4.75 mm	50	100
2.00 mm	43	99
0.425 mm	28	74
0.075 mm	9	27
Hydrometer Analysis (% passing)		
0.040 mm	6	18
0.027 mm	5	15
0.013 mm	5	13
0.005 mm	4	11
0.002 mm	4	10
% Clay (<0.002 mm)	4	10
% Silt (<0.075, >0.002 mm)	5	17
% Sand (<4.75, >0.075 mm)	41	73
% Gravel (> 4.75 mm)	50	0
Atterberg Limits		
Liquid Limit (-0.425)		
Plasticity Index (-0.425)	1	1
Linear Shrinkage (%)	0.0	0.0
PI of whole sample	0	1
Grading Modulus	2.20	1.00
TRB Classification	A-1-a (0)	A-2-4 (0)
Unified Classification	GP-GM	SM

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

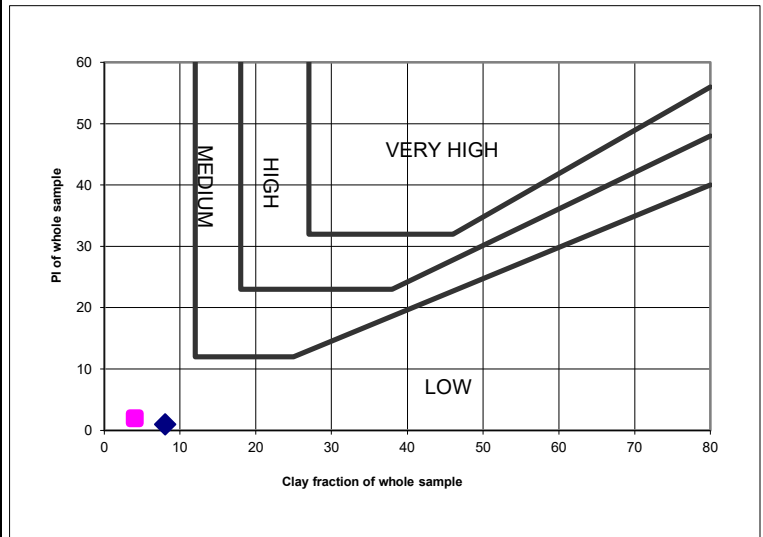
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

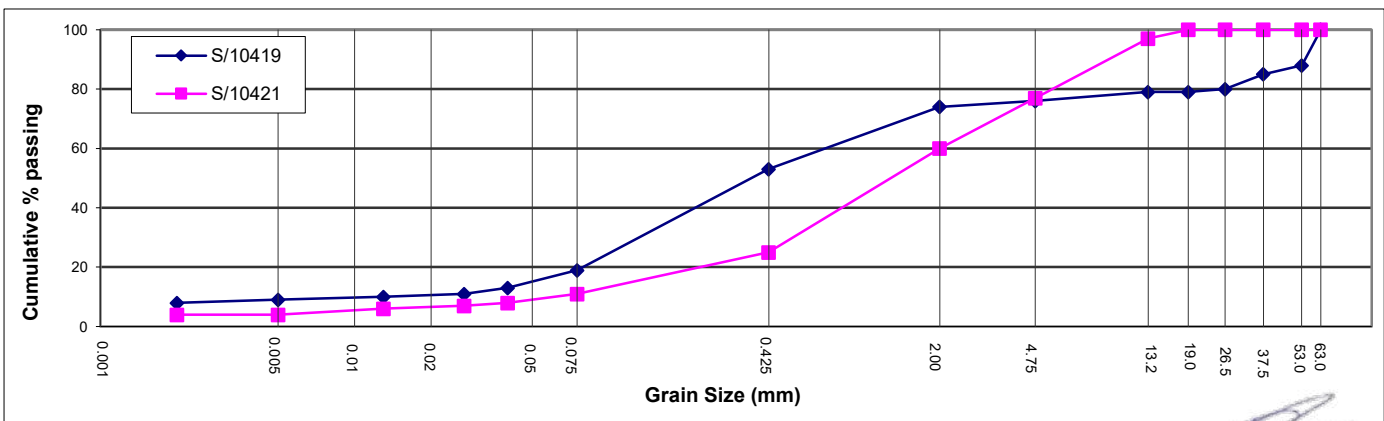
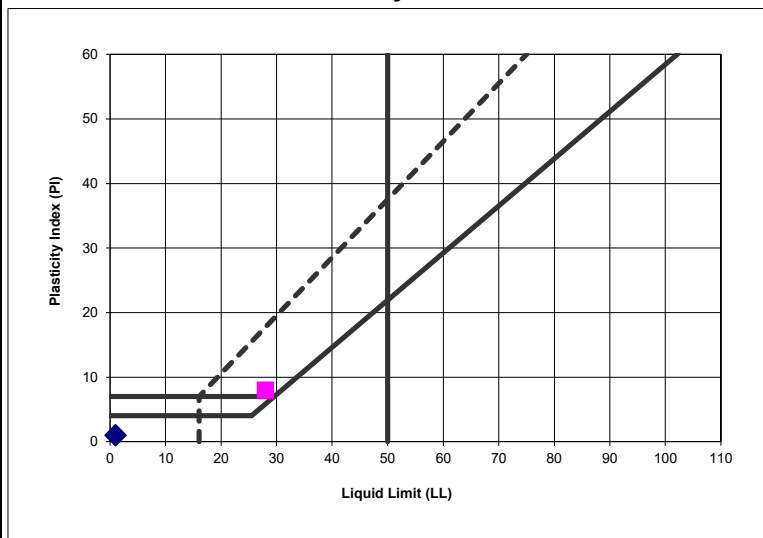
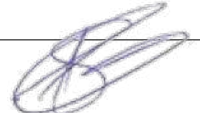
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10419	S/10421
Report No.	SG23/1616	
Depth (mm)	800-1200	1600-1800
Position	HLP-TP24	HLP-TP20
Material description	Silty sand with gravel	Well graded sand with clay and gravel
Moisture (%)		
SG	2.609	2.566
Sieve Analysis (% passing)		
63.0 mm	100	
53.0 mm	88	
37.5 mm	85	
26.5 mm	80	
19.0 mm	79	100
13.2 mm	79	97
4.75 mm	76	77
2.00 mm	74	60
0.425 mm	53	25
0.075 mm	19	11
Hydrometer Analysis (% passing)		
0.040 mm	13	8
0.027 mm	11	7
0.013 mm	10	6
0.005 mm	9	4
0.002 mm	8	4
% Clay (<0.002 mm)	8	4
% Silt (<0.075, >0.002 mm)	11	7
% Sand (<4.75, >0.075 mm)	57	66
% Gravel (> 4.75 mm)	24	23
Atterberg Limits		
Liquid Limit (-0.425)		28
Plasticity Index (-0.425)	1	8
Linear Shrinkage (%)	0.0	2.5
PI of whole sample	1	2
Grading Modulus	1.54	2.04
TRB Classification	A-2-4 (0)	A-1-b (0)
Unified Classification	SM	SW-SC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Heap Leach Pad (HLP)

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1616

Page no : 6

sample detail	Sample no.	S/10423	S/10424	S/10426			
	Chainage						
	Position (from LHS)	HLP-TP04	HL/PA-TP29	HLP-TP03			
	Depth sampled (mm)	1100-1350	1700-1900	2300-3100			
	Sample description	Light brown blotched white, sandy silty gravel. Nodular Calcrete.	Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker	Khaki to white streaked to banded black, silty sandy gravel to silty gravelly sand, calcified residual gneiss			
Method of Preparation used	Scalping	Scalping	Scalping				
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Compaction (%)							
sieve analysis (% passing)	63,0 mm	85	100	90			
	53,0 mm	64	92	70			
	37,5 mm	55	85	68			
	26,5 mm	51	76	63			
	19,0 mm	47	67	60			
	13,2 mm	45	61	58			
	4,75 mm	40	50	51			
	2,00 mm	35	45	46			
	0,425 mm	27	29	24			
	0,075 mm	16	14	11			
Grading Modulus (GM)	2.22	2.12	2.19				
Atterberg Limits	Liquid Limit (LL)	28	19	25			
	Plasticity Index (PI)	11	6	5			
	Linear Shrinkage (LS)	4.0	2.0	2.5			
Mod AASHTO & CBR values	CBR @ 100 %	43	87	44			
	CBR @ 98 %	29	64	33			
	CBR @ 95 %	16	40	21			
	CBR @ 93 %	11	29	16			
	CBR @ 90 %	6	18	10			
	Swell @ 100 %	0.0	0.0	0.1			
	Mod AASHTO (kg/m ³)	1994	2204	1974			
	O.M.C. (%)	10.7	6.3	11.0			
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture	90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC			
	Permeability cm/s	1.50 E-06	7.78 E-04	3.92 E-05			
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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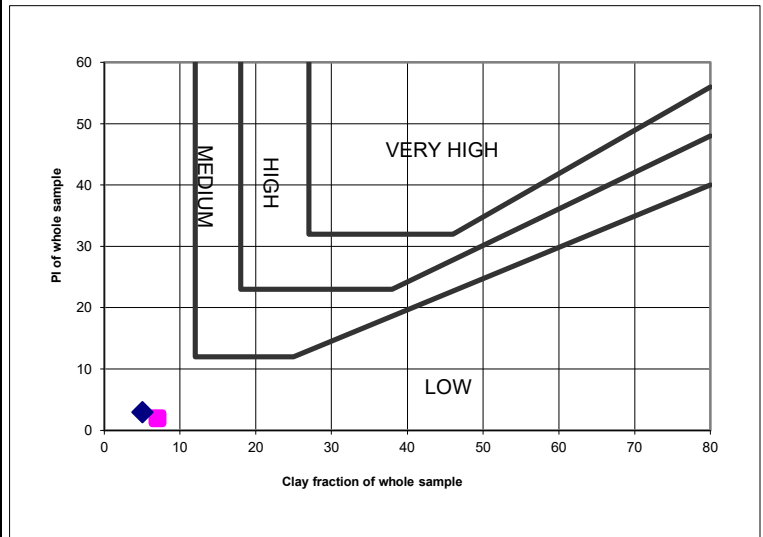
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

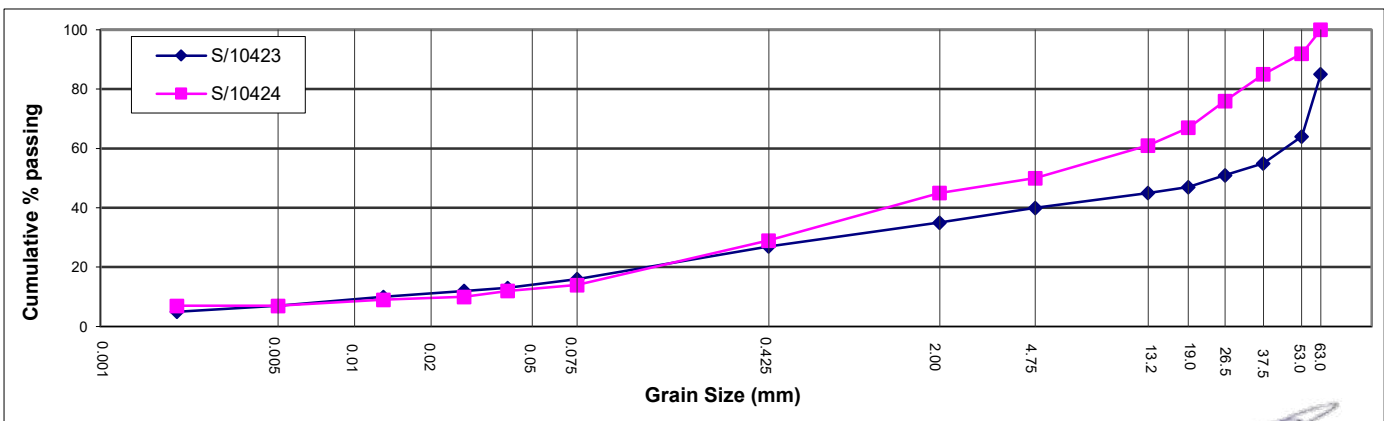
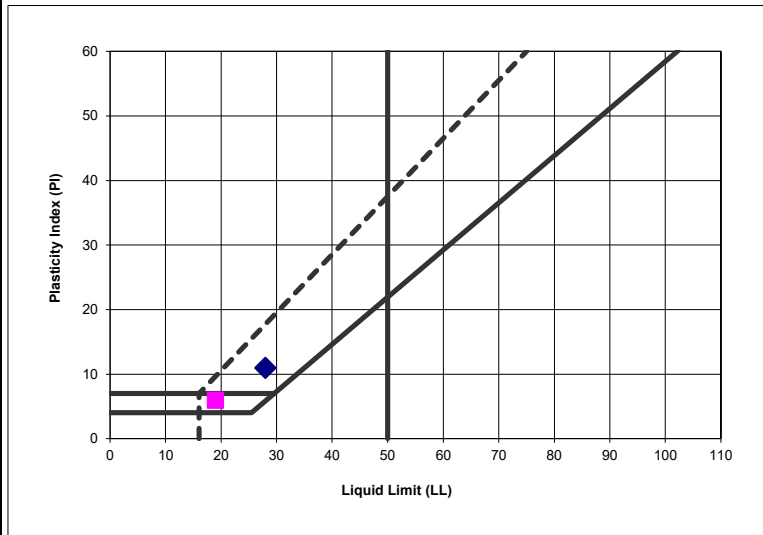
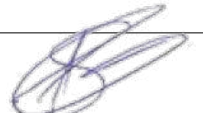
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10423	S/10424
Report No.	SG23/1616	
Depth (mm)	1100-1350	1700-1900
Position	HLP-TP04	HL/PA-TP29
Material description	Clayey gravel with sand	Silty, clayey gravel with sand
Moisture (%)		
SG	2.617	2.619
Sieve Analysis (% passing)		
63.0 mm	85	100
53.0 mm	64	92
37.5 mm	55	85
26.5 mm	51	76
19.0 mm	47	67
13.2 mm	45	61
4.75 mm	40	50
2.00 mm	35	45
0.425 mm	27	29
0.075 mm	16	14
Hydrometer Analysis (% passing)		
0.040 mm	13	12
0.027 mm	12	10
0.013 mm	10	9
0.005 mm	7	7
0.002 mm	5	7
% Clay (<0.002 mm)	5	7
% Silt (<0.075, >0.002 mm)	11	7
% Sand (<4.75, >0.075 mm)	24	36
% Gravel (> 4.75 mm)	60	50
Atterberg Limits		
Liquid Limit (-0.425)	28	19
Plasticity Index (-0.425)	11	6
Linear Shrinkage (%)	4.0	2.0
PI of whole sample	3	2
Grading Modulus	2.22	2.12
TRB Classification	A-2-6 (1)	A-1-a (0)
Unified Classification	GC	GC-GM

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

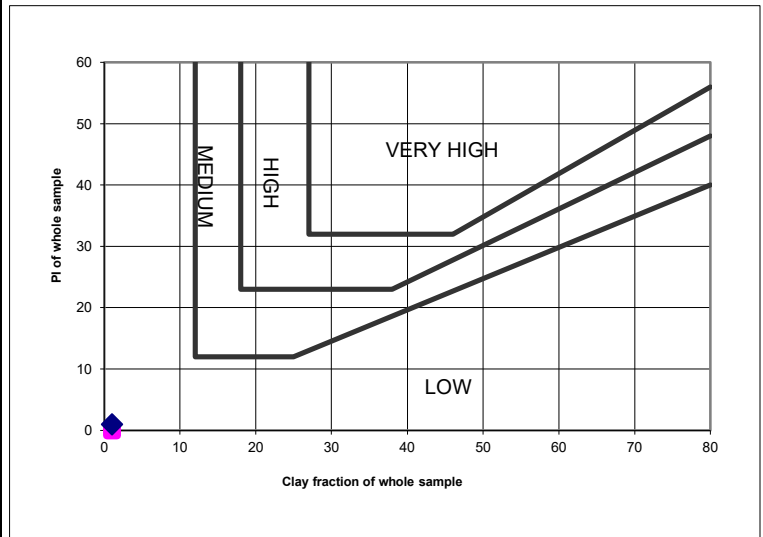
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

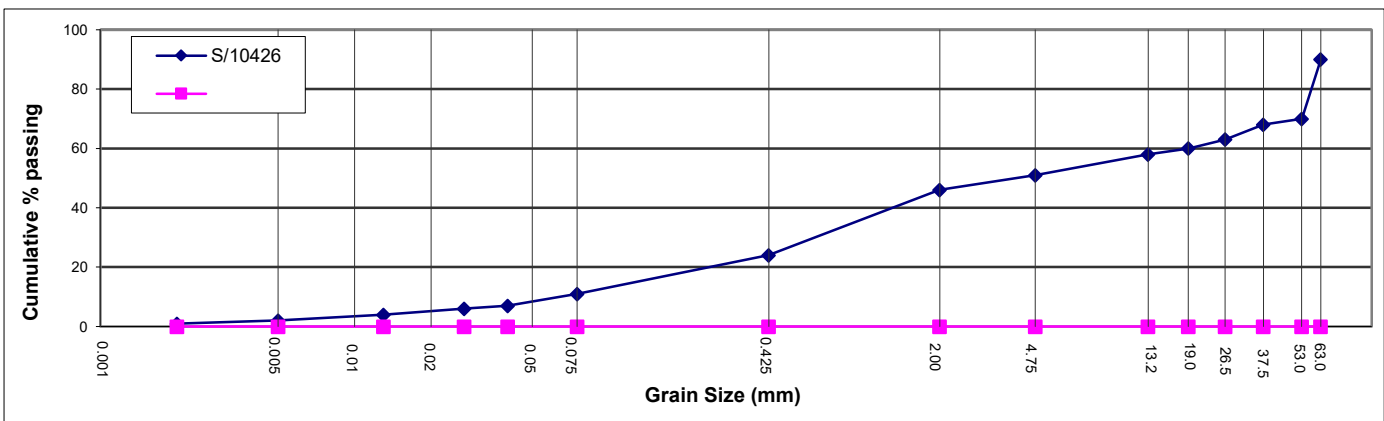
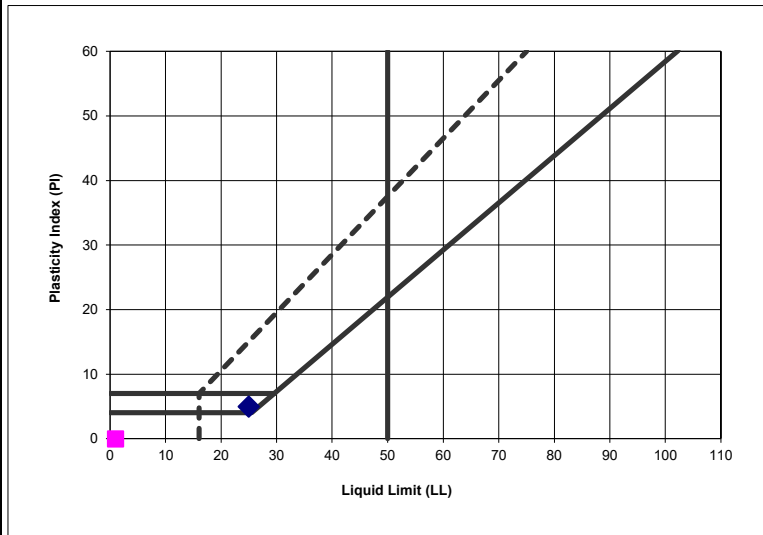
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10426
Report No.	SG23/1616
Depth (mm)	2300-3100
Position	HLP-TP03
Material description	Silty, clayey gravel with sand
Moisture (%)	
SG	2.601
Sieve Analysis (% passing)	
63.0 mm	90
53.0 mm	70
37.5 mm	68
26.5 mm	63
19.0 mm	60
13.2 mm	58
4.75 mm	51
2.00 mm	46
0.425 mm	24
0.075 mm	11
Hydrometer Analysis (% passing)	
0.040 mm	7
0.027 mm	6
0.013 mm	4
0.005 mm	2
0.002 mm	1
% Clay (<0.002 mm)	1
% Silt (<0.075, >0.002 mm)	10
% Sand (<4.75, >0.075 mm)	40
% Gravel (> 4.75 mm)	49
Atterberg Limits	
Liquid Limit (-0.425)	25
Plasticity Index (-0.425)	5
Linear Shrinkage (%)	2.5
PI of whole sample	1
Grading Modulus	2.19
TRB Classification	A-1-a (0)
Unified Classification	GC-GM

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

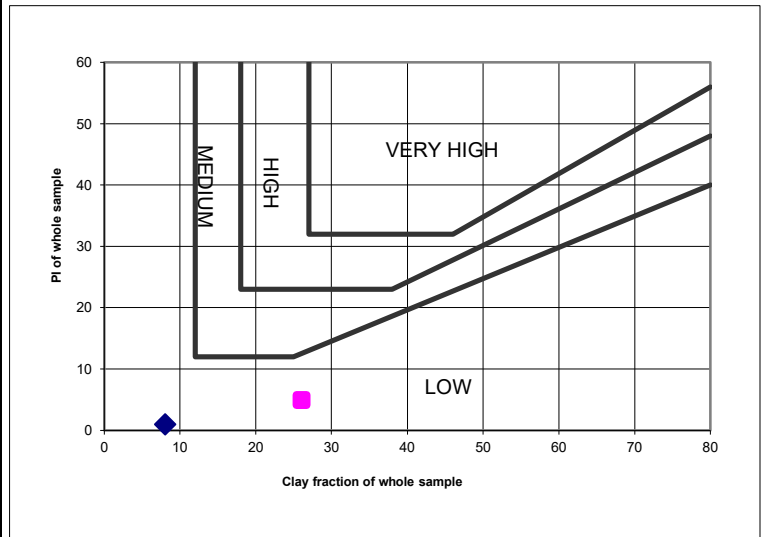
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

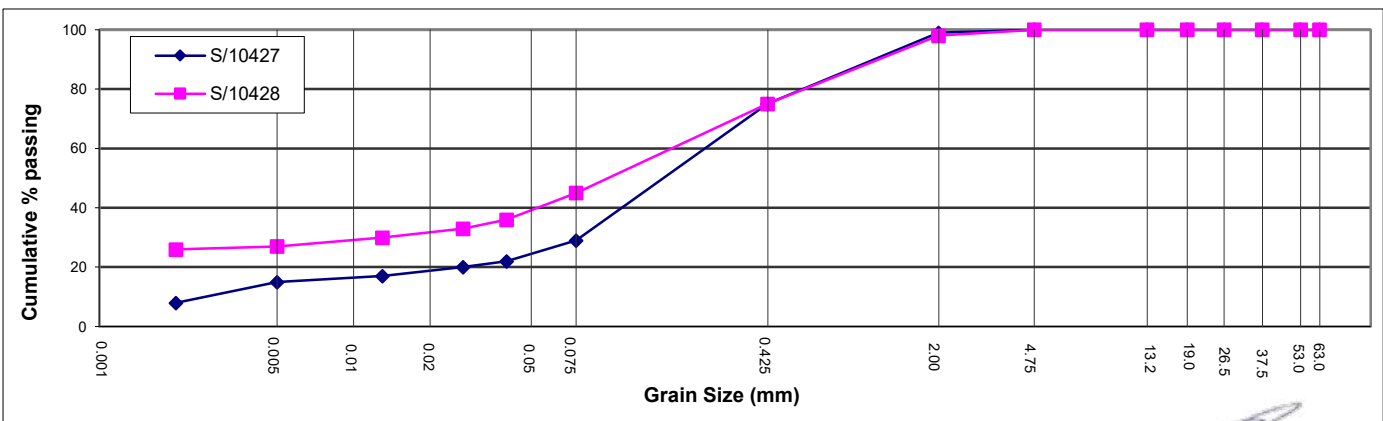
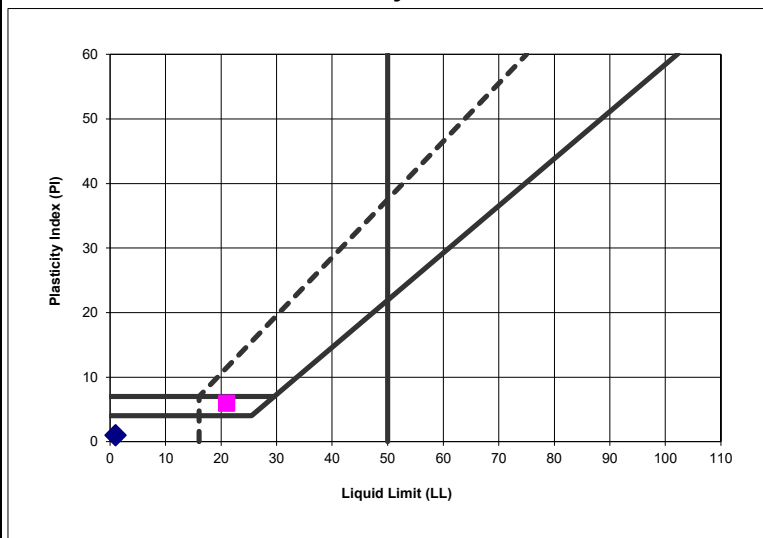
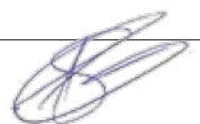
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10427	S/10428
Report No.	SG23/1617	
Depth (mm)	0-700	700-1000
Position	Plant-TP32	Plant-TP32
Material description	Silty sand	Silty clayey sand
Moisture (%)		
SG	2.609	2.515
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		
13.2 mm		100
4.75 mm	100	100
2.00 mm	99	98
0.425 mm	75	75
0.075 mm	29	45
Hydrometer Analysis (% passing)		
0.040 mm	22	36
0.027 mm	20	33
0.013 mm	17	30
0.005 mm	15	27
0.002 mm	8	26
% Clay (<0.002 mm)	8	26
% Silt (<0.075, >0.002 mm)	21	19
% Sand (<4.75, >0.075 mm)	71	55
% Gravel (> 4.75 mm)	0	0
Atterberg Limits		
Liquid Limit (-0.425)		21
Plasticity Index (-0.425)	1	6
Linear Shrinkage (%)	0.0	2.0
PI of whole sample	1	5
Grading Modulus	0.97	0.82
TRB Classification	A-2-4 (0)	A-4 (2)
Unified Classification	SM	SC-SM

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

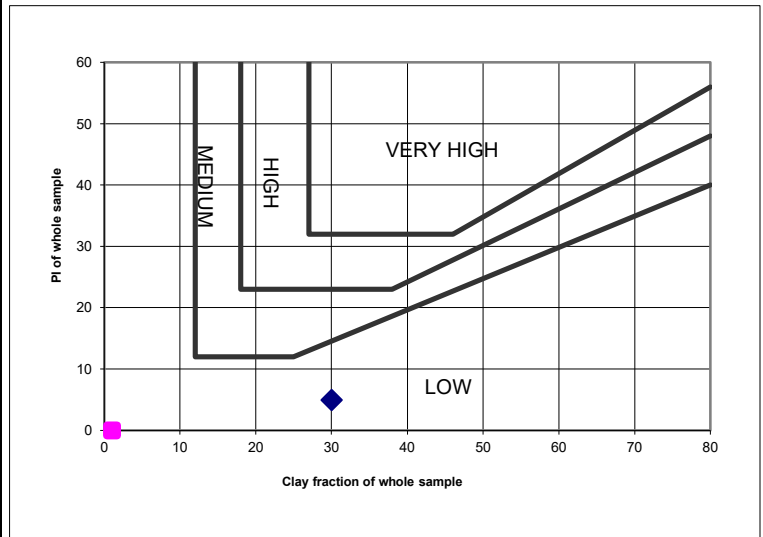
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

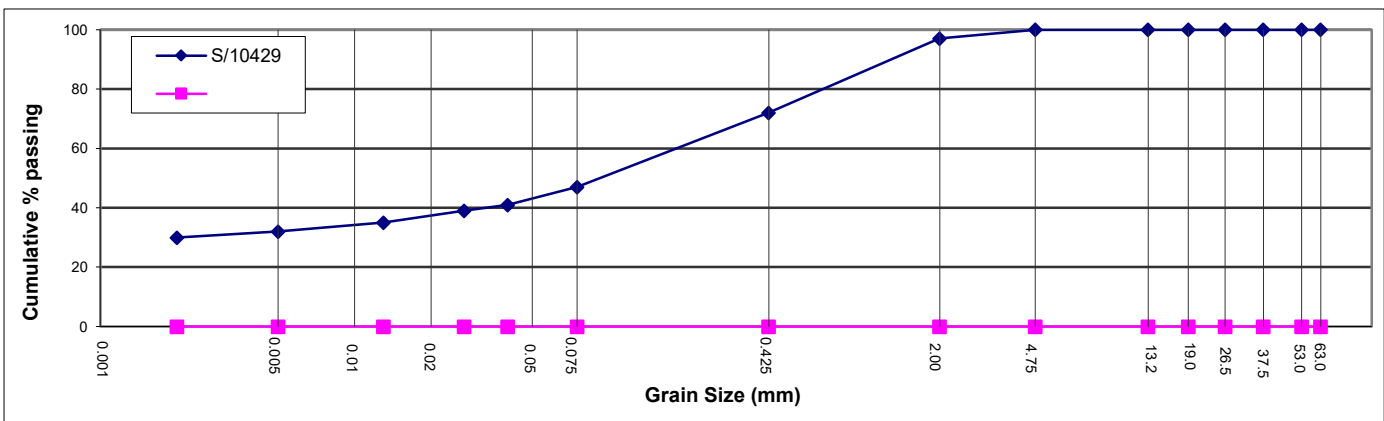
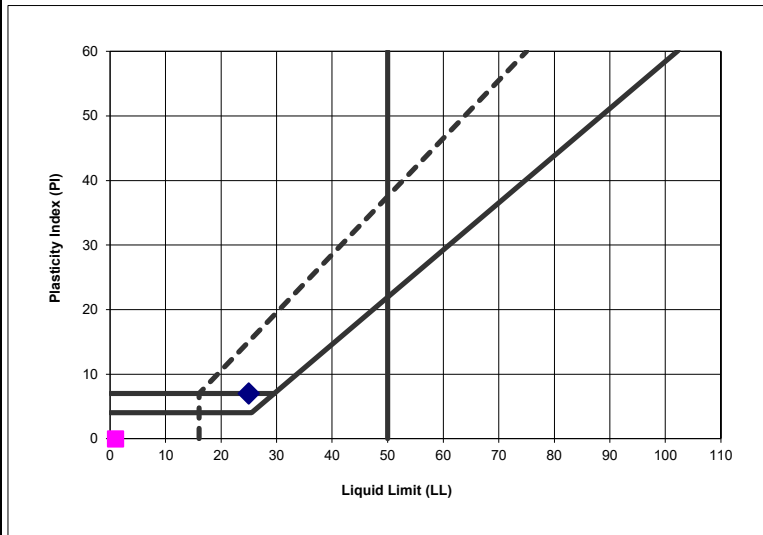
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10429
Report No.	SG23/1617
Depth (mm)	1000-1700
Position	Plant-TP32
Material description	Clayey sand
Moisture (%)	
SG	2.538
Sieve Analysis (% passing)	
63.0 mm	
53.0 mm	
37.5 mm	
26.5 mm	
19.0 mm	
13.2 mm	100
4.75 mm	100
2.00 mm	97
0.425 mm	72
0.075 mm	47
Hydrometer Analysis (% passing)	
0.040 mm	41
0.027 mm	39
0.013 mm	35
0.005 mm	32
0.002 mm	30
% Clay (<0.002 mm)	30
% Silt (<0.075, >0.002 mm)	17
% Sand (<4.75, >0.075 mm)	53
% Gravel (> 4.75 mm)	0
Atterberg Limits	
Liquid Limit (-0.425)	25
Plasticity Index (-0.425)	7
Linear Shrinkage (%)	2.5
PI of whole sample	5
Grading Modulus	0.84
TRB Classification	A-4 (3)
Unified Classification	SC

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : New Road

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1618

Page no : 1

sample detail	Sample no.	S/10432	S/10437	S/10443	S/10450	S/10452	S/10454
	Chainage						
	Position (from LHS)	RD-TP02	RD-TP04	RD-TP12	RD-TP08	RD-TP02	RD-TP14
	Depth sampled (mm)	1200-2800	600-2900	1100-4300	1000-3200	3900-4300	1400-2600
	Sample description	Brown to greyish brown, silty clayey sand with minor cobbles to boulders, coarse alluvium	Reddish brown, clayey silty coarse sand with traces of gravel, fine alluvium	Reddish brown slightly stained brown, clayey silty gravelly sand with minor fine gravel, fine alluvium	White to yellowish brown, silty gravelly sand, calcified alluvium	Completely weathered, white streaked black and gold, very soft rock, biotite gneiss	Completely weathered, dark grey streaked white stained orange, very soft rock, ferruginised biotite gneiss
Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping	
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm	89				100	
	53,0 mm	89				85	100
	37,5 mm	86				81	91
	26,5 mm	82				73	86
	19,0 mm	79			100	72	81
	13,2 mm	79			98	71	75
	4,75 mm	73	100	100	92	69	61
	2,00 mm	58	99	97	88	62	52
	0,425 mm	21	84	77	71	26	40
	0,075 mm	13	34	30	46	16	25
Grading Modulus (GM)	2.08	0.83	0.96	0.95	1.96	1.83	
Atterberg Limits	Liquid Limit (LL)	28			48	29	34
	Plasticity Index (PI)	11	1	1	20	7	9
	Linear Shrinkage (LS)	3.5			7.5	2.5	3.5
Mod AASHTO & CBR values	CBR @ 100 %		118	60	5	29	23
	CBR @ 98 %		81	46	5	20	20
	CBR @ 95 %		46	30	4	11	17
	CBR @ 93 %		32	23	4	8	15
	CBR @ 90 %		18	16	3	5	12
	Swell @ 100 %		0.1	-0.1	0.0	0.0	0.0
	Mod AASHTO (kg/m ³)		2041	2119	1640	1940	2016
	O.M.C. (%)		8.1	7.4	20.0	9.2	12.9
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

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SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : New Road

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1618

Page no : 2

sample detail	Sample no.	S/10455					
	Chainage						
	Position (from LHS)	RD-TP13					
	Depth sampled (mm)	2600-4000					
	Sample description	Yellowish brown with scattered red, white and black mottles, clayey gravelly silty fine sand with minor gravels, slightly					
	Method of Preparation used	Scalping					
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm						
	53,0 mm						
	37,5 mm						
	26,5 mm						
	19,0 mm	100					
	13,2 mm	99					
	4,75 mm	95					
	2,00 mm	90					
	0,425 mm	77					
	0,075 mm	38					
	Grading Modulus (GM)	0.95					
Atterberg Limits	Liquid Limit (LL)	27					
	Plasticity Index (PI)	9					
	Linear Shrinkage (LS)	4.5					
Mod AASHTO & CBR values	CBR @ 100 %						
	CBR @ 98 %						
	CBR @ 95 %						
	CBR @ 93 %						
	CBR @ 90 %						
	Swell @ 100 %						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

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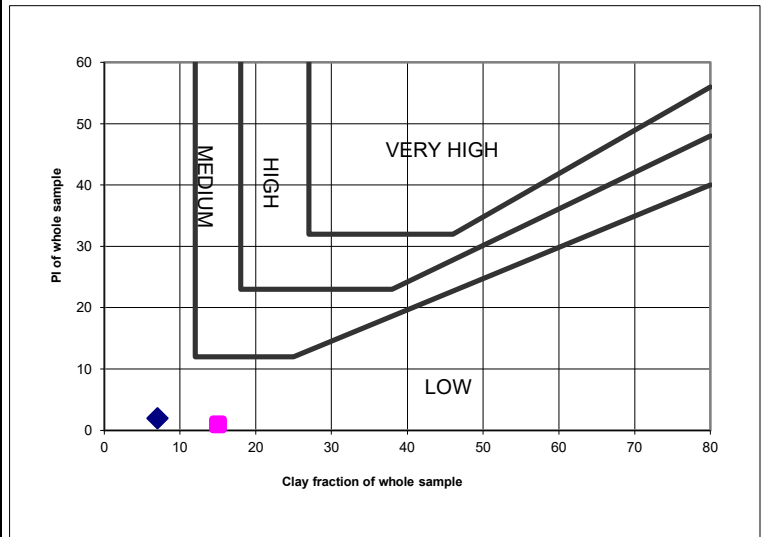
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

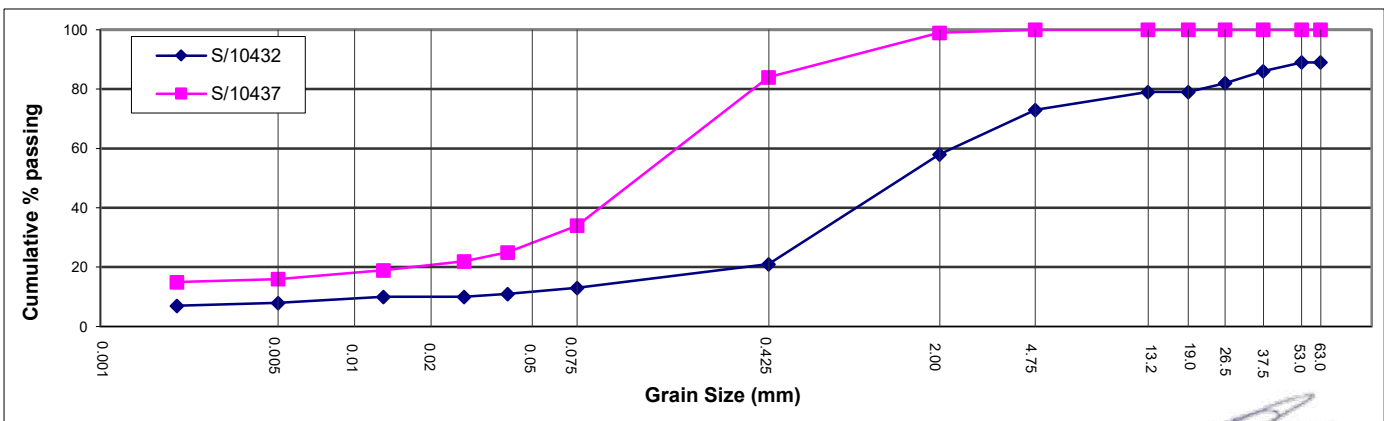
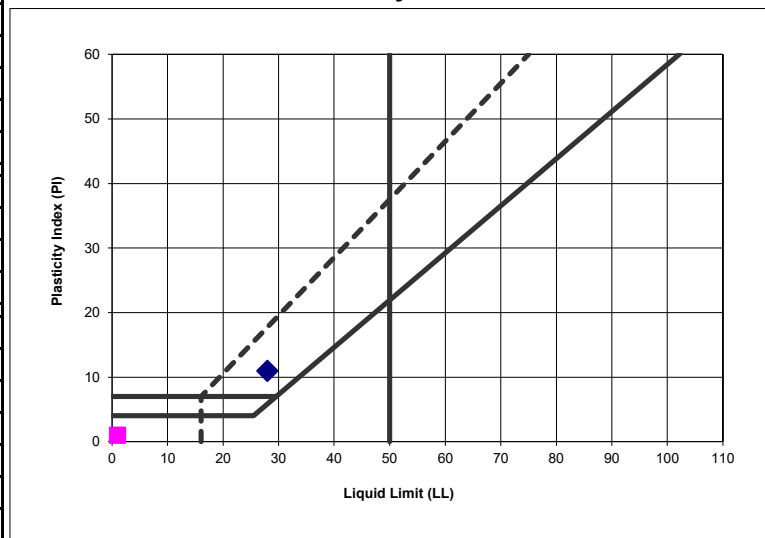
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10432	S/10437
Report No.	SG23/1618	
Depth (mm)	1200-2800	600-2900
Position	RD-TP02	RD-TP04
Material description	Clayey sand with gravel	Silty sand
Moisture (%)		
SG	2.609	2.637
Sieve Analysis (% passing)		
63.0 mm	89	
53.0 mm	89	
37.5 mm	86	
26.5 mm	82	
19.0 mm	79	
13.2 mm	79	
4.75 mm	73	100
2.00 mm	58	99
0.425 mm	21	84
0.075 mm	13	34
Hydrometer Analysis (% passing)		
0.040 mm	11	25
0.027 mm	10	22
0.013 mm	10	19
0.005 mm	8	16
0.002 mm	7	15
% Clay (<0.002 mm)	7	15
% Silt (<0.075, >0.002 mm)	6	19
% Sand (<4.75, >0.075 mm)	60	66
% Gravel (> 4.75 mm)	27	0
Atterberg Limits		
Liquid Limit (-0.425)	28	
Plasticity Index (-0.425)	11	1
Linear Shrinkage (%)	3.5	0.0
PI of whole sample	2	1
Grading Modulus	2.08	0.83
TRB Classification	A-2-6 (0)	A-2-4 (0)
Unified Classification	SC	SM

Potential Expansiveness



Plasticity Chart




Namibia Technical Services cc

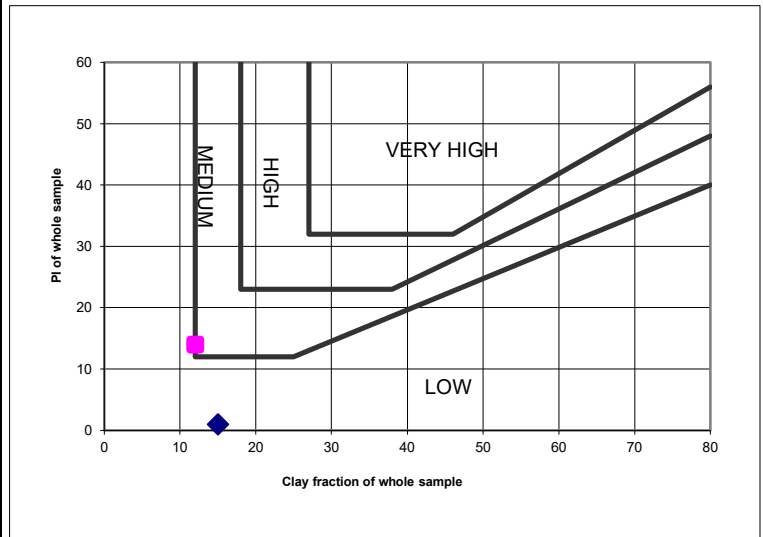
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

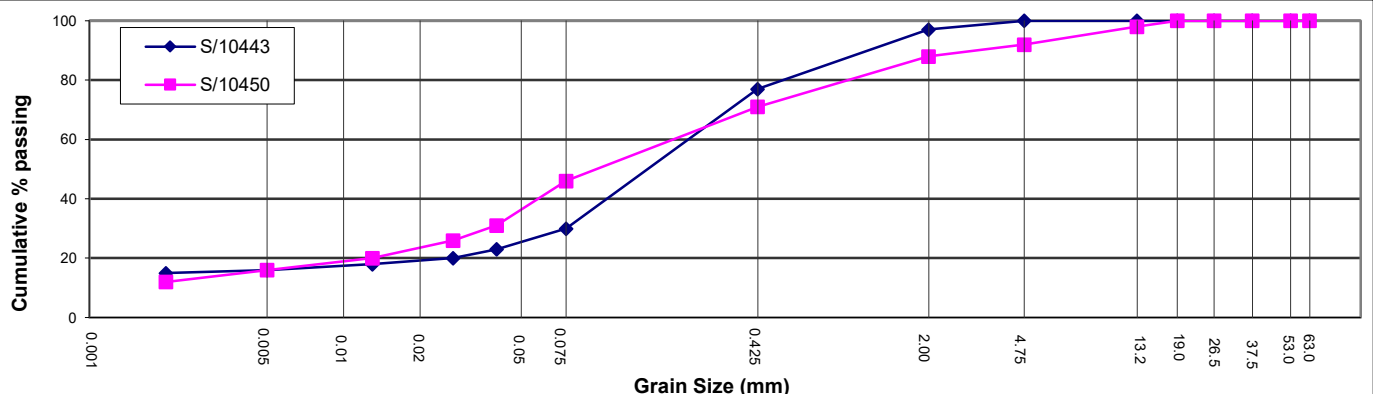
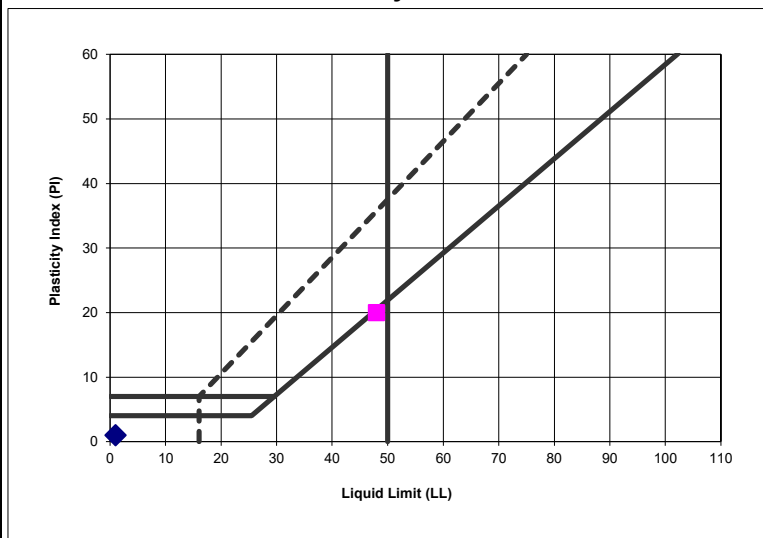
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10443	S/10450
Report No.	SG23/1618	
Depth (mm)	1100-4300	1000-3200
Position	RD-TP12	RD-TP08
Material description	Silty sand	
Moisture (%)		
SG	2.648	2.411
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		100
13.2 mm		98
4.75 mm	100	92
2.00 mm	97	88
0.425 mm	77	71
0.075 mm	30	46
Hydrometer Analysis (% passing)		
0.040 mm	23	31
0.027 mm	20	26
0.013 mm	18	20
0.005 mm	16	16
0.002 mm	15	12
% Clay (<0.002 mm)	15	12
% Silt (<0.075, >0.002 mm)	15	34
% Sand (<4.75, >0.075 mm)	70	46
% Gravel (> 4.75 mm)	0	8
Atterberg Limits		
Liquid Limit (-0.425)		48
Plasticity Index (-0.425)	1	20
Linear Shrinkage (%)	0.0	7.5
PI of whole sample	1	14
Grading Modulus	0.96	0.95
TRB Classification	A-2-4 (0)	A-7-5 (5)
Unified Classification	SM	SM

Potential Expansiveness



Plasticity Chart




Namibia Technical Services cc

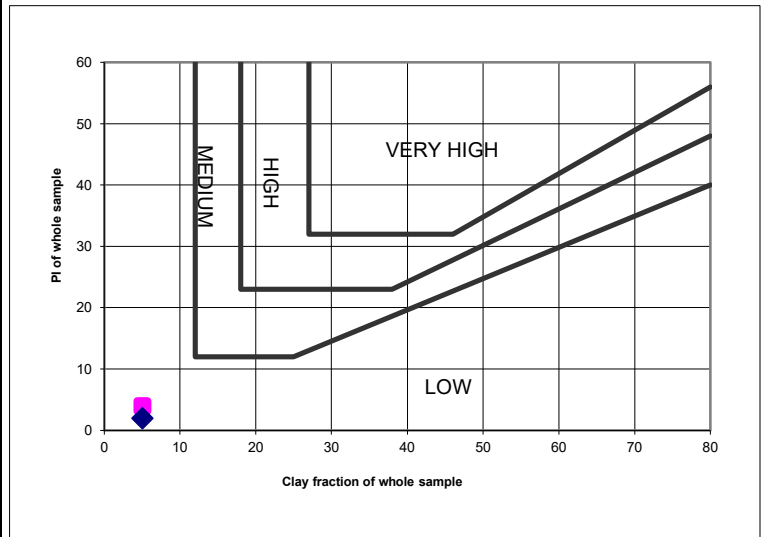
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

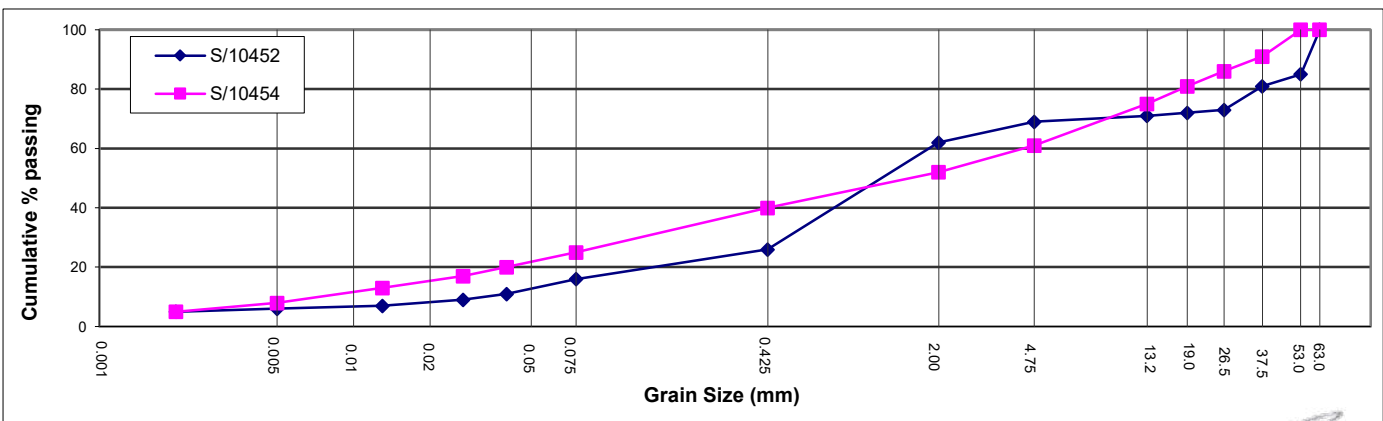
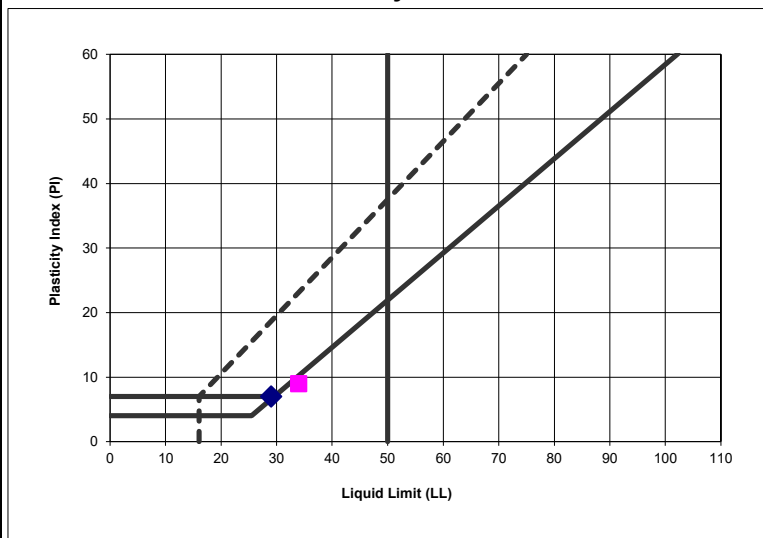
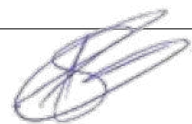
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10452	S/10454
Report No.	SG23/1618	
Depth (mm)	3900-4300	1400-2600
Position	RD-TP02	RD-TP14
Material description	Clayey sand with gravel	Silty gravel with sand
Moisture (%)		
SG	2.673	2.604
Sieve Analysis (% passing)		
63.0 mm	100	
53.0 mm	85	100
37.5 mm	81	91
26.5 mm	73	86
19.0 mm	72	81
13.2 mm	71	75
4.75 mm	69	61
2.00 mm	62	52
0.425 mm	26	40
0.075 mm	16	25
Hydrometer Analysis (% passing)		
0.040 mm	11	20
0.027 mm	9	17
0.013 mm	7	13
0.005 mm	6	8
0.002 mm	5	5
% Clay (<0.002 mm)	5	5
% Silt (<0.075, >0.002 mm)	11	20
% Sand (<4.75, >0.075 mm)	53	36
% Gravel (> 4.75 mm)	31	39
Atterberg Limits		
Liquid Limit (-0.425)	29	34
Plasticity Index (-0.425)	7	9
Linear Shrinkage (%)	2.5	3.5
PI of whole sample	2	4
Grading Modulus	1.96	1.83
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	SC	GM

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

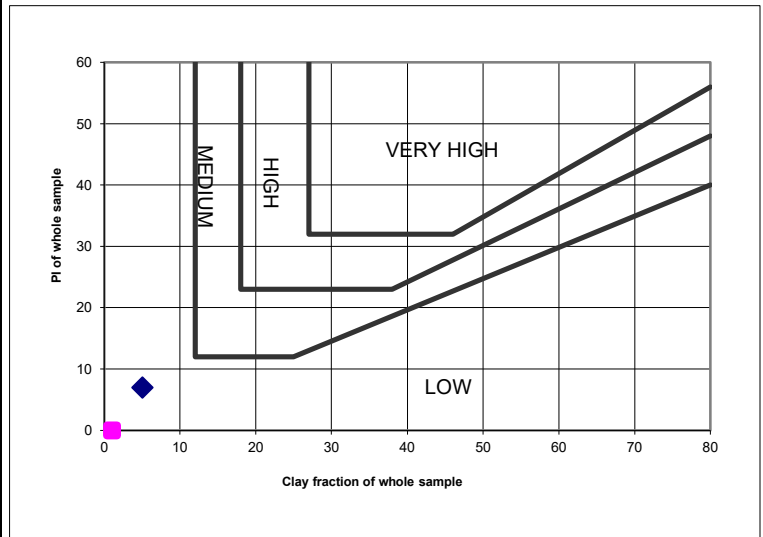
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

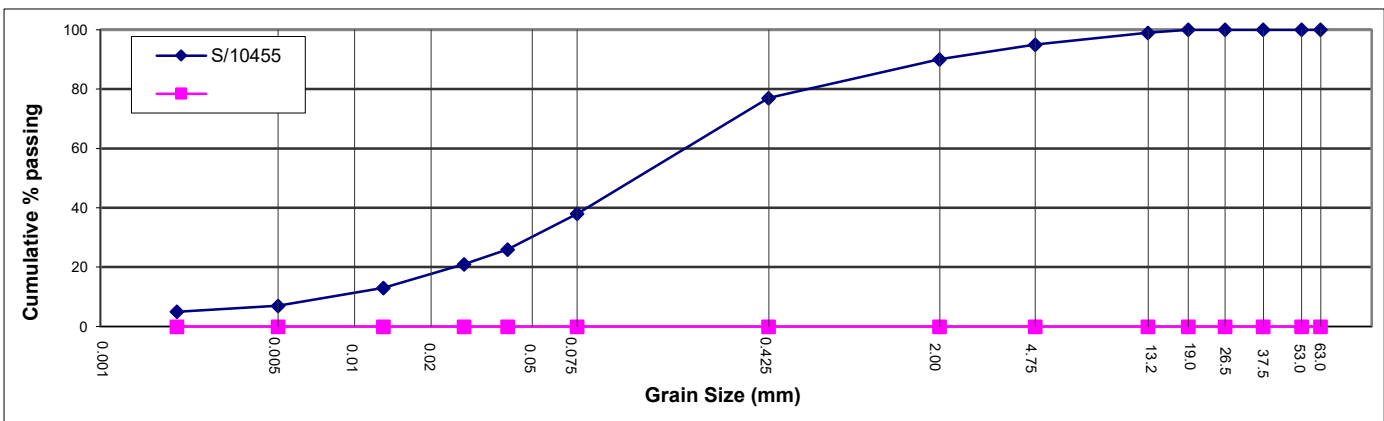
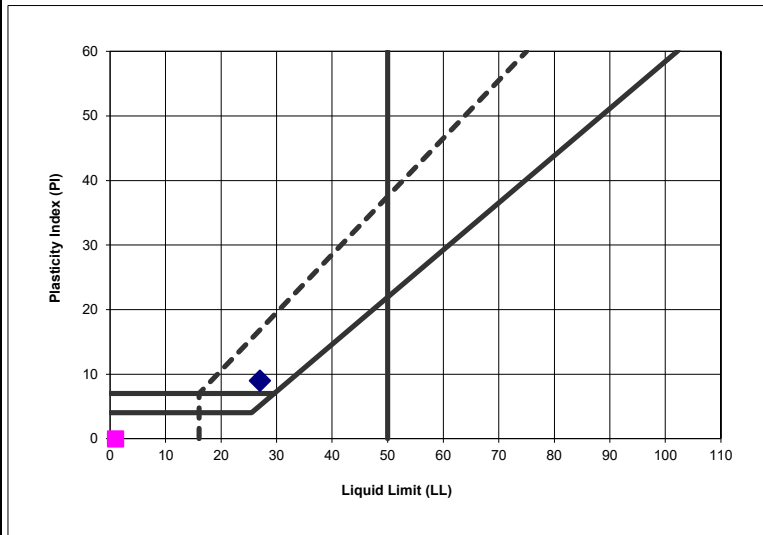
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10455
Report No.	SG23/1618
Depth (mm)	2600-4000
Position	RD-TP13
Material description	Clayey sand
Moisture (%)	
SG	2.669
Sieve Analysis (% passing)	
63.0 mm	
53.0 mm	
37.5 mm	
26.5 mm	
19.0 mm	100
13.2 mm	99
4.75 mm	95
2.00 mm	90
0.425 mm	77
0.075 mm	38
Hydrometer Analysis (% passing)	
0.040 mm	26
0.027 mm	21
0.013 mm	13
0.005 mm	7
0.002 mm	5
% Clay (<0.002 mm)	5
% Silt (<0.075, >0.002 mm)	33
% Sand (<4.75, >0.075 mm)	57
% Gravel (> 4.75 mm)	5
Atterberg Limits	
Liquid Limit (-0.425)	27
Plasticity Index (-0.425)	9
Linear Shrinkage (%)	4.5
PI of whole sample	7
Grading Modulus	0.95
TRB Classification	A-4 (1)
Unified Classification	SC

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Stormwater Diversion Channel (SWD)

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1619

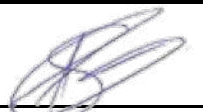
Page no : 1

sample detail	Sample no.	S/10456	S/10459	S/10463	S/10465	S/10466	S/10468
	Chainage						
	Position (from LHS)	SWD-TP02	SWD-TP15	SWD-TP23	SWD-TP17	SWD-TP08	SWD-TP19
	Depth sampled (mm)	0-700	500-2500	1200-2500	1300-2200	1000-1400	1400-2400
	Sample description	Brown mottled black , gravelly silty sand, fine alluvium	Orange brown stained red, clayey silty fine gravelly sand, fine alluvium	Dark reddish brown, sandy silty clay. Fine Alluvium	Olive brown mottled white stained orange, gravelly silty fine sand. Calcified Alluvium.	Dark reddish brown speckled white, silty sandy gravel. Ferruginized Alluvium.	Yellowish brown mottled black stained red, clayey silty gravelly sand with trace gravels, ferruginised alluvium
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm						
	53,0 mm						
	37,5 mm				100		
	26,5 mm				95	100	
	19,0 mm				92	95	
	13,2 mm			100	92	94	100
	4,75 mm	100	100	99	81	84	99
	2,00 mm	99	98	98	71	67	97
	0,425 mm	74	74	86	45	42	85
	0,075 mm	32	40	53	23	19	46
Grading Modulus (GM)	0.95	0.88	0.63	1.61	1.72	0.72	
Atterberg Limits	Liquid Limit (LL)		20	23	38	31	25
	Plasticity Index (PI)	3	8	8	16	15	8
	Linear Shrinkage (LS)	1.0	4.0	2.5	5.5	5.0	2.0
Mod AASHTO & CBR values	CBR @ 100 %		12			6	
	CBR @ 98 %		12			4	
	CBR @ 95 %		11			<3	
	CBR @ 93 %		11			<3	
	CBR @ 90 %		10			<3	
	Swell @ 100 %		0.0			0.3	
	Mod AASHTO (kg/m ³)		2055			2053	
	O.M.C. (%)		9.7			7.3	
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture		90.0% Mod AASHTO @ OMC			90.0% Mod AASHTO @ OMC	
	Permeability cm/s		3.57 E-05			4.59 E-05	
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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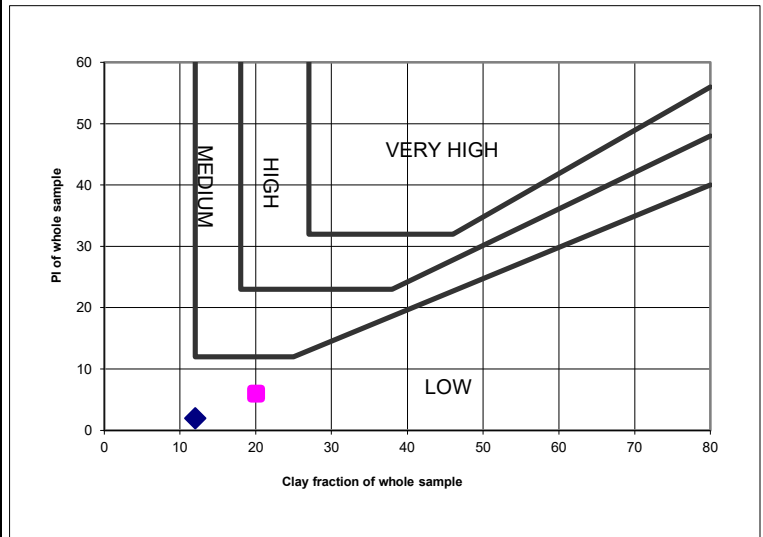
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

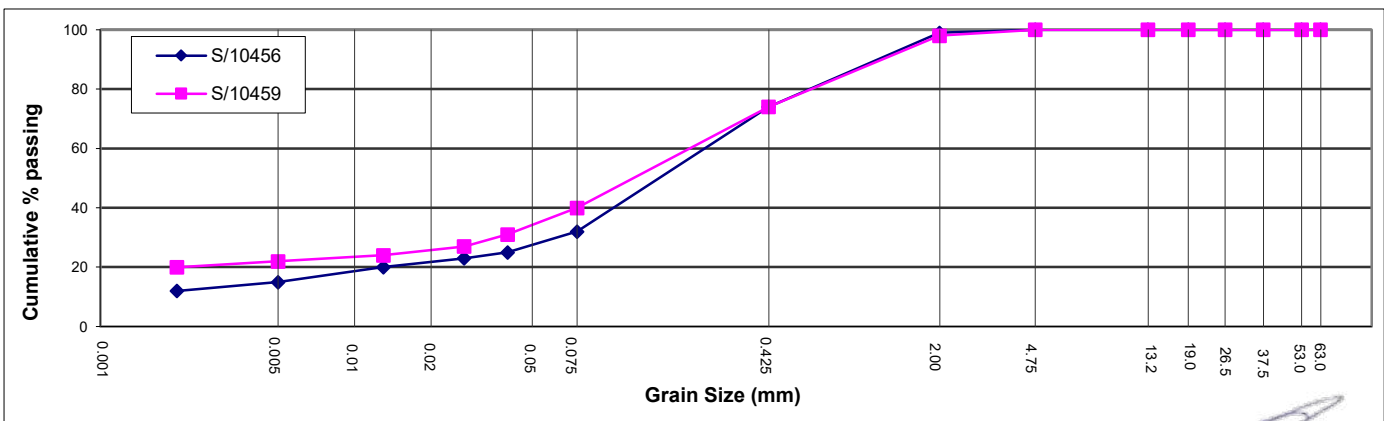
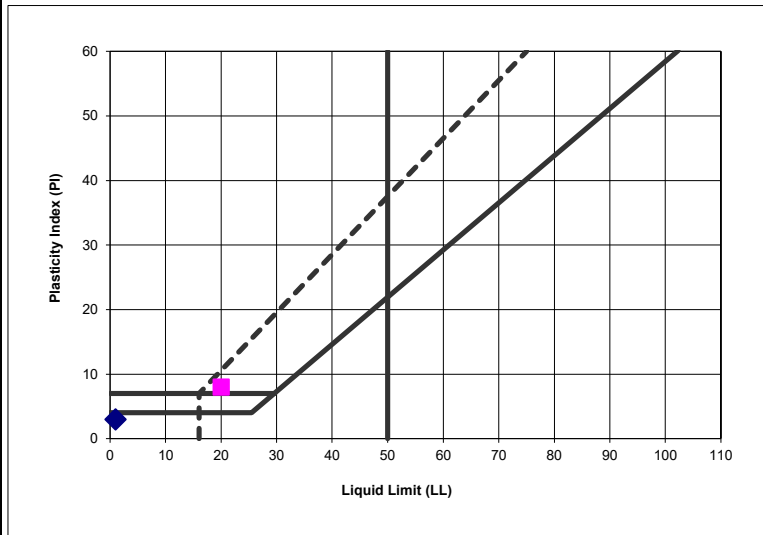
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10456	S/10459
Report No.	SG23/1619	
Depth (mm)	0-700	500-2500
Position	SWD-TP02	SWD-TP15
Material description	Silty sand	Clayey sand
Moisture (%)		
SG	2.586	2.628
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		
13.2 mm		
4.75 mm	100	100
2.00 mm	99	98
0.425 mm	74	74
0.075 mm	32	40
Hydrometer Analysis (% passing)		
0.040 mm	25	31
0.027 mm	23	27
0.013 mm	20	24
0.005 mm	15	22
0.002 mm	12	20
% Clay (<0.002 mm)	12	20
% Silt (<0.075, >0.002 mm)	20	20
% Sand (<4.75, >0.075 mm)	68	60
% Gravel (> 4.75 mm)	0	0
Atterberg Limits		
Liquid Limit (-0.425)		20
Plasticity Index (-0.425)	3	8
Linear Shrinkage (%)	1.0	4.0
PI of whole sample	2	6
Grading Modulus	0.95	0.88
TRB Classification	A-2-4 (0)	A-4 (0)
Unified Classification	SM	SC

Potential Expansiveness



Plasticity Chart




Namibia Technical Services cc

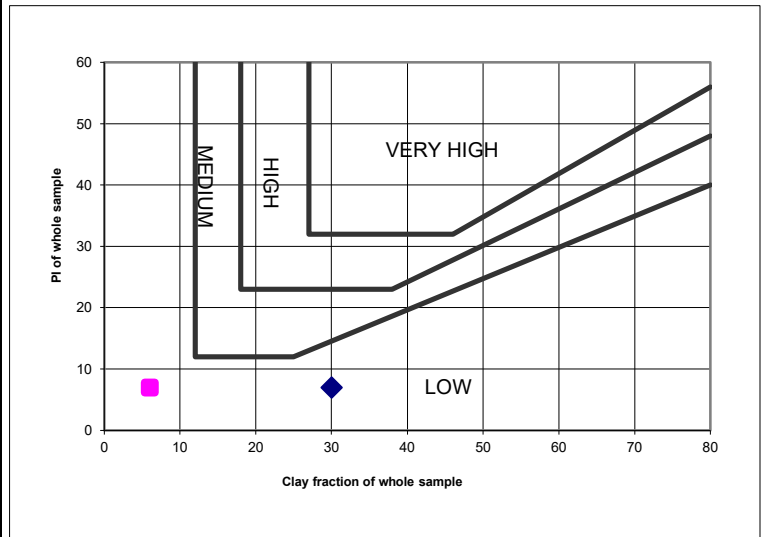
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

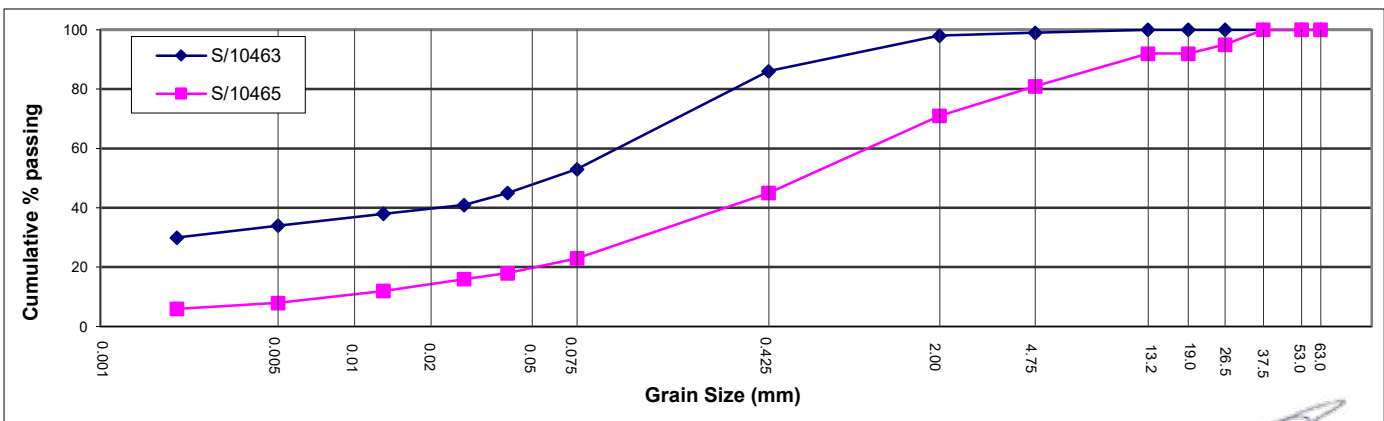
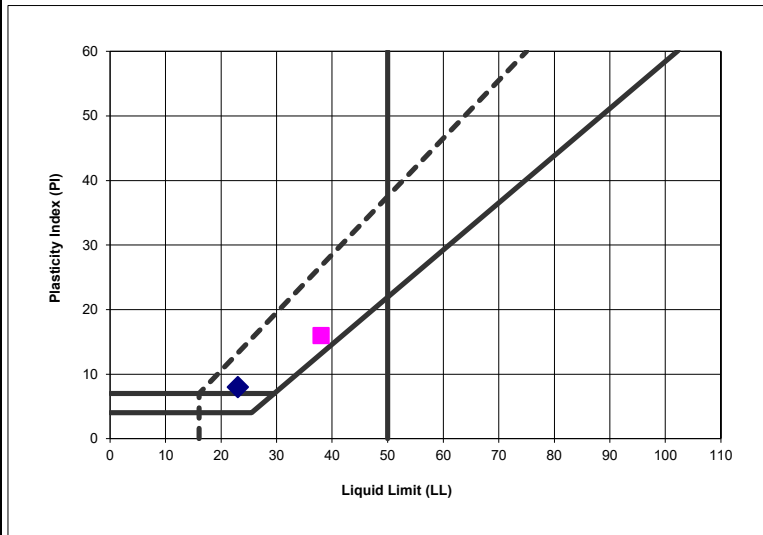
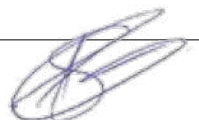
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10463	S/10465
Report No.	SG23/1619	
Depth (mm)	1200-2500	1300-2200
Position	SWD-TP23	SWD-TP17
Material description	Clayey sand	Clayey sand with gravel
Moisture (%)		
SG	2.654	2.615
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		100
26.5 mm		95
19.0 mm		92
13.2 mm	100	92
4.75 mm	99	81
2.00 mm	98	71
0.425 mm	86	45
0.075 mm	53	23
Hydrometer Analysis (% passing)		
0.040 mm	45	18
0.027 mm	41	16
0.013 mm	38	12
0.005 mm	34	8
0.002 mm	30	6
% Clay (<0.002 mm)	30	6
% Silt (<0.075, >0.002 mm)	23	17
% Sand (<4.75, >0.075 mm)	46	58
% Gravel (> 4.75 mm)	1	19
Atterberg Limits		
Liquid Limit (-0.425)	23	38
Plasticity Index (-0.425)	8	16
Linear Shrinkage (%)	2.5	5.5
PI of whole sample	7	7
Grading Modulus	0.63	1.61
TRB Classification	A-4 (4)	A-2-6 (1)
Unified Classification	SC	SC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

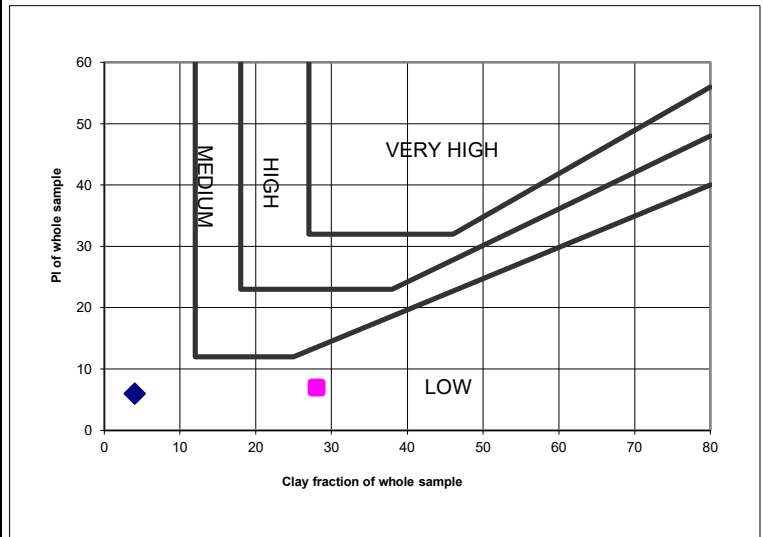
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

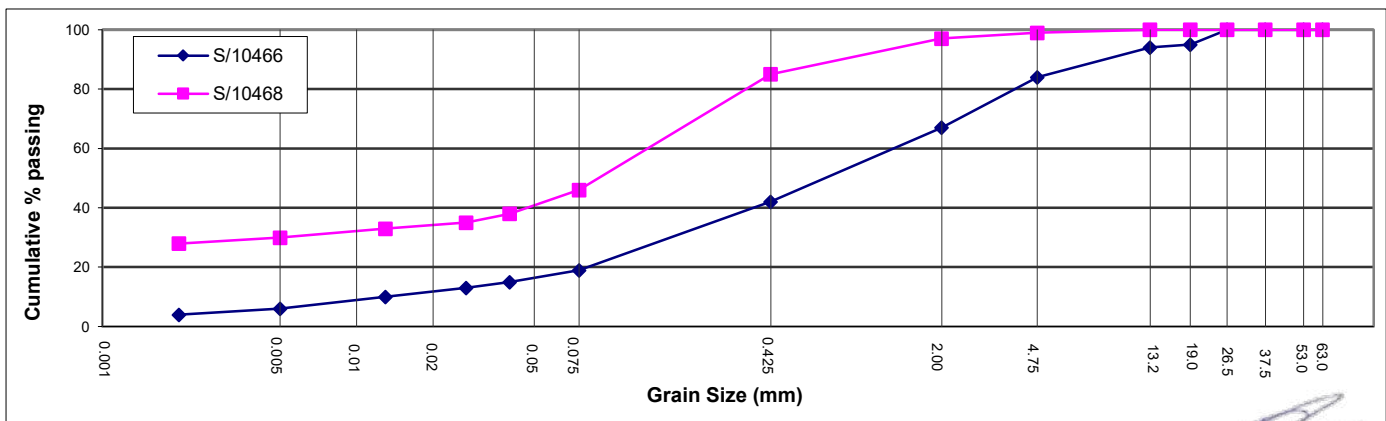
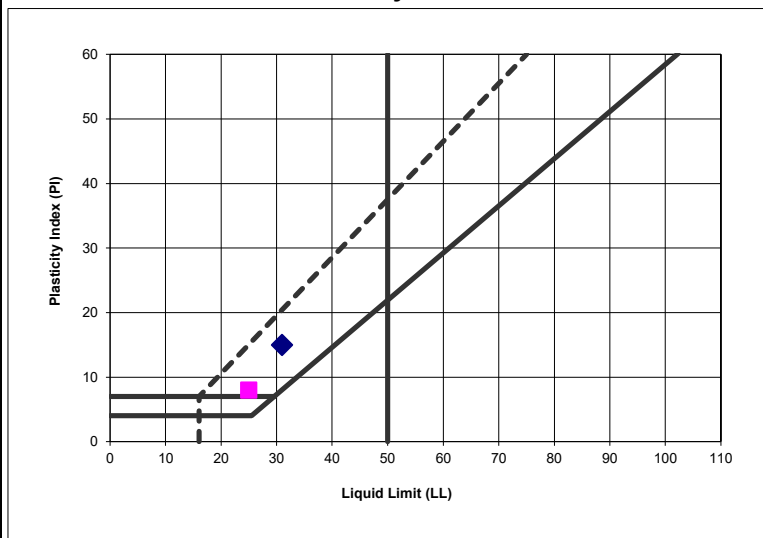
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10466	S/10468
Report No.	SG23/1619	
Depth (mm)	1000-1400	1400-2400
Position	SWD-TP08	SWD-TP19
Material description	Clayey sand with gravel	Clayey sand
Moisture (%)		
SG	2.557	2.645
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm	100	
19.0 mm	95	
13.2 mm	94	100
4.75 mm	84	99
2.00 mm	67	97
0.425 mm	42	85
0.075 mm	19	46
Hydrometer Analysis (% passing)		
0.040 mm	15	38
0.027 mm	13	35
0.013 mm	10	33
0.005 mm	6	30
0.002 mm	4	28
% Clay (<0.002 mm)	4	28
% Silt (<0.075, >0.002 mm)	15	18
% Sand (<4.75, >0.075 mm)	65	53
% Gravel (> 4.75 mm)	16	1
Atterberg Limits		
Liquid Limit (-0.425)	31	25
Plasticity Index (-0.425)	15	8
Linear Shrinkage (%)	5.0	2.0
PI of whole sample	6	7
Grading Modulus	1.72	0.72
TRB Classification	A-2-6 (1)	A-4 (1)
Unified Classification	SC	SC

Potential Expansiveness



Plasticity Chart




Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Stormwater Diversion Channel (SWD)

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1619

Page no : 5

sample detail	Sample no.	S/10470	S/10471				
	Chainage						
	Position (from LHS)	SWD-TP07	SWD-TP10				
	Depth sampled (mm)	1400-1900	1200-2100				
	Sample description	Completely weathered, green speckled to streaked black, white and gold, very soft rock, biotite gneiss	Reddish brown mottled white, silty sandy gravel. Nodular Ferricrete.				
	Method of Preparation used	Scalping	Scalping				
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm		100				
	53,0 mm		86				
	37,5 mm	100	80				
	26,5 mm	95	74				
	19,0 mm	92	66				
	13,2 mm	91	61				
	4,75 mm	75	59				
	2,00 mm	63	57				
	0,425 mm	40	49				
	0,075 mm	20	32				
Grading Modulus (GM)	1.77	1.62					
Atterberg Limits	Liquid Limit (LL)	32	29				
	Plasticity Index (PI)	12	12				
	Linear Shrinkage (LS)	3.5	6.0				
Mod AASHTO & CBR values	CBR @ 100 %	65	8				
	CBR @ 98 %	40	6				
	CBR @ 95 %	19	4				
	CBR @ 93 %	12	3				
	CBR @ 90 %	6	<3				
	Swell @ 100 %	0.0	0.0				
	Mod AASHTO (kg/m ³)	2174	1875				
	O.M.C. (%)	7.2	14.9				
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

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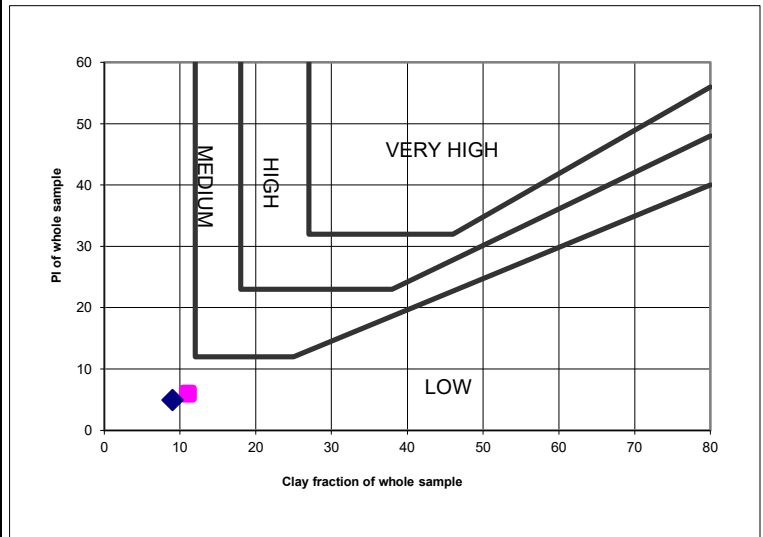
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

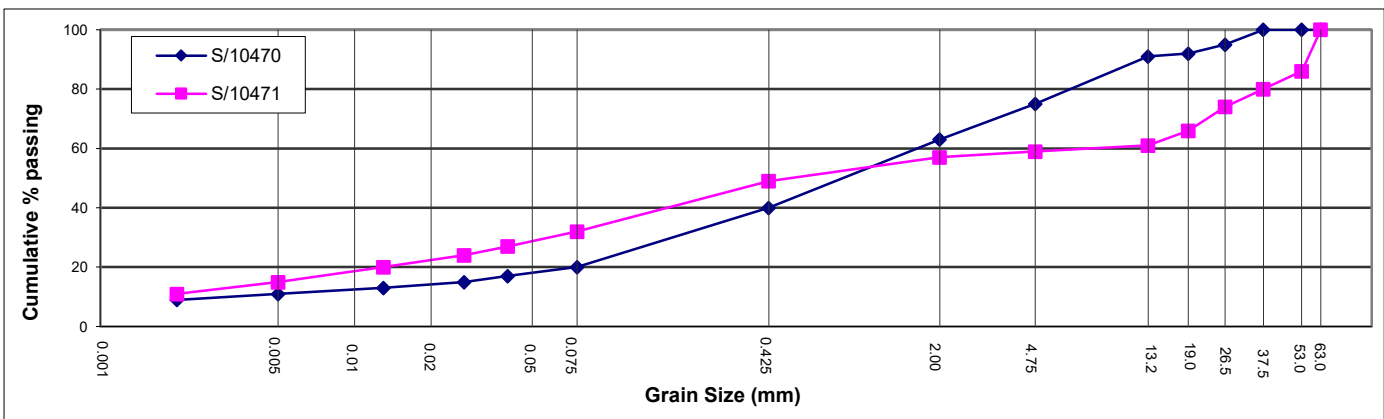
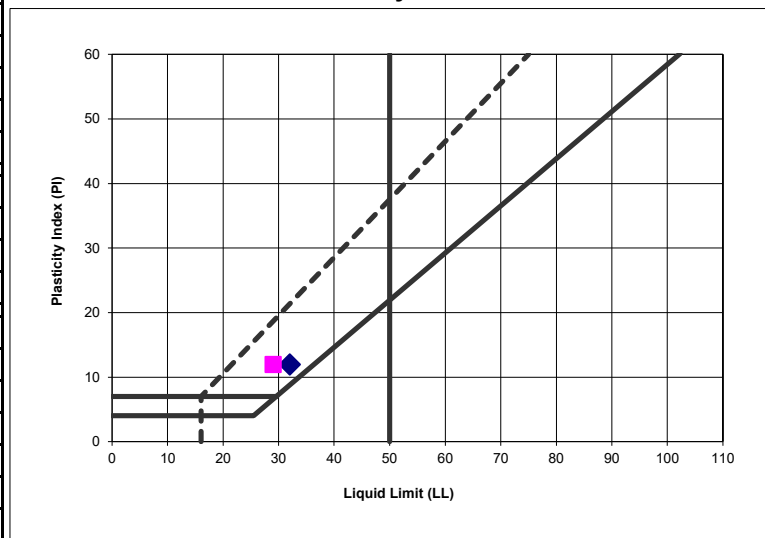
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10470	S/10471
Report No.	SG23/1619	
Depth (mm)	1400-1900	1200-2100
Position	SWD-TP07	SWD-TP10
Material description	Clayey sand with gravel	Clayey gravel with sand
Moisture (%)		
SG	2.736	2.629
Sieve Analysis (% passing)		
63.0 mm		100
53.0 mm		86
37.5 mm	100	80
26.5 mm	95	74
19.0 mm	92	66
13.2 mm	91	61
4.75 mm	75	59
2.00 mm	63	57
0.425 mm	40	49
0.075 mm	20	32
Hydrometer Analysis (% passing)		
0.040 mm	17	27
0.027 mm	15	24
0.013 mm	13	20
0.005 mm	11	15
0.002 mm	9	11
% Clay (<0.002 mm)	9	11
% Silt (<0.075, >0.002 mm)	11	21
% Sand (<4.75, >0.075 mm)	55	27
% Gravel (> 4.75 mm)	25	41
Atterberg Limits		
Liquid Limit (-0.425)	32	29
Plasticity Index (-0.425)	12	12
Linear Shrinkage (%)	3.5	6.0
PI of whole sample	5	6
Grading Modulus	1.77	1.62
TRB Classification	A-2-6 (1)	A-2-6 (1)
Unified Classification	SC	GC

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Waste Rock Dump

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1620

Page no : 1

sample detail	Sample no.	S/10472	S/10474	S/10478	S/10479	S/10488	S/10494
	Chainage						
	Position (from LHS)	WRD-TP01	WRD-TP04	WRD-TP14	WRD-TP15	WRD-TP04	WRD-TP26
	Depth sampled (mm)	1400-3500	1500-2400	2800-4900	0-700	400-1500	1200-2600
	Sample description	Dark reddish brown, silty sandy clay. Fine Alluvium.	Orange brown, gravelly silty sand. Alluvium.	Pale yellow brown, silty sandy gravel and cobbles. Coarse Alluvium.	Brown, sandy clayey silt. Fine Alluvium.	Pale brown mottled white, gravelly sandy silt. Calcified Alluvium.	White speckled to mottled black and brown, silty sandy gravel, hardpan calcrete
Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping	
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Compaction (%)							
sieve analysis (% passing)	63,0 mm						82
	53,0 mm			100			76
	37,5 mm		100	85			71
	26,5 mm		98	75			63
	19,0 mm		98	71			60
	13,2 mm	100	97	68	100	100	53
	4,75 mm	100	96	63	97	98	42
	2,00 mm	98	94	52	95	92	34
	0,425 mm	81	79	32	86	76	22
	0,075 mm	40	43	18	62	28	20
Grading Modulus (GM)	0.81	0.84	1.98	0.57	1.04	2.24	
Atterberg Limits	Liquid Limit (LL)	23	29	29	33	25	31
	Plasticity Index (PI)	9	12	11	16	6	9
	Linear Shrinkage (LS)	2.5	2.5	4.0	7.0	2.5	3.0
Mod AASHTO & CBR values	CBR @ 100 %	9	8	9		8	
	CBR @ 98 %	5	7	8		6	
	CBR @ 95 %	<3	5	8		3	
	CBR @ 93 %	<3	5	7		<3	
	CBR @ 90 %	<3	4	7		<3	
	Swell @ 100 %	0.0	0.1	0.0		0.1	
	Mod AASHTO (kg/m ³)	2035	1963	2071		1975	
	O.M.C. (%)	9.3	12.0	9.5		11.0	
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture			90.0% Mod AASHTO @ OMC		90.0% Mod AASHTO @ OMC	
	Permeability cm/s			7.44 E-05		1.19 E-05	
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Waste Rock Dump

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1620

Page no : 2

sample detail	Sample no.	S/10479					
	Chainage						
	Position (from LHS)	WRD-TP15					
	Depth sampled (mm)	0-700					
	Sample description	Brown, sandy clayey silt. Fine Alluvium.					
	Method of Preparation used						
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm						
	53,0 mm						
	37,5 mm						
	26,5 mm						
	19,0 mm						
	13,2 mm	100					
	4,75 mm	97					
	2,00 mm	95					
	0,425 mm	86					
	0,075 mm	62					
Grading Modulus (GM)	0.57						
Atterberg Limits	Liquid Limit (LL)	33					
	Plasticity Index (PI)	16					
	Linear Shrinkage (LS)	7.0					
Mod AASHTO & CBR values	CBR @ 100 %						
	CBR @ 98 %						
	CBR @ 95 %						
	CBR @ 93 %						
	CBR @ 90 %						
	Swell @ 100 %						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Proctor	Proctor MDD (kg/m ³)	1628					
	O.M.C. (%)	14.0					
Chem	pH						
	Conductivity (Sm ⁻¹)						

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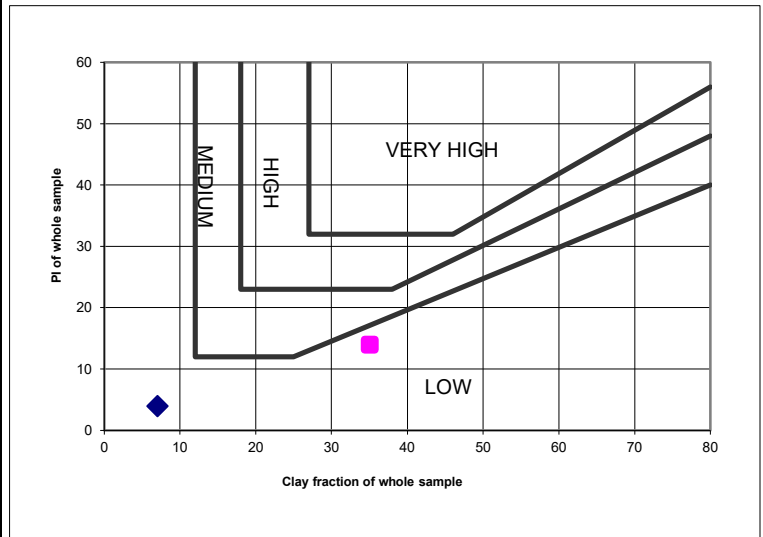
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

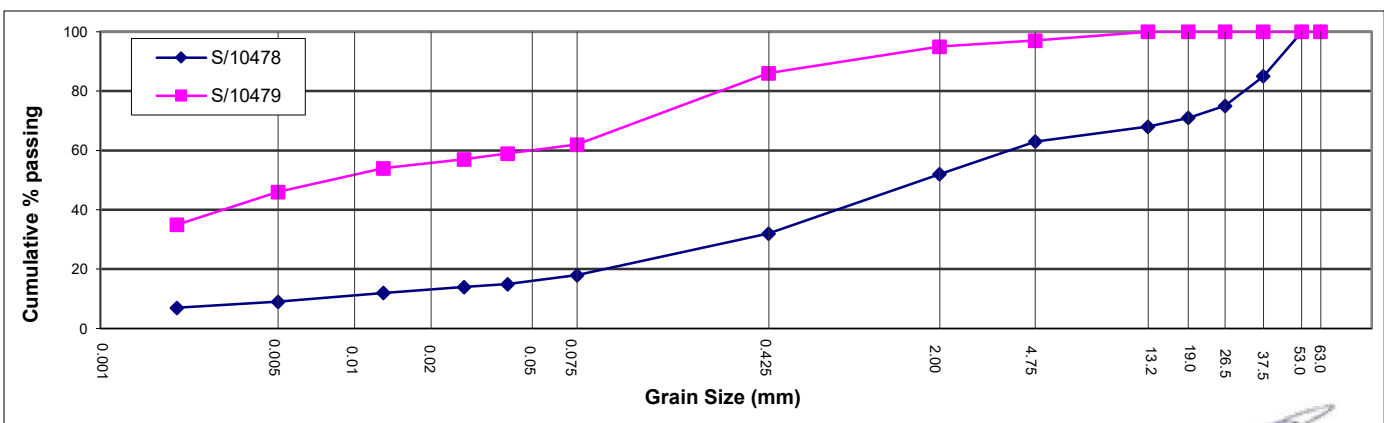
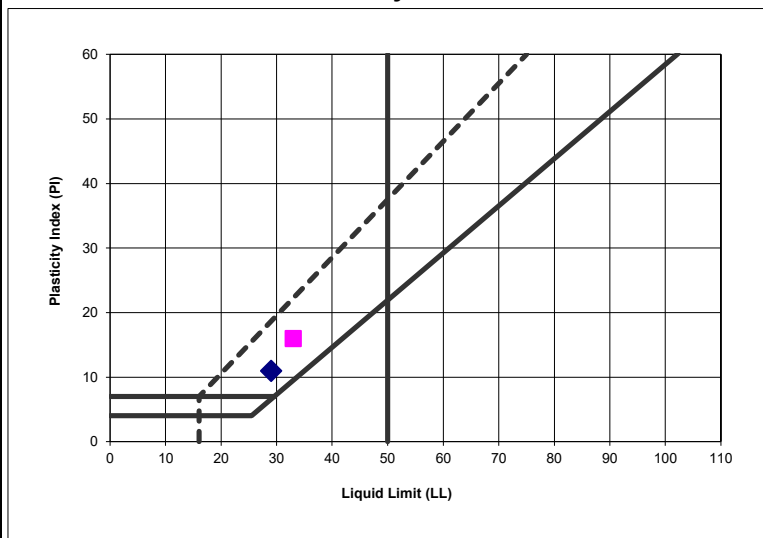
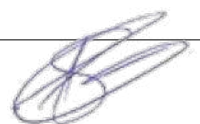
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10478	S/10479
Report No.	SG23/1620	
Depth (mm)	2800-4900	0-700
Position	WRD-TP14	WRD-TP15
Material description	Clayey sand with gravel	Sandy lean clay
Moisture (%)		
SG	2.616	2.619
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm	100	
37.5 mm	85	
26.5 mm	75	
19.0 mm	71	
13.2 mm	68	100
4.75 mm	63	97
2.00 mm	52	95
0.425 mm	32	86
0.075 mm	18	62
Hydrometer Analysis (% passing)		
0.040 mm	15	59
0.027 mm	14	57
0.013 mm	12	54
0.005 mm	9	46
0.002 mm	7	35
% Clay (<0.002 mm)	7	35
% Silt (<0.075, >0.002 mm)	11	27
% Sand (<4.75, >0.075 mm)	45	35
% Gravel (> 4.75 mm)	37	3
Atterberg Limits		
Liquid Limit (-0.425)	29	33
Plasticity Index (-0.425)	11	16
Linear Shrinkage (%)	4.0	7.0
PI of whole sample	4	14
Grading Modulus	1.98	0.57
TRB Classification	A-2-6 (0)	A-6 (7)
Unified Classification	SC	CL

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

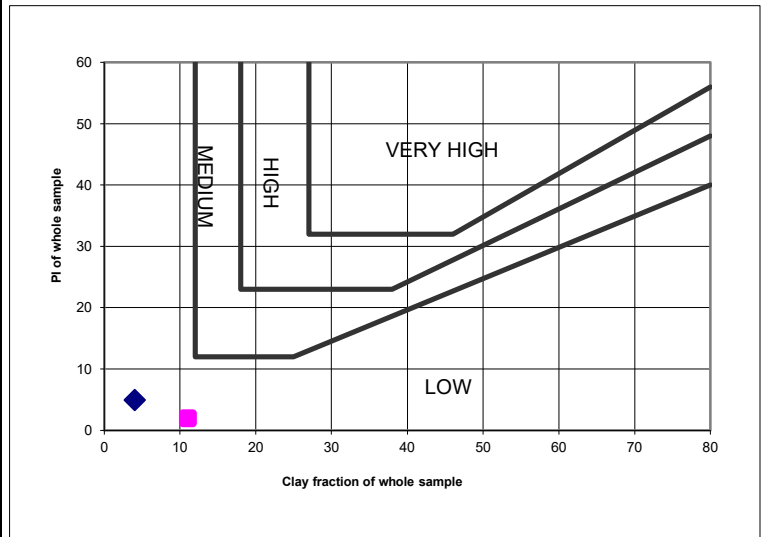
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

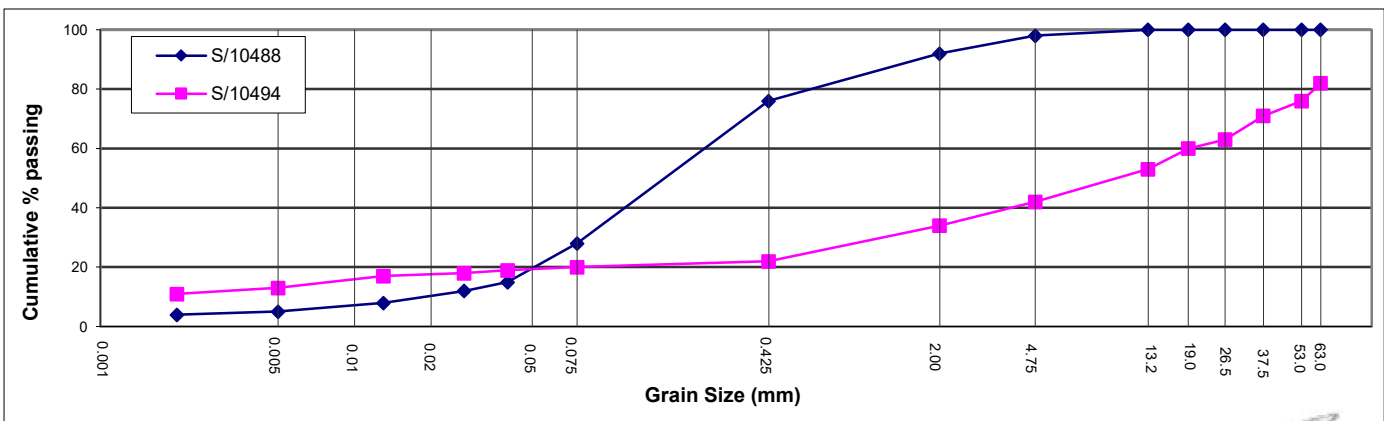
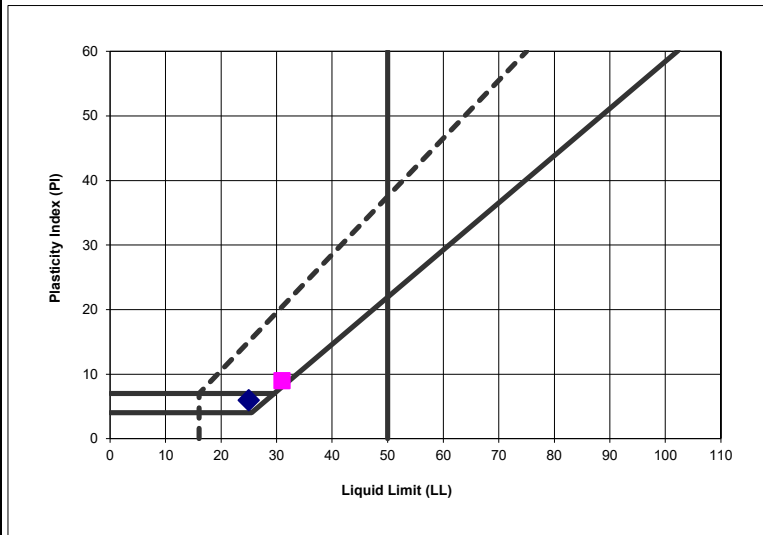
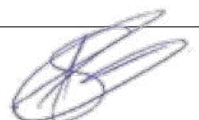
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10488	S/10494
Report No.	SG23/1620	
Depth (mm)	400-1500	1200-2600
Position	WRD-TP04	WRD-TP26
Material description	Silty, clayey sand	Clayey gravel with sand
Moisture (%)		
SG	2.502	2.588
Sieve Analysis (% passing)		
63.0 mm		82
53.0 mm		76
37.5 mm		71
26.5 mm		63
19.0 mm		60
13.2 mm	100	53
4.75 mm	98	42
2.00 mm	92	34
0.425 mm	76	22
0.075 mm	28	20
Hydrometer Analysis (% passing)		
0.040 mm	15	19
0.027 mm	12	18
0.013 mm	8	17
0.005 mm	5	13
0.002 mm	4	11
% Clay (<0.002 mm)	4	11
% Silt (<0.075, >0.002 mm)	24	9
% Sand (<4.75, >0.075 mm)	70	22
% Gravel (> 4.75 mm)	2	58
Atterberg Limits		
Liquid Limit (-0.425)	25	31
Plasticity Index (-0.425)	6	9
Linear Shrinkage (%)	2.5	3.0
PI of whole sample	5	2
Grading Modulus	1.04	2.24
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	SC-SM	GC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Waste Rock Dump

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1620

Page no : 6

sample detail	Sample no.	S/10496	S/10498	S/10500	S/10502	S/10504	S/10507
	Chainage						
	Position (from LHS)	WRD-TP35	WRD-TP39	WRD-TP37	WRD-TP32	WRD-TP16	WRD-TP37
	Depth sampled (mm)	2500-4100	1400-1500	2400-2700	1150-1450	500-1800	900-1100
	Sample description	Yellow brown, sandy silt. Ferruginous Alluvium.	Khaki stained brown to black, silty gravelly sand to silty sandy gravel, ferruginised alluvium	Completely to highly weathered, white speckled black stained orange, gneiss	Pale brown blotched white, sandy silt with abundant calcrete nodules and cobbles. Honeycomb Calcrete.	Pale brown speckled white, sandy silty gravel. Nodular Calcrete.	Yellowish brown mottled white and black, silty sandy gravel, calcified pebble marker
Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping	
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
Compaction (%)							
sieve analysis (% passing)	63,0 mm				88	88	
	53,0 mm			100	83	73	
	37,5 mm			84	76	64	
	26,5 mm			79	62	47	100
	19,0 mm		100	76	56	36	99
	13,2 mm	100	99	71	51	33	98
	4,75 mm	97	78	61	44	26	90
	2,00 mm	92	65	52	39	23	81
	0,425 mm	75	39	31	29	20	63
	0,075 mm	41	17	12	17	10	42
Grading Modulus (GM)	0.92	1.79	2.05	2.15	2.47	1.14	
Atterberg Limits	Liquid Limit (LL)	32	36	23	24	30	37
	Plasticity Index (PI)	13	10	8	8	11	13
	Linear Shrinkage (LS)	6.0	3.5	4.0	3.0	4.0	6.5
Mod AASHTO & CBR values	CBR @ 100 %	4		27		27	
	CBR @ 98 %	3		21		24	
	CBR @ 95 %	<3		14		19	
	CBR @ 93 %	<3		11		17	
	CBR @ 90 %	<3		7		14	
	Swell @ 100 %	0.1		0.1		0.2	
	Mod AASHTO (kg/m ³)	1946	Insufficient	2010		1869	
O.M.C. (%)	11.2	material	10.8		14.5		
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture		Insufficient material				
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

Namibia Technical Services cc

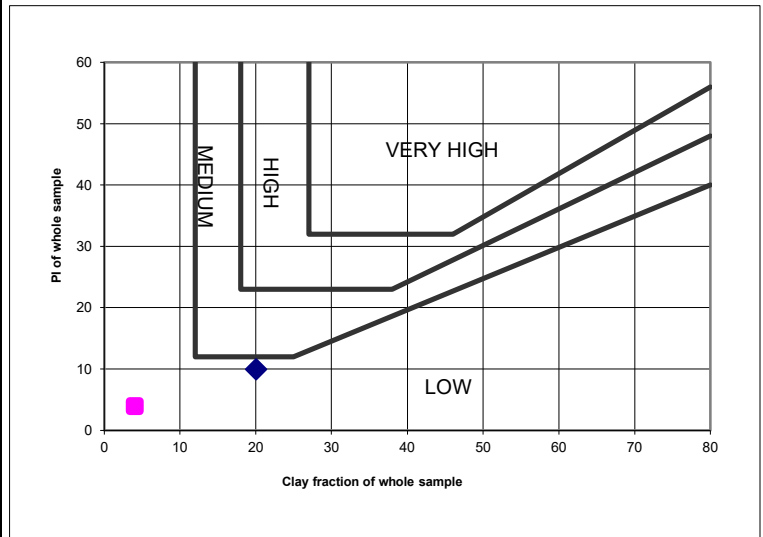
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

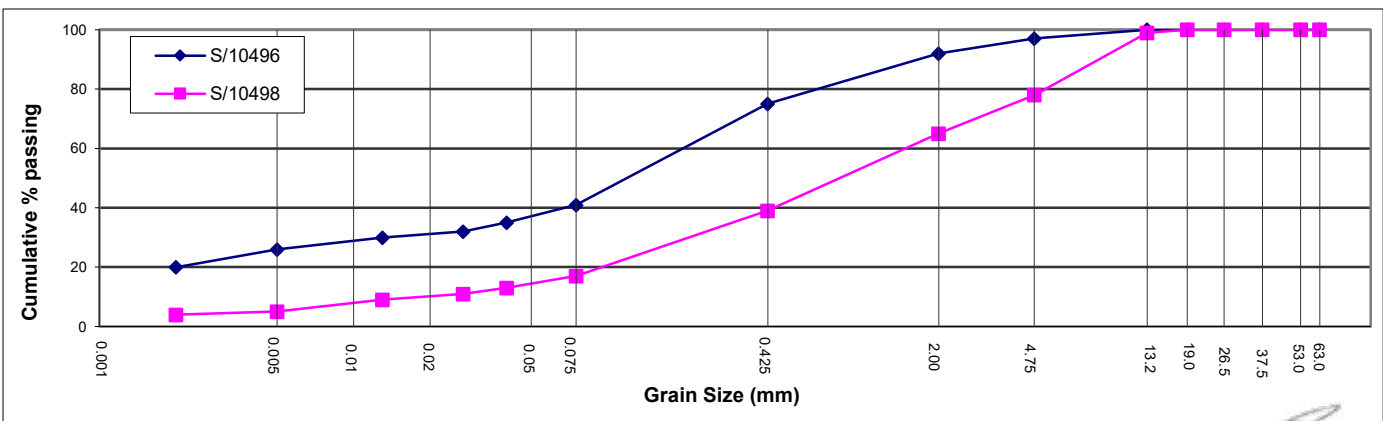
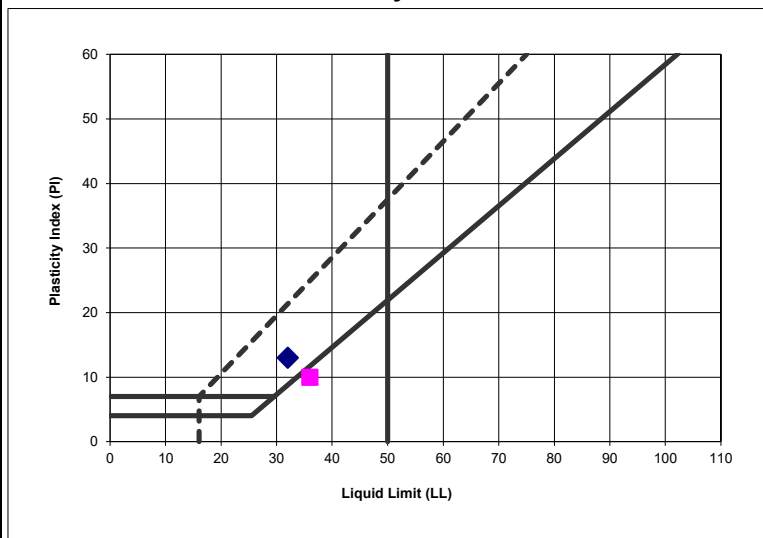
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10496	S/10498
Report No.	SG23/1620	
Depth (mm)	2500-4100	1400-1500
Position	WRD-TP35	WRD-TP39
Material description	Clayey sand	Silty sand with gravel
Moisture (%)		
SG	2.647	2.596
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		100
13.2 mm	100	99
4.75 mm	97	78
2.00 mm	92	65
0.425 mm	75	39
0.075 mm	41	17
Hydrometer Analysis (% passing)		
0.040 mm	35	13
0.027 mm	32	11
0.013 mm	30	9
0.005 mm	26	5
0.002 mm	20	4
% Clay (<0.002 mm)	20	4
% Silt (<0.075, >0.002 mm)	21	13
% Sand (<4.75, >0.075 mm)	56	61
% Gravel (> 4.75 mm)	3	22
Atterberg Limits		
Liquid Limit (-0.425)	32	36
Plasticity Index (-0.425)	13	10
Linear Shrinkage (%)	6.0	3.5
PI of whole sample	10	4
Grading Modulus	0.92	1.79
TRB Classification	A-6 (2)	A-2-4 (0)
Unified Classification	SC	SM

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

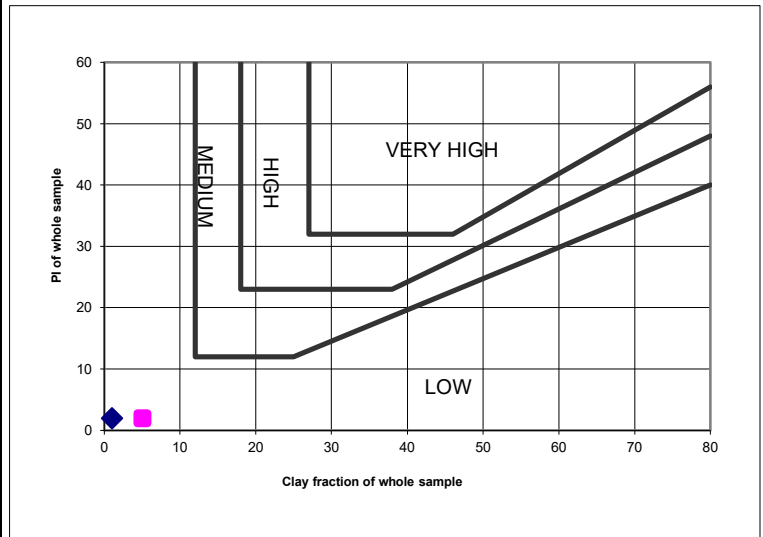
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

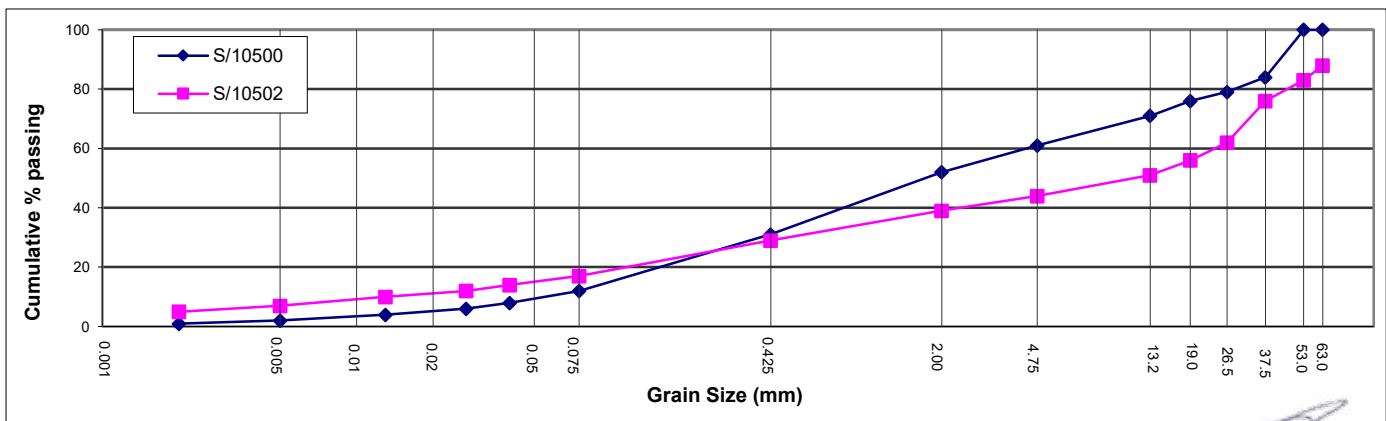
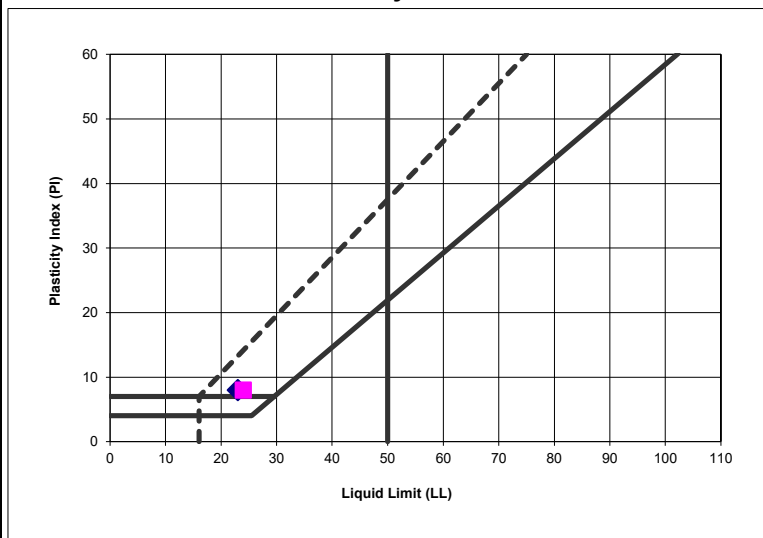
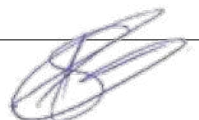
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10500	S/10502
Report No.	SG23/1620	
Depth (mm)	2400-2700	1150-1450
Position	WRD-TP37	WRD-TP32
Material description	Clayey sand with gravel	Clayey gravel with sand
Moisture (%)		
SG	2.655	2.602
Sieve Analysis (% passing)		
63.0 mm		88
53.0 mm	100	83
37.5 mm	84	76
26.5 mm	79	62
19.0 mm	76	56
13.2 mm	71	51
4.75 mm	61	44
2.00 mm	52	39
0.425 mm	31	29
0.075 mm	12	17
Hydrometer Analysis (% passing)		
0.040 mm	8	14
0.027 mm	6	12
0.013 mm	4	10
0.005 mm	2	7
0.002 mm	1	5
% Clay (<0.002 mm)	1	5
% Silt (<0.075, >0.002 mm)	11	12
% Sand (<4.75, >0.075 mm)	49	27
% Gravel (> 4.75 mm)	39	56
Atterberg Limits		
Liquid Limit (-0.425)	23	24
Plasticity Index (-0.425)	8	8
Linear Shrinkage (%)	4.0	3.0
PI of whole sample	2	2
Grading Modulus	2.05	2.15
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	SC	GC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

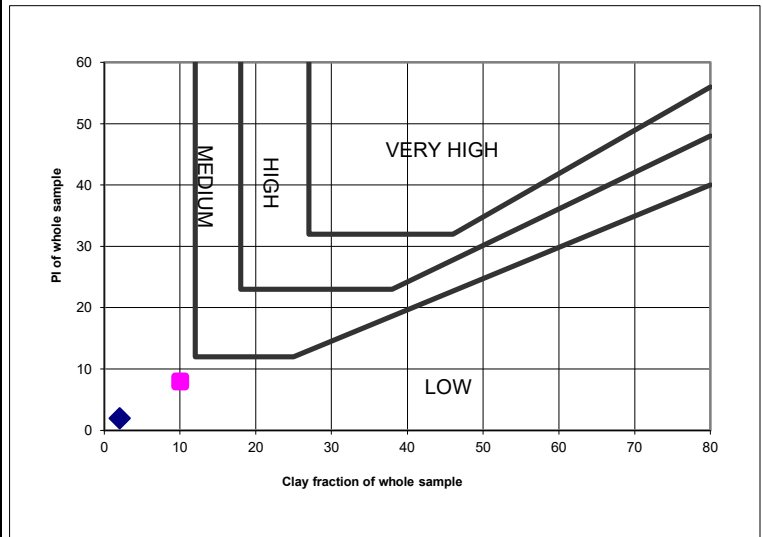
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

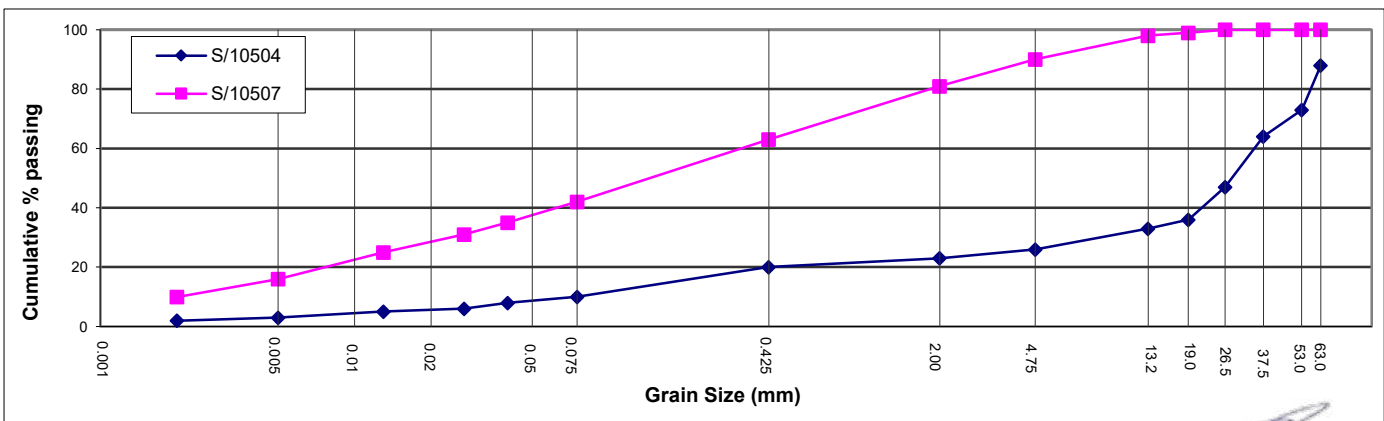
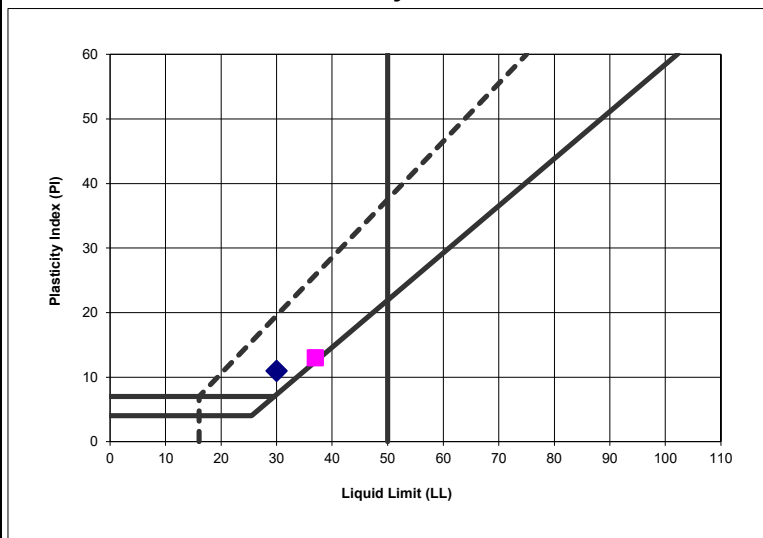
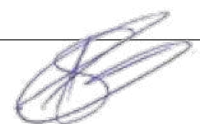
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10504	S/10507
Report No.	SG23/1620	
Depth (mm)	500-1800	900-1100
Position	WRD-TP16	WRD-TP37
Material description	Poorly graded gravel with clay and sand	Clayey sand
Moisture (%)		
SG	2.614	2.518
Sieve Analysis (% passing)		
63.0 mm	88	
53.0 mm	73	
37.5 mm	64	
26.5 mm	47	100
19.0 mm	36	99
13.2 mm	33	98
4.75 mm	26	90
2.00 mm	23	81
0.425 mm	20	63
0.075 mm	10	42
Hydrometer Analysis (% passing)		
0.040 mm	8	35
0.027 mm	6	31
0.013 mm	5	25
0.005 mm	3	16
0.002 mm	2	10
% Clay (<0.002 mm)	2	10
% Silt (<0.075, >0.002 mm)	8	32
% Sand (<4.75, >0.075 mm)	16	48
% Gravel (> 4.75 mm)	74	10
Atterberg Limits		
Liquid Limit (-0.425)	30	37
Plasticity Index (-0.425)	11	13
Linear Shrinkage (%)	4.0	6.5
PI of whole sample	2	8
Grading Modulus	2.47	1.14
TRB Classification	A-2-6 (0)	A-4 (2)
Unified Classification	GP-GC	SC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : Omitiomire

Sampling date : 23-Jun-23

Section : Waste Rock Dump

Stabilizing agent :

Layer :

From chainage :

To chainage :

Report no : SG23/1620

Page no : 10

sample detail	Sample no.	S/10509	S/10510	S/10511	S/10512		
	Chainage						
	Position (from LHS)	WRD-TP02	WRD-TP33	WRD-TP37	WRD-TP23		
	Depth sampled (mm)	1400-1600	1200-1300	1100-2400	850-1500		
	Sample description	White to grey mottled black stained orange, silty gravelly sand, calcified reworked residual gneiss	Dark brown mottled orange and black, clayey silty coarse sand, slightly ferruginised reworked residual biotite gneiss	Yellowish brown mottled black and white, gravelly silty sand, reworked residual gneiss	Pale brown blotched white, hardpan calcrete.		
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping		
field density	Testing depth (mm)						
	Dry density (kg/m ³)						
	Moisture (%)						
	Mod AASHTO (kg/m ³)						
	O.M.C. (%)						
	Compaction (%)						
sieve analysis (% passing)	63,0 mm				75		
	53,0 mm	100			62		
	37,5 mm	91			58		
	26,5 mm	80			58		
	19,0 mm	71		100	58		
	13,2 mm	66	100	100	51		
	4,75 mm	42	100	92	37		
	2,00 mm	28	96	86	31		
	0,425 mm	18	70	59	20		
	0,075 mm	5	34	24	8		
	Grading Modulus (GM)	2.49	1.00	1.31	2.41		
Atterberg Limits	Liquid Limit (LL)		22	24	35		
	Plasticity Index (PI)	1	8	6	9		
	Linear Shrinkage (LS)		3.0	3.0	4.0		
Mod AASHTO & CBR values	CBR @ 100 %				64		
	CBR @ 98 %				54		
	CBR @ 95 %				42		
	CBR @ 93 %				36		
	CBR @ 90 %				28		
	Swell @ 100 %				0.0		
	Mod AASHTO (kg/m ³)				1651		
	O.M.C. (%)				19.9		
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
	Permeability cm/s						
	% Compaction @ Moisture						
	Permeability cm/s						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

Namibia Technical Services cc

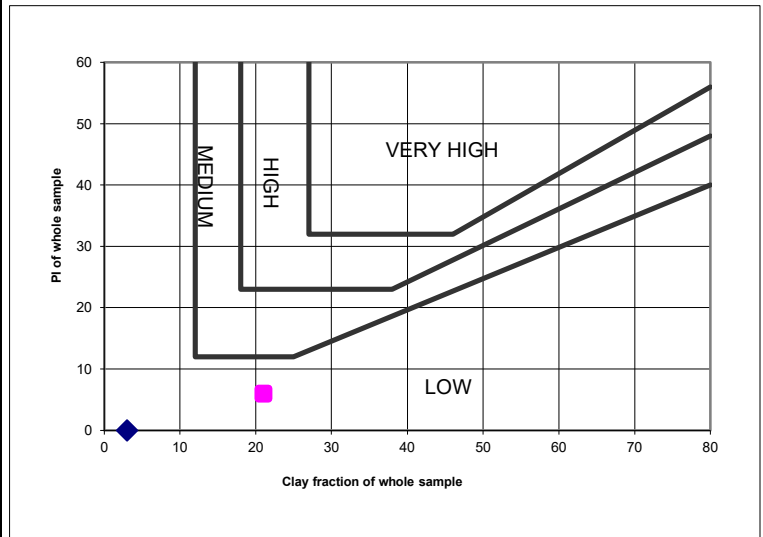
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

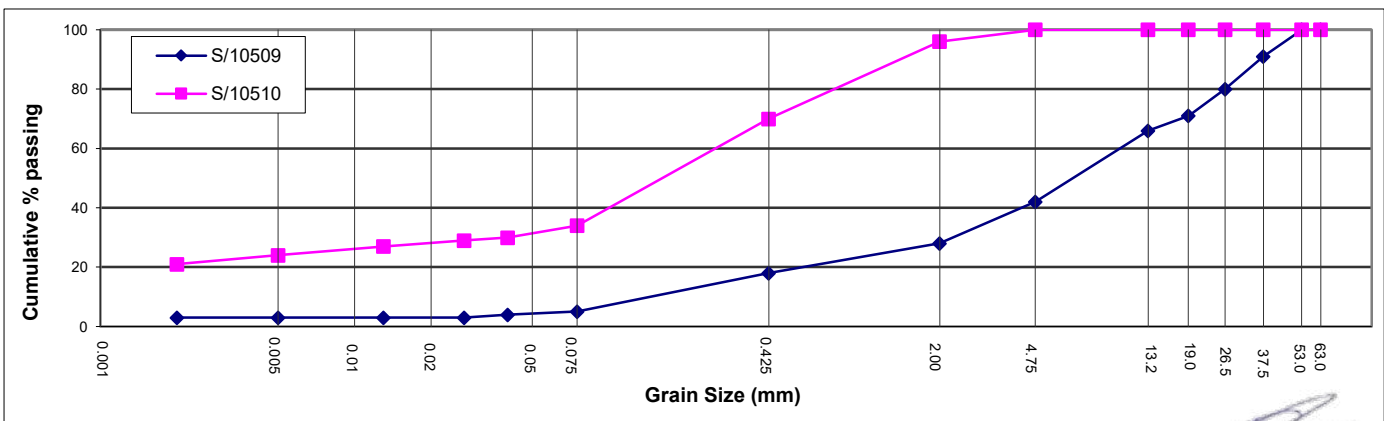
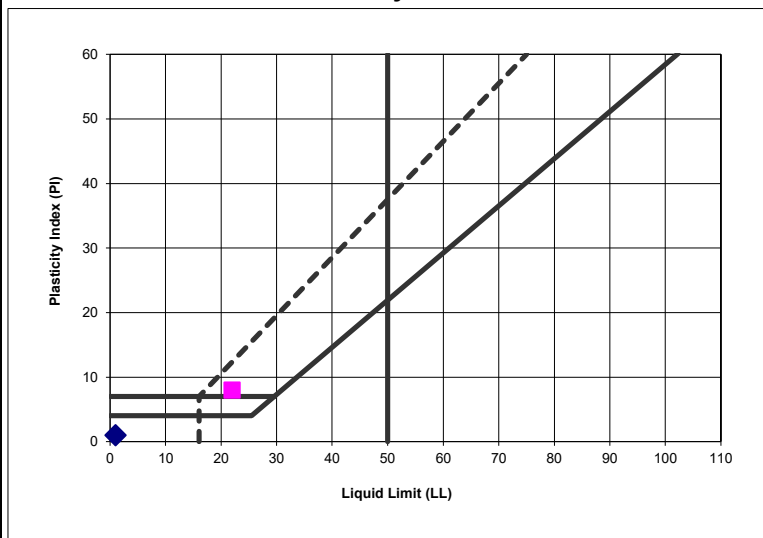
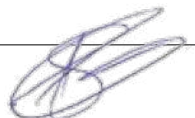
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10509	S/10510
Report No.	SG23/1620	
Depth (mm)	1400-1600	1200-1300
Position	WRD-TP02	WRD-TP33
Material description	Poorly graded gravel with silt and sand	Clayey sand
Moisture (%)		
SG	2.635	2.634
Sieve Analysis (% passing)		
63.0 mm		
53.0 mm	100	
37.5 mm	91	
26.5 mm	80	
19.0 mm	71	
13.2 mm	66	100
4.75 mm	42	100
2.00 mm	28	96
0.425 mm	18	70
0.075 mm	5	34
Hydrometer Analysis (% passing)		
0.040 mm	4	30
0.027 mm	3	29
0.013 mm	3	27
0.005 mm	3	24
0.002 mm	3	21
% Clay (<0.002 mm)	3	21
% Silt (<0.075, >0.002 mm)	2	13
% Sand (<4.75, >0.075 mm)	37	66
% Gravel (> 4.75 mm)	58	0
Atterberg Limits		
Liquid Limit (-0.425)		22
Plasticity Index (-0.425)	1	8
Linear Shrinkage (%)	0.0	3.0
PI of whole sample	0	6
Grading Modulus	2.49	1
TRB Classification	A-1-a (0)	A-2-4 (0)
Unified Classification	GP-GM	SC

Potential Expansiveness



Plasticity Chart

Namibia Technical Services cc

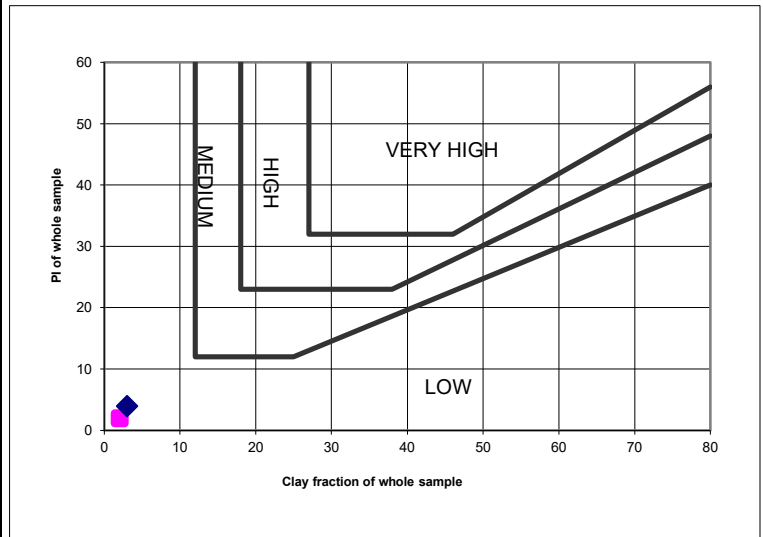
SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

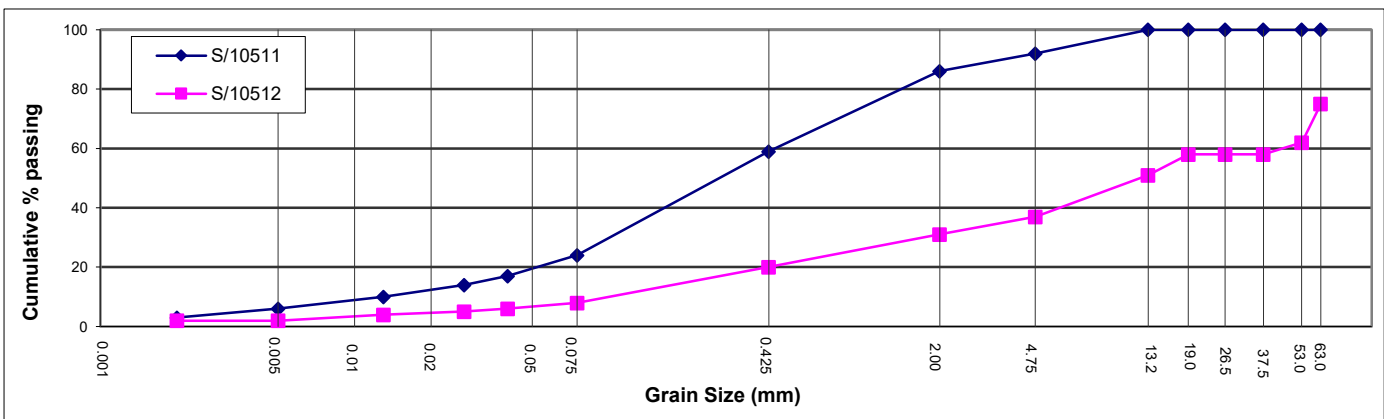
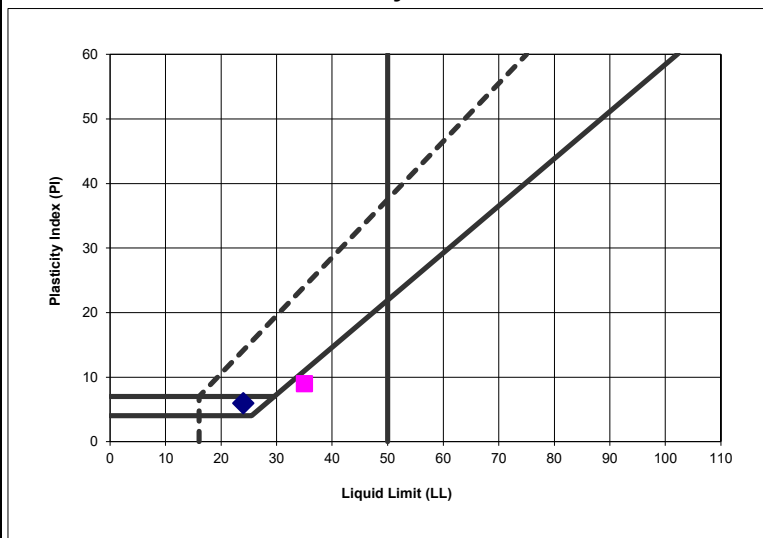
Client	Knight Piesold (Pty) Ltd (Namibia)	Project No.	10795
Project	Omitomire	Sampling Date	23-Jun-23

Sample No.	S/10511	S/10512
Report No.	SG23/1620	
Depth (mm)	1100-2400	850-1500
Position	WRD-TP37	WRD-TP23
Material description	Silty, clayey sand	Poorly graded gravel with silt and sand
Moisture (%)		
SG	2.648	2.543
Sieve Analysis (% passing)		
63.0 mm		75
53.0 mm		62
37.5 mm		58
26.5 mm		58
19.0 mm	100	58
13.2 mm	100	51
4.75 mm	92	37
2.00 mm	86	31
0.425 mm	59	20
0.075 mm	24	8
Hydrometer Analysis (% passing)		
0.040 mm	17	6
0.027 mm	14	5
0.013 mm	10	4
0.005 mm	6	2
0.002 mm	3	2
% Clay (<0.002 mm)	3	2
% Silt (<0.075, >0.002 mm)	21	6
% Sand (<4.75, >0.075 mm)	68	29
% Gravel (> 4.75 mm)	8	63
Atterberg Limits		
Liquid Limit (-0.425)	24	35
Plasticity Index (-0.425)	6	9
Linear Shrinkage (%)	3.0	4.0
PI of whole sample	4	2
Grading Modulus	1.31	2.41
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	SC-SM	GP-GM

Potential Expansiveness



Plasticity Chart



Namibia Technical Services cc

SUMMARY OF AGGREGATE TEST RESULTS

Tests done according to SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795

Project: Omitiomire

Date: 31-Aug-23

Source: PLT-BH01 : 2.80 - 9.00m

Report no: AG23/336

Order no:

Page no: 1

Sample no.	S/10595					
Description	PLT-BH01 : 2.80 - 9.00m					
Sieve Analysis	75.0 mm					
	63.0 mm					
	53.0 mm					
	37.5 mm					
	26.5 mm					
	19.0 mm					
	13.2 mm					
	9.5 mm					
	6.7 mm					
	4.75 mm					
	2.36 mm					
	1.18 mm					
	0.600 mm					
	0.425 mm					
	0.300 mm					
0.150 mm						
0.075 mm						
Fineness Modulus						
Loose Bulk Density (kg/m ³)						
Consolidated Bulk Density (kg/m ³)						
Bulk Relative Density (kg/m ³)						
Apparent Relative Density (kg/m ³)						
Water absorption (%)						
Sand Equivalent						
Organic Impurities						
Aggregate Crushing Value (dry)	19.3					
Aggregate Crushing Value (wet)	36.2					
10% FACT (dry) (kN)						
10% FACT (wet) (kN)						
Flakiness Index						
Riedel & Weber						
Average Least Dimension						
Los Angeles Abrasion (LAA)						
PI on LAA fines						
Mill Abrasion (MA)						
Deleterious Clay Content (MBT)						
Presence of Sugar						
Water Soluble Sulphates (% m/m)						
Water Soluble Salts (% m/m)						
Chloride Content (% m/m Cl)						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, nor any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

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SUMMARY OF TEST RESULTS

BASSON INDEX & BRE

Client : Knight Piesold (Pty) Ltd (Namibia)
PO Box 86062
Eros, Windhoek
Namibia

Report no : 23BI/09

Project no : 10795.0

Report date : 29-Aug-23

ALS Reference : I231538

Sample ID :

Page no : 1 of 2

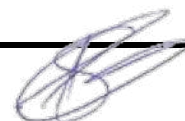
ANALYSIS	Unit	Sample Reference / description			
		S/10410	S/10418		
Sample position		HLP-TP05	HLP-TP13		
Depth from to (m)		0.0-1.1	0.3-2.25		
pH	1 / log H	6.9	6.6		
Temp at which above pH performed	°C	25.1	24.6		
pH Value at 25 °C	1 / log H	6.90	6.60		
Calcium carbonate-saturated pH (Calc) at 20 °C	1 / log H	10.90	10.70		
Electrical Conductivity at 25 °C	mS/m	1.6	2		
Total dissolved solids * (Calc)	mg/l	11	13		
Total Alkalinity as CaCO ₄	mg/l	5	5		
Total Hardness as CaCO ₃ (Calc)	mg/l	4	5		
Calcium Hardness as CaCO ₃ (Calc)	mg/l	2	3		
Calcium as Ca	mg/l	1	1.1		
Magnesium Hardness CaCO ₃ (Calc)	mg/l	2	2		
Magnesium ion (as Mg)	mg/l	0.6	0.6		
Total ammonium ion (as NH ₄)	mg/l	0.03	0.11		
Total Sulphate ion (as SO ₄)	mg/l	6	49		
Chloride ion (as Cl) (see note 1 below)	mg/l	5	5		
Total water soluble salts	mg/l	140	290		
Langelier Index at 20 °C (Calc)		-4.0	-4.2		
Ryznar Index at 20 °C (Calc)		15.0	15.0		
Corrosivity Ratio (Calc)		2.7	11.7		
Indices for standard local conditions (ie under conditions of laminar flow at 20°C)					
Leaching corrosion sub-index	LSCI	3201	3305		
Spalling corrosion sub-index	SCSI	1	5		
Aggressiveness index	Nc	3202	3310		

* TDS Calculated EC x 6.7

2:1 Distilled Water : Soil Extract

Important notes:

1. The above aggressiveness index is only applicable for conditions of laminar flow at a mean annual temperature of 20 °C.
2. For stagnant / turbulent conditions the aggressiveness index must be corrected.
3. For wet / dry cycling conditions (for example tidal zones) the aggressiveness index must be corrected.
4. For mean annual temperatures lower / higher than 20 °C the aggressiveness index must be corrected.



Namibia Technical Services cc

SUMMARY OF TEST RESULTS

BASSON INDEX & BRE

Client : Knight Piesold (Pty) Ltd (Namibia)
PO Box 86062
Eros, Windhoek
Namibia

Project no : 10799.0
Report date : 29-Aug-23
ALS Reference : I231538
Sample ID :

Report no : 23BI/09

Page no : 2 of 2

To correct for:	Multiply	By: (see Notes 2 to 5 below)
Turbulence	LSCI	1.75
Stagnance	LSCI	0.5
Temperature	LSCI,SCSI,N7	$(1+(0.05 \times (T-20)))$ where T = mean annual temperature
Wet-dry cycles	SCSI	$0.23 \times 10^{-6} \times Cl^- \times DTF \times CPA$ where DTF = dry time fraction and CPA = wet-dry cycles per annum

Note 1: Only if the concrete contains embedded steel

Note 2: To preserve the correct logical relationships when dealing with negative sub-indices (ie LSCI or SCSI having minus values) they should be multiplied by the reciprocal of the relevant factor indicated in this column.

Note 3: If more than one correction is required, multiply by the product of the individual correction factors

Note 4: Use subscript c to indicate that the index has been corrected, eg for turbulent conditions LSClc = LSCI x 1.75

Note 5: Round off corrected indices to the nearest 100

GUIDELINES FOR ASSESSING OVERALL AGGRESSIVENS (Nc):

Nc	Aggressiveness
Not greater than 300	None to mild
400 - 700	Mild to moderate
800 - 1000	High
= or > 1100	Very High

Aggressiveness Towards Concrete and Fibre Cement Pipes

Index	Aggressive	Neutral	Non-Aggressive
a) Stability pH (pHs)	> pH	= pH	< pH
b) Langelier Index	Neg. Value	Zero	Pos. Value
c) Ryznar Index	> 7.5	6 - 7	< 6

Corrosiveness Towards Metals

Corrosivity Ratio	> 0.2
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Sample Reference / description	Corrosivity Indices	Basson Index
S/10410	Corrosive	Very High
S/10418	Corrosive	Very High

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

Namibia Technical Services cc

SUMMARY OF TEST RESULTS

BASSON INDEX & BRE

Client : Knight Piesold (Pty) Ltd (Namibia)
 PO Box 86062
 Eros, Windhoek
 Namibia

Report no : 23BI/09

Project no : 10795.0
Report date : 29-Aug-23
ALS Reference : I231538
Sample ID :

Page no : 1 of 2


ANALYSIS	Unit	Sample Reference / description			
		S/10468	S/10470		
Sample position		SWD-TP19	SWD-TP07		
Depth from to (m)		1.4-2.4	1.4-1.9		
pH	1 / log H	7.8	8.1		
Temp at which above pH performed	°C	25.1	25.2		
pH Value at 25 °C	1 / log H	7.80	8.10		
Calcium carbonate-saturated pH (Calc) at 20 °C	1 / log H	8.70	9.00		
Electrical Conductivity at 25 °C	mS/m	12.1	13		
Total dissolved solids * (Calc)	mg/l	81	87		
Total Alkalinity as CaCO ₄	mg/l	55	57.5		
Total Hardness as CaCO ₃ (Calc)	mg/l	63	29		
Calcium Hardness as CaCO ₃ (Calc)	mg/l	35	18		
Calcium as Ca	mg/l	13.9	7.1		
Magnesium Hardness CaCO ₃ (Calc)	mg/l	28	11		
Magnesium ion (as Mg)	mg/l	6.9	2.7		
Total ammonium ion (as NH ₄)	mg/l	0.15	0.01		
Total Sulphate ion (as SO ₄)	mg/l	47	12		
Chloride ion (as Cl) (see note 1 below)	mg/l	5	12		
Total water soluble salts	mg/l	240	170		
Langelier Index at 20 °C (Calc)		-0.9	-0.8		
Ryznar Index at 20 °C (Calc)		9.7	9.9		
Corrosivity Ratio (Calc)		1.0	0.5		
Indices for standard local conditions (ie under conditions of laminar flow at 20°C)					
Leaching corrosion sub-index	LSCI	1040	1068		
Spalling corrosion sub-index	SCSI	6	3		
Aggressiveness index	Nc	1046	1071		

* TDS Calculated EC x 6.7

2:1 Distilled Water : Soil Extract

Important notes:

1. The above aggressiveness index is only applicable for conditions of laminar flow at a mean annual temperature of 20 °C.
2. For stagnant / turbulent conditions the aggressiveness index must be corrected.
3. For wet / dry cycling conditions (for example tidal zones) the aggressiveness index must be corrected.
4. For mean annual temperatures lower / higher than 20 °C the aggressiveness index must be corrected.



Namibia Technical Services cc

SUMMARY OF TEST RESULTS

BASSON INDEX & BRE

Client : Knight Piesold (Pty) Ltd (Namibia)
PO Box 86062
Eros, Windhoek
Namibia

Project no : 10799.0
Report date : 29-Aug-23
ALS Reference : I231538
Sample ID :

Report no : 23BI/09

Page no : 2 of 2

To correct for:	Multiply	By: (see Notes 2 to 5 below)
Turbulence	LSCI	1.75
Stagnance	LSCI	0.5
Temperature	LSCI,SCSI,N7	$(1+(0.05 \times (T-20)))$ where T = mean annual temperature
Wet-dry cycles	SCSI	$0.23 \times 10^{-6} \times CI \times DTF \times CPA$ where DTF = dry time fraction and CPA = wet-dry cycles per annum

Note 1: Only if the concrete contains embedded steel

Note 2: To preserve the correct logical relationships when dealing with negative sub-indices (ie LSCI or SCSI having minus values) they should be multiplied by the reciprocal of the relevant factor indicated in this column.

Note 3: If more than one correction is required, multiply by the product of the individual correction factors

Note 4: Use subscript c to indicate that the index has been corrected, eg for turbulent conditions $LSCIC = LSCI \times 1.75$

Note 5: Round off corrected indices to the nearest 100

GUIDELINES FOR ASSESSING OVERALL AGGRESSIVENS (Nc):

Nc	Aggressiveness
Not greater than 300	None to mild
400 - 700	Mild to moderate
800 - 1000	High
= or > 1100	Very High

Aggressiveness Towards Concrete and Fibre Cement Pipes

Index	Aggressive	Neutral	Non-Aggressive
a) Stability pH (pHs)	> pH	= pH	< pH
b) Langelier Index	Neg. Value	Zero	Pos. Value
c) Ryznar Index	> 7.5	6 - 7	< 6

Corrosiveness Towards Metals

Corrosivity Ratio	> 0.2
-------------------	-------

Sample Reference / description	Corrosivity Indices	Basson Index
S/10468	Corrosive	High to Very High
S/10470	Corrosive	High to Very High

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omission.

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	4.27	2.34	1.01	0.56	0.44	0.36
E-W	4.30	2.29	1.02	0.58	0.44	0.37

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	80	88	76	84	100	122
E-W	81	86	77	87	100	126

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	80	88	76	84	100	122
E-W	81	88	76	83	100	126

1

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	4.13	1.76	0.83	0.61	0.43	0.31
E-W	4.21	1.89	0.90	0.62	0.40	0.29

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	78	66	63	92	97	105
E-W	79	71	68	93	90	98

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	78	66	62	92	98	107
E-W	77	71	68	92	90	98

2

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.71	1.09	0.75	0.37	0.45	0.71
E-W	1.72	1.10	0.75	0.38	0.46	0.56

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	32	41	57	56	102	241
E-W	32	41	57	57	104	190

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	32	41	56	56	101	241
E-W	33	43	56	56	101	190

3

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.44	0.71	0.44	0.44	0.38	0.38
E-W	1.32	0.72	0.53	0.33	0.36	0.33

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	27	27	33	66	86	129
E-W	25	27	40	50	81	112

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	27	27	33	65	84	129
E-W	25	27	39	50	79	112

4

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.30	0.75	0.47	0.33	0.32	0.44
E-W	1.26	0.73	0.44	0.26	0.28	0.36

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	25	28	35	50	72	149
E-W	24	28	33	39	63	122

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	24	28	35	49	70	149
E-W	24	28	33	39	63	122

5

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.56	1.06	0.69	0.56	0.62	0.52
E-W	1.45	1.01	0.66	0.50	0.56	0.47

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	29	40	52	84	140	176
E-W	27	38	50	75	127	159

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	29	40	52	84	140	176
E-W	27	38	50	74	126	159

6

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.45	1.02	0.63	0.45	0.54	0.47
E-W	1.45	1.01	0.65	0.47	0.50	0.45

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	27	38	48	68	122	159
E-W	27	38	49	71	113	153

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	27	38	47	69	119	159
E-W	27	38	48	71	113	153

7

4P resistance (R)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	4.52	2.12	0.96	0.68	0.47	0.65
E-W	4.46	2.05	0.88	0.66	0.47	0.62

Wenner Resistivity (ρ) calculated from above resistance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	85	80	72	103	106	221
E-W	84	77	66	100	106	210

Wenner Resistivity (ρ) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$	Resistivity $\Omega.m$
N-S	85	80	72	103	106	221
E-W	84	77	66	99	103	210

8

Point Load Index Testing

ASTM D5731

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : W1301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : PL / 14

Page no : 1

Testing date : 28/Jul/23

Sample Preparation

Specimens cut with Diamond Table saw from Rock Core Samples

Core Size

HQ

63

Lab No	No.	Depth	Type	Orientation	W/L (mm)	D(mm)	P(kN)	D_e^2 (mm ²)	D_e (mm)	I_s (MPa)	F	$I_{s(50)}$ (MPa)	Estimated UCS Value (MPa)	C
S/10576	SWD-BH01	2.39 - 2.51	a	p	28.7	60.8	1.10	2222.88	47.15	0.49	0.97	0.48	12.01	24.91
S/10576	SWD-BH01	2.39 - 2.51	d	p	27.9	60.8	1.70	3696.64	60.80	0.46	1.09	0.50	12.51	24.91
S/10577	SWD-BH01	3.14 - 3.48	a	p	59.4	60.3	4.10	4562.83	67.55	0.90	1.14	1.03	25.53	24.82
S/10577	SWD-BH01	3.14 - 3.48	d	p	29.2	60.3	2.80	3636.09	60.30	0.77	1.09	0.84	20.79	24.82
S/10578	SWD-BH01	3.36 - 3.95	a	p	59.3	60.1	6.25	4540.04	67.38	1.38	1.14	1.57	39.02	24.78
S/10578	SWD-BH01	3.36 - 3.95	d	p	30.7	60.1	3.60	3612.01	60.10	1.00	1.09	1.08	26.83	24.78
S/10579	SWD-BH02	2.84 - 2.98	a	p	39.1	59.9	2.65	2983.55	54.62	0.89	1.04	0.92	22.87	24.74
S/10579	SWD-BH02	2.84 - 2.98	d	p	29.7	59.9	1.55	3588.01	59.90	0.43	1.08	0.47	11.59	24.74
S/10581	SWD-BH02	8.16 - 8.35	a	p	40.1	60.6	3.40	3095.62	55.64	1.10	1.05	1.15	28.67	24.87
S/10581	SWD-BH02	8.16 - 8.35	d	p	28	60.6	3.40	3672.36	60.60	0.93	1.09	1.01	25.11	24.87
S/10582	SWD-BH03	3.82 - 4.13	a	p	38.4	60.1	2.50	2939.92	54.22	0.85	1.04	0.88	21.86	24.78
S/10582	SWD-BH03	3.82 - 4.13	d	p	72.1	60.1	1.60	3612.01	60.10	0.44	1.09	0.48	11.92	24.78

d = diametrical;

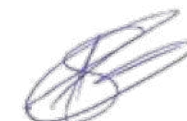
a = axial;

b = block;

i = irregular lump test;

// = parallel to planes of weakness;

p = perpendicular



Point Load Index Testing

ASTM D5731

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : WI301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : PL / 14

Page no : 2

Testing date : 28/Jul/23

Sample Preparation

Specimens cut with Diamond Table saw from Rock Core Samples

Core Size

HQ

63

Lab No	No.	Depth	Type	Orientation	W/L (mm)	D(mm)	P(kN)	D_e^2 (mm ²)	D_e (mm)	I_s (MPa)	F	$I_{s(50)}$ (MPa)	Estimated UCS Value (MPa)	C
S/10583	SWD-BH04	3.71 - 3.88	a	p	56.3	59.2	18.50	4245.81	65.16	4.36	1.13	4.91	120.83	24.62
S/10583	SWD-BH04	3.71 - 3.88	d	p	28.9	59.2	0.60	3504.64	59.20	0.17	1.08	0.18	4.55	24.62
S/10584	SWD-BH04	8.23 - 8.39	a	p	42.2	60.1	12.40	3230.85	56.84	3.84	1.06	4.07	100.76	24.78
S/10584	SWD-BH04	8.23 - 8.39	d	p	22.6	60.1	5.45	3612.01	60.10	1.51	1.09	1.64	40.62	24.78
S/10585	SWD-BH05	5.88 - 6.02	a	p	43.6	60.8	1.55	3376.92	58.11	0.46	1.07	0.49	12.23	24.91
S/10585	SWD-BH05	5.88 - 6.02	d	p	28.7	60.8	0.35	3696.64	60.80	0.09	1.09	0.10	2.58	24.91
S/10586	SWD-BH05	9.69 - 9.93	a	p	27.6	60.6	3.85	2130.65	46.16	1.81	0.96	1.74	43.36	24.87
S/10586	SWD-BH05	9.69 - 9.93	d	p	23.1	60.6	2.05	3672.36	60.60	0.56	1.09	0.61	15.14	24.87
S/10587	SWD-BH08	5.07 - 5.21	a	p	39.1	58.2	2.05	2898.88	53.84	0.71	1.03	0.73	17.86	24.43
S/10587	SWD-BH08	5.07 - 5.21	d	p	30.2	58.2	0.60	3387.24	58.20	0.18	1.07	0.19	4.63	24.43

d = diametrical;

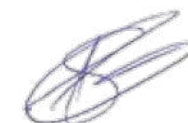
a = axial;

b = block;

i = irregular lump test;

// = parrallel to planes of weekness;

p = perpendicular



Point Load Index Testing

ASTM D5731

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : W1301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : PL / 14

Page no : 3

Testing date : 28/Jul/23

Sample Preparation

Specimens cut with Diamond Table saw from Rock Core Samples

Core Size

HQ

63

Lab No	No.	Depth	Type	Orientation	W/L (mm)	D(mm)	P(kN)	D_e^2 (mm ²)	D_e (mm)	I_s (MPa)	F	$I_{s(50)}$ (MPa)	Estimated UCS Value (MPa)	C
S/10588	HLP-BH01	6.18 - 6.31	a	p	27.2	60.9	0.85	2110.17	45.94	0.40	0.96	0.39	9.67	24.93
S/10588	HLP-BH01	6.18 - 6.31	d	p	22.8	60.9	0.50	3708.81	60.90	0.13	1.09	0.15	3.67	24.93
S/10589	HLP-BH01	8.35 - 8.60	a	p	38.3	60.3	1.75	2942.03	54.24	0.59	1.04	0.62	15.31	24.82
S/10589	HLP-BH01	8.35 - 8.60	a	p	46.5	60.3	1.60	3571.91	59.77	0.45	1.08	0.49	12.05	24.82
S/10590	HLP-BH02	5.35 - 5.65	a	p	35.9	60.6	0.95	2771.39	52.64	0.34	1.02	0.35	8.73	24.87
S/10590	HLP-BH02	5.35 - 5.65	d	p	29.8	60.6	0.65	3672.36	60.60	0.18	1.09	0.19	4.80	24.87
S/10591	HLP-BH03	2.27 - 2.41	a	p	26.1	60.1	0.40	1998.23	44.70	0.20	0.95	0.19	4.72	24.78
S/10591	HLP-BH03	2.27 - 2.41	d	p	22.1	60.1	0.35	3612.01	60.10	0.10	1.09	0.11	2.61	24.78
S/10592	HLP-BH03	3.73 - 3.93	a	p	51.3	60.3	1.45	3940.62	62.77	0.37	1.11	0.41	10.12	24.82
S/10592	HLP-BH03	3.73 - 3.93	d	p	31.9	60.3	1.00	3636.09	60.30	0.28	1.09	0.30	7.43	24.82
S/10594	HLP-BH04	5.19 - 5.35	a	p	38.5	60.6	1.95	2972.10	54.52	0.66	1.04	0.68	16.97	24.87
S/10594	HLP-BH04	5.19 - 5.35	d	p	30.1	60.6	1.05	3672.36	60.60	0.29	1.09	0.31	7.75	24.87

d = diametrical;

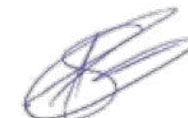
a = axial;

b = block;

i = irregular lump test;

// = parrallel to planes of weekness;

p = perpendicular



Point Load Index Testing

ASTM D5731

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : W1301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : PL / 14

Page no : 4

Testing date : 28/Jul/23

Sample Preparation

Specimens cut with Diamond Table saw from Rock Core Samples

Core Size

HQ

63

Lab No	No.	Depth	Type	Orientation	W/L (mm)	D(mm)	P(kN)	D_e^2 (mm ²)	D_e (mm)	I_s (MPa)	F	$I_{s(50)}$ (MPa)	Estimated UCS Value (MPa)	C
S/10595	PLT-BH01	2.80 - 9.00	a	p	58.6	60.1	6.55	4486.45	66.98	1.46	1.14	1.67	41.27	24.78
S/10595	PLT-BH01	2.80 - 9.00	d	p	30.1	60.1	5.25	3612.01	60.10	1.45	1.09	1.58	39.13	24.78
S/10596	PLT-BH02	4.73 - 4.94	a	p	40.5	60.3	1.60	3111.02	55.78	0.51	1.05	0.54	13.41	24.82
S/10596	PLT-BH02	4.73 - 4.94	a	p	45.1	60.3	0.90	3464.37	58.86	0.26	1.08	0.28	6.94	24.82
S/10597	PLT-BH02	8.09 - 8.37	a	p	58.5	60.8	16.00	4530.96	67.31	3.53	1.14	4.04	100.56	24.91
S/10598	PLT-BH03	1.78 - 1.98	a	p	32.3	84.4	1.55	3472.76	58.93	0.45	1.08	0.48	14.06	29.26
S/10598	PLT-BH03	1.78 - 1.98	d	p	41.2	84.4	3.25	7123.36	84.40	0.46	1.27	0.58	16.89	29.26
S/10599	PLT-BH03	6.33 - 6.55	a	p	30.4	60.2	3.90	2331.31	48.28	1.67	0.98	1.65	40.84	24.80
S/10599	PLT-BH03	6.33 - 6.55	d	//	28.1	60.2	2.70	3624.04	60.20	0.75	1.09	0.81	20.09	24.80

d = diametrical;

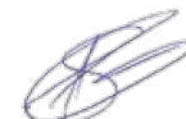
a = axial;

b = block;

i = irregular lump test;

// = parrallel to planes of weekness;

p = perpendicular





Point Load Index Testing

ASTM D5731

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : W1301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : PL / 14

Page no : 5

Testing date : 28/Jul/23

Sample Preparation

Specimens cut with Diamond Table saw from Rock Core Samples

Core Size

HQ

63

Lab No	No.	Depth	Type	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s(50)} (MPa)	Estimated UCS Value (MPa)	C
S/10601	WRD-BH01	5.13 - 5.30	a	p	28.6	60.5	0.50	2204.20	46.95	0.23	0.97	0.22	5.48	24.86
S/10601	WRD-BH01	5.13 - 5.30	d	p	27.5	60.5	0.91	3660.25	60.50	0.25	1.09	0.27	6.73	24.86
S/10602	WRD-BH01	7.75 - 7.88	a	p	42.3	60.4	0.40	3254.68	57.05	0.12	1.06	0.13	3.24	24.84

d = diametrical; a = axial; b = block; i = irregular lump test; // = parrallel to planes of weekness; p = perpendicular

CLIENT: STL

DATE: 28 August 2023

SAMPLES: 1 Sample (PO T 035)

ANALYSIS: Petrographic analysis & XRD

REPORT ON OMITIOMIRE - NTS-05

1. **SERVICE REQUESTED**

Petrographic description & XRD

2. **SAMPLE DESCRIPTION**

Rock fragment

3. **TESTS CONDUCTED**

1 x Thin Section, 1 x XRD

4. **DESCRIPTION & ANALYSIS**

A thin section was prepared from a rock fragment set in epoxy and the remainder of the sample was split and milled for XRD analysis.

NOTE:

Quantities of minerals shown in the petrographic description are largely based on amounts calculated from XRD analysis as this reflects a more representative composition.

In case the results do not correspond to the results of other analytical techniques, please let me know for further fine-tuning of XRD results.

XRD

The material was prepared for XRD analysis using a back-loading preparation method. It was analysed with a PANalytical Aeris diffractometer with a PIXcel detector and fixed slits with Fe-filtered Co-K α radiation. The phases were identified using X'PertHighscore plus software.

The relative phase amounts (weight %) were estimated using the Rietveld method. Mineral names may not reflect the actual compositions of minerals identified, but rather the mineral group. Due to crystallite size and preferred orientation effects, results may not be as accurate as shown. Mineral quantities observed in the overall sample in the thin section are in agreement with those calculated from XRD.

XRD Results:

Project Name	Omitiomire
Project no	NTS-05

Client No	Depth (m)	Our sample No	Test(s)
PLT-BH01/S/10595	2.80 - 9.0	NTS-05-44	Petrography and ASR comment

	Quartz	Microcline	Plagioclase	Biotite
S/10595	36.0	28.8	33.5	1.7

PETROGRAPHIC DESCRIPTIONS

PLT-BH01/S/10595

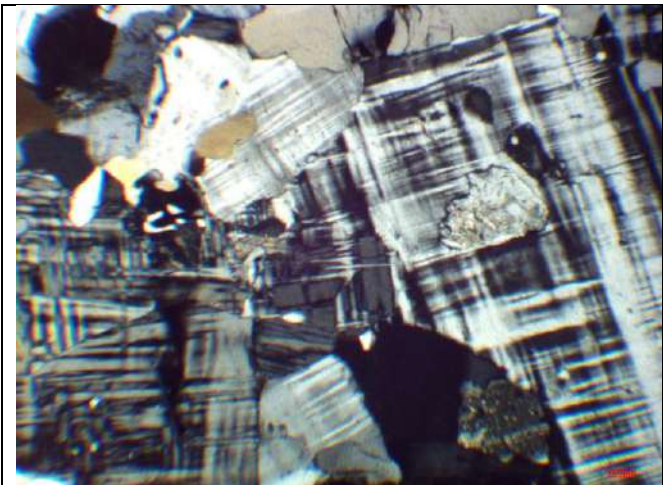
Sample type: Granite

Macroscopic description: The sample is composed of a rock fragment of a light grey colour with a small to medium grain size.

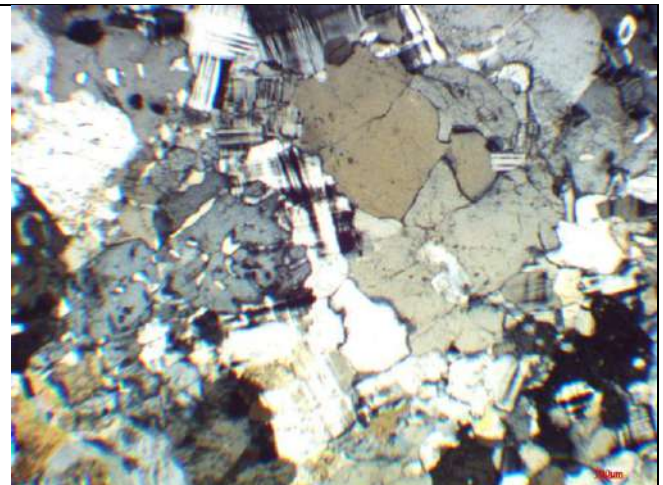
Microscopic description: The sample investigated in the thin section overall comprises quartz crystals (30-40%), plagioclase (25-35%), microcline (25-35%), and biotite (~2%).

The rock may represent granite where the largest portion is composed of roughly subhedral plagioclase (albite) with faint albite twinning. Microcline appears as anhedral crystals with cross-hatch twinning. Quartz occurs as anhedral crystals with a high degree of undulatory extinction. Isolated biotite flakes occur on grain boundaries and as small clusters.

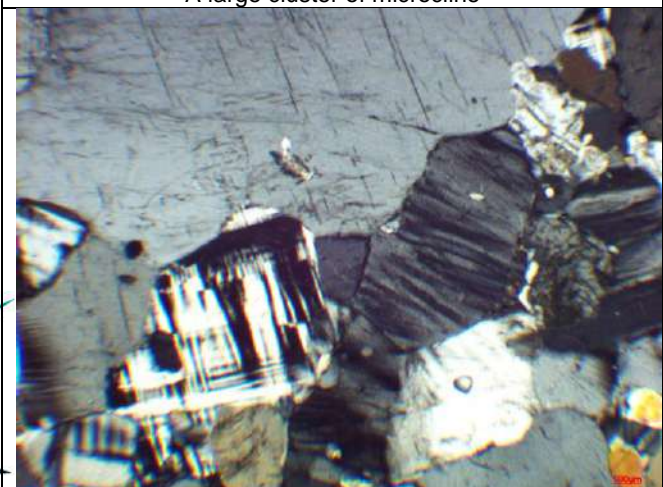
Grain size data: 100 – 1500 microns



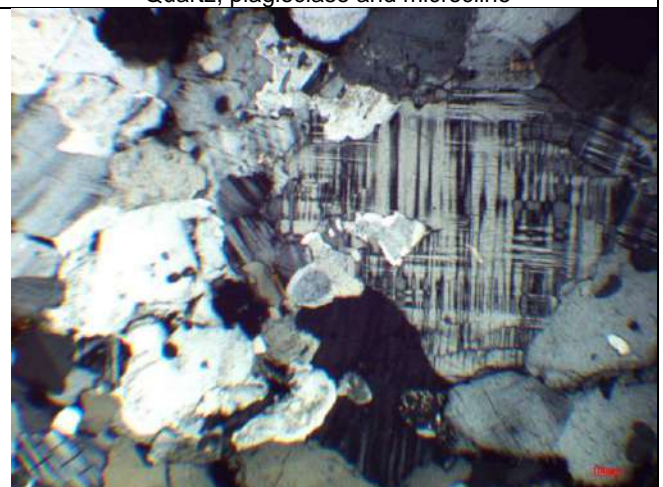
Photomicrograph: Cross-polarized light. 40x magnification
A large cluster of microcline



Photomicrograph: Cross-polarized light. 40x magnification.
Quartz, plagioclase and microcline



Photomicrograph: Cross-polarized light. 40x magnification.
Quartz, feldspar, and minor biotite



Photomicrograph: Cross-polarized light. 40x magnification.
Quartz, plagioclase and microcline

5. COMMENTS

Rock types are based on mineralogy and texture rather than stratigraphic position.

Depending on the intended use of the material the following comments can be made:

- Due to the presence of highly strained quartz and quartz with interlobate grain boundaries in the sample, a reaction with alkalis in cement can occur - unless otherwise shown by additional tests.
- The mica content consists of about 2% biotite.
- The sample may represent granite.
- It must be pointed out that the composition of rocks can vary over short distances in the field.

Analyst: Wiebke Grote



Authorized: Dr. Sabine Verryn (Pr.Sci.Nat)



Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client : Knight Piesold (Pty) Ltd (Namibia)	Job No : 10795
Project : WI301-00478/06 : OMITIOMIRE	Drilling/delivery date : 28/Jun/23
Report no : RC / 14	Testing date : 28/Jul/23
Page no : 1	

Testing detail	Core no. C / 10575	Core no. C / 10577	Core no. C / 10580	Core no. C / 10582	Core no. C / 10586
Rock type	Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	Moderately weathered, grey speckled black and white, soft to medium hard rock, gneiss	Highly to moderately weathered, grey to brown speckled black streaked white, medium hard rock, gneiss
Core marking	SWD-BH01	SWD-BH01	SWD-BH02	SWD-BH03	SWD-BH05
Core depth	2.04 - 2.26	3.14 - 3.48	7.96 - 8.16	3.82 - 4.13	9.69 - 9.93
Trimming method	cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade
Method of end preparation	grinding	grinding	grinding	grinding	grinding
Core length trimmed (mm)	135	135	126	139	124
Core length capped / ground (mm)	132	132	121	135	120
Core diameter (mm)	59	60	61	60	60
Core length / diameter ratio (2.0)	2.29	2.25	2.08	2.31	2.05
Failing load (kN)	22	66	96	34	73
Unconfined Compressive Strength (MPa)	8.0	23.5	33.5	12.0	25.5
Notes on failure	normal	normal	normal	normal	normal

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, nor for any erroneous statement or opinion contained in a report based on such tests. If a test report is published or reproduced by the client, it will be done in full, without any omission.



Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : WI301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : RC / 14

Page no : 2

Testing date : 28/Jul/23

Testing detail	Core no. C / 10590	Core no. C / 10593			
Rock type	Highly to completely weathered, brown to grey, banded white, very soft rock gneiss	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss			
Core marking	HLP-BH02	HLP-BH04			
Core depth	5.35 - 5.65	2.66 - 2.86			
Trimming method	cutting table with diamond blade	cutting table with diamond blade			
Method of end preparation	grinding	grinding			
Core length trimmed (mm)	128	132			
Core length capped / ground (mm)	122	128			
Core diameter (mm)	60	61			
Core length / diameter ratio (2.0)	2.13	2.17			
Failing load (kN)	22	21			
Unconfined Compressive Strength (MPa)	8.0	7.0			
Notes on failure	normal	normal			

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Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client : Knight Piesold (Pty) Ltd (Namibia)	Job No : 10795
Project : WI301-00478/06 : OMITIOMIRE	Drilling/delivery date : 28/Jun/23
Report no : RC / 14	Testing date : 28/Jul/23
Page no : 3	

Testing detail	Core no. C / 10597	Core no. C / 10599			
Rock type	Moderately weathered, light brown to grey speckled black and streaked white, medium hard rock, granitic gneiss	Moderately weathered, dark grey to green speckled black streaked whit, medium hard rock, gneiss			
Core marking	PLT-BH02	PLT-BH03			
Core depth	8.09 - 8.37	6.33 - 6.55			
Trimming method	cutting table with diamond blade	cutting table with diamond blade			
Method of end preparation	grinding	grinding			
Core length trimmed (mm)	126	90			
Core length capped / ground (mm)	120	84			
Core diameter (mm)	60	61			
Core length / diameter ratio (2.0)	2.10	1.48			
Failing load (kN)	119	30			
Unconfined Compressive Strength (MPa)	42.0	10.5			
Notes on failure	normal	normal			

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Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client : Knight Piesold (Pty) Ltd (Namibia)

Job No : 10795

Project : WI301-00478/06 : OMITIOMIRE

Drilling/delivery date : 28/Jun/23

Report no : RC / 14

Page no : 4

Testing date : 28/Jul/23

Testing detail	Core no. C / 10600				
Rock type	Pale brown blotched white, hardpan calcrete of soft rock strength				
Core marking	WRD-BH01				
Core depth	1.12 - 1.35				
Trimming method	cutting table with diamond blade				
Method of end preparation	grinding				
Core length trimmed (mm)	173				
Core length capped / ground (mm)	171				
Core diameter (mm)	85				
Core length / diameter ratio (2.0)	2.03				
Failing load (kN)	29				
Unconfined Compressive Strength (MPa)	5.0				
Notes on failure	normal				

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, nor for any erroneous statement or opinion contained in a report based on such tests. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



Quality | Excellence | On-Time

Client Name: Namibia Technical Services
Project Name: Omitionire
Sample: HLP-TP26
Depth: (m) 0.0 - 1.1

Job Number: NTS-05
Lab Number: NTS-05-37
Date: 08/11/2023

SHEARBOX TEST

General Test Data

Type of Test:	Slow Drained, saturated
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 2030kg/m ³ at 7.8%
Normal Stresses (kPa):	150, 300, 600
Rate of Shear (mm/min):	0.011
Comments:	-

Initial Specimen Details

		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	8.2	7.9	8.0
Dry Density	g/cm ³	1.818	1.822	1.821
Void Ratio	-	0.603	0.600	0.601
Degree of Saturation	%	39.5	38.6	38.9
Particle Density (SG)	-	2.915 - Determined		

Final Specimen Details

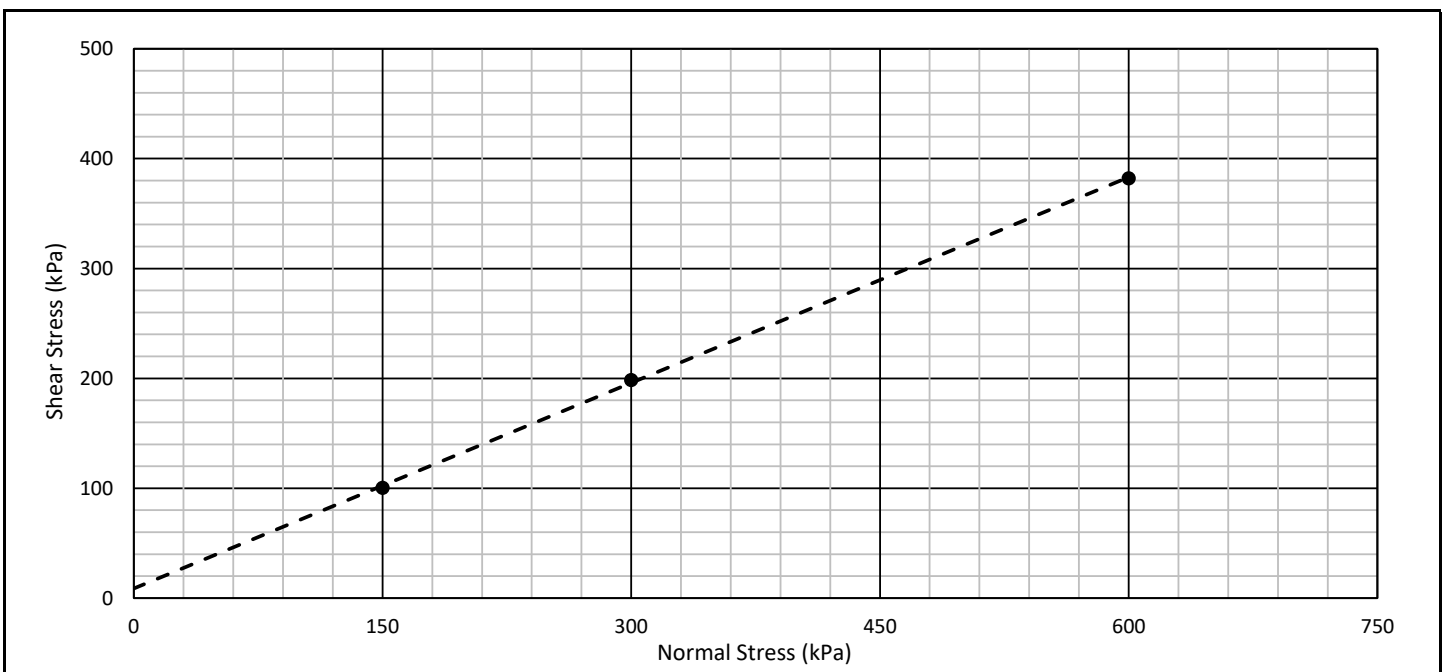
		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	14.2	16.6	15.2

Shear Details

Maximum Shear Stress:	kPa	100.6	198.6	382.2
Strain at Maximum Shear Stress:	mm	9.11	9.33	10.00

Shear Results

ϕ'	Deg.	32
c'	kPa	9



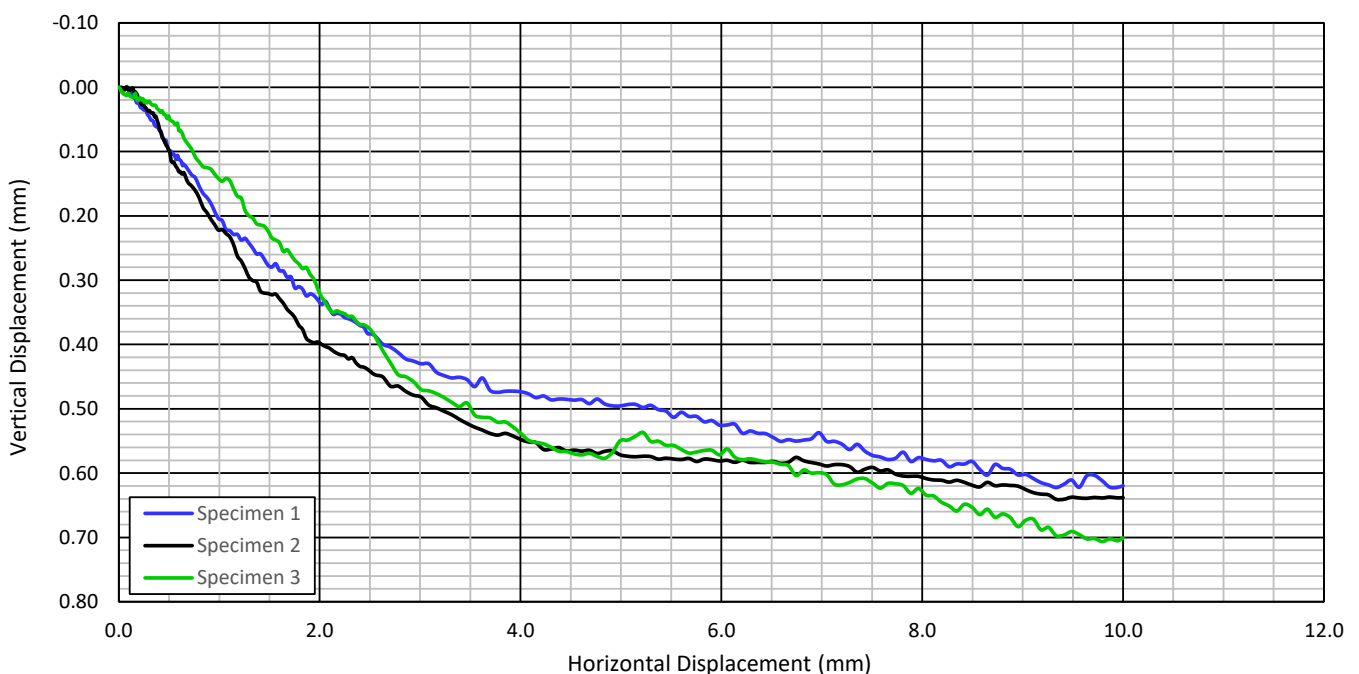
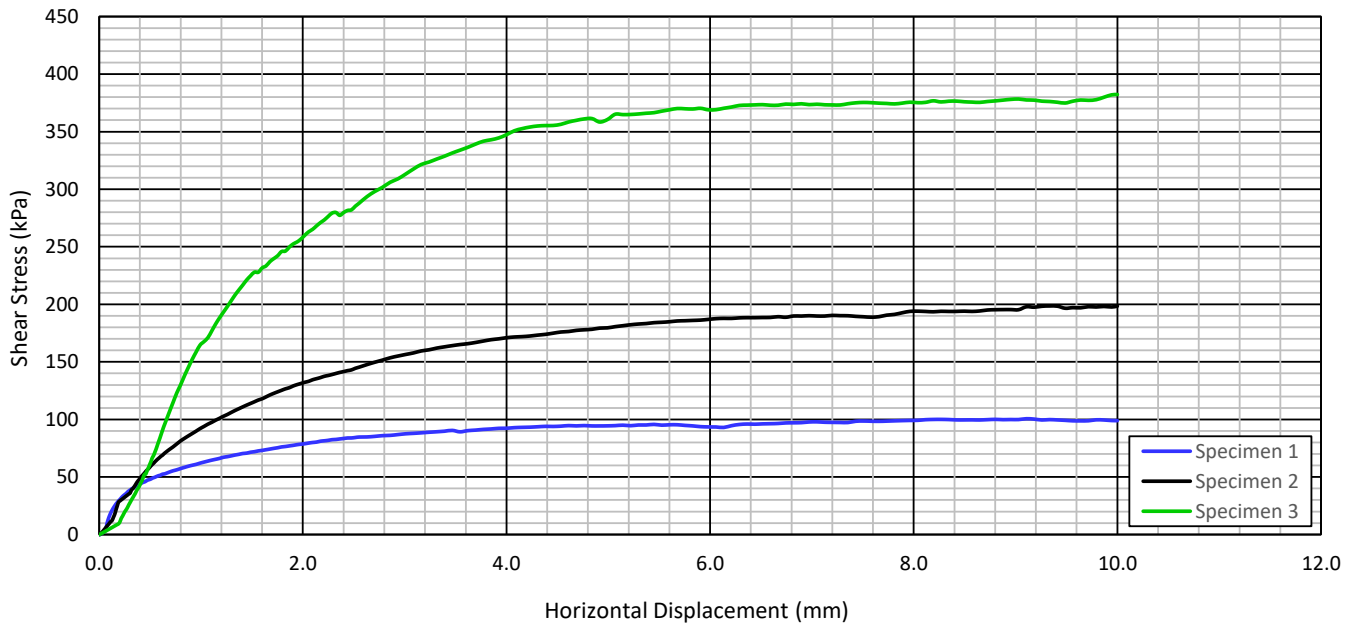


Quality | Excellence | On Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: HLP-TP26
Depth: (m) 0.0 - 1.1

Job Number: NTS-05
Lab Number: NTS-05-37
Date: 08/11/2023

SHEARBOX TEST





Quality | Excellence | On-Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: HLP-TP03
Depth: (m) 2.3 - 3.1

Job Number: NTS-05
Lab Number: NTS-05-38
Date: 08/11/2023

SHEARBOX TEST

General Test Data

Type of Test:	Slow Drained, saturated
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1974kg/m ³ at 11.0%
Normal Stresses (kPa):	150, 300, 600
Rate of Shear (mm/min):	0.025
Comments:	-

Initial Specimen Details

		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	11.1	11.5	11.1
Dry Density	g/cm ³	1.779	1.771	1.777
Void Ratio	-	0.658	0.665	0.659
Degree of Saturation	%	49.5	51.1	49.9
Particle Density (SG)	-	2.949 - Determined		

Final Specimen Details

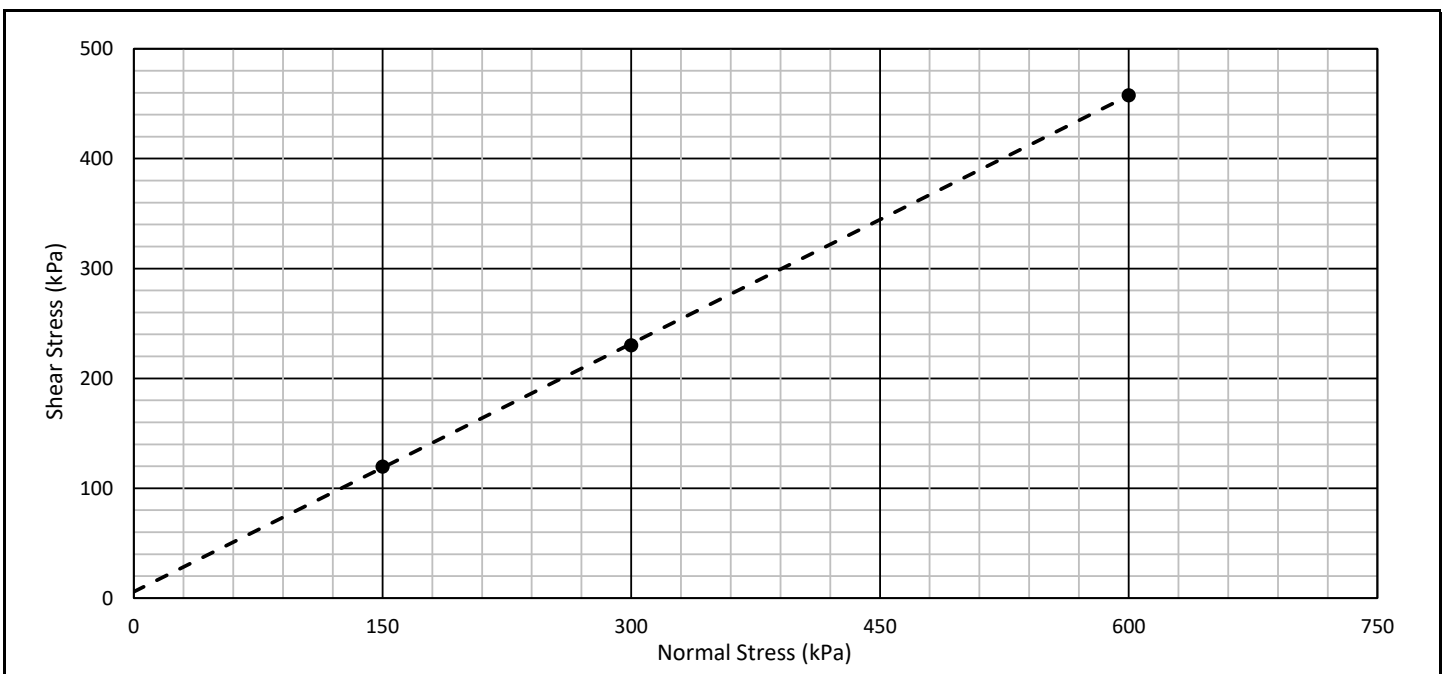
Moisture Content	%	22.2	20.5	20.1
------------------	---	------	------	------

Shear Details

Maximum Shear Stress:	kPa	119.7	230.2	457.8
Strain at Maximum Shear Stress:	mm	9.95	7.51	7.36

Shear Results

ϕ'	Deg.	37
c'	kPa	6



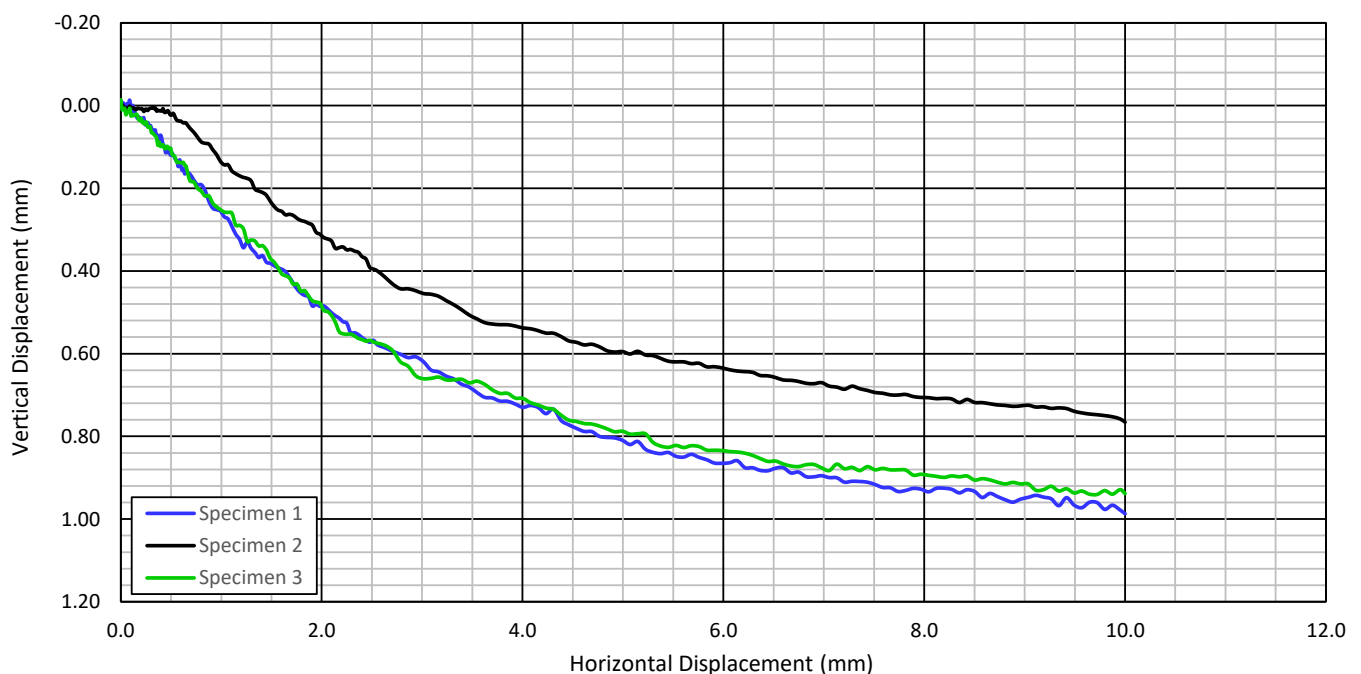
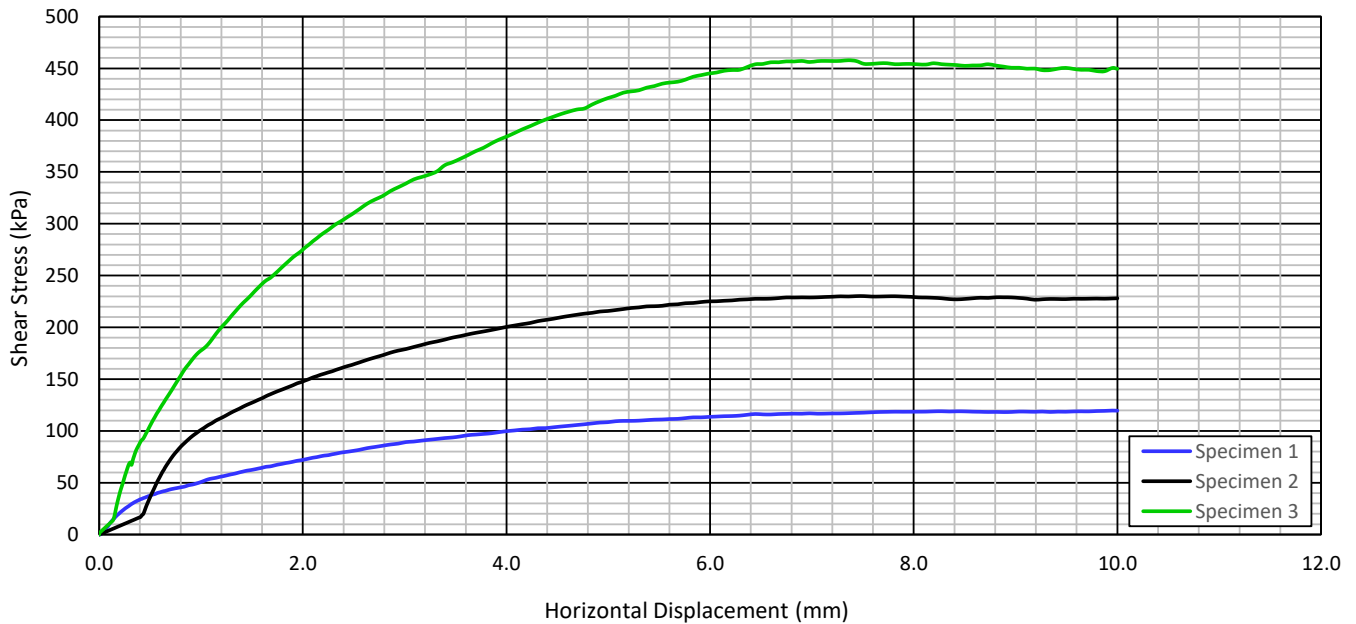


Quality | Excellence | On Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: HLP-TP03
Depth: (m) 2.3 - 3.1

Job Number: NTS-05
Lab Number: NTS-05-38
Date: 08/11/2023

SHEARBOX TEST





Quality | Excellence | On-Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: SWD-TP15
Depth: (m) 0.5 - 2.5

Job Number: NTS-05
Lab Number: NTS-05-39
Date: 08/11/2023

SHEARBOX TEST

General Test Data

Type of Test:	Slow Drained, saturated
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 2055kg/m ³ at 9.7%
Normal Stresses (kPa):	150, 300, 600
Rate of Shear (mm/min):	0.011
Comments:	-

Initial Specimen Details

		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	10.0	10.1	9.9
Dry Density	g/cm ³	1.847	1.845	1.848
Void Ratio	-	0.594	0.595	0.593
Degree of Saturation	%	49.5	49.8	49.2
Particle Density (SG)	-	2.944 - Determined		

Final Specimen Details

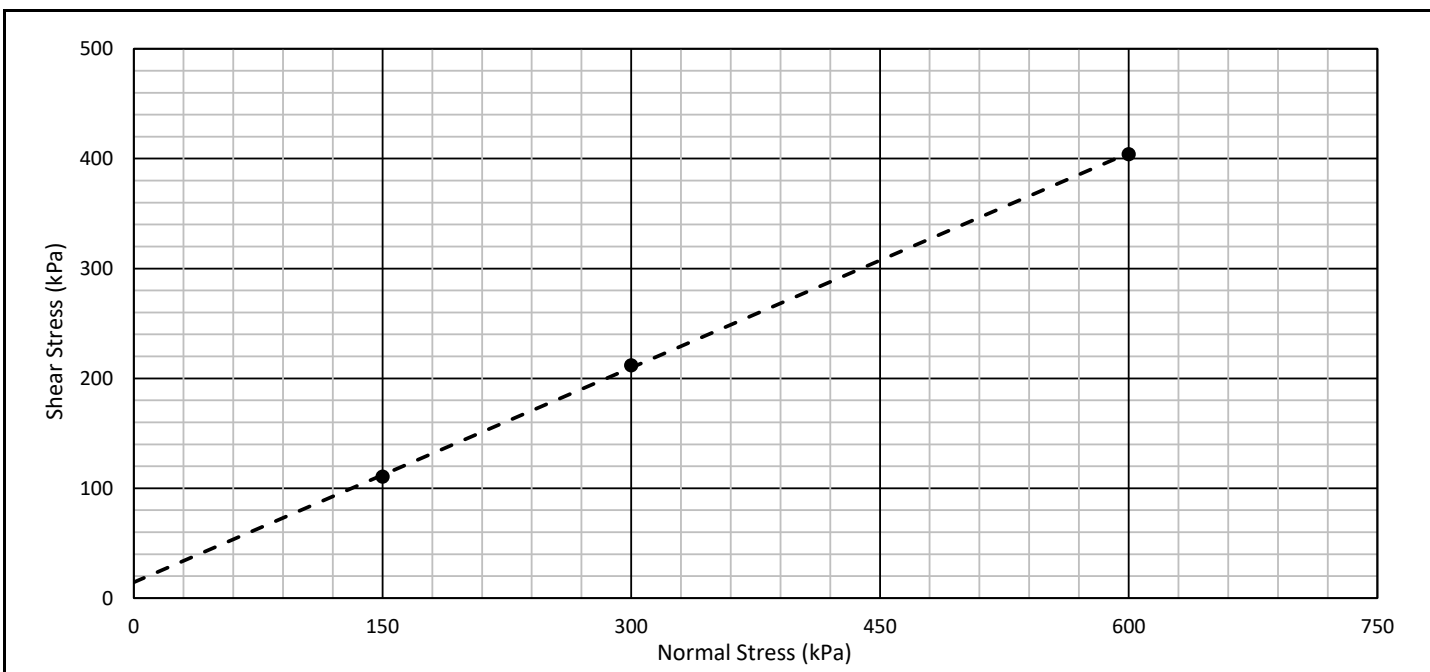
		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	18.5	17.0	17.4

Shear Details

		Specimen 1	Specimen 2	Specimen 3
Maximum Shear Stress:	kPa	110.6	212.0	404.3
Strain at Maximum Shear Stress:	mm	10.00	10.00	10.00

Shear Results

ϕ'	Deg.	33
c'	kPa	14



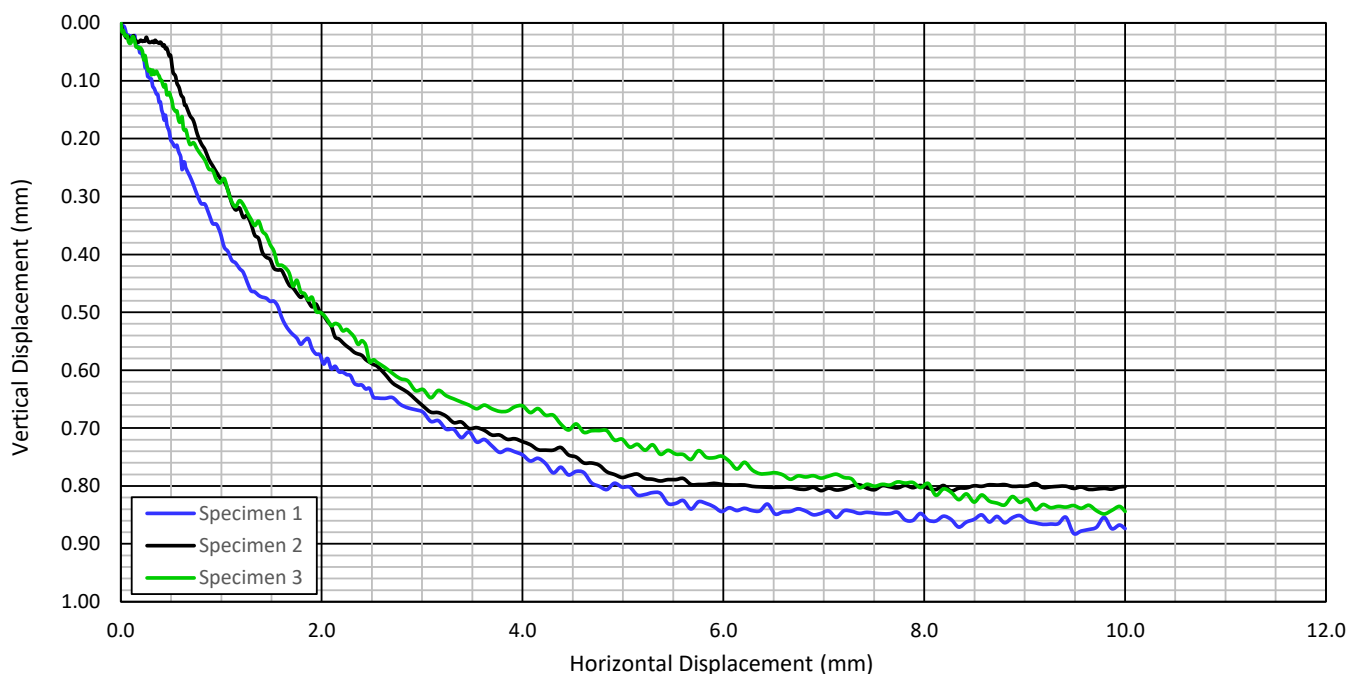
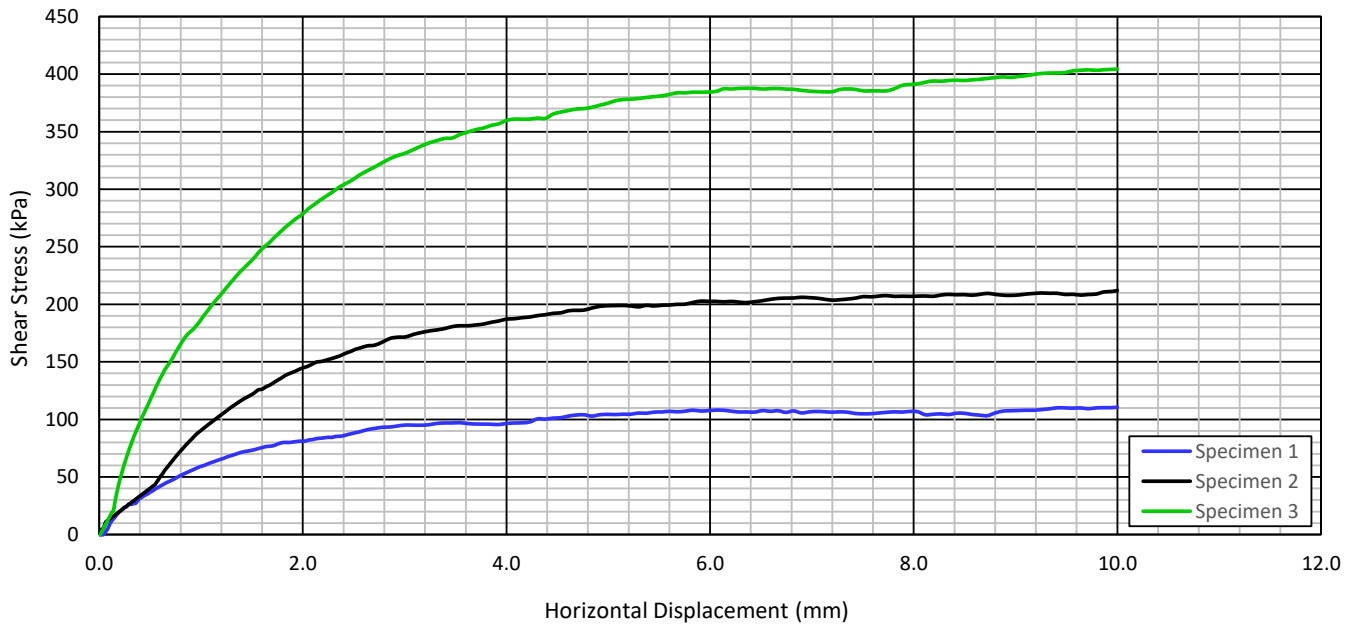


Quality | Excellence | On Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: SWD-TP15
Depth: (m) 0.5 - 2.5

Job Number: NTS-05
Lab Number: NTS-05-39
Date: 08/11/2023

SHEARBOX TEST





Quality | Excellence | On-Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: WRD-TP04
Depth: (m) 0.4 - 1.5

Job Number: NTS-05
Lab Number: NTS-05-42
Date: 08/11/2023

SHEARBOX TEST

General Test Data

Type of Test:	Slow Drained, saturated
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1975kg/m ³ at 11.0%
Normal Stresses (kPa):	150, 300, 600
Rate of Shear (mm/min):	0.025
Comments:	-

Initial Specimen Details

		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	11.3	11.5	11.1
Dry Density	g/cm ³	1.810	1.805	1.812
Void Ratio	-	0.501	0.505	0.500
Degree of Saturation	%	61.0	62.1	60.6
Particle Density (SG)	-	2.718 - Determined		

Final Specimen Details

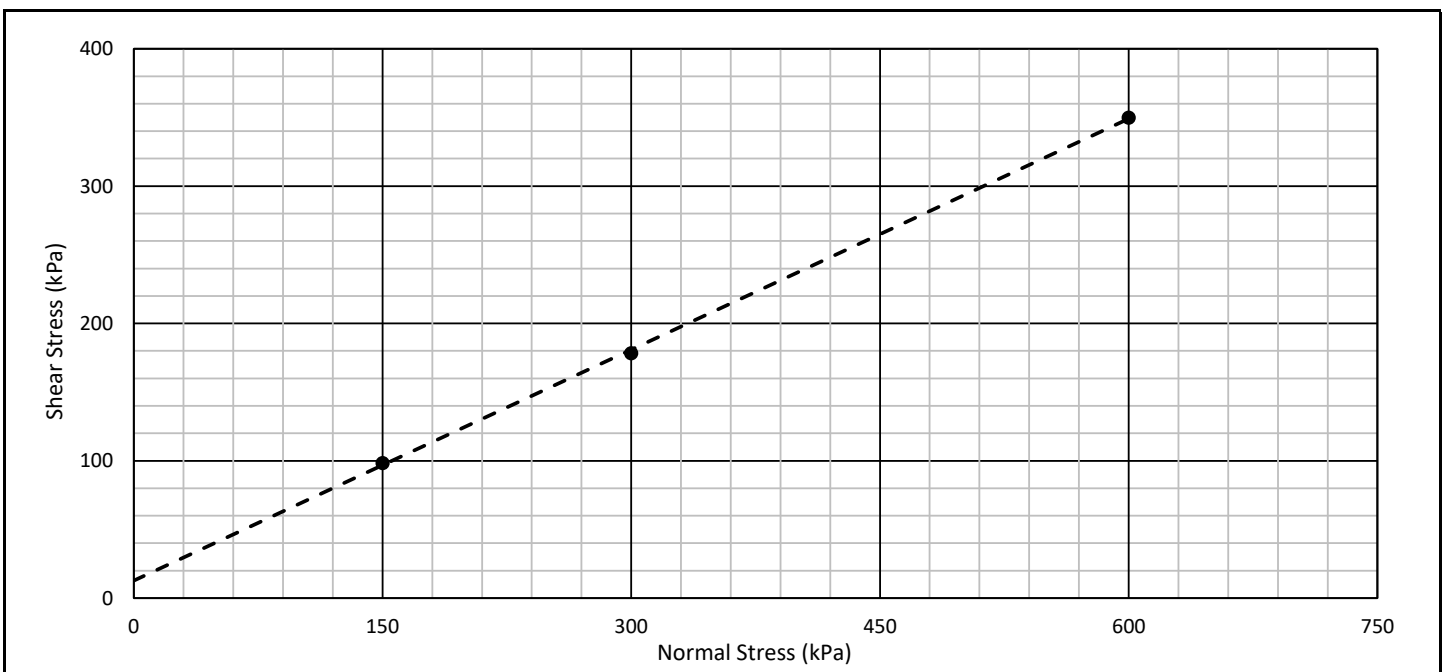
Moisture Content	%	22.2	20.7	26.1
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Shear Details

Maximum Shear Stress:	kPa	98.5	178.4	349.9
Strain at Maximum Shear Stress:	mm	9.87	10.00	9.95

Shear Results

ϕ'	Deg.	29
c'	kPa	13



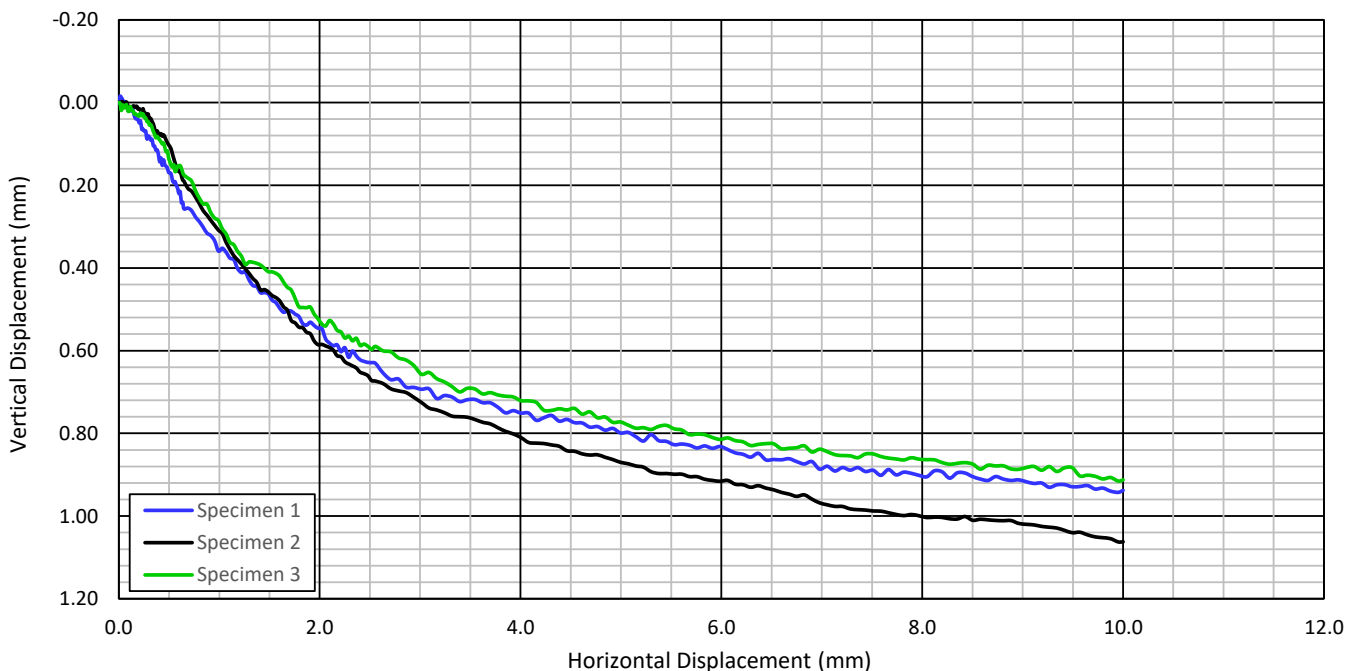
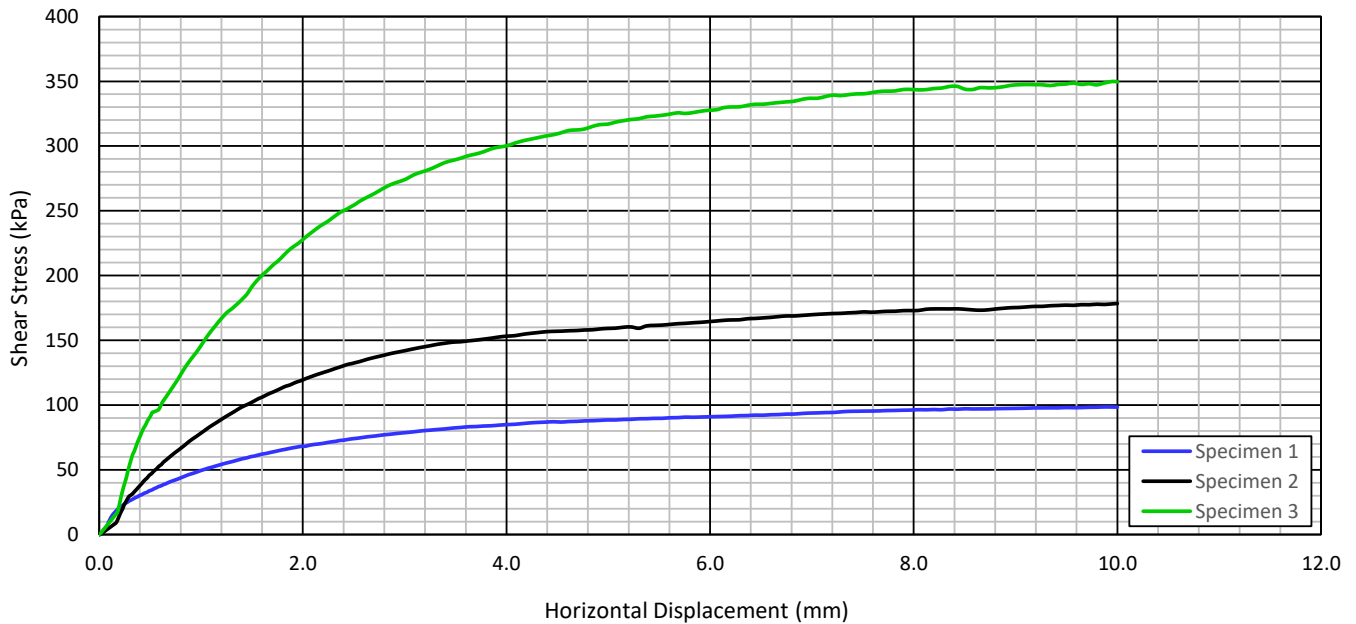


Quality | Excellence | On Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: WRD-TP04
Depth: (m) 0.4 - 1.5

Job Number: NTS-05
Lab Number: NTS-05-42
Date: 08/11/2023

SHEARBOX TEST





Quality | Excellence | On-Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: WRD-TP35
Depth: (m) 2.1 - 4.1

Job Number: NTS-05
Lab Number: NTS-05-43
Date: 10/11/2023

SHEARBOX TEST

General Test Data

Type of Test:	Slow Drained, saturated
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1946kg/m ³ at 11.2%
Normal Stresses (kPa):	150, 300, 600
Rate of Shear (mm/min):	0.011
Comments:	-

Initial Specimen Details

		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	11.4	11.3	11.5
Dry Density	g/cm ³	1.751	1.753	1.749
Void Ratio	-	0.705	0.704	0.707
Degree of Saturation	%	48.1	47.8	48.5
Particle Density (SG)	-	2.987 - Determined		

Final Specimen Details

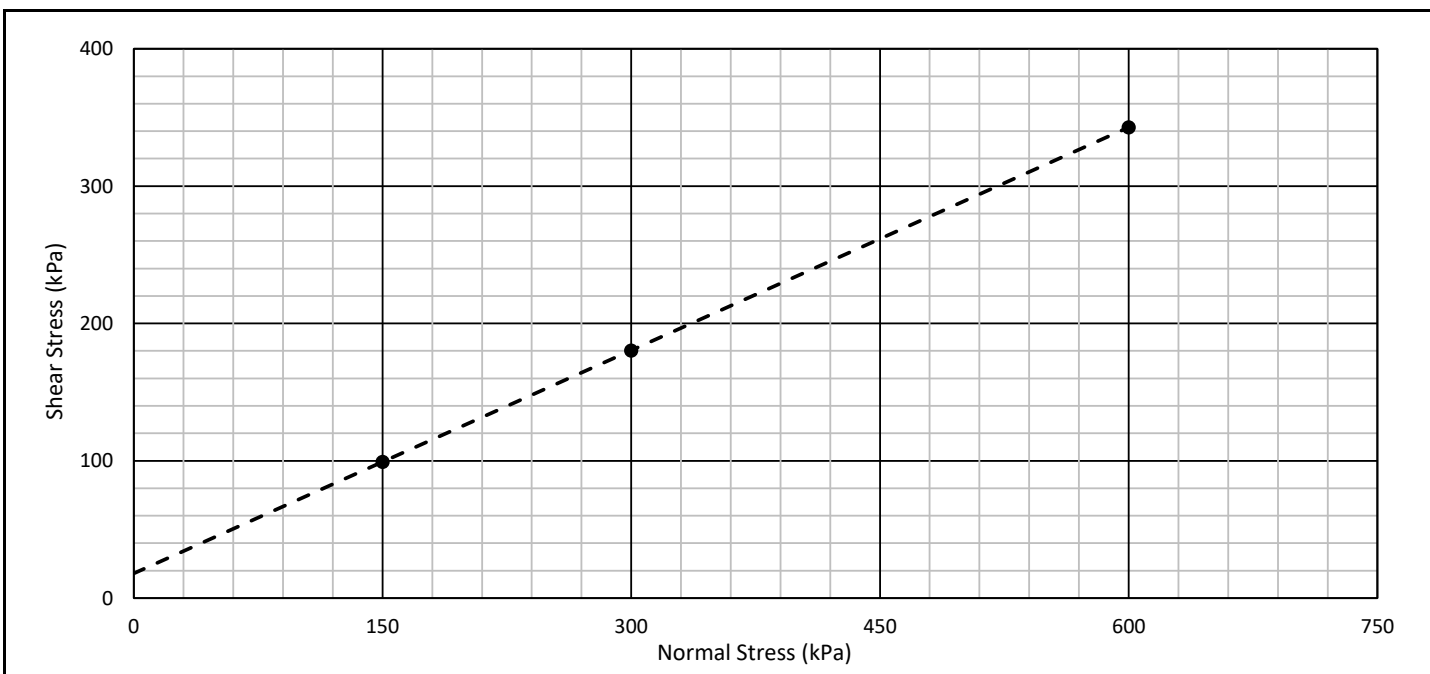
		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	23.2	20.4	20.4

Shear Details

		Specimen 1	Specimen 2	Specimen 3
Maximum Shear Stress:	kPa	99.3	180.2	342.9
Strain at Maximum Shear Stress:	mm	8.34	10.00	10.00

Shear Results

ϕ'	Deg.	28
c'	kPa	18





Quality | Excellence | On Time

Client Name: Namibia Technical Services
Project Name: Omitiomire
Sample: WRD-TP35
Depth: (m) 2.1 - 4.1

Job Number: NTS-05
Lab Number: NTS-05-43
Date: 10/11/2023

SHEARBOX TEST

