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OMITIOMIRE COPPER PROJECT: HEAP LEACH EARTHWORKS AND FOUNDATION DESIGN, PHASE 2 GROUND WATER AND SURFACE WATER STUDIES FEASIBILITY LEVEL GEOTECHNICAL INVESTIGATION - FACTUAL AND INTERPRETIVE REPORT

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TABLE OF CONTENTS

PAGE

Table	of Contents.		i
1.0	Introduction	on	1
1.1	Background	J	1
1.2	Scope of we	ork	1
1.3	Previous In	vestigations	2
2.0	Site Descri	ption	3
2.1		ation	
2.2	•	/	
2.3			
2.4	Seismicity		3
3.0	Geotechnic	cal Methods of Investigation	5
3.1		ıl Testing	
3.2		estigation	
3.3	Borehole In	vestigation	6
3.4	Laboratory	Testing	6
4.0	Geophysic	al Testing	8
5.0	Typical Pro	ofile	9
5.1	Heap Leach	n Pad (HLP)	9
	5.1.1	Alluvium	9
	5.1.2	Pedogenic soils	9
	5.1.3	Residual gneiss	. 10
	5.1.4	Gneiss bedrock	. 10
	5.1.5	General	. 10
5.2	Processing	Plant (Plant)	. 10
	5.2.1	Alluvium	. 10
	5.2.2	Pedogenic soils	. 11
	5.2.3	Residual gneiss	. 11
	5.2.4	Gneiss bedrock	. 11
	5.2.5	General	. 11
5.3	Stormwater	Diversion Channel (SWD)	. 11
	5.3.1	Alluvium	. 11
	5.3.2	Pedogenic soils	. 12
	5.3.3	Residual gneiss	. 12
	5.3.4	Gneiss bedrock	. 12
	5.3.5	General	. 13
5.4	New Road	(RD)	. 13
	5.4.1	Alluvium	. 13



	5.4.2	Pedogenic soils	14
	5.4.3	Gneiss bedrock	14
	5.4.4	General	14
5.5	Waste R	Rock Dump (WRD)	14
	5.5.1	Alluvium	14
	5.5.2	Pedogenic soils	14
	5.5.3	Residual gneiss	15
	5.5.4	Gneiss bedrock	15
	5.5.5	General	15
5.6	Potential	ll Borrow Sources (WRD and AM)	16
6.0	Laborate	ory testing	17
6.1	Alluvium	1	17
	6.1.1	Fine alluvium	17
	6.1.2	Coarse alluvium	17
6.2	Pedoger	nic soils	18
	6.2.1	Calcareous/ Calcified alluvium	18
	6.2.2	Ferruginous/ Ferriginised alluvium	18
	6.2.3	Nodular and honeycomb calcrete alluvium	19
	6.2.4	Hardpan calcrete	19
6.3	Residual	ll gneiss	19
6.4	Gneiss b	oedrock	19
	6.4.1	Uniaxial Compressive Strength (UCS)	19
6.5	Shear st	trength	20
7.0	Geotech	hnical Evaluation and Recommendations	22
7.1	Heap Le	each Pad	22
	7.1.1	Discussion	22
	7.1.2	Founding Recommendations	22
	7.1.3	Excavatability	23
7.2	HLP and	cillary structures and Ponds	23
	7.2.1	Founding Recommendations	24
	7.2.2	Excavatability	24
7.3	Processi	ing Plant	24
	7.3.1	Founding Recommendations	25
	7.3.2	Excavatability	25
7.4	Stormwa	ater Diversion Channel (SWD)	25
	7.4.1	Inlet	25
	7.4.2	Channel	27
	7.4.3	Outlet	28
	7.4.4	Volume Assessment	29
7.5	New Roa	ad (RD)	33
	7.5.1	Crossings 1 and 2	34
	7.5.2	Crossings 3 and 4	35
	7.5.3	Road Alignment	36
7.6	Waste R	Rock Dump (WRD)	39
	7.6.1	Area A	39
	7.6.2	Area B	40



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

	7.6.3	Recommendations	41
	7.6.4	Excavatability	41
7.7	Potentia	Excavatabilityl Borrow Sources	41
	7.7.1	Volume Assessment	43
	7.7.2	Excavatability	44
7.8	Material	Excavatabilityreuse	44
8.0	Referen	ces	45
9.0	Certifica	ation	46

APPENDICES

- Appendix A Geotechnical Site investigation Program
- Appendix A1 Site Locality Map
- Appendix A2 Layout of Investigated Points
- Appendix B Summary Tables for Soil Profiles, Borehole Logs, SPT Test Results and Laboratory Test Results
- Appendix B1 Summary of Test Pits
- Appendix B2 Summary of Boreholes
- Appendix B3 Summary of SPT Results
- Appendix B4 Summary Laboratory Test Results
- Appendix C Test Pit and Borehole Logs
- Appendix C1 Test Pit Profiles
- Appendix C2 Borehole Logs
- Appendix D Photographs of Test Pits and Borehole Cores
- Appendix D1 Test Pit Photographs
- Appendix D2 Borehole Core Photographs
- Appendix E Terminologies and Conventions used in Soil and Rock Logging
- Appendix F Geophysical Testing Report
- Appendix G Laboratory Test Results



ABBREVIATIONS

AAR	Alkaline Aggregate Reactivity (AAR)
	Additional Materials
BH	Borehole
CBR	California Bearing Ratio
Cu	Copper
CUT	Consolidated Undrained Triaxial
DSS	Direct Shear Strength Test
EOH	End of Hole
ERT	Electrical Resistivity Tomography
GEM	Global Earthquake Model
GSHM	Global Seismic Hazard Map
HLP	Heap Leach Pad
l _{s(50)}	Corrected Point Load Index
KP	Knight Piésold Consulting (Pty) Ltd
MDD	Maximum Dry Density
ML	Mining License
MM/A	Millimeters per annum
Myr	Million years
NGL	Natural Ground Level
OCP	Omitiomire Copper Project
OMC	Optimum Moisture Content
PGA	Peak Ground Acceleration
RD	Road
RQD	Rock Quality Designation
SPT	Standard Penetration Test
SWD	Stormwater Diversion Channel
TP	Test Pit
UCS	Uniaxial Compressive Strength
WGS	World Geodetic System 1984
WRD	Waste Rock Dump



1.0 INTRODUCTION

1.1 BACKGROUND

Knight Piésold Consulting (KP) was appointed by Craton Mining and Exploration Pty (Ltd) to provide feasibility level geotechnical investigation services to support engineering designs for the Bankable Feasibility Study of the Omitiomire Copper Project (OCP) in Namibia.

Omico through its Namibian subsidiary, Craton Mining and Exploration (Pty) Ltd (Craton), holds Mining Licence ML197 and Exclusive Prospecting Licence EPL8550, together a 30,000 Ha licence area which makes up the Omitiomire Copper Project. The mining licence is valid until March 2036. The development base case anticipates the production of 30,000 tonnes per annum of LME Grade A copper cathode for at least 15 years, targeting only open-pit mineralisation [Omico, November 2022 Press Release].

The Project is located 120km northeast from Windhoek in central Namibia and will comprise an open pit, a processing plant and heap leaching facility with associated crushing, agglomeration, stacking system, leaching process and SE/EW copper cathode extraction. The Project operation will be phased, with an estimated 104 million tonnes crushed throughput throughout the estimated 18 years Life of Mine (LOM), which is to be confirmed during the study.

1.2 SCOPE OF WORK

The purpose of this feasibility level geotechnical investigation was to characterize the foundation conditions, determine the nature and distribution of the soils and bedrock across the various sites, assess excavatability and to provide recommendations for the design of the foundations and earthworks required. In addition, areas of seepage and material sources for re-use as construction materials were investigated.

The feasibility level geotechnical and materials investigation provided in this report will aid feasibility level engineering designs for the following structures:

- Heap Leach Pad (HLP),
- Processing Plant (Plant),
- Stormwater Diversion Channel (SWD),
- New Road (RD) and
- Waste Rock Dump (WRD),

In addition, potential borrow areas (AM) and the re-use of the materials encountered across the site (AM) were investigated.

This report details the fieldwork carried out and provides the factual data retrieved from the geotechnical investigation, summary of all the laboratory results and the evaluation of all the data in order to provide feasibility level geotechnical recommendations for the project.



1.3 PREVIOUS INVESTIGATIONS

Previous geotechnical investigations undertaken by SRK focused at the TSF area (now referred to as the heap leach pads and plant area), documented in report 462312/Geotech Final, indicates that the project site is covered by a thin layer of reddish brown micaceous silty sand (assumed to be aeolian Kalahari sands). Areas close to the Black Nossob River, are underlain by a lighter-coloured alluvial soils. In some areas, where conditions are favourable, the Kalahari sands overlie a pedogenic hardpan calcrete horizon. Bedrock was noted to be highly irregular in weathering profile and varies according to fracture intensity and rock type.



2.0 SITE DESCRIPTION

2.1 PROJECT LOCATION

The Omitiomire Copper Project (OCP) is located 120 km northeast of Windhoek in the Khomas region, Namibia at latitude 802842.00 m E and longitude 7582808.00 m S (WGS84 UTM 33S). Smaller settlements border the site to the north (Hochfeld), east (Steinhausen) and south (Omitara).

The area is dominated by commercial wildlife farming with the overall proposed project footprint area measuring approximately 1000 Ha. Access to the site is via the M53 and D1435 gravel road, which is connected to the B6 National Trans Kalahari Highway from the capital city of Windhoek.

Appendix A1 provides a locality plan, whilst Appendix A2 provides the layout of the site investigated points at the stormwater diversion channel, waste rock dump, processing plant, heap leach pad, new road and additional materials investigation area.

2.2 TOPOGRAPHY

The terrain of the OCP project areas is generally flat, with flood plains and sporadic occurrences of low-laying pan / depressions containing clay soils within the central and western portions of the site.

The ephemeral Black Nossob intersects the OCP project site flowing in a general easterly direction through the southern part of farm Omitiomire.

Vegetation cover generally comprises a predominant mix of grassland plants, trees and shrubs. Soil conditions vary from transported material underlain by pedogenic soil with shallow bedrock.

The project site is generally flat lying with elevation ranging from 1650 meters above mean sea level (mamsl) to 1657 mamsl.

2.3 CLIMATE

The project area is located in a semi-arid climate region. It is characterized by tropical warm summer rainfalls and dry winter seasons. Rainfall distribution follows an irregular pattern, caused by rainfall falling in thunderstorms, generally during the months of January, February and March. Rainfall is of short duration, but intense and is characterized by extreme spatial and temporal variability. Typically, the Hochfeld area has an average precipitation rate of 400 mm/a (Mendelsohn, 2010).

2.4 SEISMICITY

A Global Earthquake Model (GEM) Global Seismic Hazard Map (GSHM) has been developed by (Pagani et al., 2018) which shows the global distribution of the Peak Ground Acceleration (PGA) with a 10% probability of exceedance (POE) in 50 years. The GSHM is based on the combination and computation of data from 30 hazard models which are distributed across the globe. Figure 2-1 shows the distribution of the PGA (g) values for the central region of Namibia. The PGA values are for a site condition with an average shear wave velocity in the upper 30 m (VS30) of 760-800 m/s. Based on the



GSHM, Omitiomire Copper Mine is situated on a low seismic hazard zone with a PGA value of approximately 0.0188227g.

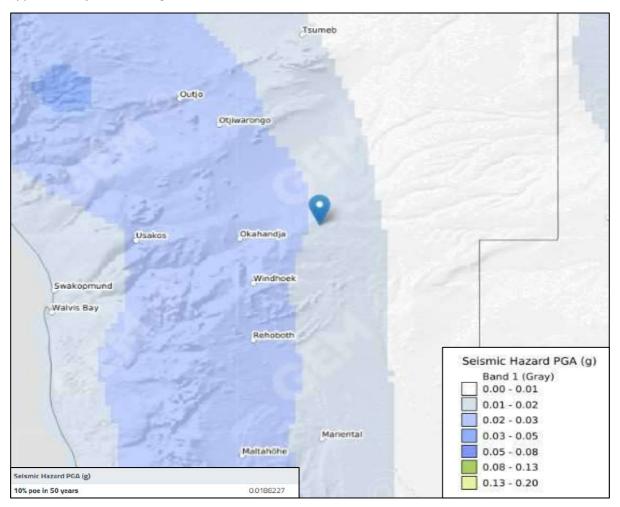


Figure 2-1: GEM Seismic Hazard Map for a Return Period of 475-years (Pagani et al., 2018)



3.0 GEOTECHNICAL METHODS OF INVESTIGATION

Details of the feasibility level geotechnical methods of investigation undertaken from March 2023 to May 2023 to investigate the scope as detailed in Section 1 are provided in the subsections below.

3.1 GEOPHYSICAL TESTING

A total of thirteen (13) Electrical Resistivity Tomography (ERT) Surveys were conducted by Gregory Symons Geophysics across the site from 3 March 2023 to 14 March 2023.

The ERT survey was performed by injecting a current into the ground and measuring a voltage. From this, the resistivity could be derived at certain points below the surface. The soil resistivity surveys were used to determine the depth of bedrock and measure the resistivity of various subsurface strata. The ERT test positions are presented in Appendix A2, the detailed report data is provided in Appendix F.

3.2 TEST PIT INVESTIGATION

A total of one-hundred and twenty seven (127) tests pits were excavated across the site from 10 May 2023 to 31 May 2023 to a maximum depth of 5m or until refusal was encountered, using a hired 20-ton Lovol (FR220D) tracked excavator from Sandworx cc. Test pitting in the SWD, WRD, Processing Plant, HLP, new road and borrow areas focused on ease of excavation for cost estimation, foundation conditions and seepage considerations, while also locating potential borrow areas for construction of platforms, roads and low permeability soils for liner materials. A layout of the test pit positions is provided in Appendix A2.

The nomenclature used for the naming of the test pits was adopted using the investigation area prefix defined in Section 1.1, followed by test pit prefix (TP) and the unique position number, e.g., SWD-TP1 refers to the stormwater diversion test pit 1.

The test pit positions planned were set out using Google Earth prior to establishment, based on the proposed footprint of the infrastructure at that time. The final positions of the test pits were adjusted on site, considering accessibility and safety factors. The test pits were logged in accordance with current standards (Guidelines for Soil and Rock Logging in South Africa, Brink and Bruin, 2002), using standard descriptors (moisture content, colour, consistency, soil structure, soil type and origin). The test pits were backfilled directly after completion of the logging, sampling and photographing.

The following test pits information is summarised in the respective appendices:

- The positions of all test pits are indicated on the site investigation plan in Appendix A2.
- A summary of the test pit logs is included in Appendix B1.
- The detailed test pit logs are provided in Appendix C1.
- Photographs of the test pit investigation are provided in Appendix D1.
- The terminology, conventions and symbols used in the logging are displayed in Appendix E.



3.3 BOREHOLE INVESTIGATION

In addition to the excavation of test pits, nineteen (19) rotary cored boreholes (BH) were drilled by RA Longstaff between 13April 2023 and 22 May 2023. The boreholes were drilled to depths ranging from 10.4 m to 25.1 m. End of hole (EOH) was determined by drilling into 6 m of bedrock with a consistency of soft rock or better or deeper to investigate the potential of geological structures which may affect the structure. A similar system was used for naming of the boreholes, by inserting a prefix of the area and borehole (BH) followed by a numeral for the unique position number.

Where conditions permitted, Standard Penetration Tests (SPT) were conducted to determine the SPT N-value and provide an indication of the material consistency of the subsurface strata.

The core samples obtained from the rotary core borehole drilling were logged according to current standards (Guidelines for Soil and Rock Logging in South Africa, Brink and Bruin, 2002).

The following borehole information is summarised in the respective appendices:

- The positions of the boreholes are shown on the site investigation plan in Appendix A2.
- A summary of the rotary core boreholes is included in Appendix B2.
- A summary of the SPT testing is included in Appendix B3.
- The detailed borehole logs are provided in Appendix C2.
- Photographs of borehole core are provided in Appendix D2.
- The terminology, conventions and symbols used in the logging of the boreholes are displayed in Appendix E.

3.4 LABORATORY TESTING

Representative soil and rock samples were taken from the test pits and boreholes and submitted to Namibia Technical Services (NTS) for the following laboratory testing:

Soils:

- 52x Foundation Indicator (particle size distribution, Atterberg limits, hydrometer including specific gravity).
- 28x California Bearing Ratio (CBR) tests.
- 28x Modified AASHTO Maximum Dry Density (MDD) Including Optimum Moisture Content (OMC).
- 3x Standard Proctor and Optimum Moisture Content.
- o 4x Basson's Index Tests.
- 5x Direct Shear Strength Test.
- o 13x Falling/ Constant Head Permeability.
- 2x Consolidated Undrained (CU) Triaxial Tests.

Rock

- o 11x Uniaxial Compressive Strength (UCS).
- o 25x Point Load Test (I-s, 50).
- 1x ACV (Wet and Dry).
- 1x Petrographic Analysis for Concrete Use.
- 1x Alkaline Aggregate Reactivity (AAR).



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

In addition to the samples collected from the boreholes for durability testing, samples will be sourced from the Open Pit pre-stripping works and submitted for laboratory testing. The results of those additional tests will be included in the interpretive report only.

A summary of the geotechnical laboratory testing schedule is provided in Appendix B4.



4.0 GEOPHYSICAL TESTING

Electrical Resistivity Tomography (ERT) Survey interpretation and results undertaken across the site in March 2023 are described below.

Thirteen (13) ERT lines were surveyed with a total traverse length of 2800 m near to and along the banks of the Nossob River and specifically at the inlet and outlet structures of the SWD. The locations of the survey lines are shown in Appendix A2. Detailed descriptions and results of the geophysical investigation are included in Appendix F.

The ERT surveys were used to provide possible depth to bedrock and identify water bearing structures using the Schlumberger array.

A summary of the geophysical report and the inverted resistivity depth sections show the following:

- A maximum of 3 overburden layers were identified. The subsurface strata are defined as follows:
 - o A top sandy resistive layer,
 - o A conductive clay bearing layer, and
 - A deeper resistive sandy layer.
- Below the overburden materials a weathered conductive basement is encountered, which transitions into resistive fresh rock.
- In places vertical to sub-vertical conductive features are observed in the weathered and fresh basement indicative of vertical faulting. The vertical faulting appears not to penetrate the overburden.
- Appendix F Figure 4 shows a structural interpretation of the area based on an aeromagnetic
 interpretation. Thrust faults are purported to be bringing in the copper (Cu) mineralization and are
 typically "old" (late Damaran 480-500 Myr). These faults are not expected to be identified in the
 resistivity data and in general this is the case with the current resistivity data set.
- Vertical faulting in a N/S and E/W direction is related to Cretaceous and late Karoo tectonics which appears to be detected by the resistivity as conductive water bearing faults and fractures.
- Overburden is Kalahari aged and the resistive layers may be related to windblown Kalahari sands.
- The conductive overburden layer appears to be associated with the Nossob River and could be related to flood events where the Nossob River has overflown its banks and deposited more clay like lithologies on the flood plain.



5.0 TYPICAL PROFILE

This section provides an overview of the material conditions encountered at the OCP site for the feasibility level geotechnical investigation undertaken. The areas investigated during the March to May 2023 program include the following investigation areas for the proposed structures detailed in Section 1.1. of this report.

A summary of the test pit and borehole logs is described for each investigation area in the subsections to follow.

5.1 HEAP LEACH PAD (HLP)

The heap leach pad area is located directly north of the plant and west of the deposit. Twenty-nine (29) test pits and four (4) boreholes were undertaken during the field investigation. The locations of the test pits are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed heap leach pad.

5.1.1 ALLUVIUM

Transported soils (mostly fine and coarse alluvium) were seen to form a surficial blanketing layer within the HLP. Fine alluvium is typically found across the HLP from surface in all the test pits and boreholes. This horizon was observed to have a variable thickness ranging from 0.3 m to 2.3 m. These soils are typically described as dry to slightly moist, loose to medium dense, brown to dark brown, silty sand with root voids.

Firm to stiff, sandy clay deposits occurring at shallow depths within the low-lying pans.

Localized occurrence of coarse alluvium is present below the fine alluvium across the HLP, and is described as reddish-brown, medium dense, matrix supported, silty sandy gravel comprising abundant fine to coarse subrounded to sub-angular quartz gravel with minor iron nodules. The alluvium is typically 0.2 m in thickness and but may be up to 1.5 m in places.

5.1.2 PEDOGENIC SOILS

The alluvium has been indurated by pedocretes of varying degrees of cementation.

These pedogenic soils predominantly comprise of calcified alluvium with sporadic occurrences of calcareous, ferruginous and ferruginised alluvium. The pedogenic soils in the HLP area extend from a depth of 0.3 m to 4.0 m, with excavator refusal commonly occurring on calcified and ferruginised alluvium between depths of 1.3 m to 2.35 m. Consistencies within the pedogenic soils are typically medium dense that become dense with depth.

Moderately to strongly cemented hardpan calcrete of medium hard rock consistency is present in the north of the HLP (HLP-BH01) as a 1 m thick horizon encountered at a depth of 3 m.



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

5.1.3 RESIDUAL GNEISS

Where encountered, the residual gneiss occurs below the alluvium and pedogenic horizons in the HLP at depths ranging between 1.4 m to 6.8 m. The residual gneiss comprises generally less than 0.5 m thick reddish brown to light brown, medium dense to dense, silty sandy gravel. With depth the residual soil becomes very dense towards bedrock.

5.1.4 GNEISS BEDROCK

The transported / pedogenic soils overly gneiss bedrock within the footprint of the HLP at depths ranging from 0.65 m to more than 6.1 m.

The typical bedrock profile is described as grey to brown speckled black banded white, medium to coarse grained, very closely jointed to widely jointed, soft to medium hard rock gneiss.

Shallow excavator refusal typically occurred on soft rock in the central and southern portion of the HLP.

Gneiss bedrock was not encountered in all test pits but was encountered in all boreholes. The HLP encountered gneiss from a minimum depth of 0.65 m to deeper than 6 m.

Joint surfaces generally infilled with silt, sand and iron oxide stained. On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric and quartz veins. Hydrothermal vein was observed within HLP-BH01 at a depth of 6.3 m to 13.3 m.

5.1.5 GENERAL

The bedrock topography is undulating across the HLP but generally expected within the upper 5 m below the dense to very dense ferruginised and calcified alluvium. Slight groundwater seepage was encountered within HLP-BH03 at a depth of 6.3 m.

5.2 PROCESSING PLANT (PLANT)

The proposed processing plant is located west of the deposit and south of the proposed Heap Leach Pad. The geotechnical site investigation aimed to obtain information on the material types and foundation conditions. Four (4) boreholes and four (4) test pits were completed at the plant site. The locations of the test pits and boreholes are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed processing plant.

5.2.1 ALLUVIUM

The typical profile at the processing plant comprises dark brown to brown, medium dense, silty sand, fine alluvium with minor sub-angular to subrounded fine gravel. This fine alluvium overlies medium dense to dense coarse alluvium (pebble marker) gravel with abundant quartz cobbles and boulders. The pebble marker was highly undulating with an average thickness of 0.3m.

Ferruginised alluvium is found southwest of the processing plant and is described as orange-brown mottled and blotched red, dense, silty gravelly sand with minor ferricrete nodules.



5.2.2 PEDOGENIC SOILS

Nodular tending to honeycomb ferricrete sporadically occurs in the southern portion of the of the Plant comprising silty sandy gravel with minor to traces of quartz cobbles. Refusal occurred on nodular tending to honeycomb calcrete at a depth ranging from 1.7m to 1.9m.

5.2.3 RESIDUAL GNEISS

The residual gneiss is very sparse in the processing plan area and was only encountered in Plant-BH02 as a thin light brown speckled white, very dense, silty sand horizon of 0.5 m.

5.2.4 GNEISS BEDROCK

Depth to bedrock at the plant site is encountered at shallow depths as soft to medium hard rock from 1.5m in (H/Plant Access-TP30). The gneiss bedrock encountered in Plant-BH2 is described as completely weathered to unweathered, grey to brown speckled black streaked and banded white, medium to coarse grained, extremely soft rock transitioning to very hard rock gneiss with depth.

The joint surfaces generally infilled with silt, sand, calcite and iron oxide staining. On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric.

5.2.5 GENERAL

Evidence of geological structures was present as breccia in Plant-BH02 from 22.9 m to 23.1 m and 23.8 m to 24.2 m. Significant pyrite mineralization was also noted along foliation in Plant-BH03.

No groundwater seepage was encountered in any of the test pits or boreholes undertaken at the Processing Plant. The presence of pedogenic materials, however, confirms the possibility of a seasonal perched water table.

5.3 STORMWATER DIVERSION CHANNEL (SWD)

The proposed Stormwater Diversion Channel is located south and east of the deposit in a north to south (N/S) direction. To the possibility of extreme flooding of the Nossob River during the rainy season flooding the pit requires the construction of the SWD prior to the commencement of mining.

Twenty-six (26) test pits and ten (10) boreholes were investigated to determine the ease of excavation for cost estimation, founding conditions along the proposed channel and suitability for material reuse.

Six (6) 63 mm diameter slotted PVC standpipes were installed at the inlet (SWD-BH02, SWD-BH03, SWD-BH04 and SWD-BH05) and outlet (SWD-BH08 and SWD-BH09) structures for monitoring of groundwater levels, with the completion zone at 20.0m to 25.1m.

The following sub-sections provide an overview of material horizons which were encountered at the proposed Stormwater Diversion Channel

5.3.1 ALLUVIUM

Transported soils comprise fine alluvium which is present in the majority of the test pits. The fine alluvium encountered at the inlet / outlet structures and along the diversion channel extends to variable



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depths varying from surface to 4.9 m, comprising brown to dark brown, clayey silty sand / sandy silty clay with minor to traces of sub-angular to subrounded quartz gravel. The consistency of this horizon is highly variable from collapsing very loose in the upper profile typically becoming dense with depth, and often from medium dense or dense at surface.

Coarse alluvium was generally found below the fine alluvium above the deep bedrock. Where found the coarse alluvium is typically 1 m thick were encountered and largely comprises medium dense to dense, brown silty sandy gravel with traces of cobbles and boulders. The coarse alluvium is sporadic in occurrence across the site and occasionally includes a pebble marker horizon.

SPT tests were exclusively carried out in the fine and coarse alluvium in the upper 4 m where penetrable. The SPT typically either refused in dry, stiff fines alluvium or coarse gravel and cobbles. SPT N-values of less than 13 were also recorded for the fine alluvium indicating soft to firm or loose to medium dense conditions.

The alluvium, as anticipated for a depositional horizon is variable and variance within the profile should be anticipated over small distances.

5.3.2 PEDOGENIC SOILS

Pedogenic horizons are highly variable within the proposed SWD and comprise predominantly ferruginised alluvium with sporadic occurrences of ferruginous, calcareous and calcified alluvium. These pedogenic soils are generally encountered below the fine and coarse alluvium and extend to depths of more than 4.7 m, with refusal commonly occurring on dense to very dense ferruginised alluvium at depths ranging from 1.1 m to 4.3 m.

Medium dense to dense nodular calcrete / ferricrete is sparse along the channel comprising abundant nodules of soft to medium hard rock strength.

It is anticipated that the pedogenic soils, much like the alluvium, will vary considerably in horizontal and vertical distribution.

5.3.3 RESIDUAL GNEISS

The residual soils predominantly located at the inlet and outlet structures in limited test pits (SWD-TP04 and SWD-TP25) and often these horizons in the boreholes are derived from the gneiss rock of the area and consists of brown to grey, firm to very stiff sandy silt with occasional large gravel to cobble fragments present.

5.3.4 GNEISS BEDROCK

Gneiss bedrock was encountered in all the boreholes drilled at the SWD. Bedrock is generally encountered within 5m from surface as soft rock, becoming medium hard rock or better with deeply weathered joints or mafic bands with depth. Medium hard rock gneiss or better is generally encountered at depths greater than 6m.

SWD- BH6 along the south-eastern bend of the channel and SWD- BH8 and SWD- BH9 at the outlet are notably more deeply weathered compared to other profiles along the channel.

The typical bedrock profile is described as follows:



- Highly to moderately weathered, grey to brown speckled black streaked and banded white, medium
 to coarse grained, closely to moderately jointed infilled with silty sand and iron oxide stained, very
 soft to medium hard rock gneiss.
- Occasionally deeper and better-quality gneiss is encountered as slightly to unweathered, grey speckled white and black or blotched red, medium to coarse grained, moderately jointed, medium hard rock to very hard rock gneiss.
- The mafic bands are generally more deeply weathered along joints.
- Shallow excavator refusal on soft to medium hard rock occurred in some test pits at the inlet (SWD-TP01, SWD-TP05, SWD-TP07) from a depth of 0.6 m.
- Shallow refusal occurred in generally medium hard rock at the outlet structure (SWD-TP20, SWD-TP23, SWD-TP24, SWD-TP25 and SWD-TP26) from a depth of 2.4 m.
- The joints are described as being infilled with silty sand and iron oxide stained and the majority of
 joints are smooth to rough and planar, often breaking along fabric.
- Gneissic folding was noted in SWD-BH06.

5.3.5 GENERAL

The geophysics survey lines identified possible fault zones in the vicinity of SWD-BH08 and SWD-BH09. These, however, was not clearly identified during the borehole drilling at the SWD however a possible breccia zone was identified in SWD-BH2. Shear zones were identified in other areas of the site.

Slight to moderate groundwater seepage was encountered within SWD-TP04, SWD-TP05, SWD-BH01, SWD-BH04 and SWD-BH05, at depths ranging between 1.5 m and 14.6 m.

5.4 NEW ROAD (RD)

The new road is located east of the SWD channel oriented in a NE/SW direction. Access constraints in the south-western part of the proposed route restricted the test pit excavations. A total of sixteen (16) test pits were excavated along the route. The locations of the test pits are presented in Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered along the new road.

5.4.1 **ALLUVIUM**

Similar to the SWD, WRD, Plant and HLP a surficial layer of fine alluvium blankets the entire road alignment in all but one the test pit. This material comprises a typically 1.4 m thick to 4.3 m thick in places brown to dark brown, medium dense to dense clayey silty sand with a root voided to pinholed soil structure. Very loose to loose zones occur in the upper profile. Excavator refusal was encountered within the fine alluvium at depths ranging from 1.4 m to 4.3 m.

Where refusal did not occur in the fine alluvium, coarse alluvium was encountered below in three test pits (RD-TP12, RD-TP13 and RD-TP14) along the new road. The coarse alluvium typically comprised yellowish brown to reddish brown mottled black and white, medium dense to dense, silty sandy gravel / clayey gravelly silty sand with minor ferricrete nodules. This horizon has an average thickness of 1.9m.



5.4.2 PEDOGENIC SOILS

Sporadic occurrences of pedocretes with little to no cementation (ferruginous / calcareous) of 0.6 m thick alluvium or indurated by cementation of 2.2 m thick calcified alluvium occur within the lower alluvium profile. These soils were profiled as having a medium dense to dense consistency.

5.4.3 GNEISS BEDROCK

Gneiss bedrock was encountered below the coarse alluvium at depths ranging from 1.4 m to 4.0 m often as a thin horizon of completely weathered gneiss becoming highly to moderately weathered, dark grey streaked white, stained orange, fine to medium grained, closely to moderately jointed, very soft to soft rock gneiss.

5.4.4 GENERAL

All but one test pit (RD-TP13) refused within the upper 5 m in alluvium or occasionally on very soft to soft rock gneiss.

5.5 WASTE ROCK DUMP (WRD)

The proposed WRD borders the proposed open pit to the north, east and south. In total, forty (40) test pits and one (1) borehole were completed during the field investigation to confirm soil/bedrock profile for founding and assess the in-situ rock and soils for use as construction materials. The locations of the test pits and borehole completed during the current feasibility level geotechnical investigation are shown on Appendix A2.

The following sub-sections provide an overview of material horizons which were encountered at the proposed Waste Rock Dump.

5.5.1 ALLUVIUM

The site of the proposed WRD facility is characterized by an approximately 1.1 m thick layer of fine alluvium, overlying a coarse alluvium. The fine alluvium consists of light brown to dark reddish brown, very loose to dense (non-cohesive soils) and firm to stiff (cohesive soils), silty sand / sandy clay or silt with presence of roots.

The underlying soil is coarse alluvium (pebble marker) comprising, typically a brown mottled black blotched white stained orange silty sandy gravel with traces subangular to subrounded cobbles and boulders extending to an average depth of 0.6m. Ferricrete nodules were observed to be present within the identified pebble marker.

5.5.2 PEDOGENIC SOILS

Below the fine and coarse alluvium horizons, pedocretes comprising ferruginous, ferruginised and calcified alluvium. Calcified alluvium is the most common of the pedocretes within the southern portion of the WRD. The calcified alluvium generally consists of brown speckled white, loose to medium dense (non-cohesive soils) or firm to stiff (cohesive soils), gravelly silty sand.



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

The central and northern portions of the WRD are characterized by well developed nodular, honeycomb and hardpan calcrete pedogenic horizons.

The nodular and honeycomb calcrete are similar, comprising pale brown blotched, medium dense to dense, white sandy silt with abundant calcrete nodules and cobbles.

Hardpan calcrete generally occurs in the northern parts of the site. The hardpan calcrete is described as moderately to very strongly cemented, light brown blotched white with very soft rock to medium hard rock consistency.

Ferruginous and ferruginised soils were encountered sporadically across the site and are anticipated to indicate localised perched water tables. These horizons are considerably more sporadic than the calcium-rich horizons. Occasional refusal occurred in ferruginised alluvium at shallow depths of approximate 1.5 m.

Refusal was typically encountered in the ferruginised alluvium or nodular, honeycomb or hardpan calcrete where encountered.

5.5.3 RESIDUAL GNEISS

Residual gneiss was encountered sporadically on site above the shallow bedrock. This residual gneiss is generally less than 0.3 m thick and described as white and brown to grey, very stiff/ dense, sandy silt to silty gravelly sand with traces of subrounded to subangular quartz gravel.

WRD-BH01 exposed the variation in weathering where soft rock was encountered then a completely weathered zone described as soil to a depth of 7.7 m.

5.5.4 GNEISS BEDROCK

Bedrock was encountered sporadically across the WRD area at shallow depths. The typical bedrock profile is described as follows:

- Highly weathered to moderately weathered, grey to brown speckled black, streaked and banded
 white, stained orange in places, medium to coarse grained, very soft rock to medium rock gneiss
 at the base of test pits.
- The borehole drilled in the northern portion of the WRD encountered very soft rock gneiss from 4.8m becoming medium hard rock with completely weathered zone of very soft rock becoming slightly to unweathered, hard to very hard rock from 14.3 m.
- Joint surfaces generally infilled with silty sand, iron oxide stained and void of any infill with depth.
- On a macro-scale, the majority of joints are smooth to rough and planar, often breaking along fabric.

5.5.5 GENERAL

Groundwater seepage was encountered at the southwestern corner (WRD-TP01) and north (WRD-BH01) of the WRD at a depth ranging between 1.4 m and 7.4 m respectively.

It should be noted that groundwater level in the area is subject to seasonal fluctuations and precipitation events. The presence of pedogenic soils, further, indicates the possibility of a perched water table.



5.6 POTENTIAL BORROW SOURCES (WRD AND AM)

Local borrow sources are required to provide materials for construction of the mine. The borrow materials are required to be geochemically innocuous (non-reactive), free of organics and non-deleterious. A total of twenty-four (24) test pits were excavated to investigate potential borrow sources. Twelve (12) test pits were undertaken on the footprint of the WRD on the southern bank of the Black Nossob river and an additional twelve (12) test pits were excavated south of the farmhouse at the northern end of the proposed new road. The main types of borrow source materials required include:

- Earthworks construction materials, including:
 - o Low permeability soil (soil liners),
 - General fill,
 - Structural fill.
- Fine and coarse aggregate for use in concrete.

The interpretive report will include a full evaluation of the re-use of materials encountered across the site in the various areas. This section will focus on the materials targeted along the river as part of the WRD and the additional materials south of the farmhouse (AM). The following observations of the typical profile are made:

- The excavatable soil profiles are shallow across the majority of the investigated area and comprise
 of upper fine alluvium soils, underlain by coarse alluvium or calcified alluvium horizons at shallow
 depths.
- The fine alluvium is typically described as less than 1 m thick gravelly silty sand with minor rootlets and root voids or pinhole voided soil structure.
- The fine alluvium generally grades into a coarse alluvium with depth comprising clayey silty sandy gravel. The coarse alluvium contains trace to minor sub-angular cobbles. The thickness of this coarse alluvium is generally 0.4 m thick in the AM area and more than 1m thick at the WRD materials area.
- Pedogenic soils of varying developmental stages occur deeper in the WRD profile. The ferruginous
 / calcareous to more commonly encountered sandy silty gravel to gravelly silty sand, calcified
 alluvium is common. Occasionally with depth nodular calcrete (WRD-TP6, WRD-TP16 and WRD-TP17) is encountered as pale brown mottled white, sandy silt with abundant calcrete nodules.
- Gneiss bedrock was rarely encountered in the materials test pits at depths from 1.3 m and bedrock topography is anticipated to be undulating along the river.
- In addition to the typical profile described for the WRD above, the AM profile comprises fine alluvium described as clayey silty sand, marginally finer than that of the WRD with limited coarse alluvium and pedogenic soil development. Refusal of the excavator occurred at a typical depth of 1.1 m in dense to very dense fine alluvium. Shallow gneiss bedrock was encountered in AM-TP02 and AM-TP04 as very soft to soft rock at a depth of approximately 1m.
- The soil profile in the AM area is notably shallower than that of the WRD materials area but is considered more consistent in material variation within the alluvium. The fine alluvium is generally a 1m thick horizon in both the AM and WRD areas. The WRD encounters the deeper profile of coarse alluvium and more frequent pedogenic soil development.
- No groundwater seepage was encountered in any of the test pits.



6.0 LABORATORY TESTING

Laboratory testing was carried out at Specialised Testing Laboratory on samples obtained during the geotechnical investigation. A summary of the laboratory results is presented in Appendix B4 and discussed below. The full results are contained in Appendix G. Triaxial tests are still ongoing due to laboratory backlog and will be issued separately as per client's request to not delay the final report.

The materials have been assessed according to material type and where necessary the variations within specific areas are referred to.

6.1 ALLUVIUM

The nature of alluvium is generally variable by type and distribution during deposition. Despite typical materials being observed and tested, it should be noted that variation within the layers is anticipated as coarse or fine lenses were encountered.

6.1.1 FINE ALLUVIUM

The fine alluvium is present across the site. It is described as a silty sand to sandy silt. The material is generally variable by grainsize within the deposited lenses. The silty sand generally comprises more than 40% fines (clay and silty) with varying clay percentage but generally low Plasticity Index (PI) values of less than 8 %. The USCS classification considers the material generally a clayey sand (SC) due to the high clay percentage and AASHTO class A-4. One sample (HLP-TP08) indicated high PI of 17 % grading as high plasticity silt (MH). The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC material typically varies from 1963 kg/m³ at 12 % to 2079 kg/m³ at 9 %. The ML sample indicates MDD and OMC values as low as 1517 kg/m³ at 23 %.

Coarser zones are encountered within the fine alluvium described as silty coarse sand or silty gravelly sand classifying as SM with low PI values and generally less than 30 % fines. The AASHTO classification generally indicates A-2-4 materials for the coarser zones. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the material vary from 2041 kg/m³ at 8 % to 2119 kg/m³ at 7 %.

The fine alluvium is anticipated to be corrosive towards steel and very highly aggressive towards concrete. It is considered generally low potential for expansiveness with specific gravity (SG) of 2.5 g/cm³ to 2.7 g/cm³.

Three samples were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of $x10^{-7}$ m/s for the alluvium classified as SC and may become less permeable with as the fines increase to $x10^{-9}$ m/s for the MH soils described above.

6.1.2 COARSE ALLUVIUM

The coarse alluvium is present across the site and includes a sporadic basal pebble marker layer. It is described as a silty sandy gravel to coarse sandy gravel with varying amounts of gravel. The material is generally variable by grainsize within the deposited lenses. The silty sand/ silty sandy gravel generally comprises less than 45% fines (clay and silty) with varying clay percentage but generally low Plasticity Index (PI) values of less than 5 %. The USCS classification considers the material generally a clayey



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

sand (SC) to Gravel of low to medium plasticity (GP to GM). The coarse alluvium is classified as AASHTO class A-1, A-2 or A-4. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC to GM material typically varies from 1858 kg/m³ at 13 % to 2204 kg/m³ at 6 %. The material has a high variability according to the COTO classification ranging from G9 to G4 quality materials.

The coarse alluvium was not tested for corrosivity but is anticipated to be corrosive towards steel and very highly aggressive towards concrete as in the fine alluvium. It is considered low potential for expansiveness with specific gravity (SG) of 2.5 g/cm³ to 2.6 g/cm³.

Three samples were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of $x10^{-7}$ m/s for the alluvium classified as SC and may more permeable gravels at $x10^{-6}$ m/s.

6.2 PEDOGENIC SOILS

6.2.1 CALCAREOUS/ CALCIFIED ALLUVIUM

Present across most of the site but not encountered in the Plant area. The pedogenic soils are at various phases of development from the weaker calcareous to stronger calcified alluvium comprising gravelly silty sand to gravelly sandy silt. It is noted that the fines content of the calcified alluvium is generally higher than that of calcareous alluvium.

The calcareous and calcified alluvium test results are within the limits of the fine alluvium, but the material typically grades as A-1 to A-2 material with one sample RD-TP08 indicating medium expansive silty sand (SM) of A-7 quality similar to that observed for HLP-TP08 discussed in Section 6.1.1 above.

The samples that were tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-values in the order of x10⁻⁶ m/s for the calcareous alluvium and x10⁻⁷ m/s for the calcified alluvium.

6.2.2 FERRUGINOUS/ FERRIGINISED ALLUVIUM

The ferruginous and ferruginised alluvium is present across the site. Testing was carried out predominantly on samples obtained from the plant, SWD and WRD due to the frequency of materials encountered. The ferruginised alluvium is described as gravelly silty sand to silty sandy gravel classified as low expansive clayey sand to silty sand (SC - SM) of AASHTO A-2 in the gravel zones and A-4 in the sandy zones. The specific gravity (SG) was recorded as 2.5 g/cm³ to 2.6 g/cm³. The AASHTO maximum dry density (MDD) and optimum moisture content (OMC) of the SC sample was recorded at 2053 kg/m³ at 7 %. The material has a low CBR strength likely due to the crushing of the poorly cemented ferricrete nodules and is thus classified as poorer than G9 according to COTO.

The ferruginised alluvium is anticipated to be corrosive towards steel and very highly aggressive towards concrete as in the fine alluvium.

One sample was tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-value in the order of $x10^{-7}$ m/s similar to the alluvium described above.

The ferruginous alluvium sample tested indicates generally similar properties to those described for the ferruginised alluvium above but has a slightly higher PI (<10%) and classifies as A-6 material.



6.2.3 NODULAR AND HONEYCOMB CALCRETE ALLUVIUM

Pedocrete is present across most of the site but not encountered in the road and borrow areas. Samples were tested from the HLP and WRD of the nodular and honeycomb calcrete described as sandy silty gravel to silty sandy gravel which has similar properties, although more gravelly, to the calcareous alluvium. The nodular calcrete was tested for permeability values and indicates generally less permeable condition than those encountered in the alluvium generally and shows typical permeability values in the order of x10-8 m/s but grading as typical G7 and G8 quality materials.

6.2.4 HARDPAN CALCRETE

Hardpan calcrete was only encountered in the HLP and WRD areas and is similar to the other pedogenic horizons. The hardpan calcrete classifies as generally a G6 quality material with MDD and OMC AASHTO values of 1651 kg/m³ at 20%.

6.3 RESIDUAL GNEISS

The residual gneiss is present across most of the site but not encountered in the test pits along the new road, along the SWD and in the borrow area.

Where encountered the residual gneiss is often described as calcified or ferruginised silty gravelly sand with a fines content varying from 5 % to 34 % and is classified as clayey to silty sand (SC-SM) to silty gravel (GM, GP or GC).

The material is classified as A-1 or A-2 according to the AASHTO classification and as G7 Quality material according to COTO with AASHTO MDD and OMC of 1973 kg/m³ and 11 %. One sample was tested for remoulded permeability at 90 % Modified AASHTO compaction density to reveal permeability k-value in the order of x10-7 m/s similar to the alluvium described above.

6.4 GNEISS BEDROCK

The very soft rock gneiss present across the site which was excavatable was tested as well. The horizons are generally thin but indicate similar results to the residual soils derived from them. The fines content is less than 25% with PI values less than 5%. The material is classified as clayey to silty sand (SC to SM) to silty gravel (GM) as typically AASHTO A-1 to A-2 and COTO G6 to G9 quality material. The very soft gneiss is anticipated to be corrosive towards steel and very highly aggressive towards concrete.

6.4.1 UNIAXIAL COMPRESSIVE STRENGTH (UCS)

According to (SAICE, Guidelines for Soil and Rock Logging in Southern Africa, 2002), very soft rock is expected to have uniaxial compressive strengths (USC) ranging between 1 MPa and 3 MPa while soft rock is between 3 MPa and 10 MPa.

Two core samples were tested for rock strength in the HLP. The first sample was obtained from borehole HLP-BH02 and the second from HLP-BH04, the results are presented in Table 6-1. Minor strength increase was noticed in very soft rock cores on desiccation and may be the reason for the over



estimation of rock strength of the sample obtained from HLP-BH2. Both HLP samples indicate soft rock conditions are encountered from a depth of 2.6 m.

Table 6-1: UCS test results

Borehole No.	Sample ID	Depth (m)	General Rock Mass Description	Failing Load (kN)	Uniaxial Compressive Strength (UCS) (MPa)	Failure Description	Rock Hardness (SAICE, 2002)
HEAP LEACH	PAD						
HLP-BH02	C/10590	5.35-5.65	Highly weathered, very soft rock gneiss	22	8	Normal failure	Soft Rock
HLP-BH04	C/10593	2.66-2.86	Highly weathered, soft rock gneiss	21	7	Normal failure	Soft Rock
SWD INLET							
SWD-BH01	C/10575	2.04-2.26	Very Soft Rock Gneiss	22	8.0	Normal	Soft Rock
SWD-BH01	C/10577	3.14-3.48	Medium Hard Rock Gneiss	66	23.5	Normal	Medium Hard Rock
SWD-BH02	C/10580	7.96-8.16	Soft Rock Gneiss	96	33.5	Normal	Hard Rock
SWD-BH03	C/10582	3.82-4.13	Soft to Medium Hard Rock Gneiss	34	12.0	Normal	Medium Hard Rock
SWD-BH05	C/10586	9.69-9.93	Medium Hard Rock Gneiss	73	25.5	Normal	Medium Hard to Hard Rock

The samples taken in the SWD are logged as generally weaker rock mass than the tested results, specifically at SWD-BH02. This was noticed where logging of the core was carried out directly after drilling in a wet state leading to lower estimated rock parameters than during testing in the dry state. This is likely attributed to the alteration minerals present in the rock core.

6.5 SHEAR STRENGTH

The shear strength parameters of the soils on site were evaluated by shearbox testing. At the time of writing two triaxial tests on clayey samples were still ongoing. In the interim, indicative values are provided based on the material types as presented in Table 6-2 below.

Table 6-2 Indicative shear strength parameters

Soil parameter	Fine alluvium	Coarse alluvium	Pedogenic horizons	Residual gneiss
Typical material classification	SC to MH	SC to GM	SC - SM	SC-SM, G
Maximum dry density (kg/m³)	1500 - 1970	1650 - 2000	1650 - 1970	1650 - 2000
Cohesion (kPa)	0 - 8	0 - 5	0 - 5	0 - 5
Friction (degrees)	28 - 32	30 - 38	30 - 34	30 - 38



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

The shearbox test results are generally inline with the upper bound parameters for the tested alluvium and residual gneiss. SWD-TP15 and WRD-TP35 indicated slightly higher values than anticipated, specifically the cohesion, this is attributed due to the fines component causing slight build up of excess pore water pressure.

WRD-TP04 indicated slightly higher values than anticipated, specifically the cohesion, this is attributed due to the "apparent" cohesion due to the sand interlock.

Triaxial tests are still ongoing due to laboratory backlog and will be issued separately as per client's request to not delay the final report.



7.0 GEOTECHNICAL EVALUATION AND RECOMMENDATIONS

The following section comprises the geotechnical evaluation of the various areas and materials based on the material type, laboratory results and site observations. Recommendations are provided for founding, excavatability and re-use of materials for construction.

Note that a general comment for all areas requiring earth works – an adequate source of water supply will be required during the earth works for the various phases of construction.

7.1 HEAP LEACH PAD

7.1.1 DISCUSSION

The test pit and borehole data indicate that the majority of the HLP site is covered by 0.3 m to 1.0 m of very loose to medium dense, silty sand to sandy silt, fine alluvium. The fine alluvium is generally loose in the upper 0.3 m and occasionally loose conditions are encountered to a depth of 1.45 m. Fine alluvium may occasionally extend to a depth of 3.0 m generally in the western and north-western portion.

Below the fine alluvium, generally medium dense coarse alluvium, medium dense calcareous or medium dense to dense calcified alluvium is encountered. The coarse alluvium and calcareous alluvium typically extend to depths of 2.5 m below ground level. This sandy gravel coarse alluvium has a typical medium dense consistency. The calcified alluvium is generally encountered within the upper 1.8 m. The alluvium is generally ferruginous/calcareous silty sand with variable gravel in the center of the HLP towards the north to north-western portions and becomes medium dense to dense ferruginised/calcified silty sand (increasing consistency) north of the HLP site boundary.

The coarse alluvium includes a sparsely occurring pebble marker in places at the base of the coarse alluvium.

The transported horizon generally transitions to a thin medium dense to dense, silty sandy gravel, residual gneiss from 1.4 m typically along the western border of the HLP. The varying transition depths into residual soils are generally attributed to the weathering of the underlying undulating gneiss bedrock topography. The western boundary of the HLP comprises sporadic very soft rock gneiss encountered from 2.5 m below surface.

The test results indicate that the fine and calcareous alluvium at the HLP is corrosive towards steel and very highly aggressive towards concrete.

7.1.2 FOUNDING RECOMMENDATIONS

The total anticipated load exerted by the approximately 50 m high (1.8 km x 1,1 km area constructed in three phases) HLP will be approximately 1 MPa. Soft rock gneiss which is encountered at an undulating depth deeper than 3.0 m below surface has an allowable bearing capacity of more than 1MPa. The HLP earthworks (bottom of liner and drainage layer) required to create adequate drainage of the pad is anticipated to be a maximum height of up to 8 m above ground level in the north of the facility. Due to



the amount of excavation required to reach suitable in situ founding material, it is rather recommended to in situ densify the soils on site as part of the earthworks design as detailed below:

- Excavate to stockpile 0.5 m below surface extending 2m beyond the HLP footprint.
- Localised in situ rehabilitation may be required in loose pockets.
- Compact the excavation floor at 95% Modified AASHTO compaction at OMC using impact compaction. A trial section should be carried out to determine the optimum number of passes required.
- Backfill the excavation to founding level (top of HLP earthworks platform) in 200 mm layers using minimum G7 quality material compacted to 95% Modified AASHTO compaction at OMC using conventional compaction equipment. Should thicker backfill layers be required, trial compaction tests must be carried out.
- Plate load testing should be carried out on the backfill material to determine the expected settlements and the time for settlement of the fill.
- Place the drainage layer and liner as per design.

The laboratory results indicate that the fine alluvium has low permeability when compacted to 90% Modified AASHTO. This fine alluvium can be used as a low permeability layer, but it is not regarded as a suitable source for the clay liner itself due to the variability within the alluvium. The nodular calcrete and residual gneiss generally do not meet the required permeability for liner use.

The soils at the HLP site are corrosive towards steel and very highly aggressive towards concrete. Treatment of these soils with neutralising agents may be required.

7.1.3 EXCAVATABILITY

The average test pit refusal depth in the outer areas of the HLP is less that 1.4 m while the center portion test pits typically refuse between 1.4 m and 3.1 m on shallow bedrock or the presence of very dense soil.

The HLP generally experiences soft excavation conditions to a depth of at least 1 m below surface satisfying the recommendations above. Should deeper excavation be required, localised intermediate to hard excavation may be anticipated with depth.

7.2 HLP ANCILLARY STRUCTURES AND PONDS

For the purpose of evaluation based on the significant difference in design concept the HLP ancillary structures and ponds have been treated separately from the HLP and plant area. This includes the area along the southern boundary of the HLP (HLP-TP20, HLP-22 to 27 and BH03-HLP, BH04-HLP and BH01-Plant and BH04-Plant). The typical profile of the HLP Ponds at the southern boundary of the HLP indicates that firm to stiff and loose fine alluvium is present from surface to variable depths. Medium dense alluvium is typically encountered from a depth of 0.5 m to 1.2m below surface with a typical allowable bearing capacity of at least 100 kPa and shallow very soft rock transitioning to medium hard rock gneiss underlies the HLP ponds from a depth of 0.6 m but typically deeper than 1.4 m with allowable bearing capacity of more than 500 kPa.

Based on the field observations and laboratory results, the overburden is described as fine alluvium silty to clayey sands (USCS classification "SC" and "SM") to coarse alluvium coarse sand s and gravelly sand ("GP-GM") with low plasticity and have low potential expansiveness as well as a low



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

compressibility rating (Hazelton & Murphy, 2007). Minor calcareous development was observed in TP-24 and ferruginised alluvium is present in the vicinity.

According to the AASHTO Classification system, the alluvial soils classify as A-1 to A-2 quality soils.

7.2.1 FOUNDING RECOMMENDATIONS

Based on the typical profile it is recommended that any lightly loaded structures (limited to 100 kPa bearing pressure) may be founded on medium dense alluvium encountered varying from depths of 0.6m to 1.1 m below surface with minor ground preparation for the foundations. Moderately loaded structures (up to 500 kPa bearing pressure) may be founded on very soft gneiss bedrock at typical depths ranging from 1.2 m to 1.7 m. Founding of structures on soft to medium hard rock (up to 1 MPa bearing pressure) may be encountered as shallow as 0.6 m as seen in TP25-HLP but typically from a depth of 1.4m to deeper than 2m.

The HLP ponds are anticipated to be excavated to a depth of more than 5 m below ground level encountering soft to medium hard rock gneiss within the excavation. It is recommended to consider raising the floor level to reduce intermediate to hard rock excavation, if possible to 3-4m below ground level.

A liner preparation layer should be used above the rock excavation floor comprising the 2x 150 mm fine alluvium compacted to 95% Standard Proctor at OMC before lining of the pond due to the uneven rock surface, closely jointed nature of the rock, low Rock Quality Designation (RQD) and brecciation indicative of faulting observed in HLP-BH3.

It is recommended that the foundations be inspected by a suitably qualified geo-professional to ensure no significant variation is present and that founding conditions are achieved at the required depths.

7.2.2 EXCAVATABILITY

Based on the test pit and borehole data excavation is classified as soft to a typical depth of 1.8 m, thereafter, intermediate to hard excavation anticipated in the very soft to medium hard rock below.

7.3 PROCESSING PLANT

The typical profile of the processing plant indicates that medium dense alluvium is encountered from surface with a typical allowable bearing capacity of at least 100 kPa and shallow very soft gneiss bedrock underlies the plant area from a depth of 1.5 m below ground as encountered in TP30-HLP and BH1 to BH4-Plant. The excavator generally refused on a dense coarse alluvium or a dense ferruginised alluvium in the test pits that did not reach rock within the upper 1.9 m. It is expected that the dense soils overly the shallow bedrock (allowable bearing capacity of more than 500 kPa) below.

Based on the field observations and laboratory results, the alluvium is described as silty to clayey sands (USCS classification "SC" and "SM") with low plasticity and have low potential expansiveness as well as a low compressibility rating (Hazelton & Murphy, 2007). The alluvium is occasionally ferruginised and minor ferricrete nodules and honeycomb development are encountered.

According to the AASHTO Classification system, the alluvial soils classify as A-2 to A-4 quality soils.



7.3.1 FOUNDING RECOMMENDATIONS

Based on the typical profile it is recommended that lightly loaded structures (limited to 100 kPa bearing pressure) may be founded on medium dense alluvium varying from surface with minor ground preparation required for the foundations. Moderately to heavy loaded structures (up to 500 kPa bearing pressure) may be founded on very soft rock gneiss bedrock at a depth of more than 1.5 m.

Heavy dynamic structures (e.g., crusher) may be founded on soft rock gneiss at a dynamic bearing pressure not exceeding 500 kPa. It is anticipated that suitable founding will be possible from a depth of 2 m below ground surface based on the results of Plant-BH3.

It is recommended that the foundations be inspected by a suitably qualified geo-professional to ensure no significant variation is present and that founding conditions are achieved at the required depths.

7.3.2 EXCAVATABILITY

Based on the test pits excavated at the plant area, refusal was generally encountered within the upper 1.9 m and are classified as soft excavation, thereafter, intermediate to hard excavation is anticipated in the rock below.

7.4 STORMWATER DIVERSION CHANNEL (SWD)

For the purpose of this evaluation, this section is divided into three sections, namely the inlet (west), channel (alignment length) and outlet (east) structures. The figure in Appendix A2 shows the position of the test pits and boreholes along the SWD structure.

The evaluation and recommendations are based on the data from the laboratory results of the tested materials, as well as the data from 26 test pits and 10 rotary core boreholes which are positioned along the proposed stormwater diversion channel area.

The results of the geophysical surveys correspond well with the observed profile in the test pits and boreholes at the SWD inlet (line 3) and outlet (line 7 and line 8). It is noted that the "clay" overburden horizon is likely the pedogenic horizons observed in the profile.

7.4.1 **INLET**

Based on the test pit (TP01-SWD to TP09-SWD and TP23-SWD) and borehole (BH01-SWD to BH05-SWD and BH10-SWD) data, fine alluvial soils occur generally from the surface to a maximum of 3.95 m. Coarse alluvium is generally below the fine alluvium with a maximum depth of 3.8 m. Pedogenic soils were sporadically encountered from 0.6 m within the alluvium. The residual gneiss was limited in occurrence at the river and was encountered below the transported and pedogenic soils above the bedrock at depths great than 2 m. The completely weathered gneiss zones were often described as a soil zone.

Gneiss bedrock was found in and along the river at shallow depths. The typical refusal depth at the inlet in the river course is approximately 3.5 m to dense residual gneiss/ hard rock gneiss. The test pits and boreholes on the flanks indicate that soft rock is typically encountered from a depth of 1.3 m to 3 m. A possible fault breccia zone was encountered in SWD-BH2 from 10.34 m to 12.4 m while SWD-BH5 encountered medium hard rock with soft rock schistose bands.



Figure 7-1 shows the typical anticipated materials at various depths based on the test pit and borehole data. This does not denote the frequency of occurrence of a material type with depth. For more detailed material, depth, frequency of material types refers to Appendix B.

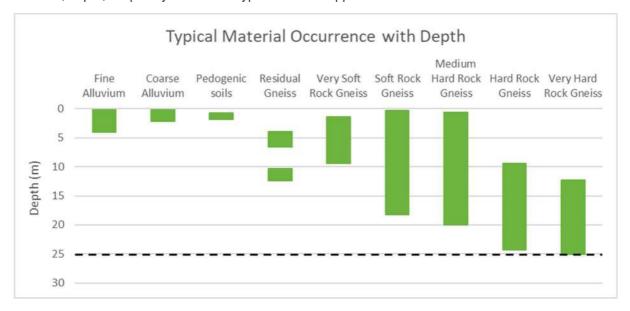


Figure 7-1: Typical Material Occurrence with Depth - SWD - Inlet

Groundwater was encountered in two test pits, TP04-SWD and TP05-SWD, from 1.5 m in boreholes BH01-SWD from 7.4 m, BH04-SWD from 14.6 m and BH05-SWD from 8.0m below ground.

The fine alluvial soils grade mainly as silty to clayey sands and have USCS classification of "SC" and "SM". The fine alluvium also classifies as "A-2" and "A-4" based on the AASHTO classification system and has low potential expansiveness.

The ferruginised and the very soft rock gneiss both have similar soil characteristics grading clayey sand "SC" and "A-2".

7.4.1.1 FOUNDATION RECOMMENDATIONS

The following recommendations are made for the SWD inlet structure:

- Excavate to 0.5 m below the SWD embankment footprint.
- Further excavate the 3 m wide cut-off trench to refusal of the excavator and batter-back to safe slope angle of 1:1.5 (V:H).
- In situ compact the excavation floor 90% Modified AASHTO compaction density. Back fill in maximum 200 mm layers using the fine alluvium compacted to 90% Modified AASHTO compaction density at OMC.
- On the downstream side of the cut-off trench, backfill the upper 0.5 m with coarse alluvium to 90%
 Modified AASHTO compaction density to create a drainage layer as per design.
- Continue the embankment earth works as per the embankment design.

7.4.1.2 EXCAVATABILITY

The river section is anticipated to experience soft excavations varying between 2.5 m and 3.9 m becoming intermediate to hard with depth. Loose soils cover the surface of the river area, and it is



recommended that the excavation walls be battered back at a safe slope of 1:2 (V:H). Seepage was also encountered in SWD-TP04 from 1.5 m indicating that dewatering of excavations should be considered.

The river flanks encounter soft excavation conditions in the upper 1.2 m, thereafter, becoming intermediate and hard excavation.

7.4.2 CHANNEL

The SWD channel was investigated by TP09-SWD to TP17-SWD and TP26-SWD as well as BH06-SWD and BH07-SWD. The test pits excavated through fine alluvium along the channel and generally indicate variable refusal depths in ferruginised or calcified alluvium most commonly between depths of 1.3 m and 4.4 m. The boreholes further indicate (very soft to medium hard) rock is encountered from 3 m to 7.6 m below ground level. There is no clear correlation with depth to refusal along the channel and variable conditions are anticipated along the route.

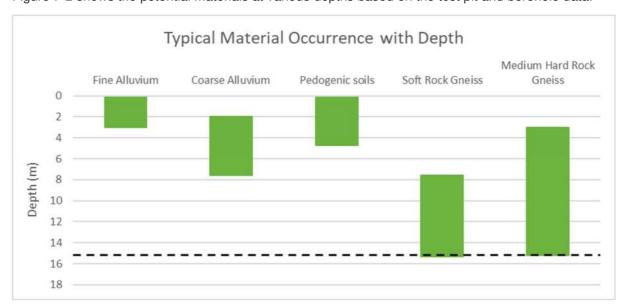


Figure 7-2 shows the potential materials at various depths based on the test pit and borehole data.

Figure 7-2: Typical Material Occurrence with Depth – SWD – Channel

The fine alluvial soils generally grade as silty sands and classify as "SC" and "A-4" based on the USCS and AASHTO classification systems. The material has low potential expansiveness and a "Good to Fair" compaction rating.

The calcified and ferruginised alluvium both classify as "SC" based on the USCS classification. Calcified alluvium grades as a gravelly sand and classifies as "A-2" while the ferruginised alluvium grades as a silty sand and classifies as "A-4". Both materials have low potential expansiveness and a "Good to Fair" compaction ratio.

The sparsely encountered nodular ferricrete grades as a silty sandy gravel "GC" and "A-2" and has low potential for expansiveness.



Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

7.4.2.1 FOUNDATION RECOMMENDATIONS

Excavate the channel to invert level (up to 10 m below ground level as per hydraulic design) battered back to a safe slope angle of 1:3 (V:H).

It is anticipated that intermediate and hard excavation will be required for the bulk of the excavation. Where soils are encountered in the channel, the following is recommended:

- in situ rip and recompact the channel
- place rip-rap or suitable erosion protection
- monitor the channel following rainfall and flooding events and remediate the channel as required.

7.4.2.2 EXCAVATABILITY

The excavatability conditions vary significantly along the channel area; however, the majority of the test pits were excavated to deeper than 2.5 m below the surface. It is anticipated that soft excavation conditions will be encountered to a depth of at least 1.1m below ground level and likely to a typical depth of 4 m. It is anticipated that intermediate and hard excavation will be encountered from a variable depth of 4m.

Groundwater was encountered only in test pit SWD-TP05 at 1.7 m below the surface. This test pit was dug in the river, and it is expected that groundwater mitigation measures may be required during construction.

7.4.3 OUTLET

The outlet area was investigated by TP20-SWD to TP24-SWD and BH08-SWD and BH09-SWD.

The boreholes drilled in the river course indicate soft rock encountered at 4.6 m to 6.8 m. The test pits excavated in the river course show refusal on coarse alluvium or soft rock to medium hard rock gneiss approximately 4.9 m.

The test pits on the flank refused at 1.1m on the southern flank and 4.3m on the northern flank in ferruginous alluvium. It is likely that the northern flank may have deeper alluvial soils. Soft conditions are anticipated above the refusal depth becoming intermediate to hard excavation from refusal and deeper.

Figure 7-3 shows the potential materials at various depths based on the test pit and borehole data.



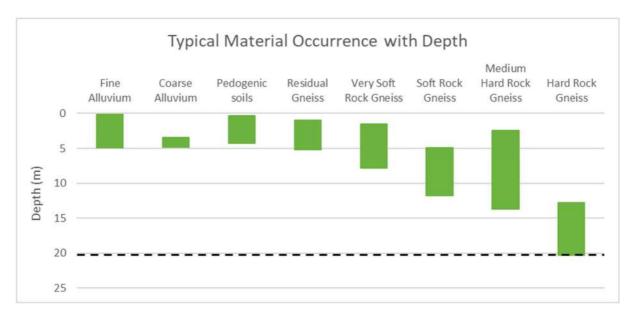


Figure 7-3: Typical Material Occurrence with Depth - SWD - Outlet

No groundwater was encountered in any of the test pits or boreholes drilled at the area of the SWD outlet.

7.4.3.1 RECOMMENDATIONS

The outlet structure requires erosion protection and is anticipated to tie in with the level created in the SWD channel. In situ compaction with rip-rap or adequate erosion protection is recommended.

7.4.3.2 EXCAVATABILITY

Based on the refusal depths of the test pit at the outlet area, it is expected that soft excavation conditions will be encountered from surface to 4 m in the river and from the surface to 2.5 m on the river flanks thereafter intermediate to hard excavation conditions are anticipated due to the presence of medium hard rock gneiss at depth.

7.4.4 VOLUME ASSESSMENT

The volumes assessment was based on the test pit and borehole data of the material at four sections along the SWD which include the inlet (Section 1), south-western portion of the channel until the bend (Section 2), from the bend towards the north-eastern portion of the channel (Section 3), from the north-eastern portion to the outlet of the channel (Section 4). Figure 7-4 presents the localities of the four sections along the SWD.



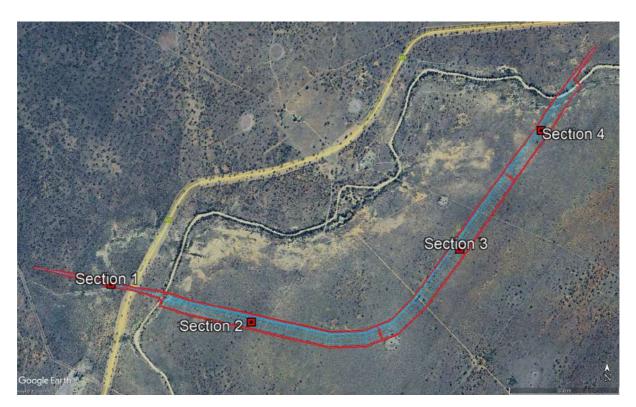


Figure 7-4: Localities of the Four Sections Along the SWD

Table 7-1: Estimated Material Volumes at Section 1

Material	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected Volume (m³)
Fine Alluvium	22,300	0.4 - 4.0	9,200 – 92,000
Coarse Alluvium	22,300	0.3 – 1.2	6,500 – 27,500
Pedogenic Soils	22,300	0.7 - 0.8	16,000 - 18,000
Residual Gneiss	22,300	0.3 – 2.1	6,500 - 48,000
Very Soft Rock Gneiss	22,300	0.5 - 2.7	11,500 - 62,000
Soft Rock Gneiss	22,300	0.3 – 8.0	6,500 – 184,000
Medium Hard Rock Gneiss	22,300	2.5 – 13.7	57,500 - 315,000
Hard Rock Gneiss	22,300	1.2 – 9.4	27,500 – 216,000
Very Hard Rock Gneiss	22,300	± 5.8	± 133,0000



Table 7-2: Estimated Material Volumes at Section 2

Material	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected Volume (m³)
Fine Alluvium	171,300	0.3 - 3.0	51,000 - 513,500
Coarse Alluvium	171,300	± 4.6	± 787,500
Pedogenic Soils	171,300	0.9 – 2.8	154,000 – 479,500
Residual Gneiss	171,300	-	-
Very Soft Rock Gneiss	171,300	-	-
Soft Rock Gneiss	171,300	± 7.7	1,319,000
Medium Hard Rock Gneiss	171,300	-	-
Hard Rock Gneiss	171,300	-	-
Very Hard Rock Gneiss	171,300	-	-

Table 7-3: Estimated Material Volumes at Section 3

Material	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected Volume (m³)
Fine Alluvium	167,000	0.5 – 1.9	83,500 - 317,000
Coarse Alluvium	167,000	0.7 – 1.9	116,500 - 317,000
Pedogenic Soils	167,000	1.0 – 2.1	167,000 – 350,500
Residual Gneiss	167,000	-	-
Very Soft Rock Gneiss	167,000	± 1.3	217,000
Soft Rock Gneiss	167,000	-	-
Medium Hard Rock Gneiss	167,000	12.2	2,037,000
Hard Rock Gneiss	167,000	-	-
Very Hard Rock Gneiss	167,000	-	-



Table 7-4: Estimated Material Volumes at Section 4

Material	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected Volume (m³)
Fine Alluvium	87,600	0.3 – 4.9	26,000 - 429,000
Coarse Alluvium	87,600	± 1.4	122,500
Pedogenic Soils	87,600	0.3 – 2.9	26,000 – 254,000
Residual Gneiss	87,600	± 0.5	43,500
Very Soft Rock Gneiss	87,600	0.9 – 2.6	78,500 – 227,500
Soft Rock Gneiss	87,600	0.1 – 4.0	8,500 – 350,000
Medium Hard Rock Gneiss	87,600	1.0 – 5.7	87,500 – 499,000
Hard Rock Gneiss	87,600	6.5 – 7.4	569,000 - 648,000
Very Hard Rock Gneiss	87,600	-	-



7.5 NEW ROAD (RD)

The evaluation and recommendations in this sub-section are based on the 16 excavated test pits of the 20 planned test pits for the new road. Four test pits (TP06-RD, TP17-RD, TP18-RD and TP19-RD) were inaccessible and thus not excavated in the western portion of the new road area.

Test pits TP02-RD, TP03-RD and TP16-RD were moved further downstream outside of the original neighbouring farm area to be excavated. This section will be divided into three subsections which will comprise the first two road-river crossings located near the south-western area of the RD site, the third and forth road-river crossings located on the north-eastern side of the RD site, and the road alignment between the river crossings. The structure coordinates and localities are indicated below in Table 7-5 and Figure 7-5, respectively.

Structure	Zone	Latitude/Northing	Longitude/Easting
Road-River Crossing 1	33 K	7578647.06 m S	800786.81 m E
Road-River Crossing 2	33 K	7579935.53 m S	801451.66 m E
Road-River Crossing 3	33 K	7583486.00 m S	806364.43 m E
Road-River Crossing 4	33 K	7583366.36 m S	806879.69 m E

Table 7-5: Road-River Crossing Positions

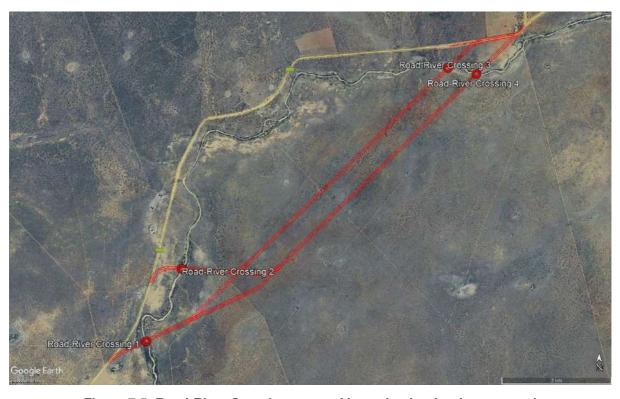


Figure 7-5: Road-River Crossing areas of investigation for the new road



7.5.1 **CROSSINGS 1 AND 2**

Laboratory results, as well as test pit data from test pits TP01-RD, TP02-RD, TP03-RD and TP16-RD were used to determine the expected foundation conditions for the structures at the river crossings at the south-western portion of the new road.

The test pits in this area refused on dense silty sand with varying gravel, alluvium to a depth of 3.9 m or in soft rock gneiss at a depth of 4.3 m in test pit TP02-RD which was dug in the middle of the river. It is expected that the rock hardness increases with depth. Figure 7-6 illustrates the typical materials encountered at various depths for this area is shown below.

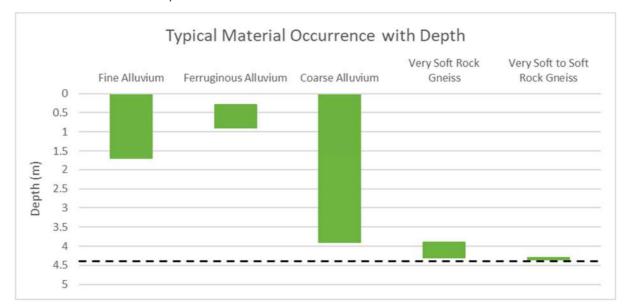


Figure 7-6: Typical Material Occurrence with Depth – Crossings 1 & 2

Based on the test pit data, the fine alluvium and ferruginous soils comprise loose to generally dense silty sand with occasional sub-angular to subrounded gravels with a typical allowable bearing capacity of 200 kPa.

The coarse alluvium as well as the very soft rock gneiss comprise mainly gravelly sands and classify both as "SC" and "A-2" according to the USCS and AASHTO classification systems. The potential expansiveness of both materials is low and has "Good to Fair" compaction ratings.

7.5.1.1 FOUNDING RECOMMENDATIONS

It is recommended that founding of any bridge embankments be placed on at least dense soils not exceeding the allowable bearing capacity of 200 kPa at depths varying from 1.7 m to 4.3 m across the area. Foundation floors should be in situ compacted to at least 90% Modified AASHTO compaction density at OMC.

Seepage was encountered from a depth of 2.3 m below surface and dewatering measures should be considered during construction.



7.5.1.2 EXCAVATABILITY

Based on the refusal depths of the test pits next to the river, it is anticipated that soft excavation conditions will be encountered to a depth of 1.7 m to 4.3 m becoming intermediate to hard with depth.

Test pit or trenched which will be dug in the river will not experience difficult excavation conditions due to the loose material. Collapse of excavation sidewalls and trench walls are expected during excavation in the upper horizons. It is recommended that the trench and excavation walls be benched or sloped at a safe slope angle of at least 1:2 (V:H).

7.5.2 **CROSSINGS 3 AND 4**

The data from test pit TP11-RD and TP13-RD and the laboratory results were used to evaluate the river crossing at points 3 and 4 of the RD site.

The soil profile near the river (TP11-RD) comprises medium dense to dense fine alluvium to a depth of 1.4 m while the soil profile always from the river towards the south comprises dense fine alluvium to 1.8 m, becoming calcareous to 2.6 m. The medium dense to dense pebble marker (coarse alluvium) extends to a depth of 4.0 m below surface whereafter very soft rock gneiss is encountered to 4.5 m before test pit refusal on soft rock. Figure 7-7 shows the typical materials at various depths for this area is shown below.

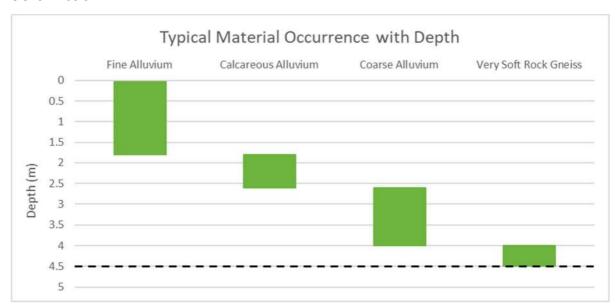


Figure 7-7: Typical Material Occurrence with Depth – Crossings 3 & 4

The silty sand coarse alluvium grades as USCS and AASHTO classification of "SC" and "A-4". This material has low potential expansiveness and has a "Good to Fair" compaction rating (Hazelton & Murphy, 2007).



7.5.2.1 FOUNDING RECOMMENDATIONS

Similar foundation conditions are present as at Crossing 1 and 2 as here in Crossing 3 and 4 and as such it is recommended that the bridge embankment foundations be founded on the dense alluvium or very soft rock gneiss with an allowable bearing capacity of at least 200 kPa.

Foundation floors should be in situ compacted to t least 90% Modified AASHTO compaction density at OMC.

7.5.2.2 EXCAVATABILITY

Similar excavation conditions as Crossings 1 and 2 are expected at Crossings 3 and 4, with test pits or trenches next to the river being having soft excavation conditions to a depth varying from 1.4 m to 4.5 m.

Excavations walls in the river should also be benched or sloped at a safe slope angle of at least 1:2 (V:H) or battered back to support the excavation walls.

Dewatering of the excavation may be required during construction.

7.5.3 ROAD ALIGNMENT

The typical soil profile between the river crossings comprises mainly medium dense to dense, with occasional loose pockets of fine alluvium from surface down a depth of 4.3 m. Coarse alluvium from 0.3 m to 1.4 m or ferruginised alluvium from 0.3 m to 0.7 m is encountered below shallow fine alluvium. Medium dense becoming dense calcified alluvium was identified in test pit TP08-RD from 1.0 m down to 3.2 m.

Very soft rock gneiss was encountered only in test pit TP14-RD from 1.4 m down to 2.6 m after which the excavator refused on soft rock gneiss. Figure 7-8 shows the materials that are typically encountered with depth across this area.

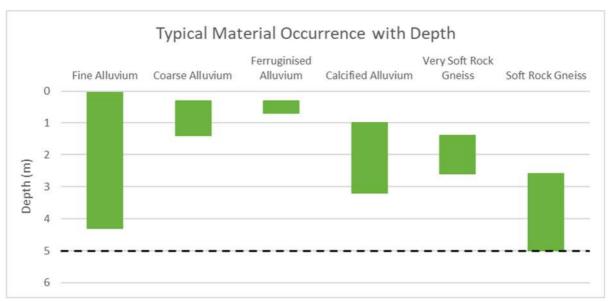


Figure 7-8: Typical Material Occurrence with Depth - Road Route



The fine and calcified alluvium grade mainly as silty or clayey sands with low gravel content in the coarse alluvial material. The fine alluvial soils comprise more than 60% sand and no gravel. The alluvium classifies as "SM" based on the USCS system; however, the fine alluvium classifies as "A-2" while the coarse alluvium classifies as "A-7" based on the AASHTO classification. The materials have a low potential expansiveness and low compressibility rating, as well as a "Good" compaction rating.

Very soft rock gneiss comprises mainly silty sandy gravel and classifies as a well graded gravel (GM) based on the USCS system. An AASHTO group of "A-2" was assigned to this material.

The plasticity index of the soils is generally below 10 %.

The compaction characteristics of the tested materials are summarised in the Table 7-6 below.

Table 7-6: CBR Values of the Tested Samples Between the Road-River Crossings

Test Pit No.	Sample Depth	Material	MDD (kg/m	OMC (%)	1	BR at MO AASHTO ompacti		% Swell	COLTO	
No.	(m)		3)	(%)		95%	98%			
TP04-RD	0.6-2.9	Fine Alluvium	2041	8.1	32	46	81	0.1	G4*	
TP08-RD	1.0-3.2	Calcified Alluvium	1640	20	4	4	5	0.0	NC	
TP12-RD	1.1-4.3	Fine Alluvium	2119	7.4	23	30	46	0.0	G6	
TP14-RD	1.4-2.6	Very Soft Rock Gneiss	2016	12.9	15	17	20	0.0	G7	

Notes:

- 1. NC Not Classifiable.
- 2. The G4 grading of TP04-RD is not considered representative of the material and is anticipated due to the uncharacteristic coarse portion of this sample.

7.5.3.1 UNPAVED WEARING COURSE FOR GRAVEL ROADS

The materials encountered on site have been further evaluated for re-use as unpaved wearing course according to THR20 [8] and the materials are plotted in Appendix B4 by material type. The classes of materials are briefly described below:

- Class A: Materials in this area generally perform satisfactorily but are finely graded and
 particularly prone to erosion by water: they should be avoided, if possible, especially on steep
 grades and sections with steep cross-falls and super-elevations. Most roads constructed from
 these materials perform satisfactorily but may require periodic labour-intensive maintenance
 over short lengths and have high gravel losses due to water erosion.
- Class B: These materials generally lack cohesion and are highly susceptible to the formation
 of loose material (ravelling) and corrugations. Regular maintenance is necessary if these
 materials are used, and the roughness is to be restricted to reasonable levels.



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- Class C: Materials in this zone generally comprise fine, gap-graded gravels lacking adequate cohesion, resulting in ravelling and the production of loose material.
- Class D: Materials with a shrinkage product in excess of 365 tend to be slippery when wet.
- Class E: Materials in this zone perform well in general, provided the oversize material is restricted to the recommended limits.

The alluvium generally classifies as material that is erodible (Class A) and which ravels and corrugates (Class B). This has been seen on the unpaved roads on site. The alluvium also classifies to a lesser extent as slippery (Class D) and good for unpaved roads (Class E).

The pedogenic soils generally follow the same trend as the alluvium classifying as material that is erodible (Class A) and which ravels and corrugates (Class B). The pedogenic alluvium (calcareous/calcified/ferruginous/ ferruginised alluvium) further classifies to as slippery (Class D) and good for unpaved roads (Class E).

The residual gneiss soils generally follow the same trend as the alluvium classifying as material that is erodible (Class A) and which ravels and corrugates (Class B).

The very soft rock gneiss that was tested reveals material that is generally good for unpaved roads (Class E), but which ravel and corrugate (Class B).

The CBR values that were tested vary significantly within each material type typically from as low as 3% to 36% when compacted at 93% Modified AASHTO. The COTO classification generally varied from poorer than G9 to G6 quality material. Occasionally, high gravel content has shown G4 quality material which is not considered to be well represented.

7.5.3.2 FOUNDING RECOMMENDATIONS

The road alignment soils are generally low plasticity soils that lack binding materials. The following is recommended:

- Excavate and spoil the upper 0.3 m organic layer.
- In situ rip and recompact the subgrade along the road alignment to 93% Modified AASHTO compaction at OMC to ensure a minimum CBR value of 7 %.
- Localised deeper compaction in loose zones is anticipated along the alignment.
- Founding of the surface wearing course on dense alluvium.
- Import a suitable surface wearing course placed as per road design.
- Ensure that the road is shaped to prevent ponding of water along the alignment and ensure that water is carried away.

7.5.3.3 EXCAVATABILITY

Based on the test pit data soft excavations conditions are expected to a typical depth of 3.5 m thereafter intermediate and hard excavation conditions are expected with localised deeper excavation nearer the river areas.



7.6 WASTE ROCK DUMP (WRD)

The waste rock dump (WRD) area covers approximately 710 ha of the mine and is divided into two areas for the purpose of this section. The first area, namely Area A (579 ha), is located on the eastern portion of the site and the second area, Area B (130 ha), which is located between the HLP and SWD. The localities of Area A and Area B (outlined in yellow) is shown below in Figure 7-9.

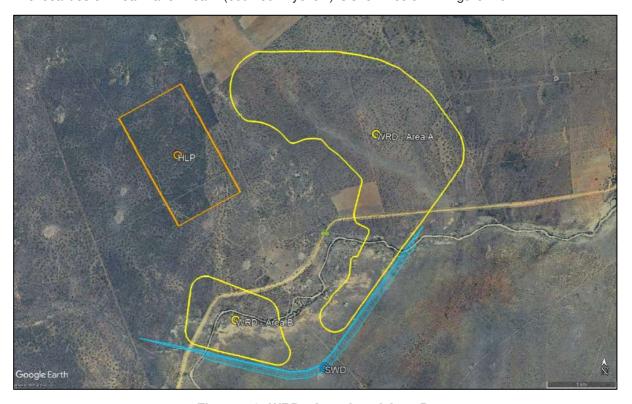


Figure 7-9: WRD - Area A and Area B

7.6.1 AREA A

The majority of Area A is covered with fine alluvium from the surface down to depths varying between 0.2 m and 1.6 m. A coarse alluvium occurs below this and extends generally between 1.1 m and 2.5 m below the surface. Deeper coarse alluvium (encountered to depths of 4.1 m) was recorded near the river area of Area A (WRD-TP17). It is anticipated that a downward coarsening trend is anticipated for the river area.

The alluvium often becomes ferruginous and calcareous with depth, which can be attributed to the existing drainage feature in the north running through Area A from the north-west to the south-east boundary of the WRD boundary joining the Black Nossob river at the new road crossings 3 and 4. The pedogenic soils was identified from as shallow as 0.6 m to as deep as about 5.0 m near the riverbed.

Hardpan calcrete was recorded in 6 test pits in the north to north-western portions of Area A and occurs from roughly 0.2 m down to 2.6 m. The depth and layer thickness of the hardpan calcrete varies significantly across the area of occurrence.



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Residual gneiss is generally observed below the transported and pedogenic soils and was encountered from depths as shallow as 1.1 m down to as deep as 2.4 m below the surface. These residual soils mainly occur mainly in the eastern and central portions of Area A.

Very soft rock gneiss occurs either below the alluvial or pedogenic soils from as shallow as 0.5 m down to as deep as 2.9 m below ground level when encountered. Soft rock to medium hard rock follow with depth down to approximately 3.4 m below surface. Refusal often occurs above the rock surface in fine or calcified alluvium, or on hardpan calcrete.

Based on the laboratory results, the alluvium classifies as clayey sand to low plasticity clays (SC to CL) according to the USCS classification and as "A-2" and "A-6" according to the AASHTO classification system. This material has low potential expansiveness and a compaction rating of "Good to Fair" based on the USCS classification. The WRD soils range between silty clayey sands and sandy gravels.

The pedogenic soils comprise mainly of sandy gravel of varying grading (GC, GP and GM) based on the USCS and as "A-2" based on the AASHTO classification. This material has a low potential expansiveness and a "Good" compaction rating.

The residual gneiss is described as silty clayey sand (SC and A-2), with sand making up more than 60 % of the total texture. In addition, the potential expansiveness of the residual gneiss is low.

7.6.2 AREA B

The northern portion outside the WRD site boundary has alluvial soils occurring from the surface down to less than 2.0 m while the southern and eastern portions have deeper alluvial horizons generally to 3.0 m but occasionally as deep as 5.0 m. This alluvial material classifies as clayey sand (SC) based on the USCS system and as "A-4" and "A-6" according to the AASHTO classification system. This material also has a "Good to Fair" compaction rating and a low potential expansiveness.

Pedogenic soils (mainly calcified alluvium) occur along and south of the riverbed from as shallow as 0.3 m to 4.4 m. The thickness of the pedogenic horizon does vary but is generally thicker than 2.5 m in this area. The pedogenic soils comprise mainly of sandy gravel with minor variations such as silty and gravelly sands. The material classifies as GC, SC and SM according to the USCS classification system and "A-2" and "A-6" in the AASHTO system. The potential expansiveness of this material is low, and it has a "Good to Fair" compaction rating based on the USCS classification.

Residual gneiss was identified near the riverbed as well as slightly north of this area of the WRD. The depth of occurrence range generally between 1.2 m and 1.6 m with the exception of test pit TP1-WRD having residual material down to 3.5 m. This residual material is generally a sand with minor amounts of clay and silt, with the exception of test pit TP2-WRD grading as a sandy gravel. It is therefore expected that the residual material closer to the riverbed be slightly finer grained and become coarser further away from the riverbed. The USCS classification of the residual soils range between "SC" and "SM" for the finer graded soils and between "GP" and "GM" for the residual soils further away from the riverbed. The AASHTO group for the finer soils is "A-2" while the coarser soils are assigned an "A-1" classification.

Very soft rock occurs from approximately 1.3 m below surface and extends to 1.8 m after which it transitions to soft rock from 1.8 m. Test pit TP6-WRD has medium hard rock gneiss occurring from 4.3 m below surface and it is therefore expected that the transition between soft rock and medium hard rock may occur from 2.5 m and 4.0 m. The very soft to soft rock gneiss grades as a gravelly sand and has a USCS classification of "SC" and an AASHTO code of "A-2" assigned to the tested material.



7.6.3 RECOMMENDATIONS

The recommendations for the waste rock dump are to scarify the surface soils to remove the organic component. In situ rip and recompact to 90% Modified AASHTO compaction density at OMC. Place the waste rock with safe side slopes of no steeper than 1:2 (V:H).

7.6.4 EXCAVATABILITY

The test pits at and around Area A of the WRD generally refused at depths shallower than 2 m towards the north with slightly deeper test pits present near the center and southern portions. Softe excavation conditions are expected to depths of 2 m becoming intermediate to hard below.

The test pits at Area B and near the riverbed generally refused or terminated at depths between 3 m and 5 m.

It is therefore expected that excavation conditions near the riverbed will be "soft" to depths of 3.0 m thereafter intermediate and hard excavation conditions are expected.

7.7 POTENTIAL BORROW SOURCES

The identified potential borrow pit area is divided into two areas namely, Area A which is located south of the road and north of the river and Area B adjacent to the river on both sides. The approximate boundary of each area is highlighted in light blue in Figure 7-10 below. Note that the typical soil profile is described below for the whole AM site and not separately for Area A and Area B.



Figure 7-10: Additional Material Source - Area A and Area B



The borrow pit area comprises mainly alluvium from the surface down to 1.0 m below surface. The majority of the test pits refuse on this layer due to the dense soil consistency of this alluvium.

Where the test pits did not refuse on this dense fine alluvium, a 0.3 m coarse alluvium layer is seen underneath it. These alluvium layers often become ferruginous or calcareous with depth and may become calcified as seen in test pit AM-TP04.

Very soft rock gneiss is observed below the alluvium and pedogenic soils from approximately 1.0 m below surface which quickly transitions to soft rock where seen in the two test pits AM-TP2 and AM-TP4 in the west of Area A.

A prominent hardpan ferricrete outcrop was identified during the field investigation along the road on the western boundary of Area A. The size of the ferricrete outcrop is estimated at 8,800 m². The occurrence of the ferricrete outcrop is highlighted in orange in Figure 7-11 below.



Figure 7-11: Ferricrete Outcrop at the AM site

Based on the laboratory results, the fine alluvium comprises mainly silty to clayey sands, with fines component (clay and silt) greater than 40 %. The clay portions of these soils typically range between 19 % and 34 % with a plasticity index (PI) of less than 12 %. A USCS classification of "SC" was generally assigned to these alluvial soils and an AASHTO classification of "A-4".

The coarse alluvium (pebble marker) comprises mainly sandy gravel with more nearly 50 % gravel. A USCS classification of "GC" and an AASHTO classification of "A-2" was assigned to this material.

The pedogenic material comprised of 75 % gravel and 21 % sand and was classified as poorly graded gravel (USCS classification: GP). The AASHTO classification of "A-1" was assigned to this material.

The gneiss bedrock comprising very soft rock comprised mainly clayey silty sand, with sand making up more than 70 % of the soil. The material is classified as non-plastic and has a low potential



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expansiveness. A USCS classification of "SC" was assigned to this material, along with an AASHTO classification of "A-1".

Where is was possible to excavate into the deeper, soft to medium hard rock gneiss, the laboratory results indicate that equal amounts (40%) of gravel and sand are present with a PI of 12 %. This material classifies as "SC" and "A-4" according to the USCS and AASHTO classifications, respectively.

7.7.1 VOLUME ASSESSMENT

The material volumes are estimated based on the area of occurrence and the layer thickness of each assessed material. Table 7-7 and Table 7-8 show the estimated volumes for the materials encountered at Area A and Area B, respectively.

Table 7-7: Estimated Material Volumes at Potential Borrow Pit Area A

Material	Position	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected In Situ Volume (m³)
Fine Alluvium	Area A	379,500	0.3 - 0.7	113,500 – 265,500
Coarse Alluvium	Area A	379,500	0.2 - 0.3	75,500 – 113,500
Pedogenic Soils	Area A	379,500	0.1 - 0.8	37,500 - 303,500
Very Soft Rock Gneiss	Area A	379,500	0.2 – 0.3	75,500 – 113,500
Soft Rock Gneiss	Area A	379,500	N/M	N/M

Notes:

Table 7-8: Estimated Material Volumes at Potential Borrow Pit Area B

Material	Position	Approximate Area Coverage (m²)	Approximate Layer Thickness (m)	Expected In Situ Volume (m³)
Fine Alluvium	Area B	330,900	0.6 - 1.2	198,500 - 397,000
Coarse Alluvium	Area B	330,900	-	N/M
Pedogenic Soils	Area B	330,900	± 0.4	± 132,000
Very Soft Rock Gneiss	Area B	330,900	-	N/M
Soft Rock Gneiss	Area B	330,900	-	N/M

Notes:

1. N/M - Not measurable.

2. "-" - Material not encountered.



^{1.} N/M – Not measurable.

^{2. &}quot;-" - Material not encountered.

7.7.2 EXCAVATABILITY

The alluvial soils covering the surface of the potential borrow pit area has a dense soil consistency on which the excavator generally refused during the field investigation. Based on (SANS634, 2012), the excavation down to refusal depths, generally the upper 1.2 m, can generally be classified as "soft" while the conditions to refusal depth can be classified as "Intermediate to hard".

"Soft" excavation conditions refer to material, which can be excavated efficiently using a back-acting excavator while "Intermediate" refers to material which can be removed using a back-acting excavator or that needs to be removed using pneumatic tools.

7.8 MATERIAL REUSE

The materials encountered across the site have been evaluated by type and discussed in the relevant sections. It should be noted that the variability within the soils on site is significant, owed both to the nature of the alluvium during its deposition causing fine or coarse lenses and due to the erratic nature of the pedogenic soils distribution both laterally and horizontally within the profile.

The materials on site are generally classified as clayey sands to gravels with varying CBR values. COTO classification generally varies between poorer than G9 and G6 quality materials classifying as A-1, A-2 and A-4 materials with occasional A-6 soils.

No distinct suitable borrow source of clay was identified during the investigation, although fine grained soils (classified SC and SM) occur throughout the site at various depths. The plasticity of the fine-grained soils is generally less than 10%.

The permeability of the materials when remoulded vary between 10^{-6} m/s and 10^{-8} m/s due to the generally fine-grained nature of the materials. The laboratory results indicate that the fine alluvium has a low permeability when compacted to 90% Modified AASHTO compaction density. This fine alluvium can be used for the construction of the clay liner preparation layer for the heap leach pad, since the permeability of such a material should typically be 1×10^{-8} m/s.

The reuse of soils for road construction has been discussed in Section 7.5.3.1 and is generally susceptible to erosion, ravelling and corrugation due to the low PI.

The alluvium can be reused as general fill during construction. The coarse alluvium and pebble marker is suitable material for the subgrade of roads, selected earthworks for foundation platforms and untreated road layers. The material can also be used for trench backfilling. The gravelly nature of the pedogenic soils as well as the residual gneiss make these materials suitable to be used as general fill, selected layers for foundation platforms, as well as subgrade and subbase material for road construction. The very soft to soft rock gneiss can also be used as subgrade and subbase for road construction. In addition, the very soft rock gneiss and the soft to medium hard rock has a "Good" compaction rating and can also be used as general fill material or as subgrade in road construction.



8.0 REFERENCES

- ASTM D3282-15. (2019). Standard Practice for Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes. West Conshohocken: ASTM International.
- Bell, F. G. (1994). Engineering Geology (Second ed.). Burlington: Elsevier Ltd.
- Brink, A. B., & Bruin, R. M. (2002). Guidelines for Soil and Rock Logging in South Africa. Geoterminology Workshop 1990, SAIEG Second Impressions.
- Byrne, G., & Berry, A. D. (2008). *A Guide to Practical Geotechnical Engineering in Southern.* Franki Africa (Pty) Ltd.
- Clayton, C. R. (1995). The standard penetration test (SPT): method and use. *Construction Industry Research and Information Association*, pp.129.
- Franki. (2008). A Guide to Practical Geotechnical Engineering in South Africa. Franki.
- González de Vallejo, L., & Ferrer, M. (2011). Geological Engineering. Boca Ranton: CRC Press.
- Hazelton, P., & Murphy, B. (2007). *Interpreting Soil Test Results: What Do All the Numbers Mean?* Collingwood: CSIRO Publishing.
- Heymann, G. (2016). Typical strength properties of South African soils. In Proceedings of the First Southern African Geotechnical Conference. 289-294. CRC Press.
- Knappett, J. A., & Graig, R. F. (2012). Craig's Soil Mechanics. Abingdon: Spon Press.
- Look, B. G. (2014). *Handbook of Geotechnical Investigation and Design Tables.* London: Taylor & Francis Group.
- Mendelsohn, J., Jarvis, A., Roberts, C., & Robertson, T. (2002). *Atlas of Namibia*. Cape Town: David Phillip.
- Owens, G. (2009). Fulton's Concrete Technology (Ninth ed.). Midrand: Cement & Concrete Institute.
- Pagani et al. (2018). Global Hazard Map. Retrieved August 30, 2023, from Gobal Earthquake Model.
- SAICE. (2002). Guidelines for Soil and Rock Logging in Southern Africa. *Geoterminology Workshop Organised by AEG, SAICE and SAIEG, 1990.* AEG SA Section, SAICE and SAIEG.
- SAICE. (2010). Site Investigation Code of Practice. *1st Edition*. South African Institution of Civil Engineering Geotechnical Division.
- SANS634. (2012). *Geotechnical Investigations for Township Establishments*. South African National Standards.
- SRK Consulting (South Africa) (Pty) Ltd. (2013). Omitiomire Oxide Project Namibia Tailings Storage Facility Definitive Faesibility Study Geotechnical Investigation, Final Report. Report Number 462312
- Weinert, H. H. (1980). The Natural Road Construction Materials of Southern Africa. Cape Town: H&R.



9.0 CERTIFICATION

This report was prepared and reviewed by the undersigned. Prepared: Christiaan Homan Junior Engineering Geologist Reviewed: Bronwen Klaas, Pr. Sci. Nat. Senior Engineering Geologist Approved: Dawid Mouton, Pr. Sci. Nat. **Technical Consultant**

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APPENDIX A

Geotechnical Site investigation Program

Appendix A1

Site Locality Map

Appendix A2

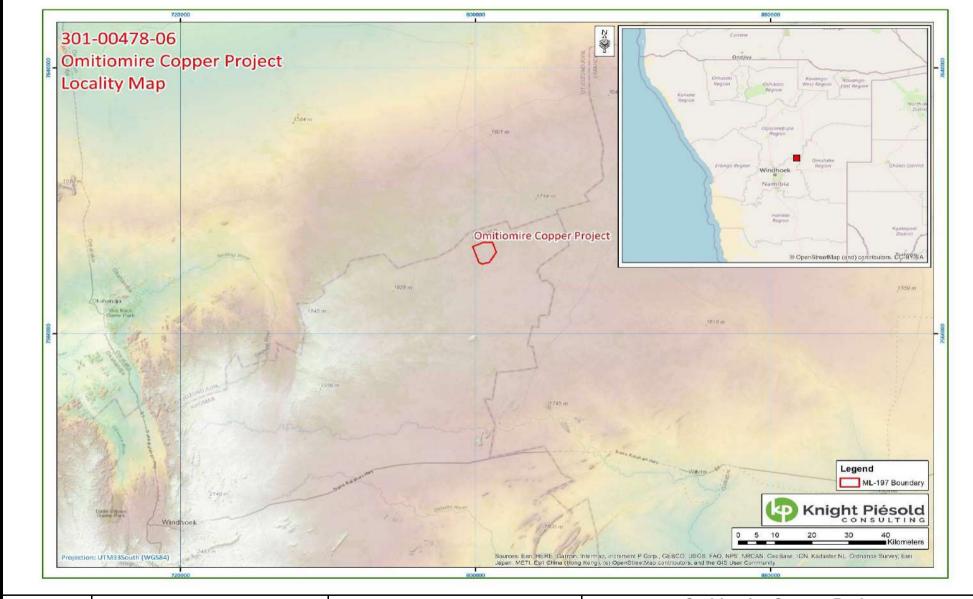
Layout of Investigated Points



APPENDIX A1

Site Locality Map







PROJECT NO. WI 301-00478/06
BY: T Oosthuizen
CHECKED: B Klaas
DRAWING DATE: 30-Jun-23
REVISION: A

Coordinate System: WGS 1984 UTM Zone 33S

Projection: Transverse Mercator Datun: WGS 1984

Units: Meter

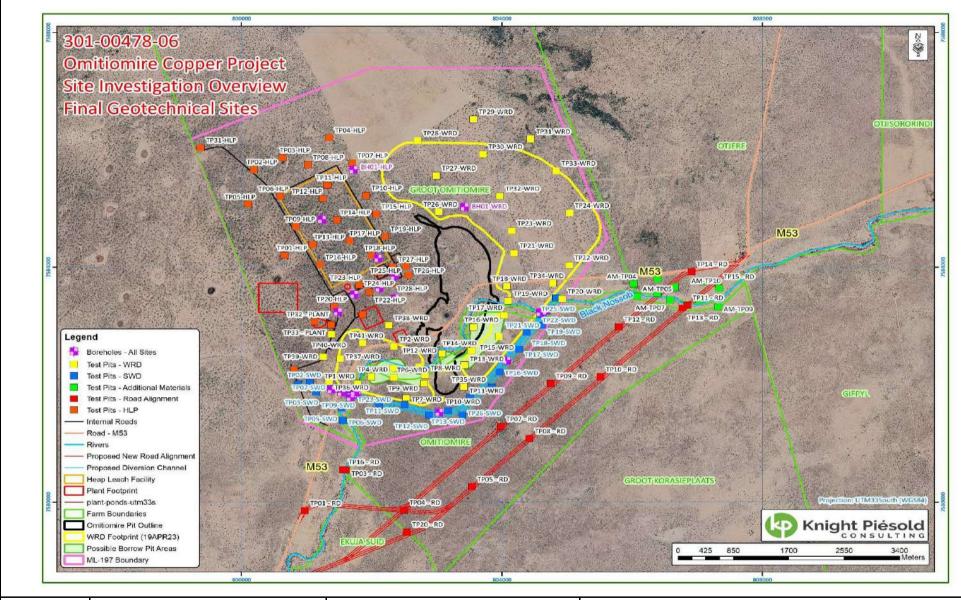
Omitiomire Copper Project: Site Locality Map

CLIENT: FIGURE NO:
Craton Mining and Exploration (Pty) Ltd A-1

APPENDIX A2

Layout of Investigated Points







PROJECT NO. WI 301-00478/06 BY: T Oosthuizen CHECKED: B Klaas DRAWING DATE: 30-Jun-23 **REVISION:**

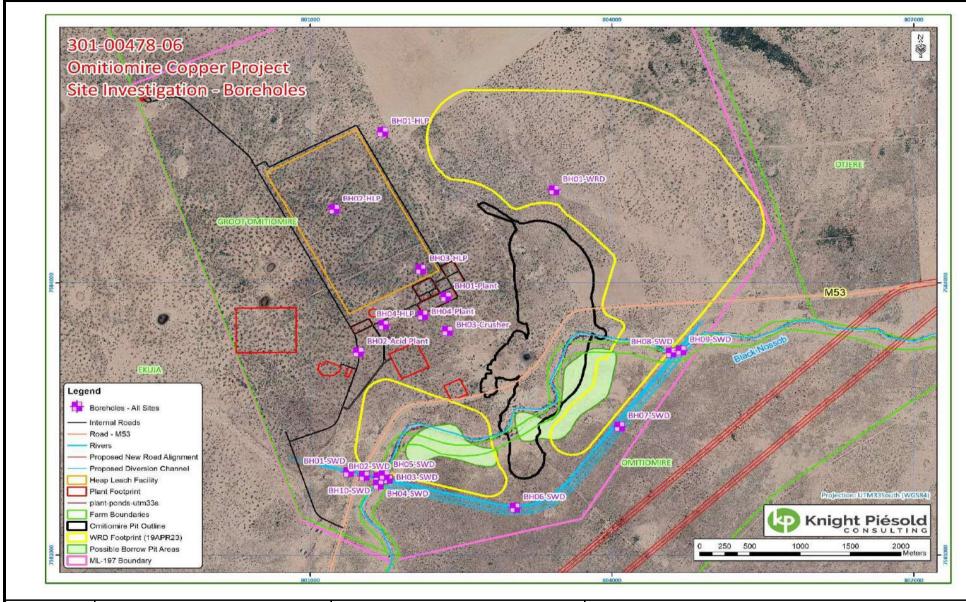
Coordinate System: WGS 1984 UTM Zone 33S Projection: Transverse Mercator

Datun: WGS 1984

Units: Meter

Omitiomire Copper Project: Location of geotechnical boreholes & test pits - All sites

CLIENT: FIGURE NO: A-2 Craton Mining and Exploration (Pty) Ltd





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BY: T Oosthuizen
CHECKED: B Klaas
DRAWING DATE: 30-Jun-23
REVISION: A

Coordinate System: WGS 1984 UTM Zone 33S

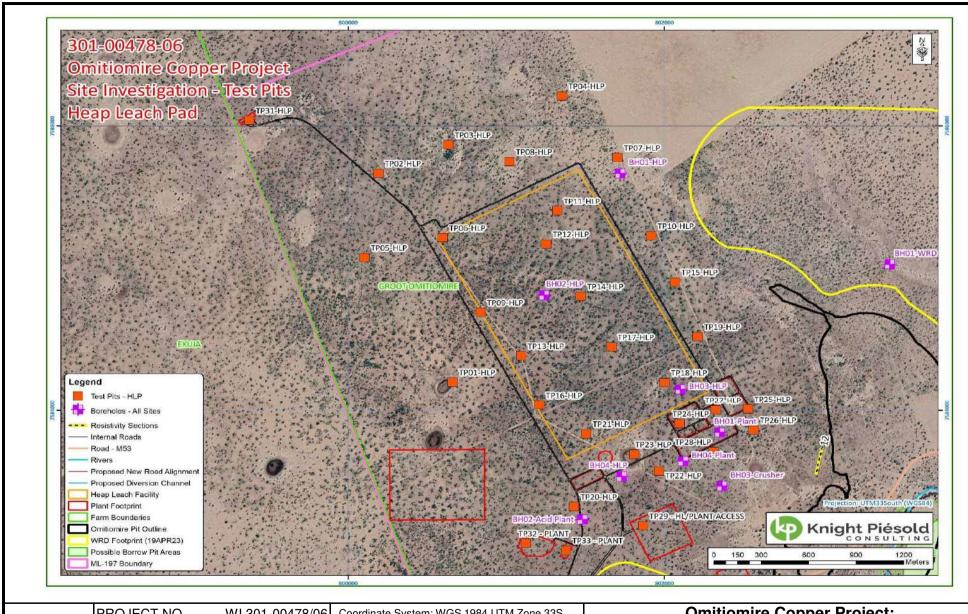
Projection: Transverse Mercator Datun: WGS 1984

Units: Meter

Omitiomire Copper Project:

Location of geotechnical boreholes - All sites

CLIENT: FIGURE NO: Craton Mining and Exploration (Pty) Ltd A-3





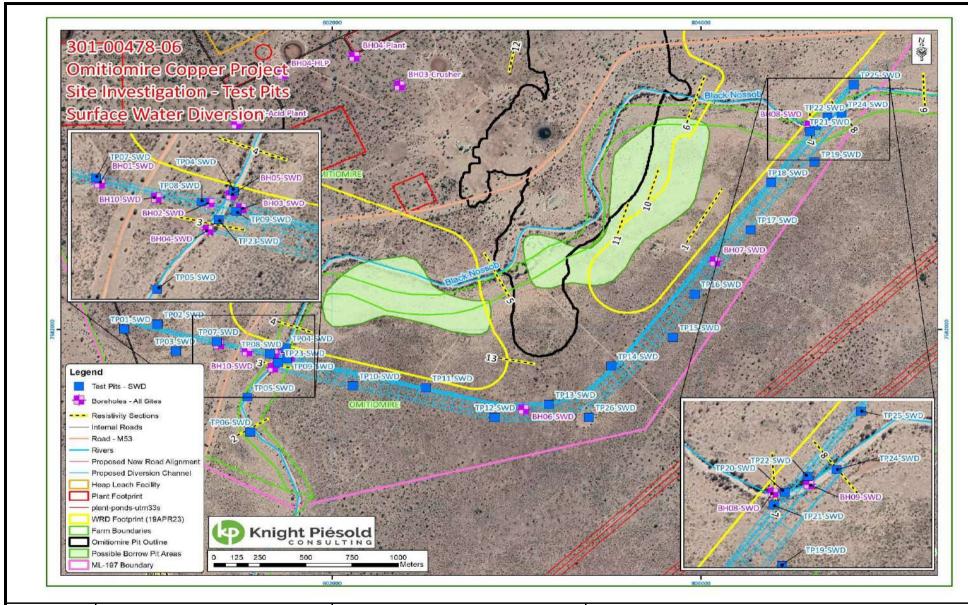
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BY:	T Oosthuizen
CHECKED:	B Klaas
DRAWING DATE:	30-Jun-23
REVISION:	Α

Coordinate System: WGS 1984 UTM Zone 33S Projection: Transverse Mercator

Datun: WGS 1984

Datun: WGS 1984 Units: Meter Omitiomire Copper Project:
Layout of geotechnical boreholes and test pits - Heap
Leach Pad (HLP) and Plant Area

CLIENT:	FIGURE NO:
Craton Mining and Exploration (Pty) Ltd	A-4





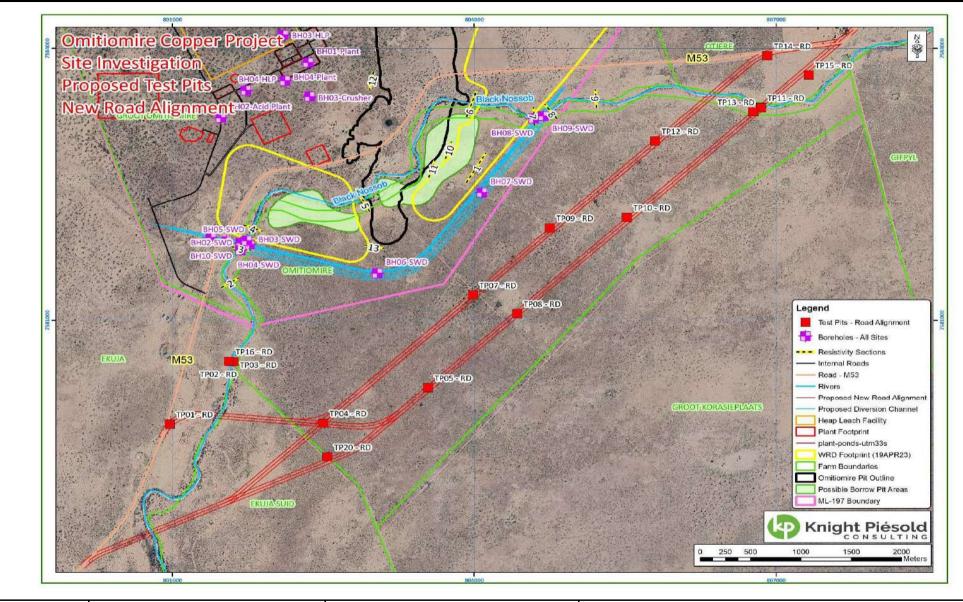
PROJECT NO. WI 301-00478/06
BY: T Oosthuizen
CHECKED: B Klaas
DRAWING DATE: 30-Jun-23
REVISION: A

Coordinate System: WGS 1984 UTM Zone 33S

Projection: Transverse Mercator

Datun: WGS 1984 Units: Meter Omitiomire Copper Project:
Layout of geotechnical boreholes and test pits Stormwater Diversion Canal (SWD)

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REVISION:	Α

Coordinate System: WGS 1984 UTM Zone 33S

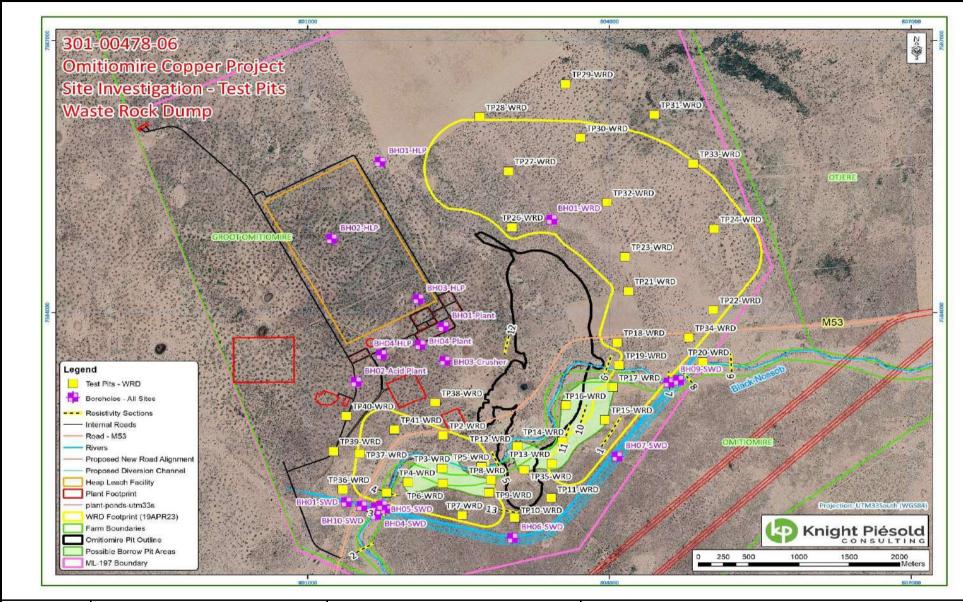
Projection: Transverse Mercator

Datun: WGS 1984 Units: Meter

Omitiomire Copper Project:

Layout of geotechnical test pits - Road diversion

CLIENT: FIGURE NO: Craton Mining and Exploration (Pty) Ltd A-6





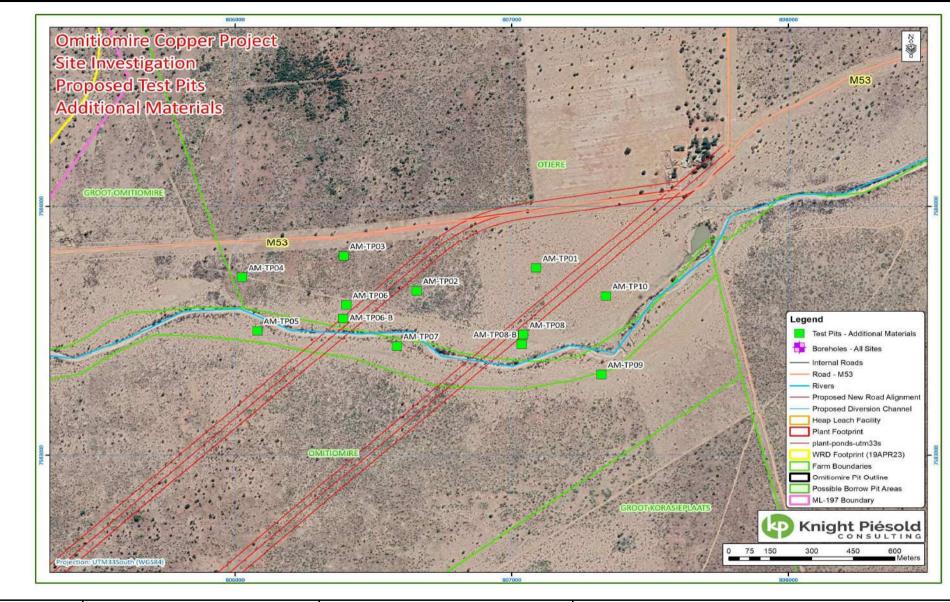
PROJECT NO. WI 301-00478/06
BY: T Oosthuizen
CHECKED: B Klaas
DRAWING DATE: 30-Jun-23
REVISION: A

Coordinate System: WGS 1984 UTM Zone 33S

Projection: Transverse Mercator

Datun: WGS 1984 Units: Meter Omitiomire Copper Project:
Layout of geotechnical boreholes and test pits - Waste
Rock Dump (WRD)

CLIENT: FIGURE NO:
Craton Mining and Exploration (Pty) Ltd A-7





PROJECT NO. WI 301-00478/06
BY: T Oosthuizen
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DRAWING DATE: 30-Jun-23
REVISION: A

Coordinate System: WGS 1984 UTM Zone 33S

Projection: Transverse Mercator

Datun: WGS 1984 Units: Meter

Omitiomire Copper Project: Layout of geotechnical test pits - Additional Materials (AM)

CLIENT: FIGURE NO: Craton Mining and Exploration (Pty) Ltd A-8

Summary Tables for Soil Profiles, Borehole Logs, SPT Test Results and Laboratory Test Results

Appendix B1

Summary Table of Test Pit Logs

Appendix B2

Summary Table of Rotary Core Boreholes

Appendix B3

Summary Table of SPT Test Results

Appendix B4

Summary Table of Laboratory Test Results



APPENDIX B1

Summary of Test Pits





CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF HEAP LEACH PAD AND PROCESSING PLANT TEST PIT PROFILES

		AS BUILT CO	OORDINATES			THICKNESS OF LAYER										
		UTM	1 33 K	TOTAL DEPTH (m)	(m)									ODOUNDWATER LEVEL		
STRUCTURE ID.	TEST PIT No.				Transported soils		Pedogenic soils				Residual Soil	Bedrock			GROUNDWATER LEVEL	
		X-COORD	Y-COORD	J2 ()	Fine Alluvium	Coarse Alluvium	Ferruginous/ *Ferruginised Alluvium	Calcareous/ *Calcified Alluvium	Nodular/ *Honeycomb Ferricrete	Nodular/ *Honeycomb Calcrete	Hardpan Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock	(m)
	TP01-HLP	800659.00	7584200.00	1.20 + R	0.30 - 1.20 + R	0 - 0.30	Allavialli	-	-	- Calcrete	_		_	_		
	TP02-HLP	800189.00	7585668.00	1.50 + R	0 - 0.60	-	0.60 - 1.50 + R	_		-	-	_	-	-	_	 -
	TP03-HLP	800632.00	7585869.00	3.10 + R	0 - 0.80	0.80 - 2.30	-	_	-	-	_	2.30 - 3.10	3.10 + R	-		_
	TP04-HLP	801350.85	7586209.65	1.35 + R	0 - 0.30	-	-	*0.30 - 1.10		1.10 - 1.35			-	-		-
	TP05-HLP	800101.00	7585074.00	1.60 + R	0 - 1.40	-	1.40 - 1.60 + R	_		*1.35 + R	_	-	-	-		-
	TP06-HLP	800597.01	7585212.77	1.80 + R	0 - 0.25	-	-	*0.25 - 1.50		-	-	1.50 - 1.80 + R	_	-		-
	TP07-HLP	801701.70	7585778.61	0.70 + R	0 - 0.70	_	_	_	_	-	0.70 + R		-	-		_
	TP08-HLP	801019.00	7585750.00	1.85 + R	0 - 1.35	-	-	*1.35 - 1.85 + R		-	-	-	-	-		-
	TP09-HLP	800837.00	7584690.00	3.15 + R	0 - 0.60	2.10 - 2.70	-	0.60 - 2.10	-	-	-	-	2.70 - 3.15	3.15 + R	-	-
	TP10-HLP	801915.33	7585227.33	1.50 + R	0 - 0.30	-	*1.25 - 1.50 + R	*0.30 - 1.25	-	-	-	-	-	-	-	-
	TP11-HLP	801321.00	7585404.00	1.35 + R	0 - 0.40	-	-	0.40 - 1.15 *1.15 - 1.35 + R	-	-	-	-	-	-	-	-
	TP12-HLP	801251.02	7585169.08	1.60 + R	0 - 0.30	-	-	*1.15 - 1.35 + R 0.30 - 1.40 *1.40 - 1.60 +R	-	-	-	-	-	-	-	-
	TP13-HLP	801093.27	7584385.61	3.00 + R	0 - 0.30	2.25 - 2.50	-	0.30 - 2.25	-	-	-	-	2.50 - 3.00	3.00 + R	-	-
	TP14-HLP	801470.07	7584802.62	2.05 + R	0 - 0.30	1.45 - 1.65	-	0.30 - 1.45	-	-	-	1.65 - 2.05 + R	-	-	-	-
Heap Leach Pad (HLP)	TP15-HLP	802066.00	7584902.00	2.05 + R	0 - 0.35	1.65 - 1.80	*1.80 - 2.05 + R	*0.35 - 1.65	-	-	-	-	-	-	-	-
	TP16-HLP	801209.00	7584039.00	3.00 + R	0 - 2.30	2.30 - 2.50	-	-	-	-	-	2.50 - 3.00	3.00 + R	-	-	-
	TP17-HLP	801663.45	7584447.86	2.35 + R	0 - 0.35	-	*2.15 - 2.35 + R	0.35 - 1.75 *1.75 - 2.15	-	-	-	-	-	-	-	-
	TP18-HLP	802000.00	7584198.00	3.15 + R	0 - 1.10	-	-	*1.10 - 1.40	-	-	-	1.40 - 2.10	2.10 - 3.15	3.15 + R	-	-
	TP19-HLP	802208.97	7584520.16	1.70 + R	0 - 0.40	-	-	*0.40 - 1.70 + R	-	-	-	-	-	-	-	-
	TP20-HLP	801428.00	7583326.00	1.80 + R	0 - 1.40	1.40 - 1.60	-	-	-	-	-	-	1.60 - 1.80	1.80 + R	-	-
	TP21-HLP	801505.04	7583835.86	1.45 + R	0 - 0.50		1.10 - 1.45 + R	*0.50 - 1.10	-	-	-	-	-	-	-	-
	TP22-HLP	801964.23	7583574.80	2.25 + R	0 - 1.80	1.80 - 2.05	-	-	-	-	-	-	2.05 - 2.25	2.25 + R	-	-
	TP23-HLP	801808.00	7583693.00	0.80 + R	0 - 0.80 + R	-	-	-	-	-	-	-		-	-	-
	TP24-HLP	802094.00	7583908.00	2.10 + R	0 - 0.80	1.20 - 1.75	-	0.80 - 1.20	-	-	-	-	1.75 - 2.10	2.10 + R	-	-
	TP25-HLP	802527.33	7584011.03	0.85 + R	0 - 0.65	-	-	-	-	-	-	-	-		0.85 + R	-
	TP26-HLP	802563.00	7583862.00	1.95 + R	0 - 1.10	1.10 - 1.40	-	-	-	-	-	-	1.40 - 1.95	1.95 + R	-	-
	TP27-HLP	802323.97	7584004.09	1.40 + R	0 - 1.05	1.05 - 1.20	-	-	-	-	-	-	1.20 - 1.40	1.40 + R	-	-
	TP28-HLP	802309.00	7583711.00	1.30 + R	0 - 0.40	-	*0.40 - 1.30 + R	-	-	-	-	-	-	-	-	-
	TP31-HLP	799373.00	7586039.00	1.30 + R	0 - 0.60	0.60 - 1.20	-	-	-	-	-	-	1.20 - 1.30	1.30 + R	-	-
	TP29 - HL/PLANT ACCESS	801863.00	7583191.00	1.90 + R	0.0 - 1.70	1.70 - 1.90	-	-	*1.90 + R	-	-	-	-	-	-	-
Divis	TP30 - HL/PLANT ACCESS	800808.00	7582253.00	1.90 + R	0.0 - 1.30	1.30 - 1.50	-	-	-	-	-	-	-	1.50 - 1.90	1.90 + R	-
Plant	TP32 - PLANT	801124.00	7583069.00	1.70 + R	0 - 0.70	0.70 - 1.00	*1.00 - 1.70	-	1.70 + R	-	-	-	-	-	-	-
	TP33 - PLANT	801376.00	7583021.00	1.40 + R	0 - 1.00	-	*1.00 - 1.40 + R	-	-	-	-	-	-	-	-	-

NOTES:

1. R. REFUSAL.
2. += MATERIAL CONTINUES AS ABOVE.
3. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.

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CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF STORMWATER DIVERSION CHANNEL TEST PIT PROFILES

	OORDINATES			THICKNESS OF LAYER										
		UTM	1 33K		(m)								CDOUNDWATER LEVEL	
	TEST PIT No.				Transported Soil		Pedogenic Soil			Residual Soil Gn		Gneiss Bedrock		GROUNDWATER LEVEL
STRUCTURE ID.		X-COORD	DRD Y-COORD	TOTAL DEPTH (m)	Fine Alluvium	Coarse Alluvium	Ferruginous/ *Ferruginised Alluvium	Calcareous/ *Calcified Alluvium	Nodular Ferricrete/ * Nodular Calcrete/ **Honeycomb Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock	(m)
	TP01-SWD	800870.00	7582003.00	0.60 + R	-	0 - 0.30	-	-	-	-	-	0.30 - 0.60	0.60 + R	-
	TP02-SWD	801050.00	7582033.00	1.50 + R	0 - 0.70	-	-	*'0.70 - 1.50	**'1.50 + R	-	-	-	-	-
	TP03-SWD	801151.00	7581871.00	1.80 + R	0 - 0.60	-	-	0.60 - 1.80 + R	-	-	-	-	-	-
	TP04-SWD	801747.00	7581888.00	3.90 + R	0 - 3.90	-	-	-	-	3.90 + R	-	-	-	1.50
Stormwater Diversion Channel (SWD)	TP05-SWD	801539.00	7581592.00	3.80 + R	0 - 2.00	2.00 - 3.80	-	-	-	-	-	-	3.80 + R	1.70
Inlet	TP06-SWD	801555.00	7581382.00	1.95 + R	0 - 1.60	-	*1.60 - 1.95 + R	-	-	-	-	-	-	-
	TP07-SWD	801373.00	7581928.00	1.90 + R	0 - 0.40	0.40 - 1.40	-	-	-	-	1.40 - 1.90	1.9 + R	-	-
	TP08-SWD	801661.00	7581854.00	1.40 + R	0 - 0.70	-	*0.70 - 1.40 + R	-	-	-	-	-	-	-
	TP09-SWD	801754.00	7581825.00	1.20 + R	0 - 1.20 + R	-	-	-	-	-	-	-	-	-
	TP23-SWD	801706.00	7581801.00	2.50 + R	0 - 2.50	-	-	-	-	-	-	-	2.50 + R	-
	TP10-SWD	802111.00	7581662.00	3.20 + R	0 - 0.35	-	*0.35 - 1.20 *2.10 - 3.00	-	1.20 - 2.10 *3.00 - 3.20 + R	-	-	-	-	-
	TP11-SWD	802507.00	7581650.00	1.40 + R	0 - 0.30	-	*0.30 - 1.40 + R	-	-	-	-	-	-	-
	TP12-SWD	802880.00	7581470.00	4.70+	0 - 0.50	-	*0.50 - 3.00	*3.00 - 4.70+	-	-	-	-	-	-
	TP13-SWD	803174.00	7581550.00	4.40+	0 - 0.45	-	*0.45 - 1.60	*1.60 - 4.40+	-	-	-	-	-	-
Stormwater Diversion Channel (SWD)	TP14-SWD	803514.00	7581781.00	1.35 + R	0 - 1.35	-	*1.35 + R	-	-	-	-	-	-	-
Channel	TP15-SWD	803848.00	7581953.00	2.50 + R	0 - 0.50	-	0.50 - 2.50 + R	-	-	-	-	-	-	-
	TP16-SWD	803965.00	7582214.00	2.80 + R	-	2.10 - 2.80 + R	0 - 2.10	-	-	-	-	-	-	-
	TP17-SWD	804269.00	7582600.00	2.50 + R	0 - 1.30	-	*1.30 - 2.50 + R	-	-	-	-	-	-	-
	TP18-SWD	804380.00	7582887.00	0.90 + R	0 - 0.90	-	-	-	-	-	-	-	-	-
	TP26-SWD	803392.00	7581470.00	4.20 + R	0 - 1.90	-	-	*1.90 - 2.90	-	-	2.90 - 4.20	4.20 + R	-	-
	TP19-SWD	804615.00	7583010.00	2.40 + R	0 - 1.00	-	1.00 - 1.40 *1.40 - 2.40 + R	-	-	-	-	-	-	-
	TP20-SWD	804622.00	7583230.00	4.80 + R	0 - 3.40	3.40 - 4.80	-	-	-	-	-	-	4.80 + R	-
Stormwater Diversion Channel (SWD)	TP21-SWD	804592.00	7583193.00	1.10 + R	0 - 0.30	-	*0.30 - 1.10 + R	-	-	-	-	-	-	-
Outlet	TP22-SWD	804685.00	7583279.00	4.30 + R	0 - 1.40	-	*1.40 - 4.30 + R	-	-	-	-	-	-	-
	TP24-SWD	804762.00	7583299.00	4.95 + R	0 - 4.90	-	-	-	-	-	-	4.90 - 4.95	4.95 + R	-
	TP25-SWD	804829.00	7583474.00	2.40 + R	0 - 0.65	-	-	-	*0.65 - 0.95	0.95 - 1.50	1.50 - 2.40	-	2.40 + R	-

NOTES

- 1. R = REFUSAL.
- 2. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.

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CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF NEW ROAD TEST PIT PROFILES

		AS BUILT COORDINATES UTM 33K					THICKNES	S OF LAYER				
						CROUNDWATER LEVEL						
STRUCTURE ID.	TEST PIT No.			TOTAL DEPTH (m)	Transpo	rted soils	Pedoge	enic soils	Bedr	ock	GROUNDWATER LEVEL	
		X-COORD	Y-COORD		Fine Alluvium	Coarse Alluvium	Ferruginous/ *Calcareous Alluvium	Calcified Alluvium	Very Soft Rock	Soft Rock	(m)	
	TP01 - RD	800971.00	7579851.00	0.90 + R	0 - 0.30	-	0.30 - 0.90 + R	-	-	-	-	
	TP02 - RD	801584.00	7580544.00	4.30 + R	-	0 - 3.90	-	-	3.90 - 4.30 + R	-	2.30	
	TP03 - RD	801605.00	7580544.00	1.70 + R	0 - 1.70 + R	-	-	-	-	-	-	
	TP04 - RD	802497.00	7579862.00	2.90 + R	0 - 2.90 + R	-	-	-	-	-	-	
	TP05 - RD	803541.00	7580258.00	3.70 + R	0 - 3.70 + R	-	-	-	-	-	-	
	TP07 - RD	803985.00	7581279.00	2.80 + R	0 - 2.80 + R	-	-	-	-	-	-	
	TP08 - RD	804425.00	7581074.00	3.20 + R	0 - 1.00	-	-	1.00 - 3.20 + R	-	-	-	
New Road	TP09 - RD	804750.00	7582018.00	3.40 + R	0 - 3.40 + R	-	-	-	-	-	-	
New Road	TP10 - RD	805515.00	7582133.00	2.80 + R	0 - 2.80 + R	-	-	-	-	-	-	
	TP11 - RD	806850.00	7583349.00	1.40 + R	0 - 1.40 + R	-	-	-	-	-	-	
	TP12 - RD	805797.00	7582981.00	4.30 + R	0 - 4.30 + R	-	-	-	-	-	-	
	TP13 - RD	806772.00	7583302.00	4.50 +	0 - 1.80	2.60 - 4.00	*1.80 - 2.60	-	4.00 - 4.50 +	-	-	
	TP14 - RD	806913.00	7583919.00	2.60 + R	0 - 0.30	0.30 - 1.40	-	-	1.40 - 2.60	2.60 + R	-	
	TP15 - RD	807325.00	7583709.00	0.70 + R	0 - 0.30	-	0.30 - 0.70 + R	-	-	-	-	
	TP16 - RD	801561.00	7580548.00	1.40 + R	0 - 1.40 + R	-	-	-	-	-	-	
	TP20 - RD	802541.00	7579491.00	2.50 + R	0 - 2.50 + R	-	-	-	-	-	-	

NOTES:

- 1. R = REFUSAL.
- 2. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.
- 3. TP06-RD, TP17-RD TP18-RD, TP19-RD WERE NOT EXCAVATED DUE TO ACCESS CONSTRAINTS.

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CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF WASTE ROCK DUMP TEST PIT PROFILES

		AS BUILT C	OORDINATES							THICKNESS OF LA	YER					
		UTN	UTM 33K		UTM 33K		(m)									1
				TOTAL	Tran	sported	Pedogenic				Residual Soil Bedrock				GROUNDWATER LEVEL	
STRUCTURE ID.	TEST PIT No.	X-COORD	Y-COORD	DEPTH (M)	Fine Alluvium	Coarse Alluvium	Ferruginous/ *Ferruginised Alluvium	Calcified Alluvium	Nodular Calcrete	Honeycomb Calcrete/* Ferricrete	Hardpan Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock	(m)
	TP1-WRD	801782.00	7582015.00	3.50 + R	0 - 3.50							3.50 + R			-	1.40
	TP2-WRD	802343.00	7582645.00	1.80 + R	0 - 1.20	1.20 - 1.40					-	1.40 - 1.60	1.60 - 1.80	1.80 + R	-	
	TP3-WRD	802332.80	7582281.00	3.30 + R	0 - 0.30	-		0.80 - 3.30 + R		-	-		-	-	-	
	TP4-WRD	801993.39	7582129.46	2.40 + R	0 - 0.40 & 1.50 2.40 + R	-	-	0.40 - 1.50	-	-	-	-	-	-	-	
	TP5-WRD	802728.00	7582301.00	3.60 + R	0 - 0.90	-	0.90 - 2.10	2.10 - 3.60 + R	-	-	-		-		-	-
	TP6-WRD	802341.11	7582119.35	4.30 + R	-	0 - 0.45	*0.45 - 2.35	3.10 - 4.30	2.35 - 3.10	-					4.30 + R	-
	TP7-WRD	802532.00	7581769.00	0.80 + R	0 - 0.80 + R	-	-	-		-					-	-
	TP8-WRD	802820.40	7582154.58	1.70 + R	0 - 0.60	-		0.60 - 1.70 + R	-	-					-	-
	TP9-WRD	802802.00	7582018.00	1.80 + R	0 - 0.80	0.80 - 1.80+R		-		-					-	-
	TP10-WRD	803055.00	7581743.00	4.40 +	0 - 4.00			4.00 - 4.40+							-	-
	TP11-WRD	803420.00	7581961.00	3.70 + R	0 - 0.30		*0.30 - 1.50	2.70 - 3.70 + R	1.50 - 2.70		-		-		-	-
	TP12-WRD	803082.00	7582525.00	1.80 + R	0 - 1.00	1.00 - 1.20		-		-	-	1.20 - 1.30	1.30 - 1.80	-	1.80 + R	-
	TP13-WRD	803424.70	7582333.22	3.40 + R	0 - 1.10	1.40 - 3.10		1.10 - 1.40		-	-		3.10 - 3.40	3.40 + R	-	
	TP14-WRD	803530.99	7582575.57	4.90+	-	2.80 - 4.90+	0.00 - 0.80 *0.80 - 2.80			-	-		-	-	-	
	TP15-WRD	803953.75	7582815.05	3.60 + R	0 - 0.70			0.70 - 3.60 + R		-	-		-	-	-	
	TP16-WRD	803563.99	7582976.05	5.00 +	0 - 0.50			1.80 - 5.00 +	0.50 - 1.80	-	-		-		-	
	TP17-WRD	804031.00	7583179.00	4.10 + R	0 - 0.90	3.60 - 4.10 + R	-	-	0.90 - 3.60	-	-		-	-	-	
	TP18-WRD	804076.74	7583671.36	2.20 + R	0 - 1.60	-	-	-	1.60 - 2.20	-	2.20 + R		-		-	-
	TP19-WRD	804095.95	7583420.70	1.40 + R	0 - 0.30		*0.30 - 1.40 + R	-		-	-		-		-	-
Waste Rock Dump (WRD)	TP20-WRD	804927.00	7583457.00	2.70 + R	0 - 0.50			-			-		0.50 - 2.70		2.70 + R	-
	TP21-WRD	804187.03	7584237.04	3.10 + R	0 - 0.90				1.10 - 3.00		-		0.90 - 1.10		3.00 - 3.10 + R	-
	TP22-WRD	805033.00	7584029.00	1.20 + R	0 - 1.20	-		-	-	-	-		-		-	-
	TP23-WRD	804153.99	7584615.84	1.50 + R	0 - 0.85						0.85 - 1.50 + R		-	-	-	-
	TP24-WRD	805039.00	7584921.00	1.30 + R	0 - 1.00	1.00 - 1.30	*1.30 + R			-	-		-	-	-	-
	TP26-WRD	803030.00	7584941.00	2.60 + R	0 - 1.00	1.00 - 1.20	-	-	-	-	1.20 - 2.60 + R		-			-
	TP27-WRD	802994.92	7585563.53	0.25 + R	0 - 0.25		-	-	-	-	0.25 + R		-	-	-	-
	TP28-WRD	802706.77	7586159.95	1.20 + R	0 - 0.45	-	-	-	0.45 - 1.20	1.20 + R						-
	TP29-WRD	803562.23	7586522.25	2.90 + R	0 - 1.25	-	-	-	1.25 - 1.60			1.60 - 2.07	2.07 - 2.90		2.90 + R	-
	TP30-WRD	803710.59	7585929.08	1.55 + R	0 - 1.25	1.25 - 1.40	-		-	-	-		-	1.40 - 1.55	1.55 + R	-
	TP31-WRD	804445.00	7586187.00	1.40 + R	0 - 0.60	0.60 - 1.20	-	-		*1.20 - 1.40 + R	-		-		-	-
	TP32-WRD	803973.00	7585217.00	1.45 + R	0 - 1.15	-	-		-	-	1.15 - 1.45 + R		-		-	-
	TP33-WRD	804832.00	7585645.00	1.30 + R	0 - 1.10	1.10 - 1.20	-			-	-	1.20 - 1.30+R			-	-
	TP34-WRD	804786.59	7583725.60	3.55 + R	0 - 1.00		-	-	1.00 - 2.35	2.35 - 3.55 + R	-				-	-
	TP35-WRD	803149.00	7582266.00	4.10 +	0 - 0.90	1.30 - 2.50	2.50 - 4.10+	0.90 - 1.30	-	-	-				-	-
	TP36-WRD	801345.00	7582051.00	1.60 + R	0 - 0.60	-	*0.60 - 1.60 + R			-	-				-	-
	TP37-WRD	801515.00	7582442.00	2.70 + R	0 - 0.90	0.90 - 1.10	-		-	-	-	1.10 - 2.40	-	2.40 - 2.70	2.70 + R	-
	TP38-WRD	802264.00	7583006.00	1.30 + R	0 - 1.00	1.00 - 1.20		-		-		1.20 - 1.30		1.30 + R	-	-
	TP39-WRD	801254.00	7582470.00	1.50 + R	0 - 0.60	-	*0.60 - 1.40	-		-	1.40 - 1.50 + R		-			-
	TP40-WRD	801382.00	7582856.00	2.10 + R	0 - 1.40	1.40 - 1.60	-	-		-		1.60 - 1.90	-		1.90 - 2.10 + R	-
	TP41-WRD	801860.00	7582706.00	1.20 + R	0 - 0.70	0.70 - 1.20 + R		-		-	-		-		-	-

- NOTES:

 1. R. R. REFUSAL.
 2. + = MATERIAL CONTINUES AS ABOVE.
 3. TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.
 4. TP25-WRD WAS NOT EXCAVATED DUE TO ACCESS CONSTRAINTS.



CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF ADDITIONAL MATERIAL SOURCES TEST PIT PROFILES

		AS BUILT COORDINATES												
		UTM	UTM 33K			(m)								
STRUCTURE ID.	TEST PIT No.			TOTAL DEPTH (M)	Trans	ported	Pedogenic			Bedrock		GROUNDWATER LEVEL		
		Ferruginous Alluvium	Calcareous Alluvium	Calcified Alluvium	Very Soft Rock	Soft Rock	(M)							
	AM-TP01	807088.00	7583751.00	0.60 + R	0 - 0.40	-	0.40 -0.60 + R	-	-	-	-	-		
	AM-TP02	806656.00	7583660.00	1.10 + R	0 - 0.70	0.70 - 0.90 + R	-	-	-	0.90 - 1.10	1.10 + R	-		
	AM-TP03	806392.00	7583799.00	1.00 + R	0 - 0.70	0.70 - 1.00 + R	1.00 + R	-	-	-	-	-		
	AM-TP04	806025.00	7583714.00	1.50 + R	-	0.00 - 0.30	-	0.30 - 1.10	1.10 - 1.20 + R	1.20 - 1.50	1.50 + R	-		
	AM-TP05	806080.00	7583500.00	1.20 + R	0 - 1.20	-	-	•	-	-	1	-		
Additional Materials (AM)	AM-TP06	806402.00	7583603.00	1.00 + R	0 - 1.00 + R	-	-	•	-	-	-	-		
Additional Materials (AM)	AM-TP06-B	806390.00	7583549.00	1.00 + R	0 - 1.00 + R	-	-	•	-	-	-	-		
	AM-TP07	806585.00	7583438.00	1.10 + R	0 - 1.10 + R	-	-	-	-	-	-	-		
	AM-TP08	807041.00	7583485.00	0.60 + R	0 - 0.60 + R	-	-	-	-	-	-	-		
	AM-TP08-B	807035.00	7583446.00	0.70 + R	0 - 0.70 + R	-	-	-	-	-	-	-		
	AM-TP09	807324.00	7583323.00	1.10 + R	0 - 0.70	-	0.70 - 1.10 + R	•	-	-	-	-		
	AM-TP10	807340.00	7583639.00	0.80 + R	0 - 0.80 + R	-	-	•	-	-	•	-		

NOTES:

^{2.} TEST PIT COORDINATES INDICATED IN WGS 84 UTM 33S AS BUILT.

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^{1.} R = REFUSAL.

APPENDIX B2

Summary of Boreholes





CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF BOREHOLE LOGS

		AS BUILT CO	OORDINATES							THICKNES	SS OF LAYERS					
		UTN	1 33S	PLANNED	ACTUAL DRULL						(m)					GROUNDWATER LEVEL
STRUCTURE ID.	Borehole No.			DRILL DEPTH (M)	ACTUAL DRILL DEPTH (M)	Transpo	orted Soils	Pedoge	nic Soils	Residual Soil		Bed	rock Gneiss (*with Pegm	atite Zones)		(*PERCHED) (m)
		X-COORD	Y-COORD	()		Fine Alluvium	Coarse Alluvium	Calcareous/*Calcified/** Powdery calcrete Alluvium	Hardpan Calcrete	Gneiss	Very Soft Rock	Soft Rock	Medium Hard Rock	Hard Rock	Very Hard Rock	
	BH01-SWD	801384.00	7581911.00	20.00	24.27	0 - 1.85	-	-			1.85 - 3.0 '5.50 - 8.20	11.25 - 14.85	3.00 - 5.50 '8.2 - 11.25	14.85 - 24.27+	-	7.4
	BH02-SWD	801681.00	7581852.00	20.00	25.11	0 - 1.05	1.05 - 2.08	-	-	2.08 - 2.34 '10.34 - 12.40	-	2.34 - 10.34 '12.40 - 18.20	-	18.20 - 19.35	19.35 - 25.11+	-
Stormwater Diversion	BH03-SWD	801768.00	7581835.00	20.00	20.23	0 - 1.33	-	-	-	-	7.83 - 9.35	1.33 - 2.20	2.2-7.83	9.35 - 12.30	12.30 - 20.23+	-
Channel (SWD) Inlet	BH04-SWD	801678.00	7581770.00	20.00	21.03	0 - 2.20	-	-	-	2.20 - 2.95 '4.52 - 6.60	-	2.95 - 4.52 '6.60 - 10.80	10.80 - 15.50	15.50 - 21.03+	-	14.60
	BH05-SWD	801740.00	7581876.00	20.00	20.01	0 - 3.95	-	-	-	3.95 - 5.35	5.35 -6.30	-	6.30 - 20.01+	-	-	8.05
	BH10-SWD	801538.00	7581868.00	10.00	10.44	0 - 0.80	0.80 - 2.00	-	-	2.00 - 3.30	-	-	3.30 - 10.44+			-
Stormwater Diversion	BH06-SWD	803037.82	7581516.63	15.00	15.26	0 - 3.00	3.00 - 7.60	-	-	-	-	7.60 - 15.26+	-	-	-	-
Channel (SWD) Channel	BH07-SWD	804077.00	7582410.00	15.00	15.22	0 - 1.10	1.10 - 3.00	-	-	-	-	-	3.00 - 15.22+	-	-	-
Stormwater Diversion	BH08-SWD	804591.00	7583229.00	20.00	20.23	0 - 4.60	-	-	-	-	-	4.60 - 7.97	7.97 - 13.70	*'13.70 - 20.23+	-	-
Channel (SWD) Outlet	BH09-SWD	804688.00	7583254.00	20.00	20.23	0 - 4.70	-	-	-	4.70 - 5.17	5.17 - 7.80	7.80 - 11.76	11.76 - 12.80	12.80 - 20.23+	-	-
Waste Rock Dump (WRD)	BH01-WRD	803426.00	7585025.00	25.00	20.28	0 - 0.19	-	**'0.19 - 0.84 '1.50 - 3.54	0.84 - 1.50	3.54 - 4.80 '5.47 - 7.75	4.80 - 7.75	-	7.75 - 14.35	14.35 - 20.28+	-	-
	BH01-Plant	802349.95	7583844.14	25.00	25.05	0 - 1.75	-	*'1.75 - 2.00	-	-	2.00 - 2.5	9.14 - 10.97 '20.67 - 25.05+	2.5 - 9.14 '10.97 - 20.67	-	-	-
Processing Plant	BH02-Acid Plant	801481.43	7583235.41	25.00	25.16	0 - 1.02	-	-	-	1.02 - 1.52 '2.60 - 3.90	1.52 - 2.60	3.90 - 5.25	5.25 - 11.95 '22.92 - 25.16+	11.95 - 22.92	-	-
r rocessing riant	BH03-Crusher	802365.16	7583468.49	25.00	25.00	0 - 1.27	1.27 - 1.78	-	-	-	14.90 - 16.33	1.78 - 6.35	6.35 - 12.60	12.60 - 14.90 '16.33 - 25.00+	-	*5.7
	BH04-Plant	802119.00	7583643.00	10.00	10.09	0 - 1.70	-	-	-	-	-	-	1.70 - 5.10 6.30 - 10.09+	5.10 - 6.30	-	-
	BH01-HLP	801721.00	7585666.00	20.00	20.07	0 - 0.69	-	0.69 - 3.00	3.00 - 4.03	4.03 - 6.18	-	*'6.18 - 18.30	18.30 - 20.07+	-	-	*7.4
Heap Leach Pad	BH02-HLP	801240.00	7584810.00	20.00	20.08	0 - 3.00	-	-	-	3.00 - 3.20	3.20 - 7.90	7.90 - 11.65	11.65 - 20.08+	-	-	-
Heap Leath Fau	BH03-HLP	802101.00	7584148.00	20.00	20.13	0 - 1.30	-	1.30 - 1.95	-	-	-	1.95 - 8.20	8.20 - 20.13+			6.30
	BH04-HLP	801731.00	7583536.00	20.00	20.23	0 - 1.05	-	1.05 - 1.70	-	1.70 - 2.00		2.00 - 4.51	4.51 - 20.23+	-	-	-

- NOTES:

 1. += PROFILE CONTINUES AS ABOVE

 2. ALL DEPTH MEASUREMENTS ARE TAKEN WITH RESPECT TO GROUND SURFACE LEVEL.

 3. BOREHOLE COORDINATES INDICATED IN WGS 84 UTM 33S.

A	5/31/2023	ISSUED WITH LETTER	SY	BK
REV	DATE	DESCRIPTION	PREP'D	REV'D

APPENDIX B3

Summary of SPT Results





APPENDIX B3

CRATON MINING AND EXPLORATION (PTY) LTD OMITIOMIRE COPPER PROJECT

OMITIOMIRE COPPER PROJECT - FEASIBILTY LEVEL GEOTECHNICAL INVESTIGATION SUMMARY OF STANDARD PENETRATION TEST RESULTS

Print Sep/18/23 7:26:52

ВН	DEPTH		BLOV	VS PER 75M	M PENETRA	ATION		N-VALUE	DRILLERS MATERIAL DESCRIPTION
SWD-BH01	1.80	10	13	11	11	8	R	R	Brown fine sand/pebbles
SWD-BH02	1.87	11	11	11	17	50	R	R	Light brown fine sand/pebbles
SWD-BH04	1.95	1	1	2	2	2	3	9	Light brown fine sand
	1.95	1	1	1	1	2	3	7	Light brown sand and clay
SWD-BH05	3.45	4	3	2	2	2	2	8	Light brown sand and clay
	3.65	2	1	1	50	R		R	Clay and pebbles
OMD DITOS	1.95	4	7	7	9	10	11	37	Red sand
SWD-BH06	3.19	8	40	45	50	R		R	Red sand with pebbles
SWD-BH08	1.95	2	3	2	3	3	5	13	Dark brown sand and clay
211D-PU09	3.45	1	2	2	2	3	3	10	Dark brown sand and clay
SWD-BH09	1.95	1	1	1	2	2	3	8	Dark brown sand
2MD-BU08	3.79	1	2	3	3	4	3	13	Dark brown sand
OWD DIAG	1.95	5	10	15	16	21	28	>50	Yellow sand
SWD-BH10	3.15	18	29	50	R			R	Light brown sand and clay
I II D DI 100	0.45	1	1	1	2	2	2	7	Dark brown sand
HLP-BH02	1.95	2	2	1	2	3	4	10	Dark brown sand
I II D DI 100	0.45	1/3	1/3	1/3	1	1	2	4	Red dark brown sand
HLP-BH03	1.65	15	22	50	R			R	Red dark brown sand
I II D DI 104	0.45	1	2	4	5	5	6	20	Dark brown sand
HLP-BH04	1.88	4	5	4	10	20	R	R	Dark brown sand and pebbles
DI ANIT DI IOA	0.45	1	1	2	2	5	5	14	Red sand
PLANT-BH01	1.75	9	12	16	20	50	R	R	Clay and weathered rock
PLANT-BH02	1.52	25	50	R				R	Weathered soft rock
DI ANT DUICO	0.45	1	1	1	2	2	3	8	Red sand
PLANT-BH03	1.78	7	16	16	30	50	R	R	Red sand and pebbles
PLANT-BH04	1.70	12	19	16	50	R		R	Light brown sand and pebbles

NOTES: 1. R = REFUSAL

ſ	A	2022/04/21	ISSUED WITH LETTER	SSYF	BK
ı	REV	DATE	DESCRIPTION	PREP'D	REV'D

Craton Mining and Exploration (Pty) Ltd
Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX B4

Summary Laboratory Test Results





APPENDIX B4.1 GEOTECHNICAL INVESTIGATION FOR HEAP LEACH PAD, WASTE ROCK DUMP, STORMWATER DIVERSION, PLANT & POTENTIAL BORROW PIT SUMMARY OF SOIL LABORATORY TEST RESULTS

	SAM	SAMPLE			GRADING		AT	TERBERG LI	MITS					STANDARI	D PROCTOR	MOD AA		CBR		TRIAXIA	L TEST (CU)	SHEAR BO Romoulded to 9 AASH	90% MDD Mod		В	ASSON INDEXT TES	ST		Coefficient of
STRUCTURE ID.		Material Description Depth	Material Origin		(%)			(%) Pi	PI	GM P	USCS	AASHTO	O SG (g/cm³)	MDD	ОМС		омс	%	% Swell COL	Internal Friction Angl	e Cohesion (kPa		Cohesion (kPa)	Leaching Corrosion	Spalling Corrosion	Overall Agressiveness	Corrosivity Index	Basson Index	Permeability (m/s) at 90% Mod AASHTO / **at 93% Proctor
	Test Pit No.	(m)		Gravel	Sand Silt	Clay		0.425m L m)	S (Whole Sample)					(kg/m³)	(%)	(kg/m³)	(%)	95 98		(φ)		(ф')		(LSCI)	(SCSI)	(N _c)			
	HLP-TP03	FP03 0.8-2.3 Yellowish brown to khaki, silty sandy gravel, calcified coarse alluvium	Alluvium	62	25 11	2	27	12 6.	0 3.0	2.31 Lo	w GC	A-2-6 (0)	2.608		-	1858	13.7 2	27 33 45	0.0 G8	-	-		-	-	-		-		-
	HLP-TP05	TP05 0.0-1.1 Orange to reddish brown, clayey silty sand, fine alluvium	Alluvium	-		-	-		-		-	-	-	-	-	-				-	-	-	-	3201	1	3202	Corrosive	Very High	-
	HLP-TP08	FP08 0.65-1.35 Brown, clayey sandy silt. Fine Alluvium	Alluvium	2	38 46	14	56	22 10	.0 17.0	0.68 Med	um MH	A-7-5 (12	2.492	•	-	1517	23.3 <	3 <3 <3	0.1 NC	-	-			-	-	-			9.26E-09
	HLP-TP24	FP24 1.2-1.75 Brown, sandy silty gravel. Coarse Alluvium.	Alluvium	50	41 5	4	-	1 0.	0.0	2.20 Lo	w GP-GM	A-1-a (0)	0) 2.564	-	-	2138	6.5 2	25 45 107	0.1 G4	-			-	-	-	•	-		2.19E-06
Hoop Londy Bod	HLP-TP26		Alluvium	-		-	-		-		-	-	-	-	-	-				-	-	32	9	-	-		-		-
Heap Leach Pad (HLP)	HLP-TP13	FP13 0.3-2.25 Light brown to brown, gravelly silty fine sand. Calcareous Alluvium.	C Alluvium	0	73 17	10	-	1 0.	0 1.0	1.00 Lo	w SM	A-2-4 (0)) 2.620	-	-	2054	7.3 2	28 38 60	0.0 G6	-	-		-	3305	5	3310	Corrosive	Very High	-
	HLP-TP24	FP24 0.8-1.2 Brown, gravelly sandy silt. Calcareous Alluvium.	C Alluvium	24	57 11	8	-	1 0.	0 1.0	1.54 Lo	w SM	A-2-4 (0)) 2.609	2085	6.7	-	-			-			-	-	•	•	-		**1.51E-06
	HLP-TP20	TP20 1.6-1.8 Completely weathered, white speckled to streaked black stained khaki to orange, very soft rock, gneiss	Gneiss	23	66 7	4	28	8 2	5 2.0	2.04 Lo	w SW-SC	A-1-b (0)	0) 2.566	-	-	-				-	-		-	-			-		-
	HLP-TP04		Nodular Calcrete	60	24 11	5	28	11 4.	0 3.0	2.22 Lo		A-2-6 (1)	2.617	-	-	1994	10.7 1	11 16 29	0.0 G8	-	-		-	-	-	•	-		1.50E-08
	HL/PA-TP29	TP29 1.7-1.9 Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker	Pebble marker		36 7	7	19	6 2		2.12 Lo) 2.619	•	-	2204		29 40 64	0.0 G6	-			-	-	•	•	-		7.78E-06
	HLP-TP03		Residual Gneiss		40 10	1	25			2.19 Lo	_				-	1974	11 1	6 21 33	0.1 G7	-	-	37	6	-	-	-	-	-	3.92E-07
	Plant-TP32		Alluvium	0	71 21	8	-			0.97 Lo		A-2-4 (0)			-	-	-			-	-	-	-	-	-	-	-	-	-
Plant	Plant-TP32		Alluvium	-	55 19	26		6 2		0.82 Lo					-	-	-			-	-	-	-	-	-		-	-	-
	Plant-TP32		F Alluvium	-	53 17	30	25	7 2.		0.84 Lc		A-4 (3)	2.538	-	-	-				-	-	-	-	-	•	•	-	-	
	RD-TP02		Alluvium		60 6	7	28			2.08 Lo	w SC	A-2-6 (0)		-	-	-	- '			-	-	-	-	-	-	•	-	•	-
	RD-TP04		Alluvium	0	66 19	15	-			0.83 Lo		A-2-4 (0)	0) 2.637	-	-	2041		32 46 81	0.1 G4		-	-	-	-			-		-
	RD-TP12		Alluvium	0	70 15	15	-	1 0		0.96 Lo		A-2-4 (0)	,	-	-	2119		23 30 46	-0.1 G6		-	-	-	-	-	•	-	-	-
New Road (RD)	RD-TP08	P08 1.0-3.2 White to yellowish brown, silty gravelly sand, calcified alluvium	C Alluvium	8	46 34	12	48	20 7.	5 14.0	0.95 Med	um SM	A-7-5 (5)	5) 2.411	-	-	1640	20 4	4 4 5	0.0 NC		-	-	-	-			-		-
	RD-TP02	P02 3.9-4.3 Completely weathered, white streaked black and gold, very soft rock, biotite gneiss	Gneiss	31	53 11	5	29	7 2	5 2.0	1.96 Lo	w SC	A-2-4 (0)	0) 2.673	-	-	1940	9.2	8 11 20	0.0 G9	-	-	-	-	-	-	-	-	-	-
	RD-TP14		Gneiss	39	36 20	5	34	9 3	5 4.0	1.83 Lo	w GM	A-2-4 (0)	0) 2.604	-	-	2016	12.9 1	15 17 20	0.0 G7	-		-		-		-	-	-	-
	RD-TP13	Yellowish brown with scattered red, white and black mottles, clayey gravelly silty fine sand with minor gravels, slightly calcified pebble marker	Pebble marker	5	57 33	5	27	9 4	5 7.0	0.95 Lo	w SC	A-4 (1)	2.669	-	-	-	-			-	-	-	-	-	-		-		-
	SWD-TP02		Alluvium	0	68 20	12	-	3 1.	0 2.0	0.95 Lo	w SM	A-2-4 (0)	2.586	-	-	-	-			-	-	-	-	-			-		-
	SWD-TP15	TP15 0.5-2.5 Orange brown stained red, clayey silty fine gravelly sand, fine alluvium	Alluvium	0	60 20	20	20	8 4	0 6.0	0.88 Lo	w SC	A-4 (0)	2.628	-	-	2055	9.7 1	11 11 12	0.0 G8	-	-	33	14	-	-		-		3.57E-07
	SWD-TP23	TP23 1.2-2.5 Dark reddish brown, sandy silty clay. Fine Alluvium	Alluvium	1	46 23	30	23	8 2	5 7.0	0.63 Lo	w SC	A-4 (4)	2.654	-	-	-	-			P	ending	-	-	-	-		-		-
Stormwater Diversion	SWD-TP17	TP17 1.3-2.2 Olive brown mottled white stained orange, gravelly silty fine sand. Calcified Alluvium.	C Alluvium	19	58 17	6	38	16 5	5 7.0	1.61 Lo	w SC	A-2-6 (1)	2.615	-	-	-	-			-	-	-	-	-		-	-	-	-
Channel (SWD)	SWD-TP08	TP08 1.0-1.4 Dark reddish brown speckled white, silty sandy gravel. Ferruginized Alluvium.	F Alluvium	16	65 15	4	31	15 5	0 6.0	1.72 Lo	w SC	A-2-6 (1)	1) 2.557	-	-	2053	7.3 <	<3 <3 4	0.3 NO		-	-	-	-			-		4.59E-07
	SWD-TP19	TP19 1.4-2.4 Yellowish brown mottled black stained red, clayey silty gravelly sand with trace gravels, ferruginised alluvium	F Alluvium	1	53 18	28	25	8 2	0 7.0	0.72 Lo	w SC	A-4 (1)	2.645	-	-	-	-			-	-	-	-	1040	5	1046	Corrosive	High to Very High	-
	SWD-TP07	TP07 1.4-1.9 Completely weathered, green speckled to streaked black, white and gold, very soft rock, biotite gneiss	Gneiss	25	55 11	9	32	12 3.	5 5.0	1.77 Lo	w SC	A-2-6 (1)) 2.736	-	-	2174	7.2 1	2 19 40	0.0 G8	-	-	-	-	1068	3	1071	Corrosive	High to Very High	-
	SWD-TP10	TP10 1.2-2.10 Reddish brown mottled white, silty sandy gravel. Nodular Ferricrete.	Nodular Ferricrete	41	27 21	11	29	12 6.	0 6.0	1.62 Lo	w GC	A-2-6 (1)) 2.629	-	-	1875	14.9	3 4 6	0.0 NC				-	•	•	•	-		-
	WRD-TP01	TP01 1.4-3.5 Dark reddish brown, silty sandy clay. Fine Alluvium.	Alluvium	0	60 17	23	23	9 2	5 7.0	0.81 Lo	w SC	A-4 (1)	2.624	-	-	2035	9.3 <	3 <3 5	0.0 NC	P	ending		-	-			-		-
	WRD-TP04	TP04 1.5-2.4 Orange brown, gravelly silty sand. Alluvium.	Alluvium	4	53 28	15	29	12 2.	5 9.0	0.84 Lo	w SC	A-6 (3)	2.641		-	1963	12 5	5 5 7	0.1 NC	-	-	-	-	-	-	-	-	-	
	WRD-TP14	TP14 2.8-4.9 Pale yellow brown, silty sandy gravel and cobbles. Coarse Alluvium.	Alluvium	37	45 11	7	29	11 4.	0 4.0	1.98 Lo	w SC	A-2-6 (0)	2.616		-	2071	9.5	7 8 8	0.0 G9	-	-	-	-	-	-	-	-	-	7.44E-07
	WRD-TP15	TP15 0.0-0.7 Brown, sandy clayey silt. Fine Alluvium.	Alluvium	3	35 27	35	33	16 7.	0 14.0	0.57 Lo	w CL	A-6 (7)	2.619	1628	14.0	-				-	-	-	-	-			-	-	-
	WRD-TP04	TP04 0.4-1.5 Pale brown mottled white, gravelly sandy silt. Calcified Alluvium.	C Alluvium	2	70 24	4	25	6 2	5 5.0	1.04 Lo	w SC-SM	A-2-4 (0)	0) 2.502		-	1975	11 <	3 3 6	0.1 NC	-	-	29	13	-	-		-	-	1.19E-07
	WRD-TP26	TP26 1.2-2.6 White speckled to mottled black and brown, silty sandy gravel, hardpan calcrete	Calcrete	58	22 9	11	31	9 3.	0 2.0	2.24 Lo	w GC	A-2-4 (0)	2.588		-	-				-	-	-	-	-	-		-	-	-
	WRD-TP35	TP35 2.5-4.1 Yellow brown, sandy silt. Ferruginous Alluvium.	F Alluvium	35	6 21	20	32	13 6.	0 10.0	0.92 Lo	_	A-6 (2)	2.647		-	1946	11.2 <	3 <3 3	0.1 NO	-	-	28	18				-	-	-
Waste Rock	WRD-TP39	TP39 1.4-1.5 Khaki stained brown to black, silty gravelly sand to silty sandy gravel, ferruginised alluvium	F Alluvium	22	61 13	4	36	10 3.	5 4.0	1.79 Lo	w SM	A-2-4 (0)	2.596		-	-				-	-	-	-	-	-		-	-	<u> </u>
Dump (WRD)	WRD-TP37		Gneiss	39	49 11	1	23	8 4.	0 2.0	2.05 Lo	w SC	A-2-4 (0)) 2.655	•	-	2010	10.8 1	11 14 21	0.1 G8	-	-	-	-	-	-	-	-	-	
	WRD-TP32	TP32 1.15-1.45 Pale brown blotched white, sandy silt with abundant calcrete nodules and cobbles. Honeycomb Calcrete.	Honeycomb Calcrete	56	27 12	5	24	8 3.	0 2.0	2.15 Lo	w GC	A-2-4 (0)	0) 2.602		-	-				-	-	-	-	-	-	•	-		-
	WRD-TP16	TP16 0.5-1.8 Pale brown speckled white, sandly silty gravel. Nodular Calcrete.	Nodular Calcrete	74	16 8	2	30	11 4.	0 2.0	2.47 Lo	w GP-GC	A-2-6 (0)) 2.614		-	1869	14.5 1	17 19 24	0.2 G7	-	-	-	-	•	-	•	-		-
	WRD-TP37	TP37 0.9-1.1 Yellowish brown mottled white and black, silty sandy gravel, calcified pebble marker	Pebble marker	10	48 32	10	37	13 6.	5 8.0	1.14 Lo	w SC	A-4 (2)	2.518		-	-	-			-	-	-	-	-	-	-	-	-	-
	WRD-TP02	TP02 1.4-1.6 White to grey mottled black stained orange, silty gravelly sand, calcified reworked residual gneiss	Residual Gneiss	58	37 2	3	-	1 0	0.0	2.49 Lo	w GP-GM	A-1-a (0)	2.635	-	-	-	-			-	-	-	-	-	-	-	-	-	-
	WRD-TP33	TP33 1.2-1.3 Dark brown mottled orange and black, clayey silty coarse sand, slightly ferruginised reworked residual biotite gneiss	Residual Gneiss	0	66 13	21	22	8 3	0 6.0	1.00 Lo	w SC	A-2-4 (0)	2.634	-		-	-	- - -		-	<u> </u>	-	-	-	-	-	-	-	-
	WRD-TP37	TP37 1.1-2.4 Yellowish brown mottled black and white, gravelly silty sand, reworked residual gneiss	Residual Gneiss	8	68 21	3	24	6 3	0 4.0	1.31 Lo	w SC-SM	A-2-4 (0)	2.648		-	-	-			-		-	-	-	-		-	-	-
	WRD-TP23	TP23 0.85-1.50 Pale brown blotched white, hardpan calcrete.	Hardpan Calcrete	63	29 6	2	35	9 4	0 2.0	2.41 Lo	w GP-GM	A-2-4 (0)	2.543	-	-	1651	19.9 3	36 42 54	0.0 G6	-	-	-	-	-	-	-	-	-	-

NOTES

1.1.L. LOUID LIMIT; 2. PI. PLASTICITY INDEX; 3. LS - LINEAR SHRINKAGE; 4. GM - GRADING MODULUS 5, PE - POTENTIAL EXPANSIVENESS; 6. USCS - UNIFIED SOIL CLASSIFICATION SYSTEM 7. MID - MAXIMUM DRY DENSITY 8. OMC - OPTIMUM MOISTURE CONTENT; 9. CBR - CALIFORNIA BEARING RATIO; 10. KPA - KILOPASCAL; 11. AASHTO - AMERICAN ASSOCIATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NC - NC CLASSIFICATION STATE HIGHWAY TRANSPORT OFFIC HIGHW

REV DATE DESCRIPTION PREPD REVO		Α	8/1/2023	ISSUED WITH LETTER	SY	BK
·		REV	DATE	DESCRIPTION	PREPD	REVD
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APPENDIX B4.2 GEOTECHNICAL INVESTIGATION FOR HEAP LEACH PAD, WASTE ROCK DUMP, STORMWATER DIVERSION, PLANT & POTENTIAL BORROW PIT SUMMARY OF SOIL LABORATORY TEST RESULTS

Test PI No. Core Send	SC SC-SM SC SC MH - SM SM	A-4 (0) A-4 (0) A-4 (0) A-4 (0) -	STANDARE STANDARE	OMC (%) (%) (%) (%) (%) (%) (%) (%) (%) (%)	MOD AASHTO COMPACTION MDD OMC (%)	93 95 5 9 32 46	98 Swell 98	- - - - - -	TRIAXIAL TI	Cohesion (kPa)	Angle (\$\phi\$) (Leaching Corrosion (LSCI)	Spalling	Overall Agressiveness (N,)	Corrosivity Index		Coefficient of Permeability (Mod AASHTO / **at 90% (Mod AASHTO / **at 93% Proctor
Test PINO Test	SC SC-SM SC SC-SM SC	A-4 (0) A-4 (0) A-4 (0) A-4 (0) A-7-5 (12) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (0) A-4 (0) A-4 (4)	MDD (kg/m²) 2.528	OMC (%) (%) (%	MDD OMC kg/m²) (%)	93 95 5 9 5 9 32 46	98 Swell 98	- G9 - NC	Friction	(kPa)	Friction Angle (\$\phi\$)	hesion (kPa) Corrosion (LSCI)	Corrosion (SCSI)	Agressiveness (N _c)	Index	Basson Index	at 90% Mod AASHTO / "at 93% Proctor - - - 1.29E-07
Test PN No. Crow Set Crow C	SC SC-SM SC	A-4 (0) A-4 (0) A-4 (0) A-4 (0) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	(kg/m²) 2.528 - 2.697 - 2.741 - 2.552 - 2.620 - 2.492 - 2.609 - 2.637 - 2.648 - 2.586 - 2.528 -	(%) (k	kg/m³) (%)	93 95 5 9 32 46	98		Friction	(kPa)	Angle (\$\phi\$) ((kPa) Corrosion (LSCI)	(SCSI)	- (N _c)	Index	-	
AM-TPDI 0.4 6.8 Brown stained crange brown motited black, clayey sity sand, slightly forruginised fine alluvium AM-TPDI 0.0-6.7 Reddsith brown, clayey sity sand with traces of subarquiter to subrounded fine gravel, fine alluvium Fine Alluvium 1 S3 22 M4 48 29 9 3 3 7.0 0.74 Low 10 MA-TPDI 0.0-6.7 Reddsith brown, clayey sity sand fine alluvium Fine Alluvium 5 Fine Alluvium 5 Fine Alluvium 6 S5 Low 1 Low 2 S5 Low 1 S5 Low 1 Low 2 S5 Low 2 Low 2 S5 Low 2 S5 Low 2 S5 Low 2 S5 Low 2 Low 2 Low 2 S5 Low 2 Low 2 Low 2 Low 2 S5 Low 2 Low 2 Low 2 Low 2 S5 Low 2 Low 2 Low 2 Low 2 S5 Low 2 Low 2 Low 2 Low 2 S5 Low 2 S5 Low 2	SC SC-SM SC	A-4 (0) A-4 (0) A-4 (0) A-4 (0) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.528		2079 9.2 	5 9		- NC	-					-	Corrosive		
AM-TPDS 0.0-0.0 Reddish brown, clayey sity sand with traces of subarquist to subrounded tine gravel, fine alluvium Fine Alluvium 0 59 22 19 41 19 5 3 7.0 0.74 Low 1.0 AM-TPDS 0.0-0.0 S Brown, clayey sity sand, fine alluvium Fine Alluvium 0 59 22 19 41 19 5 3 5.0 0.67 Low 50 1.0 AM-TPDS 0.0-0.0 S Brown, clayey sity sand, fine alluvium Fine Alluvium 0 59 22 19 24 40 22 7 3 5 6.0 0.0 C Low 50 1.0 AM-TPDS 0.0-0.1 Date brown with traces of subarquist to subrounded gravel to fine gravel, fine alluvium Fine Alluvium 0 52 19 22 45 22 45 22 7 3 5 6.0 0.0 C Low 50 1.0 AM-TPDS 0.0-0.1 Date brown with traces of subarquist to subrounded gravel to fine gravel, fine alluvium Fine Alluvium 0 52 19 29 46 25 9 4 6 50 0.0 C Low 50 1.0 Date brown with traces of subarquist to subrounded gravel to fine gravel, fine alluvium Fine Alluvium 0 52 19 29 46 25 9 4 6 50 0.0 C Low 50 1.0 Date brown brown, sayly sity sand, fine alluvium Fine Alluvium 0 52 19 29 46 25 9 4 6 50 0.0 Low 50 1.0 Date brown to brown, sayly sity sand, fine alluvium Fine Alluvium 0 7 0 10 10 10 10 10 10 10 0.0 Low 50 1	SC SC-SM SC	A-4 (0) A-4 (0) A-4 (0) A-4 (0) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.697					- NC	-	-				-	Corrosive	- Very High	
AM-TPOS 0.0-0.6 Brown. clayey silly sand, fine alluvium Fine Alluvium 0 59 22 19 41 19 5 3 5.0 0.67 Low St. AM-TPOS 0.4-1.0 Rodshib brown, clayey silly sand, fine alluvium Fine Alluvium 0 57 21 22 43 22 7 3 3 6.0 0.75 Low 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	SC-SM SC SC - MH - SM SM SM SM SM SC SC SC SC SC	A-4 (0) A-4 (0) A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (0) A-4 (0) A-4 (0)	2.741					- NC	-	-					Corrosive	Very High	
AM-TPGS	SC SC - MH - SM SM SM SM SC SC SC SC	A-4 (0) A-4 (0) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.652					- NC		-			- - 1	-	Corrosive	- Very High	
AM-TP08-B 0.5-0.7 Dark brown with orange staining, clayey sity sand, fine alluvium Fine Alluviu	SC - MH - SM SM SM SM SC SC SC SC	A-4 (0) - A-7-5 (12) - A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.620					- NC	-	-	32		1 -	3202	- Corrosive	- Very High	
HLP-TPOS 0.0-1.1 Orange to reddish brown, clayey sifty sand, fine alluvium Fine Alluvium 2 38 46 14 60 56 22 10.0 17.0 0.68 Medium 1 HLP-TPOS 0.0-1.1 Pale brown to brown, sandy silt. Fine Alluvium Fine Alluvium 2 38 46 14 60 56 22 10.0 17.0 0.68 Medium 1 HLP-TPOS 0.0-1.1 Pale brown to brown, sandy silt. Fine Alluvium Fine Alluvium 0 71 21 8 29 - 1 0.0 1.0 0.97 Low 1 End Alluvium 0 71 21 8 29 - 1 0.0 1.0 0.97 Low 1 End Alluvium 1 End Alluvium 0 66 19 15 34 - 1 0.0 1.0 0.97 Low 1 End Alluvium 1 End Alluvium 1 End Alluvium 0 66 19 15 34 - 1 0.0 1.0 0.95 Low 1 End Alluvium 1 End Alluviu	MH SM SM SM SM SC SC SC SC	A-7-5 (12) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.492	- - - - :	2041 8.1	32 46	81 0.1		-	-	32		1 -	3202	Corrosive	Very High	-
HLP-TP08	SM SM SM SM SC SC SC SC SC	A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.609 - 2.637 - 2.648 - 2.586 - 2.628 -	- - - - :	2041 8.1	32 46	81 0.1		-	-	32		-	-	-	- very riigii	0.00= 00
HLP-TP26	SM SM SM SM SC SC SC SC SC	A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.609 - 2.637 - 2.648 - 2.586 - 2.628 -	- - - - :	2041 8.1	32 46	81 0.1		-		32	9 -			_		
Plant-TP32 0.0-0.7 Brown to orange brown, clayey silty coarse sand, fine alluvium Fine Alluvium 0 71 21 8 29 . 1 0.0 1.0 0.97 Low 1.0 1.0 0.97 Low 1.0 1.0 0.97 Low 1.0 1.0 0.97 Low 1.0 1.0 0.98 Low 1.0 0.0 0.98 Low 0.98 Low 1.0 0.0 0.0 0.98	SM SM SM SC SC SC SC	A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.637 - 2.648 - 2.586 - 2.628 -	- :				- G4	-	-	-				1 -		3.202-03
RD-TP04 0.6-2.9 Reddish brown, clayey silty carse sand with traces of gravel, fine alluvium Fine Alluvium 0 66 19 15 34 - 1 0.0 1.0 0.83 Low 1.0 RD-TP12 1.1-4.3 Reddish brown slightly stained brown, clayey silty gravely sand with minor fine gravel, fine alluvium Fine Alluvium 0 70 15 15 30 - 1 0.0 1.0 0.96 Low 1.0 SWD-TP02 0.0-0.7 Brown motted black , gravely silty sand, fine alluvium Fine Alluvium 0 68 20 12 32 - 3 1.0 2.0 0.95 Low 1.0 SWD-TP15 0.5-2.5 Orange brown stained red, clayey silty fine gravelly sand, fine alluvium Fine Alluvium 0 60 20 20 40 20 8 4.0 6.0 0.88 Low 1.0 SWD-TP03 1.2-2.5 Dark reddish brown, sandy silty clay. Fine Alluvium Fine Alluvium 1 46 23 30 53 23 8 2.5 7.0 0.63 Low 1.0 WRD-TP01 1.4-3.5 Dark reddish brown, silty sandy clay. Fine Alluvium Fine Alluvium 4 53 28 15 43 29 12 2.5 9.0 0.84 Low 1.0 WRD-TP04 1.5-2.4 Orange brown, gravelly silty sand, Alluvium. Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low WRD-TP05 0.0-0.7 Brown, sandy clayey silt. Fine Alluvium. Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low 1.0 WRD-TP04 1.5-2.4 Orange brown to khaki, silty sandy gravel, calcified coarse alluvium Coarse Alluvium 62 2.5 11 2 13 2.7 12 6.0 3.0 2.31 Low 1.0 WRD-TP04 1.5-2.8 Brown to gravely silty sand to coarse sand with gravels, coarse alluvium Coarse Alluvium 7 60 6 7 13 28 11 3.5 2.0	SM SM SM SC SC SC SC	A-2-4 (0) A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.637 - 2.648 - 2.586 - 2.628 -	- :				G4				- 1 -	_			 	
RD-TP12 1.1-4.3 Reddish brown slightly stained brown, clayey silty gravelly sand with minor fine gravel, fine alluvium Fine Alluvium 0 70 15 15 30	SM SM SC SC SC SC	A-2-4 (0) A-2-4 (0) A-4 (0) A-4 (4)	2.648 - 2.586 - 2.628 -	- :												 	
SWD-TP02 0.0-0.7 Brown motited black, gravelly silty sand, fine alluvium Fine Alluvium 0 68 20 12 32 - 3 1.0 2.0 0.95 Low 15 SWD-TP15 0.5-2.5 Orange brown stained red, clayey silty fine gravelly sand, fine alluvium Fine Alluvium 0 60 20 20 40 20 8 4.0 6.0 0.88 Low 15 SWD-TP23 1.2-2.5 Dark reddish brown, sandy silty clay. Fine Alluvium Fine Alluvium 1 46 23 30 53 23 8 2.5 7.0 0.83 Low 16 SWD-TP01 1.4-3.5 Dark reddish brown, silty sandy clay. Fine Alluvium Fine Alluvium 0 60 17 23 40 23 9 2.5 7.0 0.81 Low 17 SWD-TP04 1.5-2.4 Orange brown, gravelly silty sand. Alluvium. Fine Alluvium 4 53 28 15 43 29 12 2.5 9.0 0.44 Low 18 SWD-TP05 0.0-0.7 Brown, sandy clayey silt. Fine Alluvium Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low 18 SWD-TP05 0.8-2.3 Yelkowish brown to Alwaki, silty sandy gravel, calcified coarse alluvium Coarse Alluvium 50 41 5 4 9 1 0.0 0.0 0.0 2.20 Low 18 SWD-TP04 1.2-1.75 Brown, sandy silty gravel coarse alluvium Coarse Alluvium 50 41 5 4 9 1 0.0 0.0 0.0 2.20 Low 18 SWD-TP05 1.2-28 Brown to greyish brown, clayey silty sand to coarse sand with gravels, coarse alluvium Coarse Alluvium 27 60 6 7 13 28 11 3.5 2.0 2.08 Low WRD-TP14 2.8-4.9 Pale yellow brown, silty sandy gravel with cobbles, ferruginised / calcified pebble marker Pebble marker 49 36 7 8 15 23 8 3.5 2.0 2.23 Low 18 Low 19 SWD-TP05 1.7-1.9 Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker Pebble marker 50 36 7 7 14 19 6 2.0 2.0 2.0 2.12 Low 66	SM SC SC SC SC	A-2-4 (0) A-4 (0) A-4 (4)	2.586 - 2.628 -	- :	-		46 -0.1		-	-						-	
SWD-TP15 0.5-2.5 Orange brown stained red, clayey silty fine gravelly sand, fine alluvium Fine	SC SC SC	A-4 (4)		- :	J.			-	-	-							
SWD-TP23 1.2-2.5 Dark reddish brown, sandy silty clay. Fine Alluvium	SC SC SC	A-4 (4)			2055 9.7	11 11	12 0.0	G8		-	37	6 -					3.57E-07
WRD-TP01 1.4-3.5 Dark reddish brown, silty sandy clay. Fine Alluvium. Fine Alluvium 0 60 17 23 40 23 9 2.5 7.0 0.81 Low WRD-TP04 1.5-2.4 Orange brown, gravelly silty sand. Alluvium. Fine Alluvium 4 53 28 15 43 29 12 2.5 9.0 0.84 Low WRD-TP15 0.0-0.7 Brown, sandy clayey silt. Fine Alluvium. Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low WRD-TP15 0.8-23 Yellowish brown to khaki, silty sandy gravel, caicified coarse alluvium Coarse Alluvium 62 25 11 2 13 27 12 6.0 3.0 2.31 Low WRD-TP14 1.2-1.75 Brown, sandy silty gravel. Coarse Alluvium Coarse Alluvium 50 41 5 4 9 - 1 0.0 0.0 0.2 2.0 Low Grave Alluvium Coarse Alluvium 0 55 19 26 45 21 6 2.0 5.0 0.82 Low Grave Alluvium Coarse Alluvium 2.7 2.8-4.9 Pale yellow brown, silty sandy gravel and cobbles to boulders, coarse alluvium Coarse Alluvium 37 45 11 7 18 29 11 4.0 4.0 1.98 Low WRD-TP14 2.8-4.9 Pale yellow brown, sally sandy gravel and cobbles. Coarse Alluvium Coarse Alluvium 37 45 11 7 18 29 11 4.0 4.0 1.98 Low 4.0	SC	A 4 (4)		-				-	Pendi	ng	-		-			-	
WRD-TP04 1.5-24 Orange brown, gravelly silty and. Alluvium. Fine Alluvium 4 53 28 15 43 29 12 2.5 9.0 0.84 Low MRD-TP15 0.0-0.7 Brown, sandy clayey silt. Fine Alluvium. Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low MRD-TP15 0.8-23 Yellowish brown to khaki, silty sandy gravel, calcified coarse alluvium Coarse Alluvium 62 25 11 2 13 27 12 6.0 3.0 2.31 Low MRD-TP24 1.2-1.75 Brown, sandy silty gravel. Coarse Alluvium. Coarse Alluvium 50 41 5 4 9 . 1 0.0 0.0 0.2 2.0 Low Gravel Coarse Alluvium Coarse Alluvium 0 55 19 26 45 21 6 2.0 5.0 0.82 Low Standard Coarse Coarse Alluvium Coarse Alluvium 27 60 6 7 13 28 111 3.5 2.0 2.0 Low Standard Coarse Coarse Alluvium Coarse Alluvium 37 45 11 7 18 29 11 4.0	SC	A-4 (1)	2.624 -	- :	2035 9.3	<3 <3	5 0.0	NC	Pendi		-			-		-	-
WRD-TP15 0.0-0.7 Brown, sandy clayey sit. Fine Alluvium. Fine Alluvium 3 35 27 35 62 33 16 7.0 14.0 0.57 Low	CL	A-6 (3)	2.641 -		1963 12		7 0.1	NC	-	-	-			-	-	-	
HLP-TP03 0.8-2.3 Yellowish brown to khaki, silty sandy gravel, calcified coarse alluvium Coarse Alluvium 62 25 11 2 13 27 12 6.0 3.0 2.31 Low 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		A-6 (7)	2.619 1628	14.0				-	-	-	-		-	-	-	-	
HLP-TP24 1.2-1.75 Brown, sandy sitty gravel. Coarse Alluvium. Coarse Alluvium 50 41 5 4 9 . 1 0.0 0.0 2.20 Low Gif	GC	A-2-6 (0)	2.608 -	-	1858 13.7	27 33	45 0.0	G8	-	-	-		-	-	-	-	-
RD-TP02 1.2-2.8 Brown to greyish brown, silty clayey sand with minor cobbles to boulders, coarse alluvium Coarse Alluvium 27 60 6 7 13 28 11 3.5 2.0 2.08 Low	GP-GM	A-1-a (0)	2.564 -	- :	2138 6.5	25 45	107 0.1	G4	-	-	-		-		-	-	2.19E-06
WRD-TP14 2.8-4.9 Pale yellow brown, silty sandy gravel and cobbles. Coarse Alluvium. Coarse Alluvium 37 45 11 7 18 29 11 4.0 4.0 1.98 Low AM-TP03 0.7-1.0 Reddish brown, clayey silty sandy gravel with cobbles, ferruginised / calcified pebble marker Pebble marker 49 36 7 8 15 23 8 3.5 2.0 2.23 Low 10 HUPA-TP29 1.7-1.9 Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker Pebble marker 50 36 7 7 14 19 6 2.0 2.0 2.12 Low 60	SC-SM	A-4 (2)	2.515 -	-					-	-	-		-	-		-	
AM-TP03 0.7-1.0 Reddish brown, clayey silty sandy gravel with cobbles, ferruginised / calcified pebble marker	SC	A-2-6 (0)	2.609 -	-				-	-	-	-	-	-		-	-	-
HLPA-TP29 1.7-1.9 Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker Pebble marker 50 36 7 7 14 19 6 2.0 2.0 2.12 Low GC	SC	A-2-6 (0)	2.616 -	- :	2071 9.5	7 8	8 0.0	G9	-	-	-		-	-			7.44E-07
Toda name Toda n	GC	A-2-4 (0)	2.658 -	-					-	-	-		-	-		-	
	GC-GM	A-1-a (0)	2.619 -	- :	2204 6.3	29 40	64 0.0	G6	-	-	-		-	-	-	-	7.78E-06
RD-TP13 2.6-4.0 Vellowish brown with scattered red, white and black mottles, clayey gravelly sitly fine sand with minor gravels, slightly cacdified pebble marker 5 5 7 33 5 38 27 9 4.5 7.0 0.95 Low slightly cacdified pebble marker	SC	A-4 (1)	2.669 -	-					-	-	-		-	-	-	-	
	SC	A-4 (2)	2.518 -	-					-	-	-		-	-	-	-	
HLP-TP13 0.3-2.25 Light brown to brown, gravelly silty fine sand. Calcareous Alluvium. Calcareous Alluvium 0 73 17 10 27 - 1 0.0 1.0 1.00 Low	SM	A-2-4 (0)	2.620 -	- :	2054 7.3	28 38	60 0.0	G6	-	-	-	- 3305	5	3310	Corrosive	Very High	
HLP-TP24 0.8-1.2 Brown, gravelly sandy silt. Calcareous Alluvium. Calcareous Alluvium 24 57 11 8 19 - 1 0.0 1.0 1.54 Low	SM	A-2-4 (0)	2.609 2085	6.7					-	-	-		-	-	-		**1.51E-06
AM-TP04 0.3-1.1 Reddish brown, clayey silty sandy gravel, calcareous alluvium Calcareous Alluvium 75 21 2 2 4 - 1 0 0.0 2.57 Low	GP	A-1-a (0)	2.624 -	- :	2149 6.5	92 105	129 0.0	G4	-	-	-		-	-	-	-	1.01E-06
RD-TP08 1.0-3.2 White to yellowish brown, silty gravelly sand, calcified alluvium	SM	A-7-5 (5)	2.411 -	-	1640 20	4 4	5 0.0	NC	-	-	-	-	-	-	-	-	-
SWD-TP17 1.3-2.2 Olive brown mottled white stained orange, gravelly sitly fine sand. Calcified Alluvium. Calcified Alluvium 19 58 17 6 23 38 16 5.5 7.0 1.61 Low	SC	A-2-6 (1)	2.615 -	-				-	-	-	-	-	-		-	-	-
WRD-TP04 0.4-1.5 Pale brown mottled white, gravelly sandy sit. Calcified Alluvium. Calcified Alluvium 2 70 24 4 28 25 6 2.5 5.0 1.04 Low S0	SC-SM	A-2-4 (0)	2.502 -	-	1975 11	<3 3	6 0.1	NC	-	-	33	14 -	-	-	-	-	1.19E-07
HLP-TP04 1.1-1.35 Light brown blotched wihite, sandy silty gravel. Nodular Calcrete. Nodular Calcrete 60 24 11 5 16 28 11 4.0 3.0 2.22 Low	GC	A-2-6 (1)	2.617 -	-	1994 10.7	11 16	29 0.0	G8	-	-	-		-	-	-	-	1.50E-08
WRD-TP16 0.5-1.8 Pale brown speckled white, sandly silty gravel. Nodular Calcrete. Nodular Calcrete 74 16 8 2 10 30 11 4.0 2.0 2.47 Low Gi	GP-GC	A-2-6 (0)	2.614 -	-	1869 14.5	17 19	24 0.2	G7	-	-	-		-	-	-	-	-
SWD-TP10 1.2-2.10 Reddish brown mottled white, sity sandy gravel. Nodular Ferricrete. Nodular Ferricrete 41 27 21 11 32 29 12 6.0 6.0 1.62 Low	GC	A-2-6 (1)	2.629 -	-	1875 14.9	3 4	6 0.0	NC	-	-	-		-	-	-	-	-
WRD-TP32 1.15-1.45 Pale brown blotched white, sandy silt with abundant calcrete nodules and cobbles. Honeycomb Calcrete. Honeycomb Calcrete 56 27 12 5 17 24 8 3.0 2.0 2.15 Low	GC	A-2-4 (0)	2.602 -	-				-	-	-	-		-	-	-	-	-
WRD-TP23 0.85-1.50 Pale brown blotched white, hardpan calcrete. Hardpan Calcrete 63 29 6 2 8 35 9 4.0 2.0 2.41 Low Gif	GP-GM	A-2-4 (0)	2.543 -	-	1651 19.9	36 42	54 0.0	G6	-	-	-		-	-	-	-	
WRD-TP26 1.2-2.6 White speckled to mottled black and brown, sitty sandy gravel, hardpan calcrete Hardpan Calcrete 58 22 9 11 20 31 9 3.0 2.0 2.24 Low	GC	A-2-4 (0)	2.588 -	-					-	-	-		-	-	-	-	
	SC	A-4 (3)	2.538 -	-				-	-	-	-		-	-	-	-	-
SWD-TP08 1.0-1.4 Dark reddish brown spackled white, silty sandy gravel. Ferruginized Alluvium. Ferruginised Alluvium 16 65 15 4 19 31 15 5.0 6.0 1.72 Low	SC	A-2-6 (1)	2.557 -	- :	2053 7.3	<3 <3	4 0.3	NC	-	-	-		-	-	-	- I link to Man	4.59E-07
SWD-TP19 1.4-2.4 Yellowish brown mottled black stained red, clayey silty gravelly sand with trace gravels, ferruginised alluvium 1 53 18 28 46 25 8 2.0 7.0 0.72 Low	SC	A-4 (1)	2.645 -	-					-	-	-	- 1040	5	1046	Corrosive	High to Very High	
WRD-TP39 1.4-1.5 Khaki stained brown to black, sity gravelly sand to sith sandy gravel, ferruginised alluvium 22 61 13 4 17 36 10 3.5 4.0 1.79 Low	SM	A-2-4 (0)	2.596 -	-				-	-	-	-		-	-	-	-	-
WRD-TP35 2.5-4.1 Yellow brown, sandy silt. Ferruginous Alluvium. Ferruginous Alluvium 35 6 21 20 41 32 13 6.0 10.0 0.92 Low	SC	A-6 (2)	2.647 -	-	1946 11.2	<3 <3	3 0.1	NC	-	-	29	13 -	-	-		-	
HLP-TP03 2.3-3.1 Khaki to white streaked to banded black, silty sandy gravel to silty gravelly sand, calcified residual gneiss Residual Gneiss 49 40 10 1 11 25 5 2.5 1.0 2.19 Low GG	GC-GM	A-1-a (0)	2.601 -	-	1974 11	16 21	33 0.1	G7	-	-	28	18 -	-	-		-	3.92E-07
WRD-TP02 1.4-1.6 White to grey mottled black stained orange, sity gravelly sand, calcified reworked residual gneiss Residual Gneiss 58 37 2 3 5 - 1 0.0 0.0 2.49 Low Gr	GP-GM	A-1-a (0)	2.635 -						-	-							
Dayly brown method groups and black, alcount alth approximated an additional provided residual highly		A-2-4 (0)		-						-						-	
WRD-TP37 1.1-2.4 Yellowish brown mottled black and white, gravelly silty sand, reworked residual gneiss Residual Gneiss 8 68 21 3 24 24 6 3.0 4.0 1.31 Low SC			2.648 -	_				_	_						+ .		
			2.629 -					-					-	-		 	
				-	0000	40	- · ·		-	-			-		 	++	
			2.571 -	- :	2020 8.8	18 28	51 0.0		-	-	-			-	-		-
HLP-TP20 1.6-1.8 Completely weathered, white speckled to streaked black stained khaki to orange, very soft rock, gneiss Gneiss 23 66 7 4 11 28 8 2.5 2.0 2.04 Low SV			2.566 -	-				-	-	-	-			-	-	-	
			2.673 -		1940 9.2		20 0.0		-	-	-		•	-	-		
			2.604 -			15 17			-	-	-		•	-	-	- High to Very	
	SC	A-2-6 (1)	2.736 -		2174 7.2	12 19	40 0.0	G8				1000	3	1071	Corrosive	Light to Very	-
WRD-TP37 2.4-2.7 Completely to highly weathered, white speckled black stained orange, gneiss Gneiss 39 49 11 1 12 23 8 4.0 2.0 2.05 Low			2.655 -		2010 10.8	-	21 0.1		-	-		- 1068	1 -	l	+	High	

NOTES:

1. LL - LIQUID LIMIT; 2. PI - PLASTICITY INDEX; 3. LS - LINEAR SHRINKAGE; 4. GM - GRADING MODULUS 5, PE - POTENTIAL EXPANSIVENESS; 6. USCS - UNIFIED SOIL CLASSIFICATION SYSTEM 7. MID - MAXIMUM DRY DENSITY 8. OMC - OPTIMUM MOISTURE CONTENT; 9. CBR - CALIFORNIA BEARING RATIO; 10. KPA - KILOPASCAL; 11. AASHTO - AMERICAN ASSOCIATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION SYSTEM 7. MID - MAXIMUM DRY DENSITY 8. OMC - OPTIMUM MOISTURE CONTENT; 9. CBR - CALIFORNIA BEARING RATIO; 10. KPA - KILOPASCAL; 11. AASHTO - AMERICAN ASSOCIATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 12. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 13. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDRALIC CONDUCTIVITY; 13. NO - NOT CLASSIFICATION STATE HIGHWAY TRANSPORT OFFICIALS; 14. K-VALUE - HYDR

A 81/2022 ISSLEO WITHLETTER SY BK.
REY DATE DESCRIPTION PREPD REYO



APPENDIX B4

CRATON MINING AND EXPLORATION (PTY) LTD

HEAP LEACH PAD AND PONDS FEASIBILITY DESIGN GEOTECHNICAL INVESTIGATION FOR HEAP LEACH PAD, WASTE ROCK DUMP, STORMWATER DIVERSION & PLANT SUMMARY OF ROCK LABORATORY TEST RESULTS

	SAI	MPLE		Uniaxial Compr (UC	essive Strength		Poin	t Load Index Te	est	
STRUCTURE ID.	Borehole No.	Depth (m)	Rock Description	UCS (MPa)	Failure Type	Туре	Orientation	Failure Load P	I _{s (50)} (MPa)	Estimated UCS (MPa)
		2.04 - 2.26	Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	8.0	normal	-	-	-	-	-
		2.39 - 2.51	Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	-	-	а	р	0.97	0.48	12.01
	SWD-BH01				_	а	p p	1.09	1.03	12.51 25.53
		3.14 - 3.48	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	23.5	normal	d	р	1.09	0.84	20.79
		3.36 - 3.95	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	-	-	a d	p p	1.14	1.57	39.02 26.83
		2.84 - 2.98	Highly to moderately weathered, brown to grey speckled black banded white, soft to medium hard rock, gneiss	-	-	a d	p p	1.04	0.92	22.87 11.59
	SWD-BH02	7.96 - 8.16	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	33.5	normal	-	-	-	-	-
Stormwater		8.16 - 8.35	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	-	1	a	р	1.05	1.15	28.67
Diversion Channel (SWD)	CWD DLIOS	3.82 - 4.13		40.0		а	р	1.04	0.88	21.86
	SWD-BH03	3.82 - 4.13	Moderately weathered, grey speckled black and white, soft to medium hard rock, gneiss	12.0	normal	d	р	1.09	0.48	11.92
		3.71 - 3.88	Moderately weathered, grey speckled black streaked white, soft to medium hard rock, gneiss	-	-	a d	p p	1.13	0.18	120.83 4.55
	SWD-BH04	8.23 - 8.39	Moderately weathered, grey speckled black streaked white, soft to medium hard rock, gneiss	-	-	a	р	1.06	4.07	100.76
						а	р	1.07	0.49	12.23
	SWD-BH05	5.88 - 6.02	Highly to completely weathered, dark grey to dark brown speckled black, very soft rock, gneiss	-	-	d	р	1.09	0.10	2.58
		9.69 - 9.93	Highly to moderately weathered, grey to brown speckled black streaked white, medium hard rock, gneiss	25.5	normal	a d	р	0.96	0.61	43.36 15.14
	SWD-BH08	5.07 - 5.21	Highly weathered, grey to brown speckled black streaked white, soft rock, gneiss	_	_	а	р	1.03	0.73	17.86
	300-6100	3.07 - 3.21	Trigrily weathered, grey to brown speckled black streaked writte, soft rock, griefss	-	-	d	р	1.07	0.19	4.63
	HLP-BH01	6.18 - 6.31	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	a d	р	0.96 1.09	0.39	9.67 3.67
	HLP-BH01	8.35 - 8.60	Highly weathered, brown banded red speckled black streaked white, soft rock, gneiss	-	-	а	р	1.04	0.62	15.31
						d a	р	1.08	0.49	12.05 8.73
	HLP-BH02	5.35 - 5.65	Highly to completely weathered, brown to grey, banded white, very soft rock gneiss	8.0	normal	d	р	1.09	0.19	4.80
Heap Leach Pad (HLP)	HLP-BH03	2.27 - 2.41	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	а	р	0.95	0.19	4.72 2.61
						а	р	1.11	0.41	10.12
	HLP-BH03	3.73 - 3.93	Highly weathered, brown to green speckled black streaked white, soft rock, gneiss	-	-	d	р	1.09	0.3	7.43
	HLP-BH04	2.66 - 2.86	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	7.0	normal	-	-	-	-	-
	HLP-BH04	5.19 - 5.35	Moderately weathered, grey speckled black streaked white, medium hard rock, gneiss	-	-	a d	p p	1.04	0.68	16.97 7.75
	PLT-BH01	2.80 - 9.00	Slightly weathered, grey speckled black streaked white, medium hard rock, granitic gneiss	-	-	а	р	1.14	1.67	41.27
						d	р	1.09	0.54	39.13 13.41
	PLT-BH02	4.73 - 4.94	Highly weathered, brown to grey, soft rock, gneiss	-	-	d	Р	1.05	0.54	6.94
Plant Area (PLT)	PLT-BH02	8.09 - 8.37	Moderately weathered, light brown to grey speckled black and streaked white, medium hard rock, granitic gneiss	42.0	normal	а	р	1.14	4.04	100.56
	PLT-BH03	1.78 - 1.98	Completely weathered, yellow brown speckled black, calcified soft rock, gneiss	-	-	a d	p p	1.08	0.48	14.06 16.89
	PLT-BH03	6.33 - 6.55	Moderately weathered, dark grey to green speckled black streaked whit, medium hard rock, gneiss	10.5	normal	а	р	0.98	1.65	40.84
	w=					d	р	1.09		20.09
Wasta Bask Ba	WRD-BH01		Pale brown blotched white, hardpan calcrete of soft rock strength	5.0	normal -	a	- p	0.97	0.22	5.48
Waste Rock Dump (WRD)	WRD-BH01	5.13 - 5.30	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss	-	-	d	р	1.09	0.27	6.73
	WRD-BH01	7.75 - 7.88	Slightly weathered to moderately weathered, dark brown to grey streaked and banded white, medium hard rock, gneiss	-	-	а	р	1.06	0.13	3.24

NOTES:

1. D = DIAMETRICAL LOADING; 2. A = AXIAL LOADING; 3. UCS = UNIAXIAL COMPRESSIVE STRENGTH; 4, P = PERPENDICULAR.

A	8/1/2023	ISSUED WITH LETTER	SY	BK
REV	DATE	DESCRIPTION	PREP'D	REV'D

Evaluation of unpaved wearing course materials according to THR20

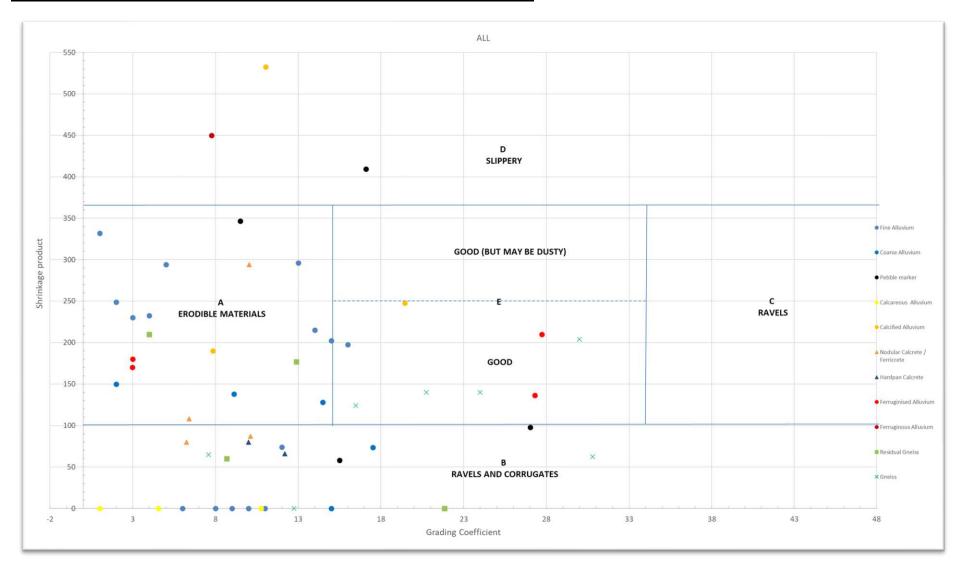


Figure 1 Evaluation of all materials unpaved wearing course according to THR20

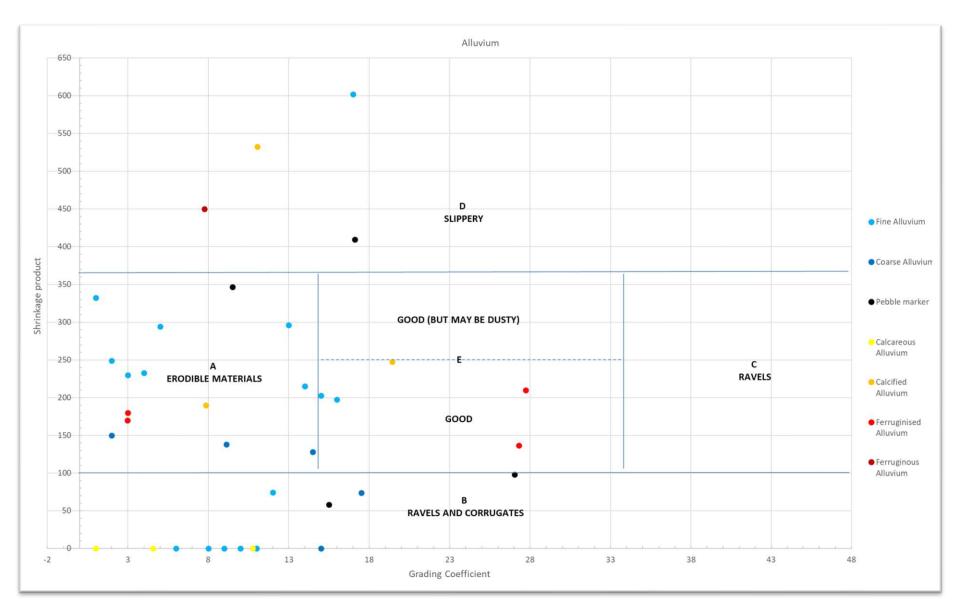


Figure 2 Evaluation of alluvium materials unpaved wearing course according to THR20

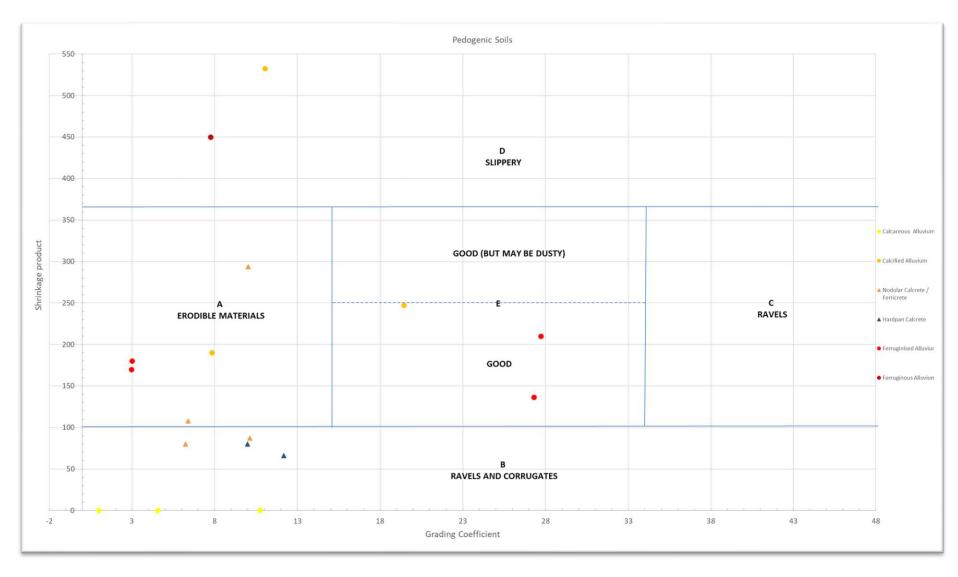


Figure 3 Evaluation of pedogenic materials unpaved wearing course according to THR20

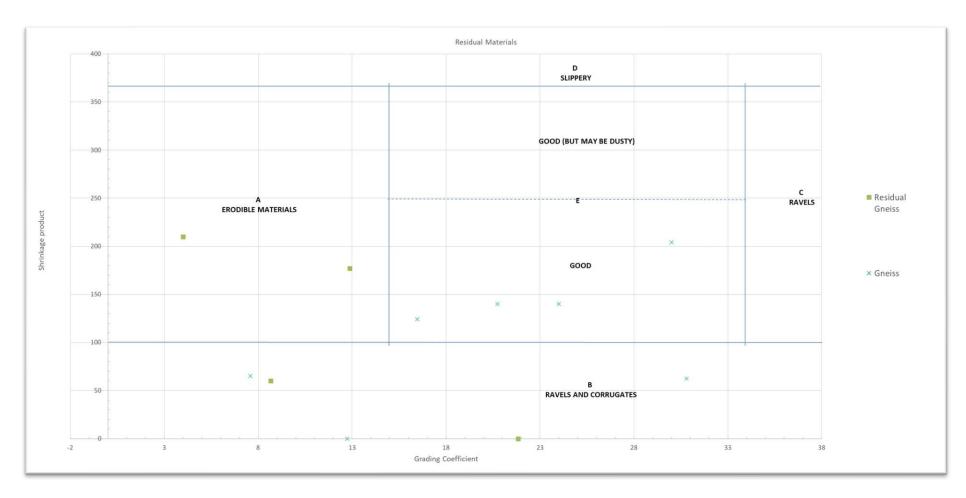


Figure 4 Evaluation of residual and very soft rock materials unpaved wearing course according to THR20

Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX C

Test Pit and Borehole Logs

Appendix C1

Test Pit Profiles

Appendix C2

Boreholes Logs



Craton Mining and Exploration (Pty) Ltd
Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX C1

Test Pit Profiles

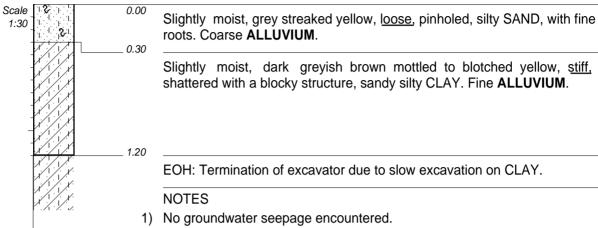




GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP01 Sheet 1 of 1

JOB: 3010047806



1) 140 groundwater seepage encour

2) Stable sidewalls.

3) No sample taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: Chris Homan
TYPE SET BY: EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 22 May 2023 DATE : 22 May 2023

DATE: 04/07/2023 13:44

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COORDINATE SYSTEM: UTM, WGS84 (33K)

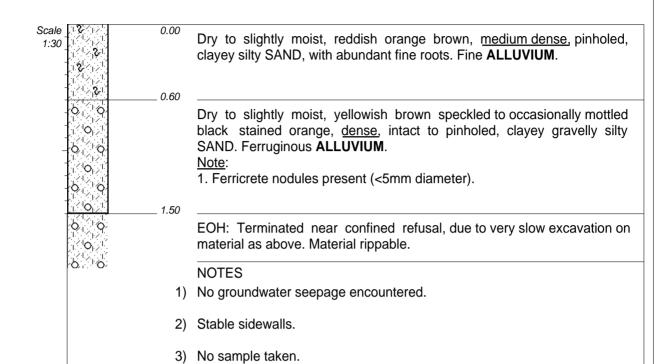
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP02 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . Chris Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 23 May 2023

DATE: 23 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

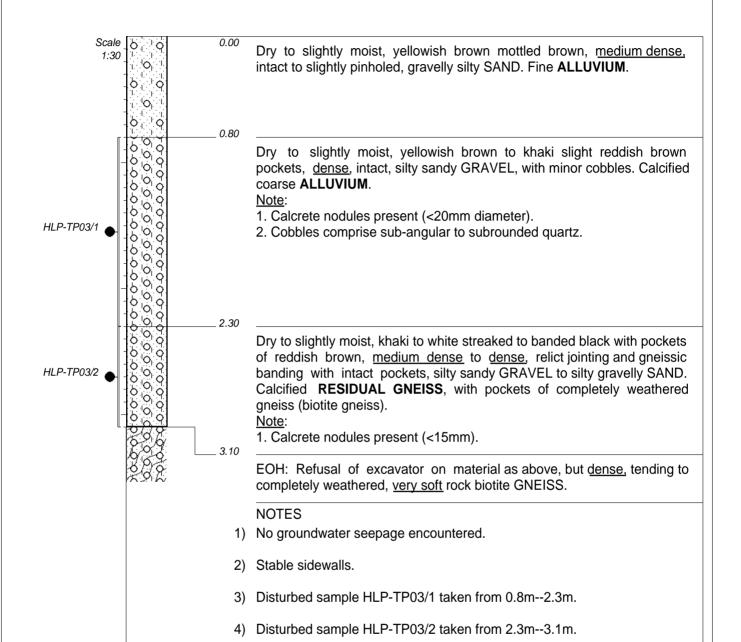
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP03 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D DRILLED BY: Joseph

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 22 May 2023 DATE : 22 May 2023

DATE: 04/07/2023 13:44

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COORDINATE SYSTEM: UTM, WGS84 (33K)

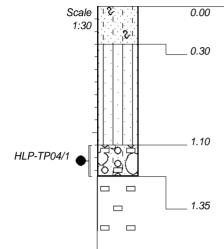
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP04 Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown, <u>very loose</u>, intact, silty fine to medium SAND, with roots. Fine **ALLUVIUM**.

Slightly moist, dark brown to reddish brown mottled white, weakly to moderately cemented, <u>medium dense</u>, sandy SILT, with scattered traces of calcrete nodules. Calcified **ALLUVIUM**.

Slightly moist, light brown blotched white, <u>medium dense</u>, sandy silty GRAVEL, comprising abundant calcrete nodules and cobbles. **NODULAR CALCRETE** tending to **HONEYCOMB CALCRETE**.

EOH: Refusal of excavator on very strongly cemented, HONEYCOMB CALCRETE of medium hard rock consistency.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP04/1 taken at 1.1m--1.35m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance
TYPE SET BY: EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 19 May 2023 DATE : 19 May 2023

DATE: 04/07/2023 13:44

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COORDINATE SYSTEM: UTM, WGS84 (33K)

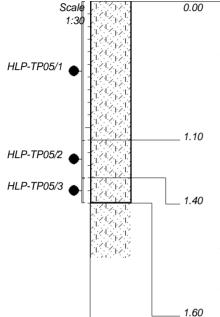
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP05 Sheet 1 of 1

JOB: 3010047806



Dry, orange to reddish brown, $\underline{\text{medium dense}}$, pinholed, clayey silty SAND. Fine ALLUVIUM.

Note:

1. Abundant roots present at top 200mm.

Dry, yellowish brown, <u>medium dense</u> to <u>dense</u>, intact to pinholed, clayey silty SAND, with traces of sub-angular to subrounded quartz fine gravel. Fine **ALLUVIUM**.

Dry, dark greyish brown mottled black mottled to blotched yellow to orange, <u>dense</u>, intact to slightly honeycombed, clayey silty SAND, with traces of sub-angular to subrounded quartz gravel. Ferruginous **ALLUVIUM**.

Note:

1. Ferricrete nodules present (<7mm diameter).

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but <u>dense</u>.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP05/1 taken from 0.0m--1.1m.
- 4) Disturbed sample HLP-TP05/2 taken from 1.1m--1.4m.
- 5) Disturbed sample HLP-TP05/3 taken from 1.4m--1.6m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY : Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 23 May 2023

DATE: 23 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

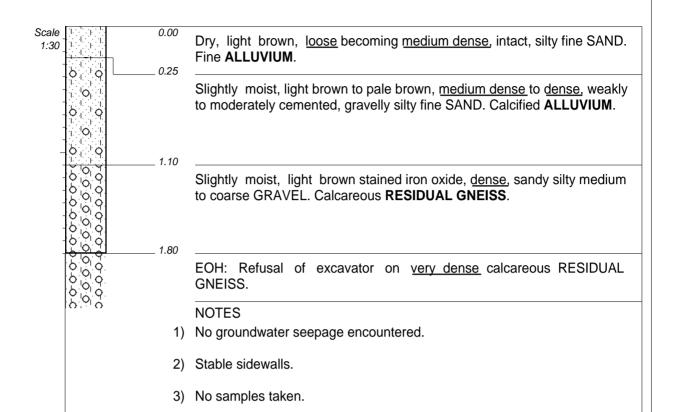
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP06 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 20 May 2023 DATE : 20 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

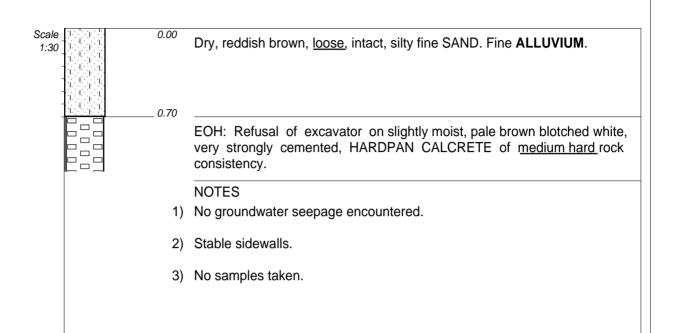
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP07 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 19 May 2023 DATE : 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

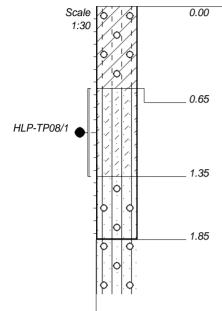
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP08 Sheet 1 of 1

JOB: 3010047806



Slightly moist to moist, dark brown, firm, blocky shattered, gravelly silty CLAY. Fine ALLUVIUM.

Note:

1. Gravel comprises minor calcrete nodules, with subrounded fine to medium quartz.

Slightly moist, brown, firm, clayey sandy silt. Fine ALLUVIUM.

Slightly moist, pale brown, dense to very dense, moderately to strongly cemented, gravelly sandy silt. Calcified **ALLUVIUM**.

EOH: Refusal of excavator on very dense, very strongly cemented, calcified ALLUVIUM of soft rock strength.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP08/1 taken at 0.65m--1.35m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 19 May 2023

DATE: 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

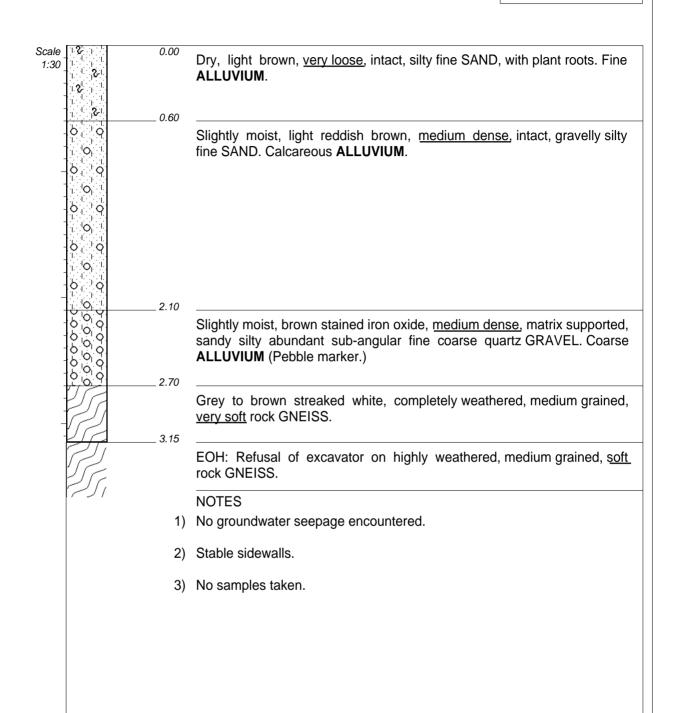
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP09 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 20 May 2023

DATE: 20 May 2023

DATE: 04/07/2023 13:44
TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

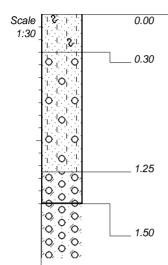
X-COORD: 800837.00 *Y-COORD*: 7584690.00



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP10 Sheet 1 of 1

JOB: 3010047806



Slightly moist, light brown, very loose to loose, intact, silty fine to medium SAND, with plant roots. Fine ALLUVIUM.

Slightly moist, light brown, medium dense to dense, intact, poorly to moderately cemented, gravelly silty fine to medium SAND. Calcified ALLUVIUM.

Slightly moist, dark brown stained orange, moderately cemented, sandy clayey subrounded fine to medium GRAVEL. Ferruginised ALLUVIUM of soft rock strength.

EOH: Refusal of excavator on strongly cemented, ferruginised ALLUVIUM of soft to medium hard rock strength.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx MACHINE:: Excavator FR220D DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 19 May 2023 DATE: 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

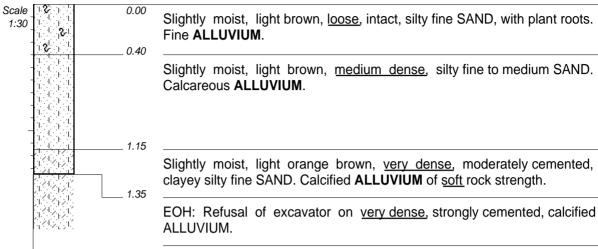
x-coord: 801915.33 Y-COORD: 7585227.33



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP11 Sheet 1 of 1

JOB: 3010047806



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 19 May 2023 DATE : 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

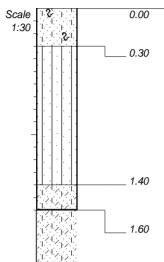
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OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP12 Sheet 1 of 1

JOB: 3010047806



Slightly moist, light brown, <u>loose</u>, intact, silty fine SAND, with plant roots. Fine **ALLUVIUM**.

Slightly moist, light brown, $\underline{\text{firm}}$, intact, sandy SILT. Calcareous **ALLUVIUM**.

Slightly moist, light orange brown, moderately cemented, <u>dense</u>, clayey silty fine SAND. Calcified **ALLUVIUM**.

EOH: Refusal of excavator on <u>very dense</u>, strongly cemented, calcified **ALLUVIUM**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 19 May 2023 DATE: 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

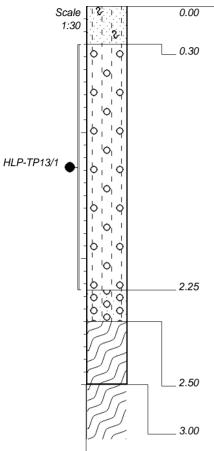
X-COORD : 801251.02 Y-COORD : 7585169.08



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP13 Sheet 1 of 1

JOB: 3010047806



Dry, light brown, loose, intact, silty fine SAND, with plant roots. Fine ALLUVIUM.

Slightly moist, light brown to brown, medium dense, intact, gravelly silty fine SAND> Calcareous ALLUVIUM.

Slightly moist, brown stained iron oxide, dense, matrix supported, sandy silty GRAVEL, comprising fine to medium, subrounded to sub-angular quartz. Coarse ALLUVIUM.

Note:

1. Traces of ferricrete nodules.

Grey to brown streaked and banded white, completely weathered, very soft rock GNEISS.

EOH: Refusal of excavator on highly weathered, soft rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- Stable sidewalls.
- 3) Disturbed sample HLP-TP13/1 taken at 0.3m--2.25m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

SETUP FILE: KPTP8.SET

TYPE SET BY: EM

INCLINATION: Vertical

DIAM:

DATE: 20 May 2023 DATE: 20 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

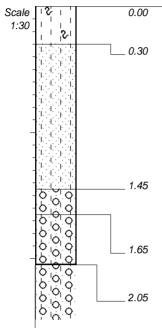
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OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP14 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, light brown, <u>very loose</u>, intact, silty fine AND, with roots. Fine **ALLUVIUM**.

Slightly moist, light brown, loose to medium dense, silty medium SAND. Calcareous **ALLUVIUM**.

Slightly moist, brown stained iron oxide, <u>medium dense</u>, matrix supported, sandy silty GRAVEL, comprising abundant subangular fine to coarse quartz gravel. Coarse **ALLUVIUM**.

Dark brown stained iron oxide, <u>dense</u>, silty sandy subangular fine grained GRAVEL. **RESIDUAL GNEISS**.

EOH: Refusal of excavator on very dense RESIDUAL GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 19 May 2023 DATE : 19 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

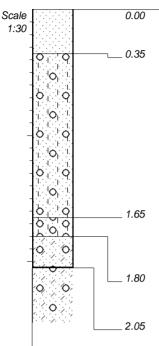
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP15 Sheet 1 of 1

JOB: 3010047806



Slightly moist, light brown, very loose, intact, silty fine SAND. Fine ALLUVIUM.

Slightly moist, light brown, medium dense to dense, poorly to moderately cemented, gravelly silty fine to medium SAND. Calcified ALLUVIUM.

Slightly moist, brown stained iron oxide, medium dense, matrix supported, silty sandy GRAVEL, comprising abundant medium sub-angular quartz gravel. Coarse ALLUVIUM.

Slightly moist, dark brown stained orange, dense, moderately cemented, gravelly clayey medium SAND. Ferruginised ALLUVIUM.

EOH: Refusal of excavator on very dense, strongly cemented, ferruginised ALLUVIUM.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM:

DATE: 18 May 2023 DATE: 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

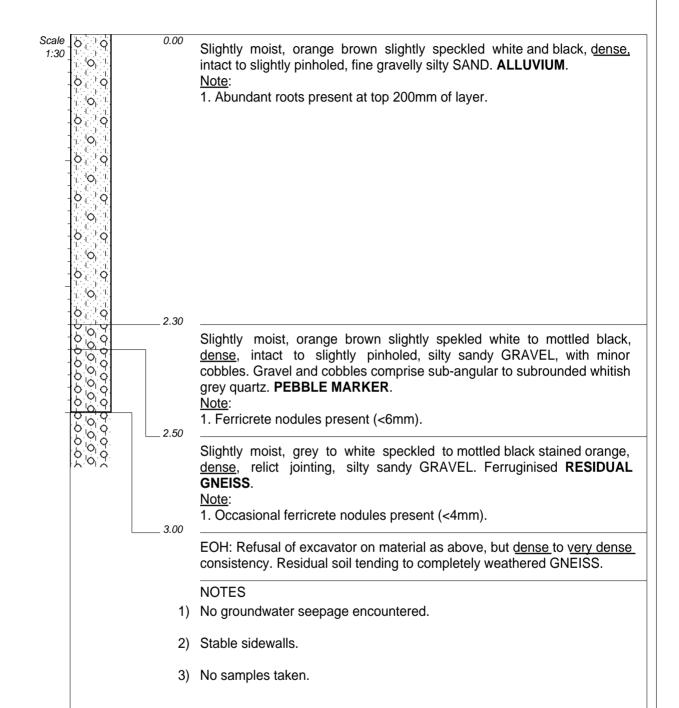
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP16 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D DRILLED BY: Joseph

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 22 May 2023 DATE : 22 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

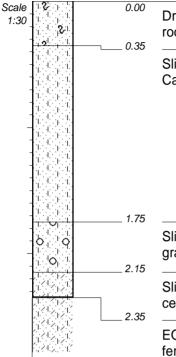
X-COORD: 0801209 Y-COORD: 7584039



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP17 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, light brown, loose, silty fine to medium SAND, with roots. Fine ALLUVIUM.

Slightly moist, pale brown, medium dense, intact, silty fine SAND. Calcareous ALLUVIUM.

Slightly moist, pale brown, dense to very dense, moderately cemented, gravelly silty SAND. Calcified ALLUVIUM.

Slightly moist, brown stained iron oxide, dense, moderately to strongly cemented, clayey silty medium SAND. Ferruginised ALLUVIUM.

EOH: Refusal of excavator on very dense, strongly cemented, ferruginised ALLUVIUM.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

SETUP FILE: KPTP8.SET

TYPE SET BY: EM

INCLINATION: Vertical

DIAM:

DATE: 19 May 2023 DATE: 19 May 2023

DATE: 04/07/2023 13:44 TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

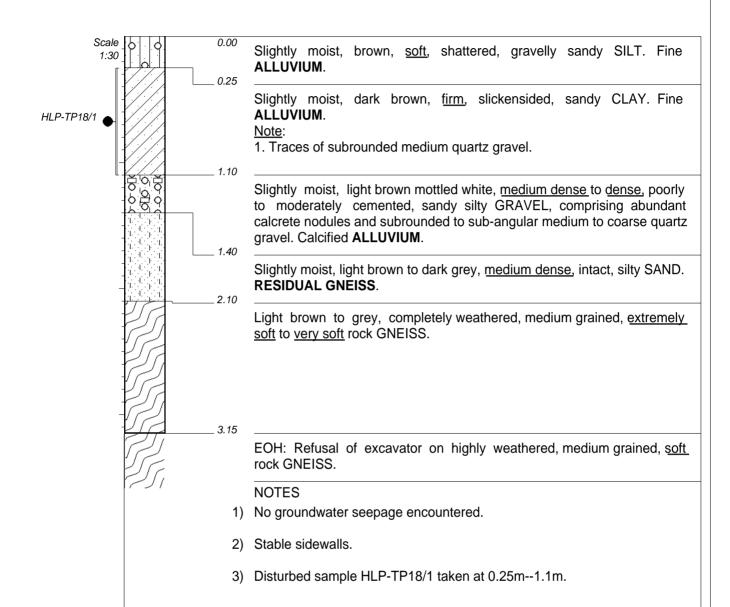
x-coord: 801663.45 Y-COORD: 7584447.86



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP18 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 18 May 2023 DATE: 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

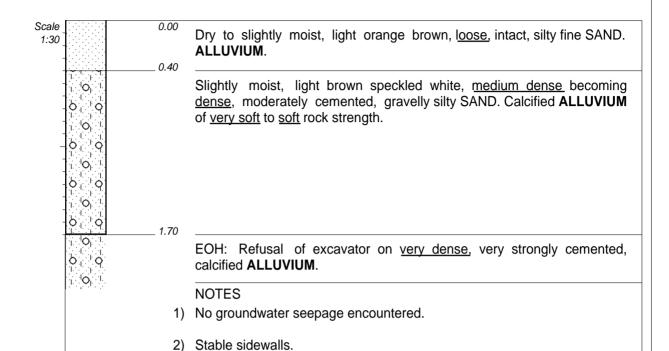
X-COORD: 802000.00 Y-COORD: 7584198.00



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP19 Sheet 1 of 1

JOB: 3010047806



3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 18 May 2023 DATE: 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

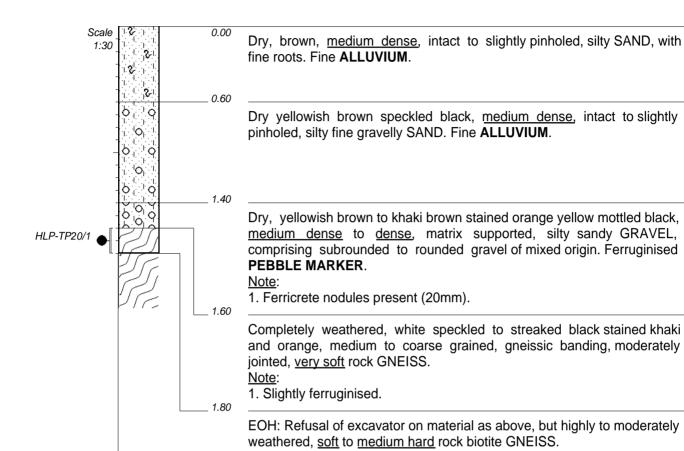
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GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP20 Sheet 1 of 1

JOB: 3010047806



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP20/1 taken from 1.6m--1.8m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 23 May 2023 DATE : 23 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

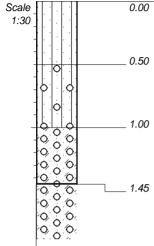
X-COORD: 0801428 Y-COORD: 7583326



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP21 Sheet 1 of 1

JOB: 3010047806



Dry, brown to light brown, \underline{soft} to \underline{firm} , intact, sandy SILT, with traces of plant roots. **ALLUVIUM**.

Slightly moist, grey to brown, <u>firm</u> to <u>stiff</u>, moderately cemented, gravelly sandy SILT. Calcified **ALLUVIUM** of <u>very soft</u> rock strength.

Slightly moist, grey to brown, <u>dense</u>, moderately cemented, sandy clayey abundant, sub-angular, medium to coarse GRAVEL. Ferruginised **ALLUVIUM**.

EOH: Refusal of excavator on very dense, ferruginised ALLUVIUM.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 20 May 2023 DATE : 20 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

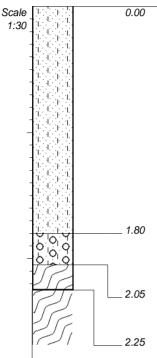
X-COORD : 801505.04 Y-COORD : 7583835.86



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP22 Sheet 1 of 1

JOB: 3010047806



Dry, light brown, \underline{loose} to $\underline{medium\ dense}$, intact, silty fine SAND. Fine **ALLUVIUM**.

Note:

1. Presence of plant roots to a depth of 300mm.

Dry, brown stained iron oxide, <u>medium dense</u>, matrix supported, sandy silty GRAVEL, comprising abundant fine to medium, sub-angular quartz gravel. Coarse **ALLUVIUM**. (Pebble marker.)

Brown to grey speckled black streaked white, completely to highly weathered, <u>very soft</u> to <u>soft</u> rock GNEISS.

EOH: Refusal of excavator on highly weathered, brown to grey streaked white, <u>soft</u> to <u>medium hard</u> rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 20 May 2023 DATE : 20 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

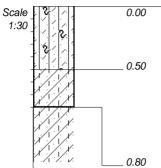
X-COORD: 801964.23 Y-COORD: 7583574.80



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP23 Sheet 1 of 1

JOB: 3010047806



Dry, dark brown, <u>soft</u> to <u>firm</u>, fissured, clayey sandy SILT, with abundant plant roots. Fine **ALLUVIUM**.

Slightly moist, dark brown, $\underline{\text{firm}}$ to $\underline{\text{stiff}}$, shattered, silty sandy CLAY. Fine **ALLUVIUM**.

Note:

1. Scattered traces of subrounded to sub-angular medium quartz gravel.

EOH: Refusal of excavator on very stiff material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No Disturbed samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 20 May 2023 DATE: 20 May 2023

DATE: 04/07/2023 13:44

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COORDINATE SYSTEM: UTM, WGS84 (33K)

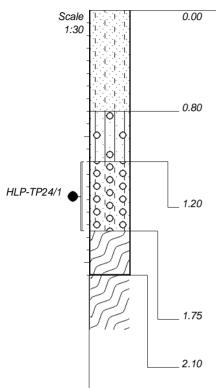
X-COORD: 801808.00 Y-COORD: 7583693.00



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP24 Sheet 1 of 1

JOB: 3010047806



Slightly moist, light brown, loose, intact, silty fine SAND. Fine ALLUVIUM.

Slightly moist, pale brown, $\underline{\text{firm}}$, poorly cemented, gravelly sandy SILT. Calcareous **ALLUVIUM**.

Note:

1. Gravel comprises coarse subrounded to sub-angular, fine to medium grained quartz.

Slightly moist, brown, <u>medium dense</u>, matrix supported, sandy silty abundant subrounded to sub-angular, medium to coarse grained, quartz GRAVEL. Coarse **ALLUVIUM**. (Pebble marker.)

- 1. Minor scattered cobbles at base of strata.
- 2. Traces of ferricrete nodules.

Brown to grey, completely to highly weathered, medium grained, <u>very soft</u> to <u>soft</u> rock GNEISS.

EOH: Refusal of excavator on brown to grey, highly weathered, medium grained, <u>soft</u> rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed samples HLP-TP24/1 taken at 1.2m--1.75m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 18 May 2023 DATE : 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

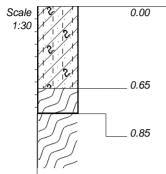
X-COORD: 802094.00 Y-COORD: 7583908.00



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP25 Sheet 1 of 1

JOB: 3010047806



Slightly moist to dry, brown to dark brown, $\underline{\text{firm}}$, shattered, sandy silty CLAY, with roots. Fine **ALLUVIUM**.

Grey to brown, highly to completely weathered, very closely jointed, medium grained, soft rock GNEISS.

EOH: Refusal of excavator on medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 18 May 2023 DATE: 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

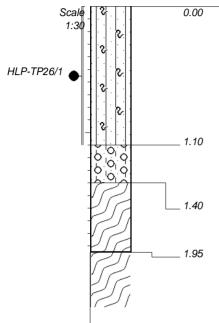
X-COORD: 802527.33 Y-COORD: 7584011.03



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP26 Sheet 1 of 1

JOB: 3010047806



Slightly moist, pale brown to brown, firm, fissured, sandy SILT, with roots. Fine ALLUVIUM.

Slightly moist, brown mottled iron oxide, medium dense, matrix supported, silty sandy GRAVEL, comprising medium, subrounded to sub-angular quartz GRAVEL. Coarse ALLUVIUM. (Pebble Marker.)

Brown to grey speckled and streaked white, completely weathered, medium grained, very soft rock GNEISS.

EOH: Refusal of excavator on brown to grey, completely weathered, medium grained, soft to medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- Stable sidewalls.
- 3) Disturbed sample HLP-TP26/1 taken at 0.0m--1.1m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 18 May 2023 DATE: 18 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

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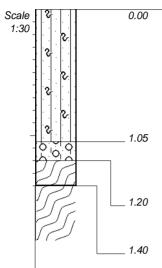
HOLE No: HLP-TP26



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP27 Sheet 1 of 1

JOB: 3010047806



Slightly moist, light brown to brown, <u>soft</u> becoming <u>firm</u> with depth, shattered, sandy SILT, with plant roots. Fine **ALLUVIUM**.

Slightly moist, brown stained iron oxide, <u>medium dense</u>, matrix supported, silty sandy GRAVEL, comprising abundant fine to medium, subrounded to sub-angular quartz GRAVEL. Coarse **ALLUVIUM**. (Pebble Marker.)

Brown grey speckled and streaked white, completely weathered, medium grained, <u>very soft</u> rock GNEISS.

EOH: Refusal of excavator on brown to grey, completely weathered, <u>soft</u> rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

PROFILED BY : S YaFrance TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 18 May 2023 DATE : 18 May 2023

DATE: 04/07/2023 13:44

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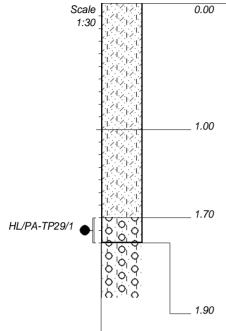
HOLE No: HLP-TP27



GEOTECHNICAL INVESTIGATION

HOLE No: HL/PA-TP29 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u>, intact with horizon slightly honeycomb in upper 100mm, clayey silty coarse SAND. Fine **ALLUVIUM**. Note:

1. Scattered fine roots present.

Dry, yellowish brown speckled black, <u>medium dense</u>, intact to slightly pinholed, clayey silty SAND, with minor coarse sand to fine gravel. Fine **ALLUVIUM**.

Dry, yellowish brown to khaki blotched black slightly stained orange, <u>dense</u>, matrix supported, silty sandy GRAVEL, with trace to minor subrounded cobbles of quartz. Ferruginous **PEBBLE MARKER**. Note:

1. Ferruginisation at nodular to honeycomb stage, very weakly cemented.

EOH: Refusal of excavator on material as above, but cementation stronger. **HONEYCOMB FERRICRETE**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Scattered small termite mounds near test pit.
- 4) Disturbed sample HL/PA-TP29/1 taken from 1.7m--1.9m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: Chris Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 23 May 2023

DATE: 23 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

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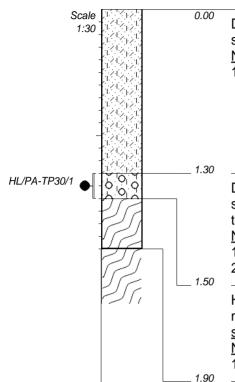
HOLE No: HL/PA-TP29



GEOTECHNICAL INVESTIGATION

HOLE No: HL/PA-TP30 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u>, pinholed, clayey silty SAND, with minor sub-angular to subrounded fine gravel of mixed origin. Fine **ALLUVIUM**. Note:

1. Scattered roots present.

Dry brown mottled black slightly stained red, <u>dense</u>, matrix supported, silty sandy GRAVEL, with abundant sub-angular to subrounded cobbles to boulders of quartz. **PEBBLE MARKER**.

Note:

- 1. Ferricrete nodules present (<40mm).
- 2. Very undulating horizon.

Highly weathered, white speckled to streaked black stained orange, medium to coarse grained, gneissic banding, widely to very widely jointed, soft rock GNEISS.

Note:

1. Very undulating horizon.

EOH: Refusal of excavator on material as above, but moderately weathered, medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HL/PA-TP30/1 taken from 1.3m--1.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D DRILLED BY: Joseph

PROFILED BY: Chris Homan

SETUP FILE: KPTP8.SET

TYPE SET BY : FM

INCLINATION: Vertical

DIAM: DATE: 25 May 2023 DATE: 25 May 2023

DATE: 04/07/2023 13:44

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COORDINATE SYSTEM: UTM, WGS84 (33K)

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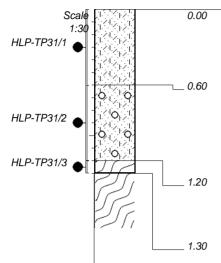
HOLE No: HL/PA-TP30



GEOTECHNICAL INVESTIGATION

HOLE No: HLP-TP31 Sheet 1 of 1

JOB: 3010047806



Dry, brown to yellowish brown, $\underline{\text{medium dense}}$, pinholed, clayey silty SAND. Fine ALLUVIUM.

Note:

1. Fine roots present.

Dry, dark greyish brown mottled yellow and orange, <u>dense</u>, intact, clayey silty gravelly SAND, with minor subrounded gravel of mixed origin. Ferruginised coarse **ALLUVIUM**.

Note:

1. Ferricrete nodules present.

Highly weathered with pockets of completely weathered, whitish grey speckled to banded black speckled gold, medium to coarse grained, gneissic banding, moderately jointed, very soft rock GNEISS.

EOH: Refusal of excavator on material as above, but moderately weathered.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample HLP-TP31/1 taken from 0.0m--0.6m.
- 4) Disturbed sample HLP-TP31/2 taken from 0.6m--1.2m.
- 5) Disturbed sample HLP-TP31/3 taken from 1.2m--1.3m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY : Chris Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 23 May 2023

DATE: 23 May 2023

DATE: 04/07/2023 13:44

TEXT: ..51\PROFILES\PKFHLPTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

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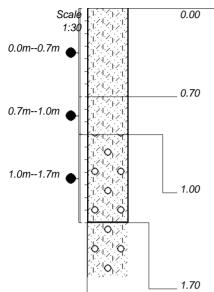
HOLE No: HLP-TP31



GEOTECHNICAL INVESTIGATION

HOLE No: Plant-TP32 Sheet 1 of 1

JOB: 3010047806



Dry, brown to orange brown, $\underline{\text{medium dense}}$, intact to pinholed, clayey silty coarse SAND. Fine $\mathbf{ALLUVIUM}$.

Note:

1. Roots present at top 200mm.

Dry, orange to yellowish brown, <u>medium dense</u> to <u>dense</u> with depth, pinholed, clayey silty fine to coarse SAND, with minor fine gravel. Coarse **ALLUVIUM**.

Note:

1. Scattered fine roots present.

Dry, orange brown mottled to blotched red and dark grey slightly mottled black, <u>dense</u>, intact to pinholed, clayey silty gravelly SAND. Ferruginised **ALLUVIUM**.

Note:

1. Ferricrete nodules present (<20mm).

EOH: Refusal of excavator due to slow excavation on material as above, but more dense/cemented. Nodular ferruginised **ALLUVIUM**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Sample Plant-TP32/1 taken from 0.0m--0.7m.
- 4) Sample Plant-TP32/2 taken from 0.7m--1.0m.
- 5) Sample Plant-TP32/3 taken from 1.0m--1.7m.

CONTRACTOR:

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE :

DATE: 24 May 2023

DATE: 10/07/2023 12:40

TEXT: ..\PROFILES\PKFPLANTTP.TXT

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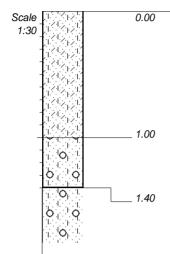
HOLE No: Plant-TP32



GEOTECHNICAL INVESTIGATION

HOLE No: Plant-TP33
Sheet 1 of 1

JOB: 3010047806



Dry, orange brown, <u>medium dense</u> to <u>dense</u> with depth, intact to pinholed, clayey silty SAND with coarse sand to fine gravel. Fine **ALLUVIUM**. Note:

1. Roots present at top 200mm.

Dry, brown mottled black blotched orange slightly speckled white, <u>dense</u>, pinholed to honeycombed, fine gravelly silty SAND. Ferruginised **ALLUVIUM**.

EOH: Refusal of excavator due to slow excavation on material as above, but more ferruginised, dense. Ferruginised **ALLUVIUM**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.

CONTRACTOR:

MACHINE:: Excavator FR220D DRILLED BY: Joseph

PROFILED BY: Chris Homan

SETUP FILE : KPTP8.SET

TYPE SET BY: EM

INCLINATION: Vertical

DIAM : DATE :

DATE: 24 May 2023

DATE: 10/07/2023 12:40

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COORDINATE SYSTEM: UTM, WGS84 (33K)

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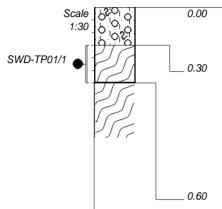
HOLE No: Plant-TP33



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP01 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>dense</u>, matrix supported, silty sandy GRAVEL, with minor sub-angular to subrounded quartz cobbles. Coarse **COLLUVIUM**, with roots. Interpreted as surface pebble marker.

Highly weathered, white speckled and streaked black stained orange, medium to coarse grained, gneissic banding, closely jointed, <u>soft</u> to <u>medium hard</u> rock with depth, GNEISS.

Note:

- 1 Very undulating horizon.
- 2. Brown, clayey silty sand infilling on joints, <2mm thick with occasional pockets in places.

EOH: Refusal of excavator on material as above, but moderately weathered, medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP01/1 taken at 0.3m--0.6m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 25 May 2023 DATE: 25 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

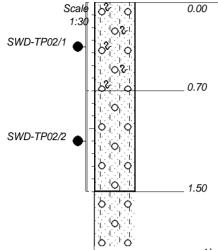
X-COORD: 0800870 Y-COORD: 7582003



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP02 Sheet 1 of 1

JOB: 3010047806



Dry, brown occasional mottled black, dense, intact, gravelly silty SAND with roots, with occasional thick roots throughout horizon. Fine ALLUVIUM.

Dry, yellowish brown streaked to blotched white and occasionally black, medium dense to dense, intact, gravelly silty SAND, with calcrete nodules (<20mm), with honeycomb formation in places, very weakly to weakly cemented. Calcified ALLUVIUM.

EOH: Refusal of excavator on material as above, moderately cemented.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP02/1 taken at 0.0m--0.7m.
- 4) Disturbed sample SWD-TP02/2 taken at 0.7m--1.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 24 May 2023

DATE: 24 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

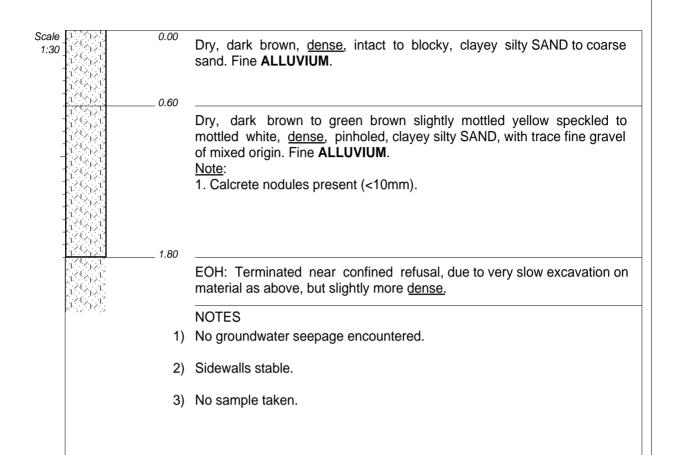
X-COORD: 801049 Y-COORD: 7582030



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP03 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY : C Homan

TYPE SET BY : EM

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 24 May 2023 DATE : 24 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

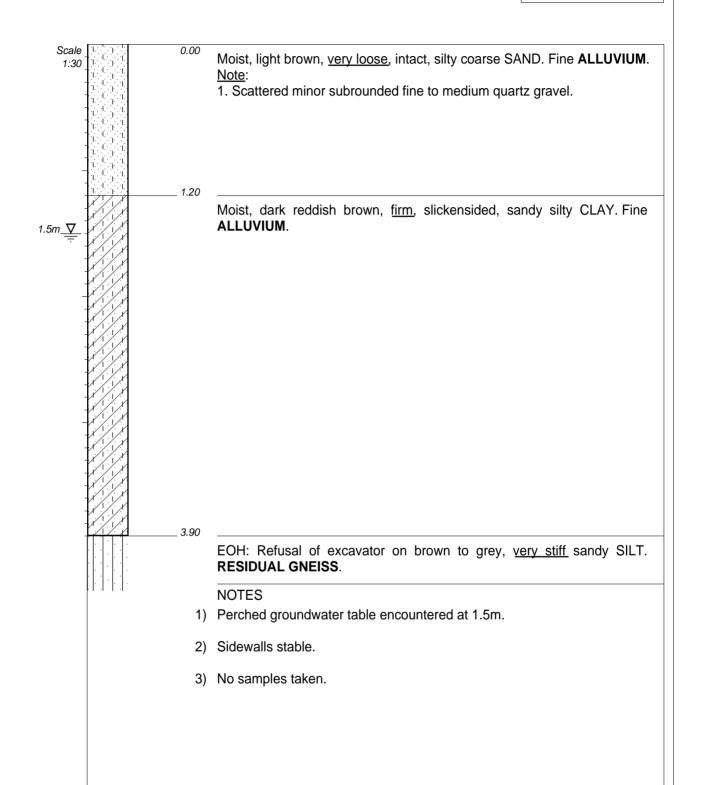
X-COORD: 0801151 Y-COORD: 7581871



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP04 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET **INCLINATION:** Vertical

DIAM: DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

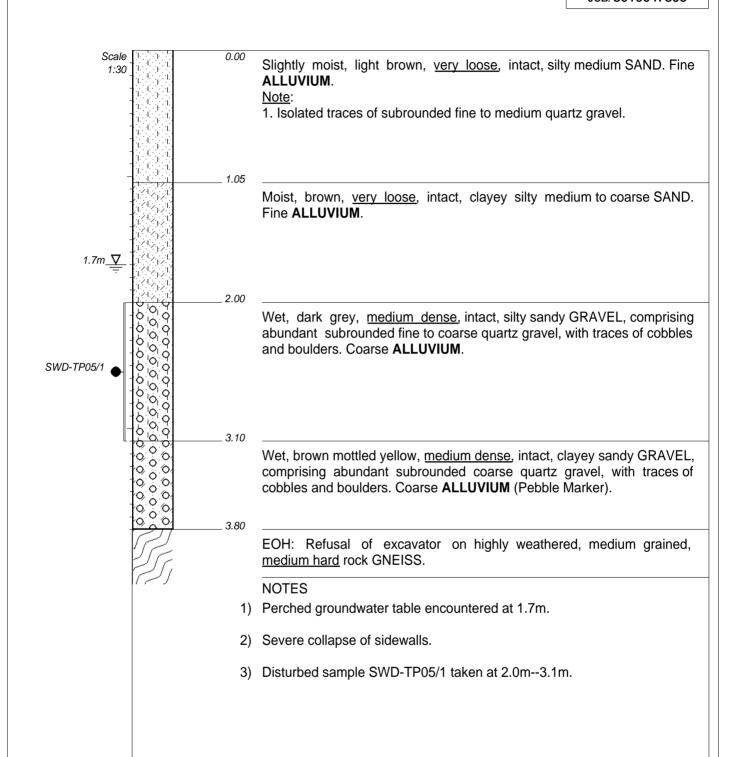
X-COORD: 801747 Y-COORD: 7581888



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP05 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM:

DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

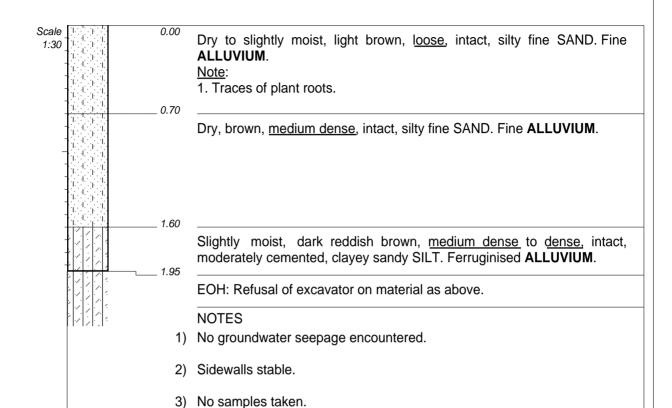
X-COORD: 801539 Y-COORD: 7581592



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP06 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION: Vertical**

DIAM:

DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

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HOLE No: SWD-TP06

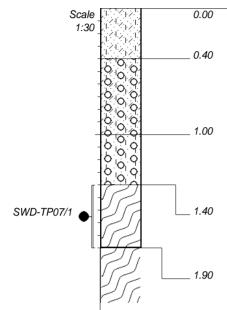
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP07 Sheet 1 of 1

JOB: 3010047806



Dry, dark brown, <u>dense</u>, intact to blocky, clayey silty SAND to coarse sand. Fine **ALLUVIUM**.

Dry, dark brown speckled white, <u>dense</u>, intact to slightly pinholed, clayey silty SAND, with abundant fine to medium GRAVEL. Gravel comprises sub-angular to subrounded gravel of mixed origin. Coarse **ALLUVIUM**.

Dry, brown to green brown slightly mottled yellow, white and black, <u>medium dense</u> to <u>dense</u>, matrix supported, silty sandy GRAVEL, with traces of sub-angular to subrounded cobbles to boulders of gneiss and quartz. **PEBBLE MARKER**.

Completely weathered, green speckled to streaked black and white and gold, slightly streaked orange, medium to coarse grained, gneissic banding, extremely closely jointed, <u>very soft</u> rock biotite GNEISS.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but highly weathered, <u>soft</u> rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP07/1 taken at 1.4m--1.9m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 24 May 2023

DATE: 24 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

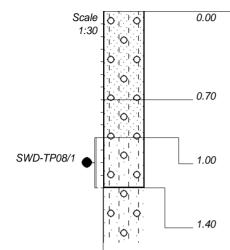
x-coord: k33 0801373 *y-coord*: 7581928



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP08 Sheet 1 of 1

JOB: 3010047806



Dry, light brown, <u>medium dense</u>, intact, gravelly silty fine SAND. Fine **ALLUVIUM**.

Note:

1. Traces of plant roots.

Slightly moist, reddish brown, <u>dense</u>, intact, poorly to moderately cemented, gravelly silty fine to medium SAND. Ferruginised **ALLUVIUM** of <u>very soft</u> to <u>soft</u> rock strength.

Slightly moist, dark reddish brown speckled white, <u>medium dense</u> to <u>dense</u>, intact, moderately cemented, silty sandy fine to medium sub-angular quartz gravel

EOH: Refusal of excavator on very dense ferruginised ALLUVIUM.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP08/1 taken at 1.0m--1.4m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

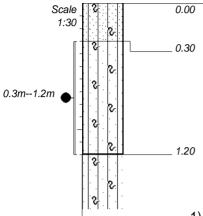
X-COORD: 801661 Y-COORD: 7581854



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP09 Sheet 1 of 1

JOB: 3010047806



Dry, pale brown, \underline{loose} , intact, fine sandy SILT / silty fine SAND, with medium to fine plant roots. **ALLUVIUM**.

Slightly moist, pale brown, $\underline{\text{stiff}}$, intact, slightly gravelly sandy SILT, with fine plant roots. **ALLUVIUM**.

EOH: Refusal of excavator in stiff material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample taken from 0.3m--1.2m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY : B Klaas

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 10 May 2023 DATE: 10 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

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HOLE No: SWD-TP09

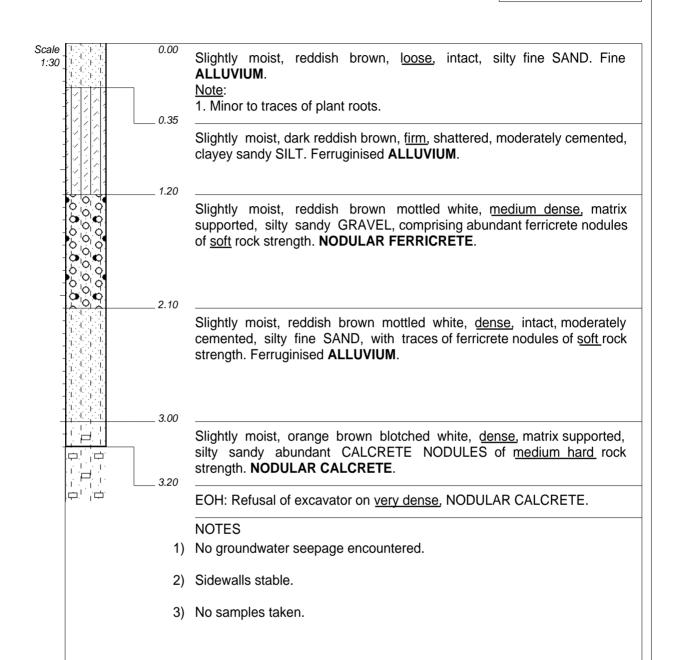
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP10 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM:

DATE: 16 May 2023

DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

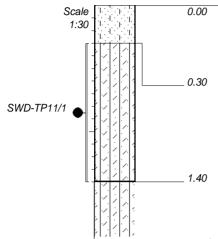
X-COORD: 802111 Y-COORD: 7581662



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP11 Sheet 1 of 1

JOB: 3010047806



Dry, red to orange brown, \underline{loose} , intact, silty fine to medium SAND. Fine **ALLUVIUM**.

Note:

1. Traces of plant roots.

Slightly moist, dark reddish brown, firm, fissured, clayey sandy SILT. Ferruginised **ALLUVIUM**.

EOH: Refusal of excavator on very stiff, ferruginised ALLUVIUM.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP11/1 taken at 0,3m--1.4m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET **INCLINATION:** Vertical

DIAM : DATE : 16 May 2023 DATE : 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

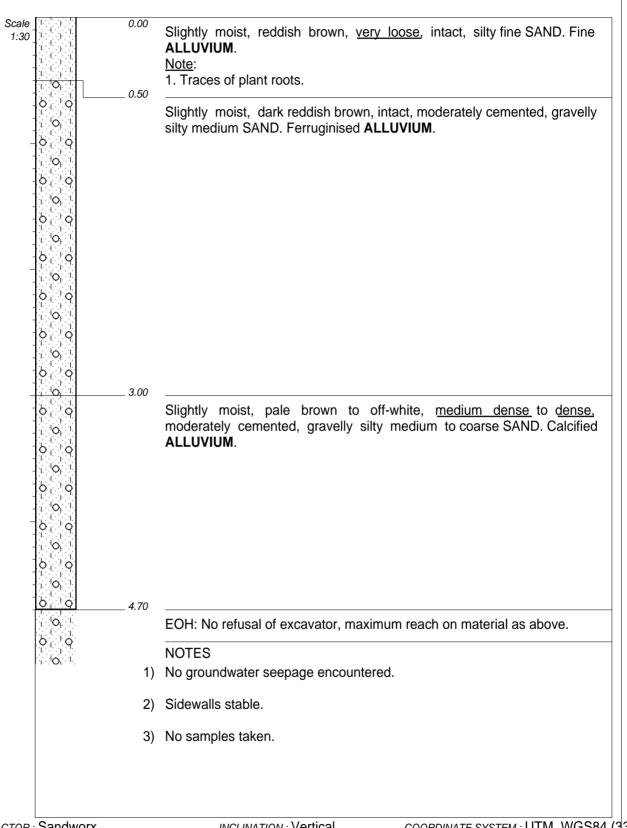
X-COORD: 802507 Y-COORD: 7581650



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP12 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx MACHINE:: Excavator FR220D DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

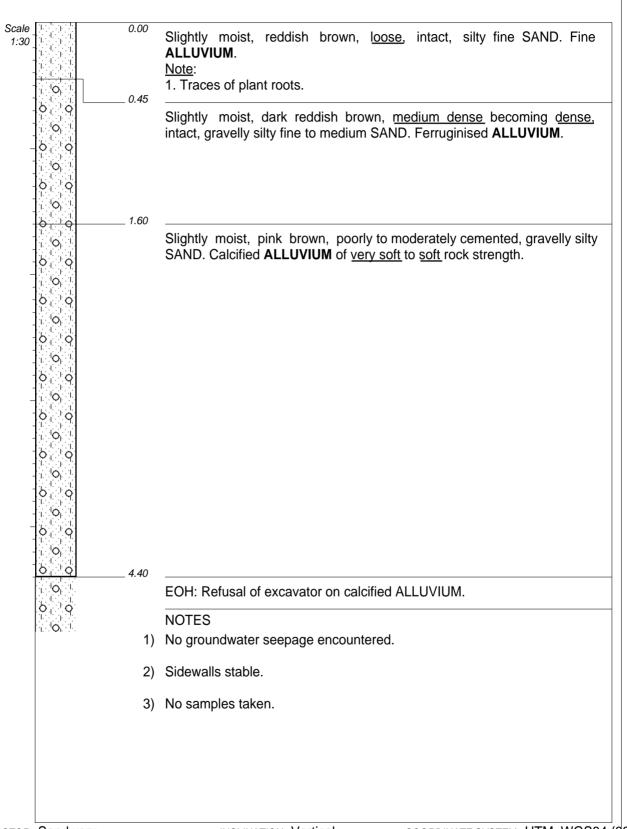
X-COORD: 802880 Y-COORD: 7581470



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP13 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 16 May 2023 DATE : 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

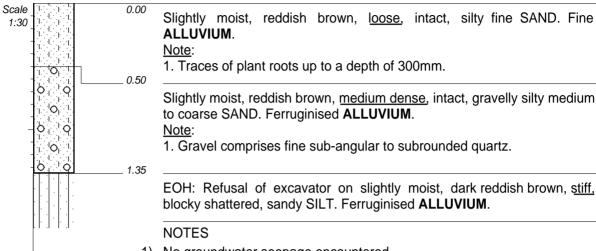
X-COORD: 803174 Y-COORD: 7581550



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP14 Sheet 1 of 1

JOB: 3010047806



- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) No samples taken.

CONTRACTOR: Sandworx MACHINE:: Excavator FR220D DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

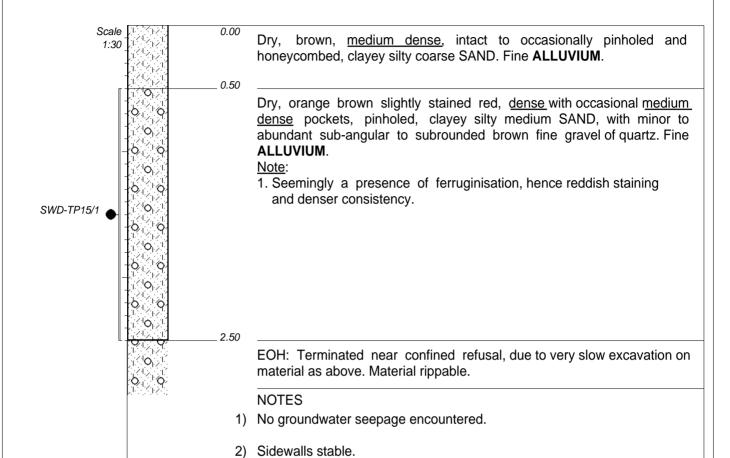
X-COORD: 803514 Y-COORD: 7581781



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP15 Sheet 1 of 1

JOB: 3010047806



3) Disturbed sample SWD-TP15/1 taken at 0.5m--2.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 25 May 2023

DATE: 25 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

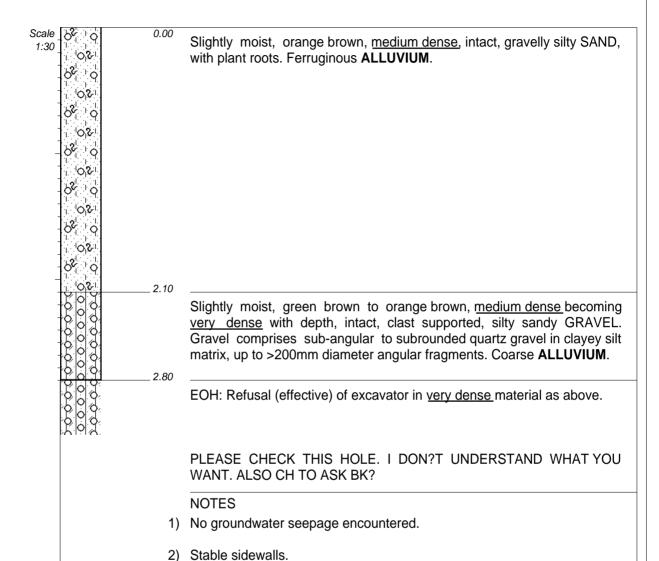
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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP16 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

3) No samples taken.

INCLINATION: Vertical

DIAM: DATE: 11 May 2023

DATE: 11 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 803965 Y-COORD: 7582214

HOLE No: SWD-TP16

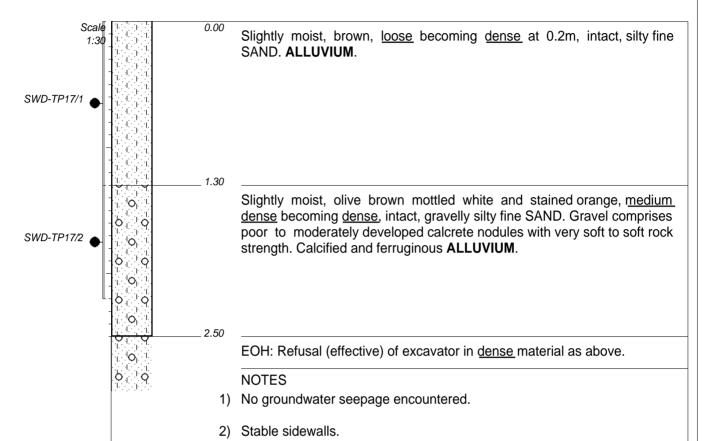
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP17 Sheet 1 of 1

JOB: 3010047806



- 3) Disturbed sample SWD-TP17/1 taken from 0.0m--1.3m.
- 4) Disturbed sample SWD-TP17/2 taken from 1.3m--2.2m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 11 May 2023 DATE: 11 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

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HOLE No: SWD-TP17

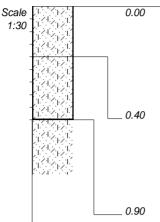
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP18 Sheet 1 of 1

JOB: 3010047806



Dry, brown to dark brown, <u>medium dense</u> to <u>dense</u>, intact to slightly pinholed to honeycombed openings, clayey silty coarse SAND, with trace fine gravel, comprising sub-angular to subrounded brown quartz. Fine **ALLUVIUM**.

Note:

1. Roots present at top 100mm.

Dry, dark brown with slight orange to red staining, <u>dense</u>, intact but breaks in a blocky manner, clayey silty SAND, with traces of coarse sand. Fine **ALLUVIUM**.

Note:

1. Seemingly very slightly ferruginised.

EOH: Terminated due to very slow excavation on material as above, but slightly denser. Interpreted as slightly more ferruginised.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: C Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 26 May 2023 DATE: 26 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

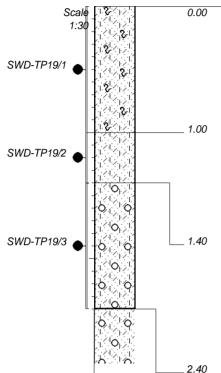
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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP19 Sheet 1 of 1

JOB: 3010047806



Dry, brown to reddish brown, <u>medium dense</u>, intact to occasionally pinholed to honeycombed openings, clayey silty SAND, with roots present throughout horizon. Fine **ALLUVIUM**.

Dry, yellowish brown with occasional mottled black, <u>dense</u>, intact to slightly pinholed, clayey silty SAND, with trace sub-angular to subrounded gravel to cobbles of gneiss and quartz. Fine **ALLUVIUM**. Note:

- 1. Occasional ferricrete nodules present.
- 2. Horizon slightly ferruginised.

Dry, yellowish brown mottled black slightly stained red, <u>medium dense</u>, intact to honeycombed, clayey silty gravelly SAND, with trace sub-angular to subrounded gravel of quartz. Ferruginous **ALLUVIUM**. Notes:

- 1. Ferricrete nodules present.
- 2. Ferruginisation between nodular to honeycombed ferricrete, which is weakly cemented.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly more ferruginised. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) Disturbed sample SWD-TP19/1 taken at 0.0m--1.0m.
- 4) Disturbed sample SWD-TP19/2 taken at 1.0m--1.4m.
- 5) Disturbed sample SWD-TP19/3 taken at 1.4m--2.4m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 26 May 2023

DATE: 26 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

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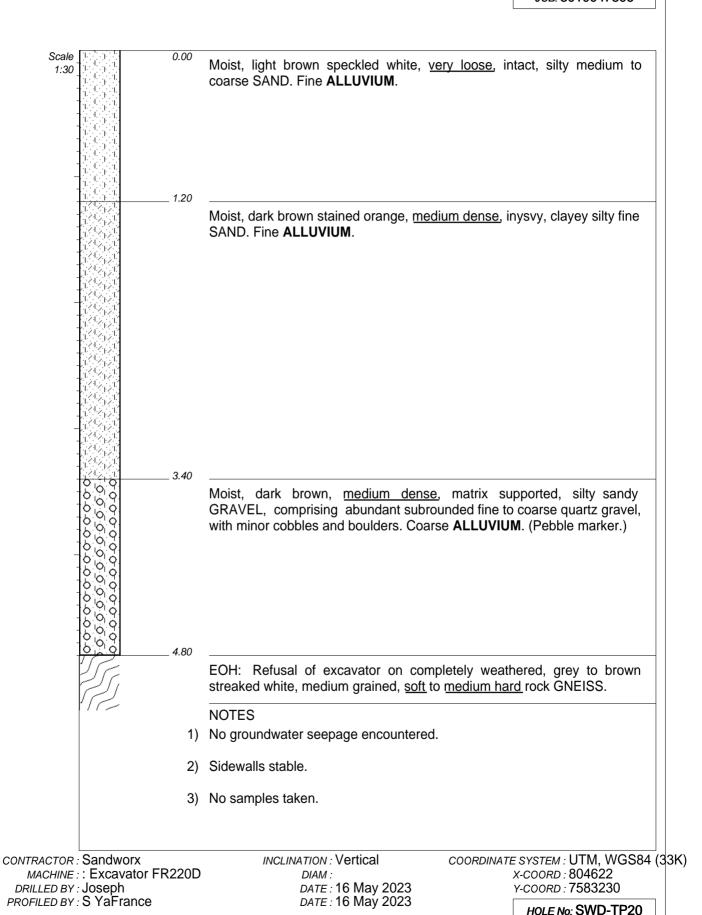
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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP20 Sheet 1 of 1

JOB: 3010047806



D079 E Mouton dotPLOT 7022 PBpH7

DATE: 07/07/2023 14:15

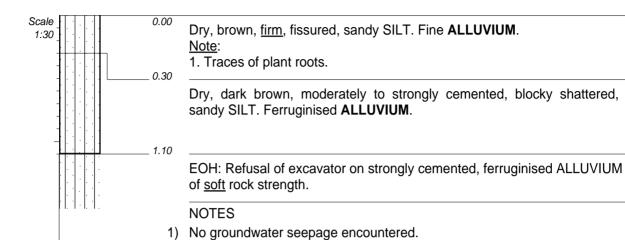
TEXT: ..51\PROFILES\PKFSWDTP.TXT



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP21 Sheet 1 of 1

JOB: 3010047806



2) Sidewalls stable.

3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 804592 Y-COORD: 7583193



TYPE SET BY: EM

SETUP FILE: KPTP8.SET

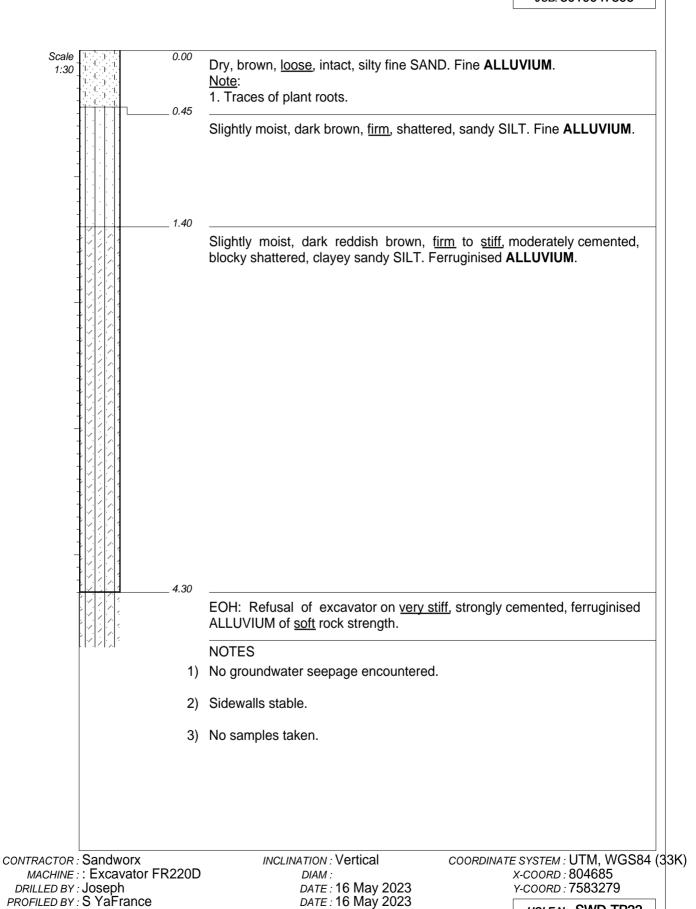
OMITIOMIRE COPPER PROJECT

GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP22 Sheet 1 of 1

JOB: 3010047806

HOLE No: SWD-TP22



D079 E Mouton dotPLOT 7022 PBpH7

DATE: 16 May 2023

DATE: 07/07/2023 14:15

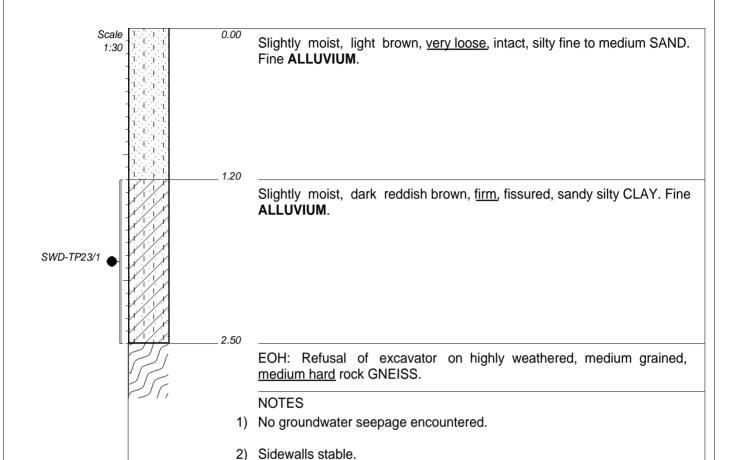
TEXT: ..51\PROFILES\PKFSWDTP.TXT



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP23 Sheet 1 of 1

JOB: 3010047806



3) Disturbed sample SWD-TP23/1 taken at 1.2m--2.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 16 May 2023

DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 801706 Y-COORD: 7581801

HOLE No: SWD-TP23

D079 E Mouton dotPLOT 7022 PBpH7



TYPE SET BY: EM

SETUP FILE: KPTP8.SET

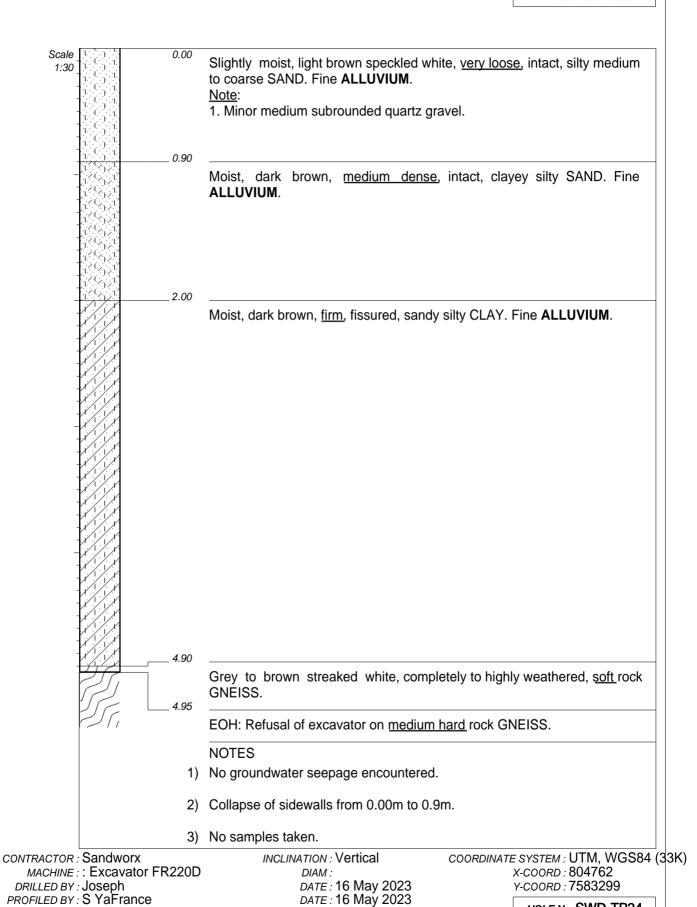
OMITIOMIRE COPPER PROJECT

GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP24 Sheet 1 of 1

JOB: 3010047806

HOLE No: SWD-TP24



D079 E Mouton dotPLOT 7022 PBpH7

DATE: 07/07/2023 14:15

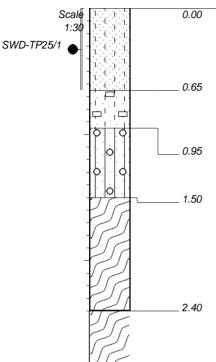
TEXT: ..51\PROFILES\PKFSWDTP.TXT



GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP25 Sheet 1 of 1

JOB: 3010047806



Slightly moist, dark brown, loose, intact, silty fine SAND. Fine ALLUVIUM.

1. Traces of plant roots.

Slightly moist, pale brown mottled white, medium dense, matrix supported, silty sandy abundant CALCRETE NODULES of soft rock strength. NODULAR CALCRETE.

Slightly moist, grey blotched white, medium dense, intact, gravelly sandy SILT. Calcareous RESIDUAL GNEISS.

Highly weathered, grey to brown streaked white, medium grained, very soft to soft rock GNEISS.

EOH: Refusal of excavator on highly weathered, grey to brown, medium grained, medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample SWD-TP25/1 taken at 0.0m--0.65m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: 16 May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

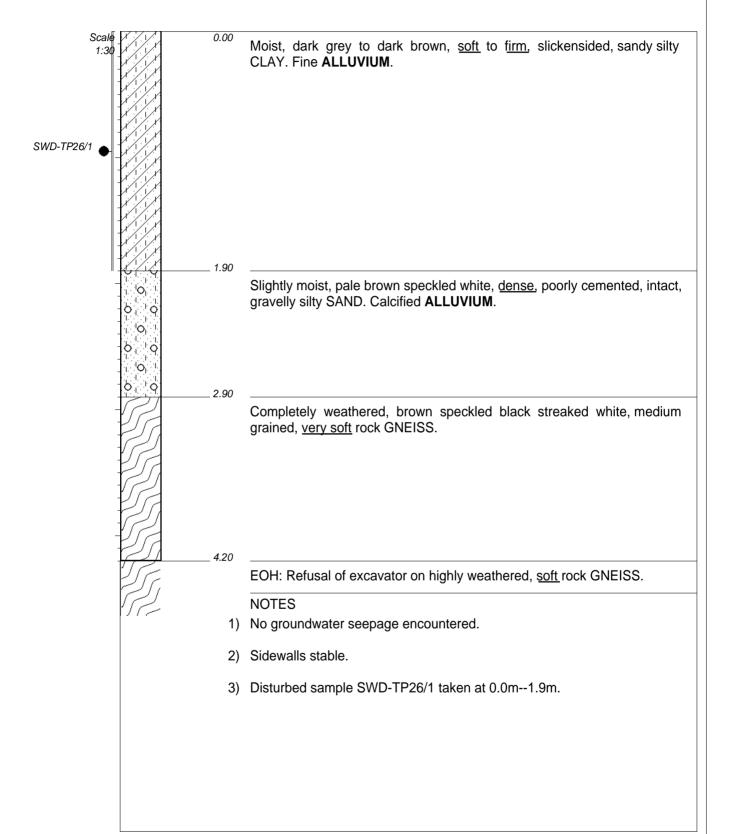
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GEOTECHNICAL INVESTIGATION

HOLE No: SWD-TP26 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 16 May 2023 DATE : 16 May 2023

DATE: 07/07/2023 14:15

TEXT: ..51\PROFILES\PKFSWDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

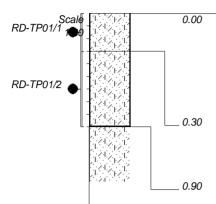
X-COORD: 803392 Y-COORD: 7581470



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP01 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, brown, <u>medium dense</u>, pinholed, clayey silty SAND, with trace angular to sub-angular, fine grained gravel of quartz. Fine **ALLUVIUM**.

Notes:

- 1. Scattered fine roots present at top 200mm.
- 2. Thick roots present throughout (20mm diameter).

Dry to slightly moist, brown to dark brown stained orange, <u>dense</u>, intact to slightly pinholed and slight shattered, clayey silty SAND, with trace angular to sub-angular fine gravel. Slightly ferruginised fine **ALLUVIUM**.

EOH: Refusal of excavator on material as above, but slightly more dense and ferruginised. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP01/1 taken at 0.0m--0.3m.
- 4) Disturbed sample RD-TP01/2 taken at 0.3m--0.9m.

CONTRACTOR: Sandworx

MACHINE:: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION : Vertical DIAM :

DIAM : DATE : May 2023 DATE : 30 May 2023

DATE: 04/07/2023 13:52 TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 0800971 Y-COORD: 7579851

HOLE No: RD-TP01



MACHINE:: Lovol Fr220D

DRILLED BY: Simon

TYPE SET BY : FM

PROFILED BY: Chris Homan

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GEOTECHNICAL INVESTIGATION

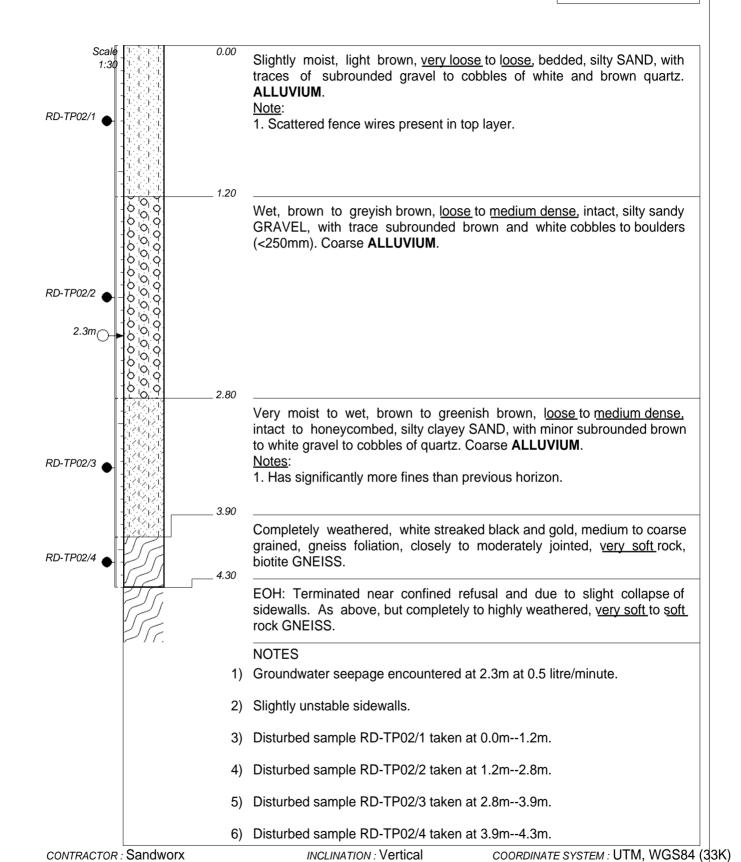
HOLE No: RD-TP02 Sheet 1 of 1

JOB: 3010047806

X-COORD: 0801584

Y-COORD: 7580544

HOLE No: RD-TP02



SETUP FILE: KPTP8.SET

D079 E Mouton

TEXT: ..P51\PROFILES\PKFRDTP.TXT

dotPLOT 7022 PBpH7

DATE: 30 May 2023

DATE: 30 May 2023

DATE: 04/07/2023 13:52

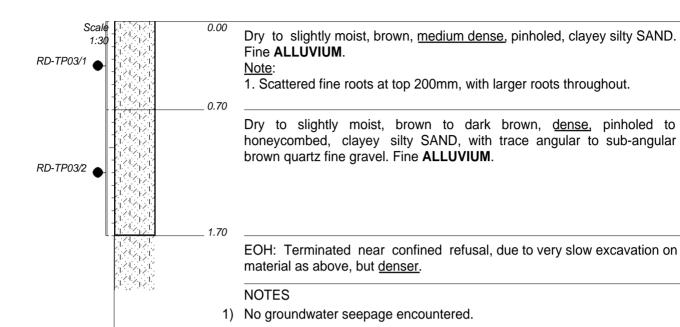
DIAM:



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP03 Sheet 1 of 1

JOB: 3010047806



2) Stable sidewalls.

- 3) Disturbed sample RD-TP03/1 taken at 0.0m--0.7m.
- 4) Disturbed sample RD-TP03/2 taken at 0,7m--1.7m.

CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 29 May 2023 DATE : 29 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

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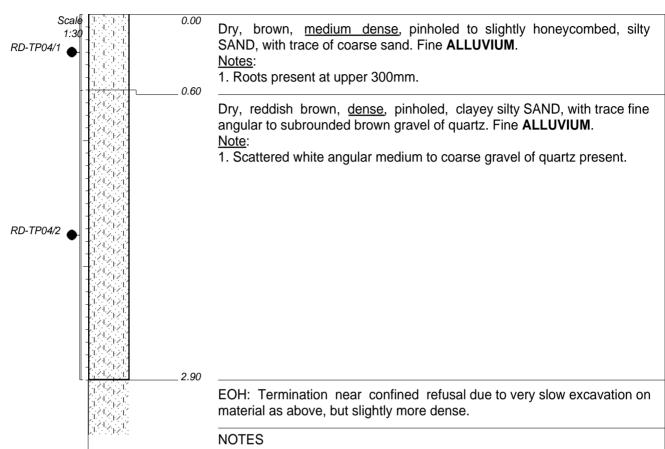
HOLE No: RD-TP03



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP04 Sheet 1 of 1

JOB: 3010047806



1) No groundwater seepage encountered.

- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP04/1 taken at 0.0m--0.6m.
- 4) Disturbed sample RD-TP04/2 taken at 0.6m--2.9m.

CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: 30 May 2023 DATE: 30 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

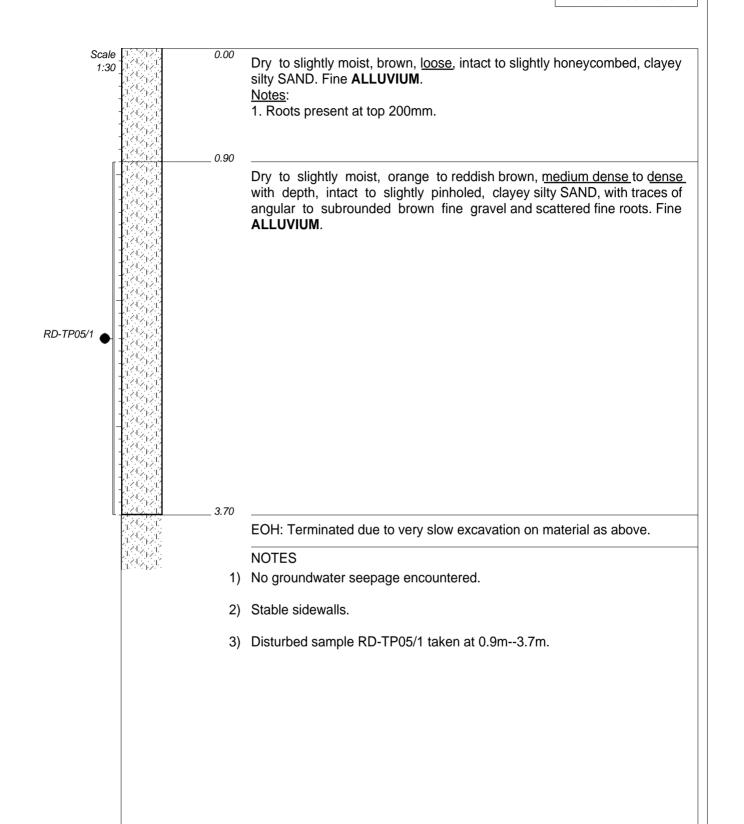
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP05 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 30 May 2023 DATE : 30 May 2023

DATE: 04/07/2023 13:52 TEXT:..P51\PROFILES\PKFRDTP.TXT

WALE W. DD TDOE

COORDINATE SYSTEM: UTM, WGS84 (33K)

Y-COORD: 7580258

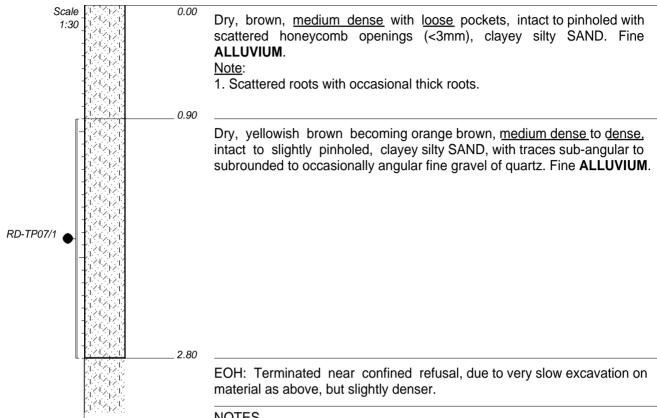
X-COORD: 08033541



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP07 Sheet 1 of 1

JOB: 3010047806



NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP07/1 taken at 0.9m--2.8m.

CONTRACTOR: Sandworx MACHINE:: Lovol Fr220D DRILLED BY: Simon PROFILED BY: Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION: Vertical**

DIAM: DATE: 29 May 2023 DATE: 29 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

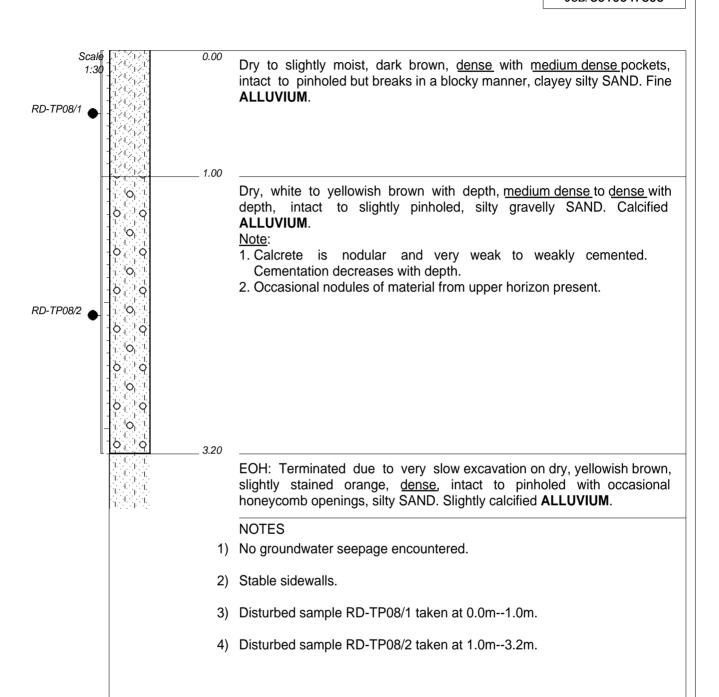
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP08 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 29 May 2023 DATE : 29 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

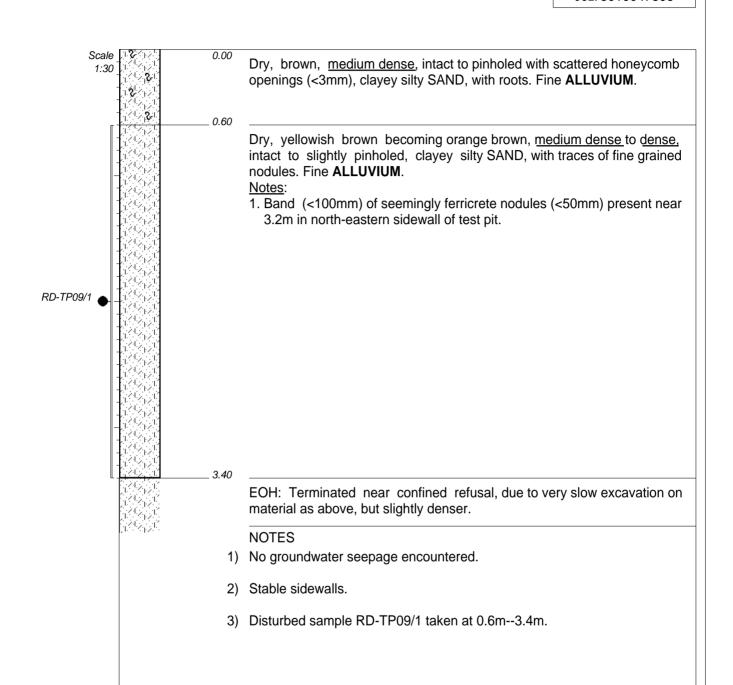
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP09 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

DIAM : DATE : 29 May 2023 DATE : 29 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 0804750 Y-COORD: 7582018

HOLE No: RD-TP09

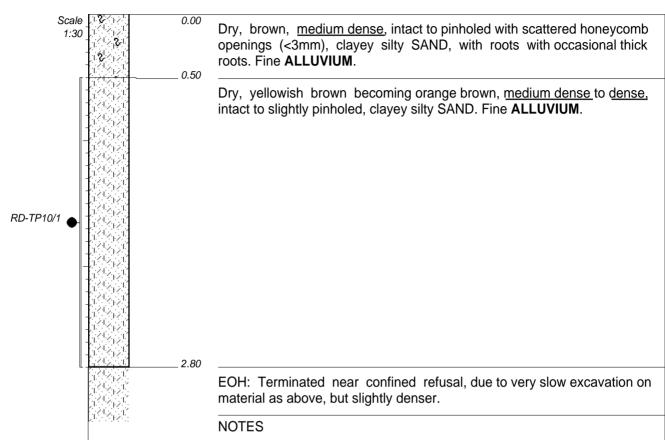
INCLINATION: Vertical



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP10 Sheet 1 of 1

JOB: 3010047806



- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP10/1 taken at 0.5m--2.8m.

CONTRACTOR: Sandworx

MACHINE:: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 29 May 2023 DATE : 29 May 2023

DATE: 04/07/2023 13:52

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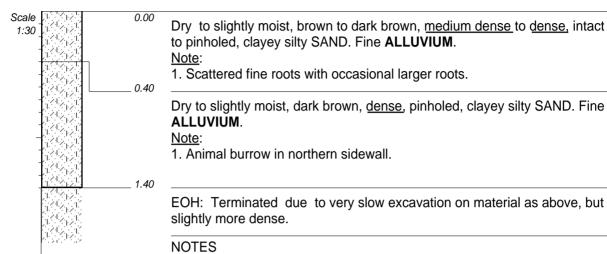
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP11
Sheet 1 of 1

JOB: 3010047806



1) No groundwater seepage encountered.

- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 27 May 2023 DATE : 27 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

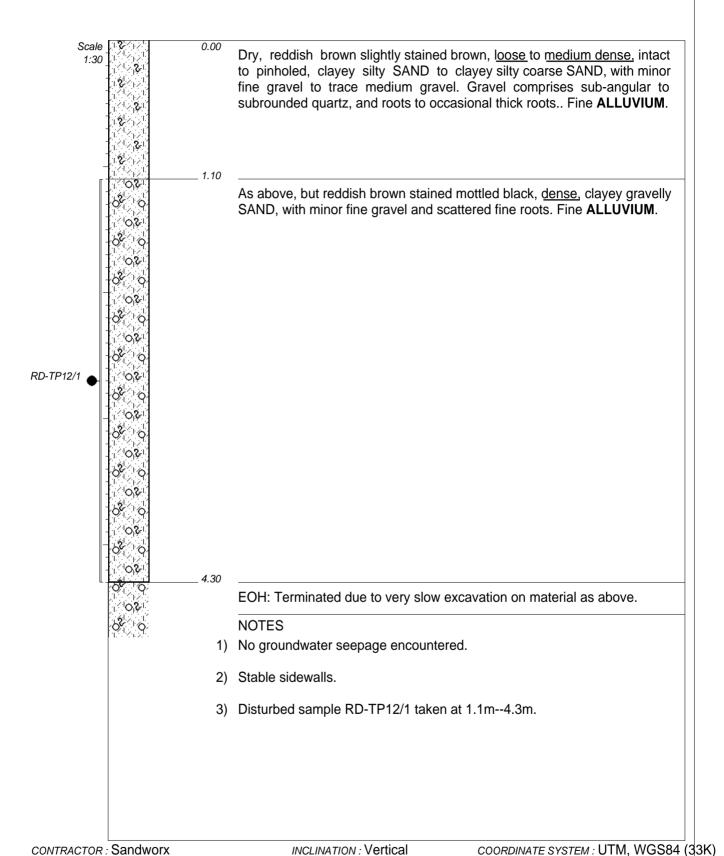
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP12 Sheet 1 of 1

JOB: 3010047806



MACHINE:: Lovol Fr220D
DRILLED BY: Simon
PROFILED BY: Chris Homan
TYPE SET BY: EM
SETUP FILE: KPTP8.SET

DIAM : Vertical
DIAM :
DATE : 27 May 2023
DATE : 27 May 2023
DATE : 04/07/2023 13:52

DATE: 04/07/2023 13:52 TEXT: ..P51\PROFILES\PKFRDTP.TXT HOLE No: RD-TP12

X-COORD: 0805797

Y-COORD: 7582981

D079 E Mouton dotPLOT 7022 PBpH7



PROFILED BY: Chris Homan

SETUP FILE: KPTP8.SET

TYPE SET BY : FM

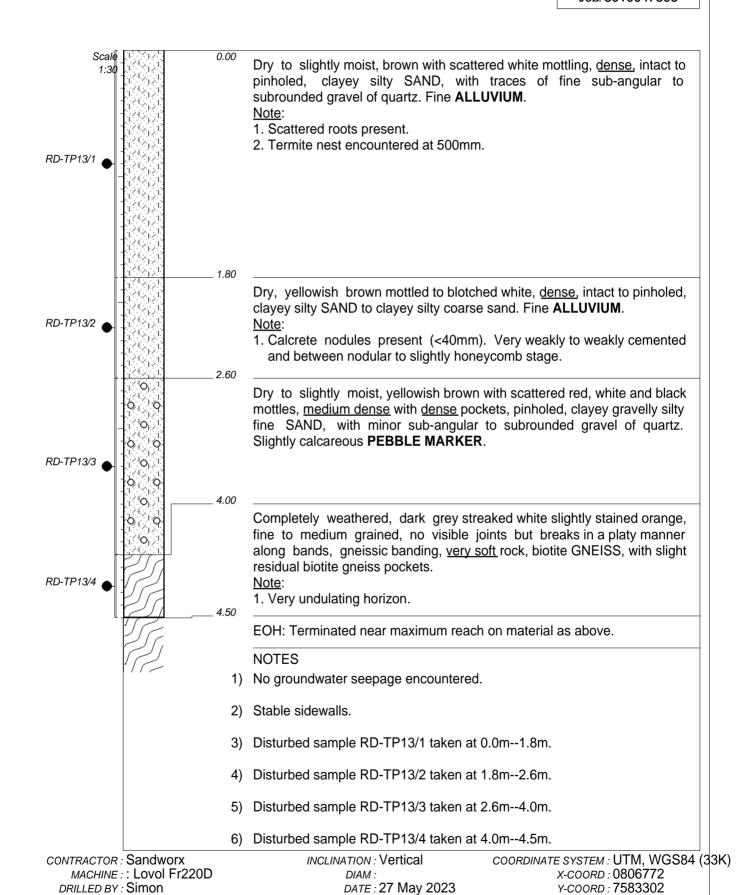
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GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP13
Sheet 1 of 1

JOB: 3010047806

HOLE No: RD-TP13



DATE: 27 May 2023

DATE: 04/07/2023 13:52

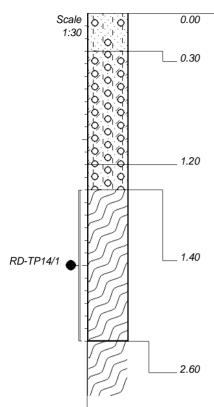
TEXT: ..P51\PROFILES\PKFRDTP.TXT



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP14
Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown, <u>dense</u>, intact to slightly pinholed, gravelly silty SAND, with trace sub-angular to subrounded gravel of guartz. Fine **ALLUVIUM**.

Dry, reddish brown to dark reddish brown, <u>dense</u>, clayey silty sandy fine GRAVEL. Gravel comprises sub-angular to subrounded with occasional angular quartz. Coarse **ALLUVIUM**.

Dry to slightly moist, reddish brown mottled black stained yellow, <u>dense</u>, matrix supported, silty sandy GRAVEL, with trace sub-angular to subrounded cobbles of quartz. Ferruginised **PEBBLE MARKER**.

Note:

1. Ferricrete nodules present (<15mm).

Completely weathered, dark grey streaked white stained orange, fine to medium grained, gneissic banding, extremely to very closely jointed, <u>very soft</u> rock, biotite GNEISS, with pockets of ferruginised residual gneiss present.

Note:

1. Horizon shows distinct signs of folding. Very undulating horizon.

EOH: Refusal of excavator on material as above, but highly to moderately weathered, <u>soft</u> to <u>medium hard</u> rock biotite GNEISS. Material breaks in platy manner.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP14/1 taken at 1.4m--2.6m.

CONTRACTOR: Sandworx

MACHINE:: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 27 May 2023 DATE: 27 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

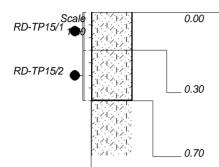
X-COORD: 0806913 Y-COORD: 7583919



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP15 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u>, intact to pinholed, clayey silty SAND. Fine **ALLUVIUM**.

Note:

1. Ant nests present in upper soils.

Dry, brown to dark brown slightly stained orange and red, <u>dense</u>, intact to pinholed with scattered openings (<2mm), clayey silty SAND. Slightly ferruginised **ALLUVIUM**.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly more dense and ferruginised. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP15/1 taken at 0.0m--0.3m.
- 4) Disturbed sample RD-TP15/2 taken at 0.3m--0.7m.

CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 27 May 2023 DATE : 27 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

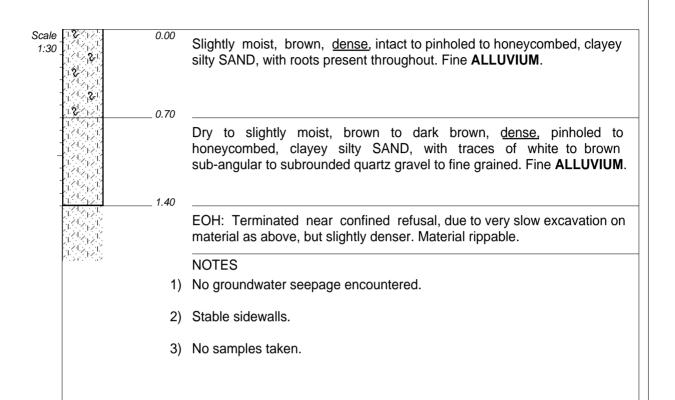
X-COORD: 0807324 Y-COORD: 7583709



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP16 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : 30 May 2023 DATE : 30 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

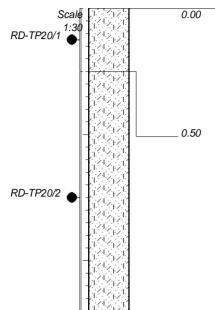
X-COORD: 0801561 Y-COORD: 7580548



GEOTECHNICAL INVESTIGATION

HOLE No: RD-TP20 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist with upper 100mm moist, brown, loose to medium dense, intact to occasionally pinholed to honeycombed, clayey silty SAND, with trace angular to sub-angular brown fine gravel of quartz. Fine **ALLUVIUM**.

Note:

- 1. Roots present in top 300mm.
- 2. Horizon moisture possibly affected by rain from previous evening.

Dry to slightly moist, reddish brown, <u>medium dense</u> with <u>dense</u> pockets, intact to pinholed, clayey silty SAND, with traces of angular to sub-angular brown to white fine gravel of quartz. Fine **ALLUVIUM**. Note:

- 1. Scattered roots present.
- 2. Scattered thick roots (<30mm diameter) present in top 500mm.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly denser.

NOTES

2.50

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample RD-TP20/1 taken at 0.0m--0.5m.
- 4) Disturbed sample RD-TP20/2 taken at 0.5m--2.5m.
- 5) Rained previous evening on site.

CONTRACTOR: Sandworx

MACHINE: Lovol Fr220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : 27 May 2023 DATE : 27 May 2023

DATE: 04/07/2023 13:52

TEXT: ..P51\PROFILES\PKFRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

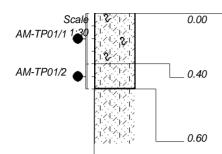
X-COORD: 0802541 Y-COORD: 7589491



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP01 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, brown to dark brown, <u>medium dense</u>, pinholed with occasional root channel openings, clayey silty SAND, with roots. Fine **ALLUVIUM**.

Dry to slightly moist, brown with orange brown staining mottled black, <u>dense</u>, pinholed to slightly honeycombed and material breaks in a blocky manner, clayey silty SAND. Slightly ferruginised fine **ALLUVIUM**.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP01/1 taken at 0.0m--0.4m.
- 4) Disturbed sample AM-TP01/2 taken at 0.4m--0.6m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Simon

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : 1200 DATE : 31 May 2023 DATE : 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

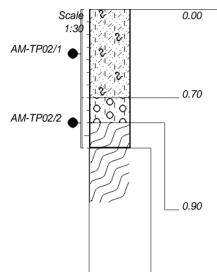
X-COORD: 0807088 Y-COORD: 7583751



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP02 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, reddish brown, <u>dense</u>, pinholed, clayey silty SAND, with traces of angular to sub-angular fine gravel of quartz, with thick roots. Fine **ALLUVIUM**.

Dry to slightly moist, reddish brown slightly mottled black stained orange, <u>medium_dense</u>, matrix supported, silty sandy GRAVEL, with trace sub-angular white quartz cobbles. Slightly ferruginised **PEBBLE MARKER**.

Note:

1. Horizon undulating, with variable thickness.

Completely weathered with highly weathered pockets, dark grey streaked white to reddish brown stained orange and black, extremely closely jointed, fine to medium grained, gneissic banding, <u>very</u> soft rock with <u>soft</u> to <u>medium hard</u> rock pockets, biotite GNEISS.

Notes:

1.10

- 1. Material is ferruginised and breaks in a platy manner.
- 2. Horizon undulating.

EOH: Refusal of excavator on material as above, but highly weathered, soft to medium hard rock, ferruginised biotite GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP02/1 taken at 0.0m--0.7m.
- 4) Disturbed sample AM-TP02/2 taken at 0.7m--1.1m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

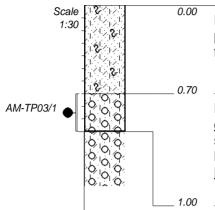
X-COORD: 0806656 Y-COORD: 7583660



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP03 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, reddish brown, <u>medium dense</u> to <u>dense</u> with depth, pinholed, clayey silty coarse SAND, with traces of angular to sub-angular fine gravel of quartz, with roots. Fine **ALLUVIUM**.

Dry to slightly moist, reddish brown with white quartz mottled black, <u>dense</u>, matrix supported, clayey silty sandy GRAVEL, with trace sub-angular cobbles of greyish white quartz. Ferruginised **PEBBLE MARKER**, with traces of calcrete.

Note:

1. Ferricrete nodules present (<20mm).

EOH: Refusal of excavator on material as above, but denser due to stronger cementation. Ferruginised / calcified **PEBBLE MARKER**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP03/1 taken at 0.7m--1.0m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

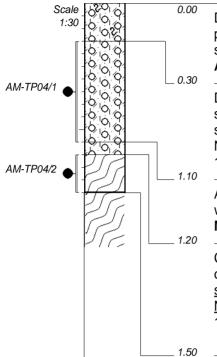
X-COORD: 0806392 Y-COORD: 7583799



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP04 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, brown to reddish brown, <u>medium dense</u> to <u>dense</u>, pinholed with open root channels, clayey silty sandy GRAVEL, with minor sub-angular to subrounded cobbles of quartz, with roots. Coarse **ALLUVIUM**.

Dry to slightly moist, reddish brown blotched khaki, <u>dense</u>, matrix supported, clayey silty sandy GRAVEL, with minor sub-angular to subrounded cobbles of quartz. Calcareous **ALLUVIUM**.

1. Calcrete nodules present (<60mm).

As above, but silty sandy GRAVEL, with minor sub-angular to subrounded white to purple orange dark grey quartz cobbles. Calcified **PEBBLE MARKER**.

Completely to highly weathered, white speckled to streaked black stained orange, medium to coarse grained, gneissic banding, closely jointed, <u>very soft</u> rock biotite GNEISS.

Notes:

1. Brown clayey silty sand infilling (<5mm to pockets). Prominent feldspar pockets in places (<40mm).

EOH: Refusal of excavator on material as above, but highly weathered, <u>soft</u> rock biotite GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP04/1 taken at 0.3m--1.1m.
- 4) Disturbed sample AM-TP04/2 taken at 1.2m--1.5m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

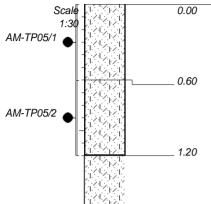
X-COORD: 0806025 Y-COORD: 7583714



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP05 Sheet 1 of 1

JOB: 3010047806



Slightly moist, brown, <u>medium dense</u> with <u>dense</u> pockets, pinholed to occasionally honeycombed, clayey silty SAND. Fine **ALLUVIUM**. Note:

1. Roots present at top 300mm.

Slightly moist, brown mottled dark brown, <u>dense</u>, intact to pinholed with occasional honeycombed openings but breaks in a blocky manner, clayey silty SAND. Fine **ALLUVIUM**.

EOH: Terminated due to very slow excavation on material as above, but slightly more dense.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP05/1 taken at 0.0m--0.6m.
- 4) Disturbed sample AM-TP05/2 taken at 0.6m--1.2m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM : 1200 DATE : 31 May 2023 DATE : 30 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

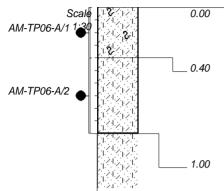
X-COORD: 0806080 Y-COORD: 7583500



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP06-A Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, brown, <u>medium dense</u>, pinholed with occasional open root channels, clayey silty SAND, with traces of angular to sub-angular fine gravel of quartz, with roots. Fine **ALLUVIUM**.

Dry to slightly moist, reddish brown, <u>dense</u>, pinholed to honeycombed, clayey silty coarse SAND, with trace sub-angular to subrounded brown to white fine gravel of quartz. Fine **ALLUVIUM**. Note:

1. Scattered roots present.

EOH: Terminated due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP06-A/1 taken at 0.0m--0.4m.
- 4) Disturbed sample AM-TP06-A/2 taken at 0.4m--1.0m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 30 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

X-COORD: 0806402 Y-COORD: 7583603

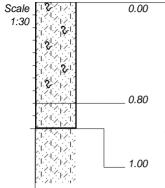
HOLE No: AM-TP06-A



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP06-B Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u> to <u>dense</u>, pinholed with scattered openings (>2mm), clayey silty SAND, with roots. Fine **ALLUVIUM**.

Dry to slightly moist, dark brown mottled orange red, <u>dense</u>, pinholed with very slightly polished surfaces and material breaks in a blocky manner, clayey silty SAND. Fine **ALLUVIUM**.

EOH: Terminated due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

CONTRACTOR: Sandworx

MACHINE:: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : 1200 DATE : 31 May 2023 DATE : 30 May 2023

DATE: 04/07/2023 13:47 TEXT:...P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM,WGS84(33K)

X-COORD: 0806389 Y-COORD: 7583550

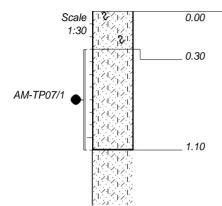
HOLE No: AM-TP06-B



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP07 Sheet 1 of 1

JOB: 3010047806



Dry, brown, medium dense to dense, intact to pinholed, clayey silty SAND, with fine roots. Fine ALLUVIUM.

Dry to slightly moist, dark brown, dense, pinholed and slickensided and material breaks in a blocky manner, clayey silty SAND. Fine **ALLUVIUM**.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- Stable sidewalls.
- 3) Disturbed sample AM-TP07/1 taken at 0.3m--1.1m.

CONTRACTOR: Sandworx MACHINE:: Lovol FR220D DRILLED BY: Joseph Hosain PROFILED BY . Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION: Vertical**

DIAM: 1200 DATE: 31 May 2023 DATE: 30 May 2023

DATE: 04/07/2023 13:47 TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K) X-COORD: 0806585

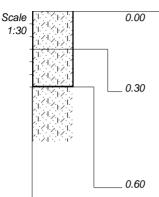
Y-COORD: 7583438



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP08-A Sheet 1 of 1

JOB: 3010047806



Dry, brown slightly stained orange, <u>dense</u> with <u>medium dense</u> pockets, intact to pinholed, clayey silty coarse SAND. Fine **ALLUVIUM**. Note:

1. Scattered fine roots present.

Dry to slightly moist, brown to dark brown, <u>dense</u>, pinholed and material breaks in a blocky manner, clayey silty SAND, with traces of angular to sub-angular coarse sand of quartz. Fine **ALLUVIUM**. Note:

1. Scattered roots present.

EOH: Terminated due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

CONTRACTOR: Sandworx

MACHINE:: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

X-COORD: 0807041 Y-COORD: 7583485

HOLE No: AM-TP08-A

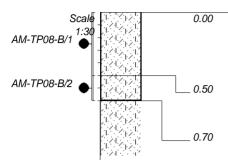
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP08-B Sheet 1 of 1

JOB: 3010047806



Moist, brown to dark brown, <u>medium dense</u>, pinholed to blocky, silty clayey SAND. Fine **ALLUVIUM**. Note:

1. Scattered fine roots present.

Moist, dark brown with orange staining, <u>dense</u>, intact to pinholed, clayey silty SAND. Fine **ALLUVIUM**.

EOH: Terminated due to very slow excavation in material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Sample AM-TP08-B/1 taken at 0.0m--0.5m.
- 4) Sample AM-TP08-B/2 taken at 0.5m--0.7m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47

TEXT: ..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

X-COORD: 0807035 Y-COORD: 7583446

HOLE No: AM-TP08-B

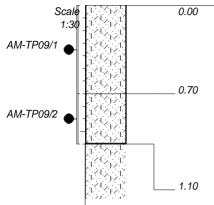
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP09 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u> with <u>dense</u> pockets, pinholed to honeycombed and breaks in a blocky manner, clayey silty SAND. Fine **ALLUVIUM**. Note:

1. Slightly undulating horizon.

Slightly moist to moist, brown mottled orange and black, <u>dense</u>, pinholed, clayey silty SAND. Fine **ALLUVIUM**. <u>Note</u>:

- 1. Seemingly very slightly ferruginised.
- 2. Ferricrete nodules present (<3mm).

EOH: Terminated due to very slow excavation on material as above, but denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample AM-TP09/1 taken from 0.0m--0.7m.
- 4) Disturbed sample AM-TP09/2 taken from 0.7m--1.1m.

CONTRACTOR: Sandworx

MACHINE: Lovol FR220D

DRILLED BY: Joseph Hosain

PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: 1200 DATE: 31 May 2023 DATE: 31 May 2023

DATE: 04/07/2023 13:47 TEXT:..P51\PROFILES\PKFAMTP.TXT

COORDINATE SYSTEM: UTM, WGS84(33K)

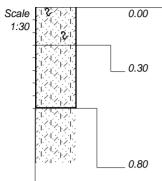
X-COORD: 0807324 Y-COORD: 7583323



GEOTECHNICAL INVESTIGATION

HOLE No: AM-TP10 Sheet 1 of 1

JOB: 3010047806



Dry, brown, medium dense to dense with depth, pinholed and breaks in a blocky manner, clayey silty SAND, with traces of coarse sand of quartz, and roots. Fine ALLUVIUM.

Dry to slightly moist, brown to dark brown, dense, pinholed, clayey silty SAND, with trace of coarse sand to fine gravel of sub-angular to subrounded quartz. Fine ALLUVIUM. Note:

1. Scattered fine roots present.

EOH: Terminated near confined refusal, due to very slow excavation on material as above, but slightly denser. Material rippable.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

CONTRACTOR: Sandworx MACHINE:: Lovol FR220D DRILLED BY: Joseph Hosain PROFILED BY . Chris Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION: Vertical**

DIAM: 1200 DATE: 31 May 2023 DATE . 31 May 2023

DATE: 04/07/2023 13:47 TEXT: ..P51\PROFILES\PKFAMTP.TXT

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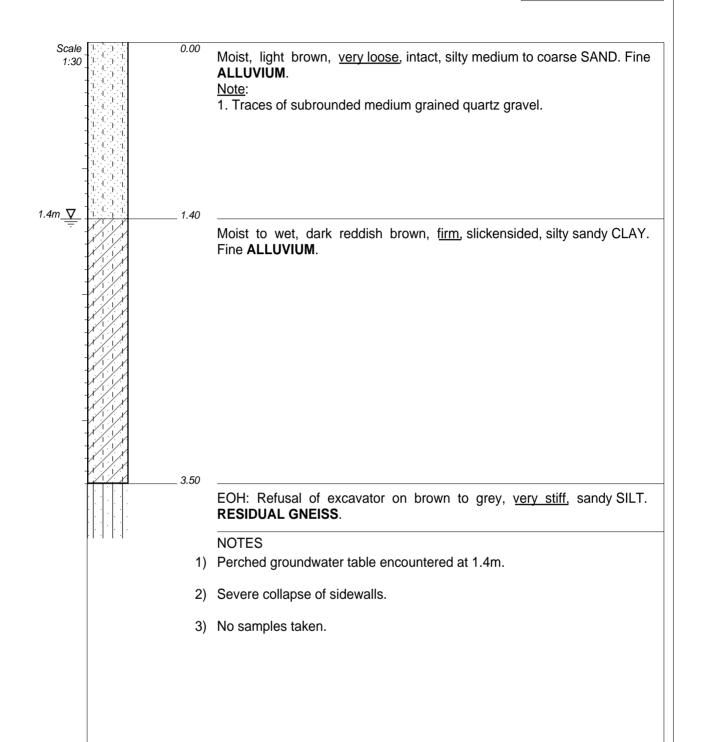
Y-COORD: 7583630



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP01 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: 13 May 2023 DATE: 13 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

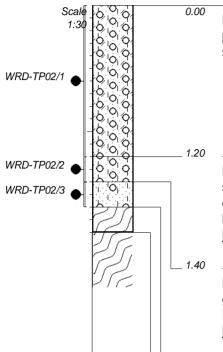
X-COORD: 801782.00 Y-COORD: 7582015.00



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP02 Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown becoming yellowish brown with depth, <u>medium dense</u>, pinholed, clayey silty sandy fine GRAVEL, with sub-angular to subrounded brown gravel of guartz. Coarse **ALLUVIUM**.

Dry, brown mottled black blotched white stained orange, <u>dense</u>, matrix supported, silty sandy GRAVEL, with trace sub-angular to subrounded cobbles and boulders of quartz, ferruginised and calcified **PEBBLE MARKER**.

Note:

1. Ferricrete nodules present (<20mm).

Dry, white to grey mottled black stained orange, <u>dense</u>, intact, silty gravelly SAND, with trace sub-angular to subrounded quartz gravel. Reworked calcified **RESIDUAL GNEISS**.

Note:

1.60

1.80

1. Ferricrete nodules present (<10mm).

Completely weathered, white speckled to streaked black stained orange, medium to coarse grained, gneissic banding, widely jointed, <u>very soft</u> rock biotite GNEISS.

EOH: Refusal of excavator on highly to moderately weathered, <u>soft</u> rock biotite GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP02/1 taken at 0.0m--1.2m.
- 4) Disturbed sample WRD-TP02/2 taken at 1.2m--1.4m.
- 5) Disturbed sample WRD-TP02/3 taken at 1.4m--1.6m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: May 2023

DATE: 26 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

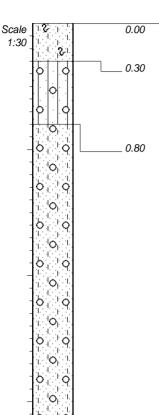
X-COORD: 0802343 Y-COORD: 7582645



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP03 Sheet 1 of 1

JOB: 3010047806



Slightly moist, pale brown, <u>loose</u>, intact, silty fine SAND, with plant roots and biotic voids. **ALLUVIUM**.

Slightly moist, pale brown speckled white, <u>firm</u> to <u>stiff</u>, intact, gravelly sandy SILT. Gravel comprises weakly to moderately well cemented calcrete nodules and up to 40mm diameter angular quartz. Calcified **ALLUVIUM**.

Slightly moist becoming dry, pale orange brown, firm to stiff with loose pockets, intact, gravelly silty SAND. Ferruginous and irregularly calcified **ALLUVIUM**.

EOH: Refusal of excavator in material as above, but becoming <u>very stiff</u> (before teeth change).

NOTES

3.30

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.
- 4) Excavator teeth are not sharp causing premature refusal.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023

DATE: 10 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

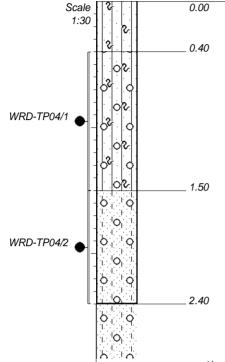
X-COORD: 80233280 Y-COORD: 7582281.00



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP04 Sheet 1 of 1

JOB: 3010047806



Slightly moist, pale brown, <u>firm</u>, shattered, slightly gravelly sandy SILT, with plant roots. **ALLUVIUM**.

Slightly moist to dry, pale brown mottled white, <u>stiff</u>, intact, gravelly (calcrete nodules) fine to medium sandy SILT, with plant roots. Calcified **ALLUVIUM**.

Slightly moist, orange brown, <u>medium dense</u> with <u>dense</u> pockets, intact, gravelly silty SAND. **ALLUVIUM**, with pockets and lenses of very fine grained laminated deposits and coarse grained sand pockets.

EOH: Refusal of excavator in dense material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) No further penetration by excavator (before teeth change).
- 3) Disturbed samples WRD-TP04/1 taken from 0.4m--1.5m.
- 4) Disturbed samples WRD-TP04/2 taken from 1.5m--2.4m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY : B Klaas

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 10 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

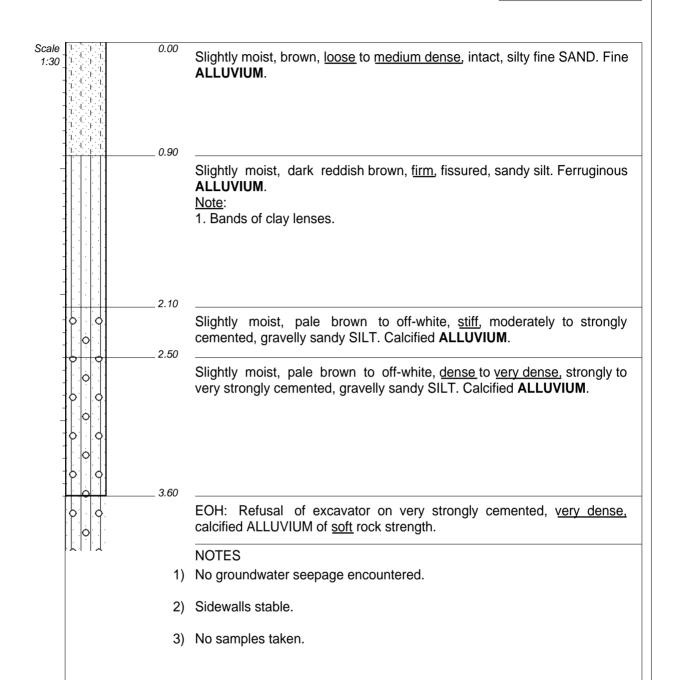
x-coord: 801993.39 y-coord: 7582129.46



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP05 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 13 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

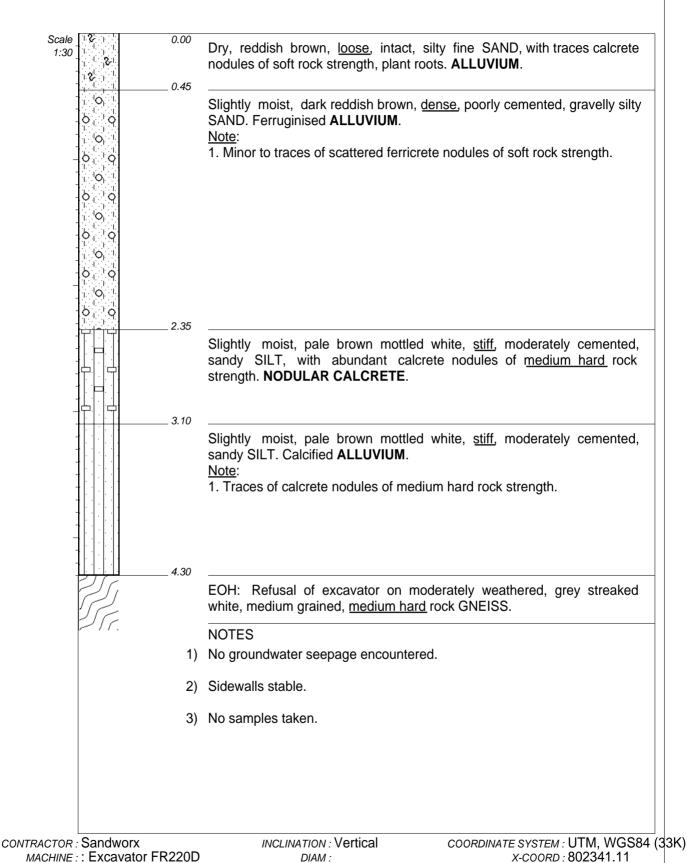
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GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP06 Sheet 1 of 1

JOB: 3010047806



DRILLED BY: Joseph
PROFILED BY: S YaFrance

DATE: May 2023
DATE: 13 May 2023

 TYPE SET BY : EM
 DATE : 07/07/2023 14:24

 SETUP FILE : KPTP8.SET
 TEXT : ..51\PROFILES\PKFWRDTP.TXT

07/07/2023 14:24 HOLE No: WRD-TP06

Y-COORD: 7582119.35

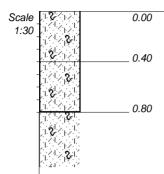
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP07 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u> to <u>dense</u> with depth, intact with scattered openings, clayey silty SAND, with roots. Fine **ALLUVIUM**.

Dry, brown to dark brown, <u>dense</u>, intact but breaks in a blocky nature, clayey silty coarse SAND, with very fine roots. Fine **ALLUVIUM**.

EOH: Terminated due to very slow excavation on material as above, but tending to <u>very dense</u>.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D DRILLED BY: Joseph

PROFILED BY : C Homan

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 26 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD : 0802532 *Y-COORD* : 7581769

HOLE No: WRD-TP07

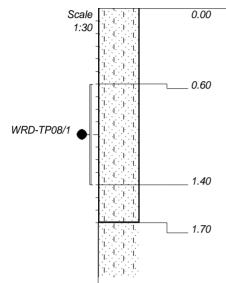
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP08 Sheet 1 of 1

JOB: 3010047806



Slightly moist, pale brown, loose to medium dense, intact, slightly gravelly silty SAND. **ALLUVIUM**.

Note:

Gravel up to 50mm diameter rounded gneiss fragments.

Slightly moist, pale brown speckled white, <u>medium dense</u>, intact, silty fine SAND, with minor fine calcrete gravel. Calcified **ALLUVIUM**

Slightly moist, pale orange brown, <u>medium dense</u> becoming <u>dense</u>, silty SAND. Ferruginous and calcified **ALLUVIUM**.

EOH: Refusal of excavator in dense material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP08/1 taken from 0.6m--1.4m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : **May 2023**

DATE: 10 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

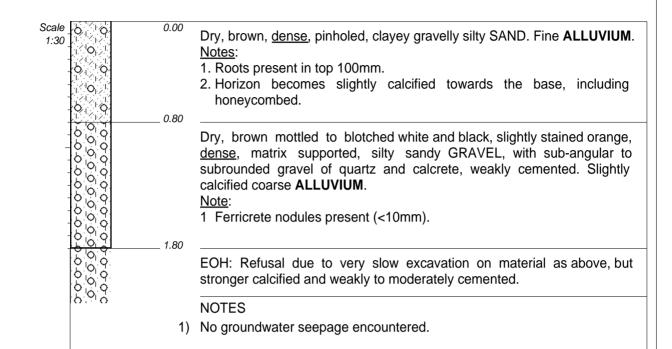
X-COORD : 802820.40 Y-COORD : 7582154.58



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP09 Sheet 1 of 1

JOB: 3010047806



2) Stable sidewalls.

3) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: **DATE:** May 2023

DATE: 26 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

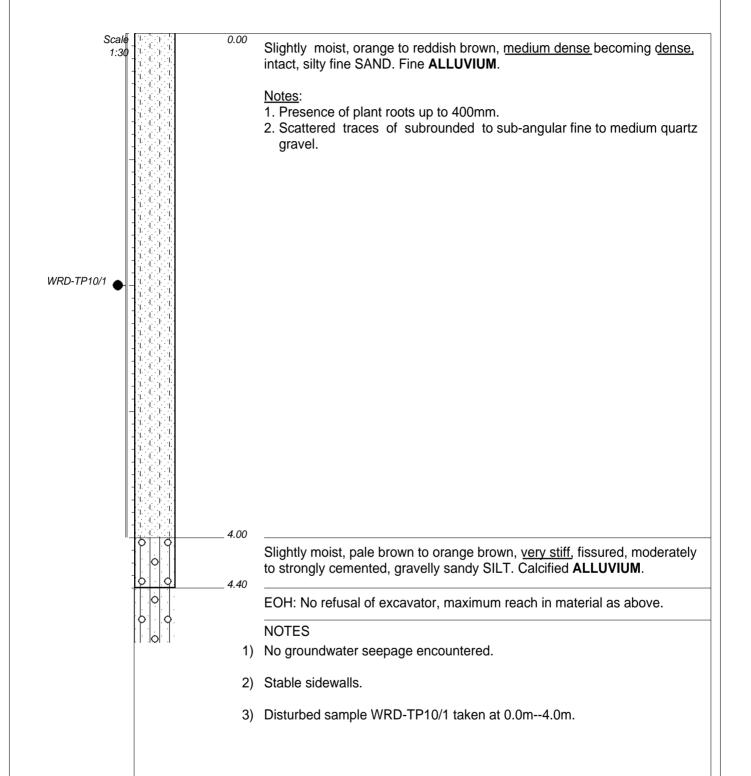
X-COORD: 0802802 Y-COORD: 7582018



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP10 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

SETUP FILE: KPTP8.SET

TYPE SET BY: EM

INCLINATION: Vertical

DIAM: DATE: May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

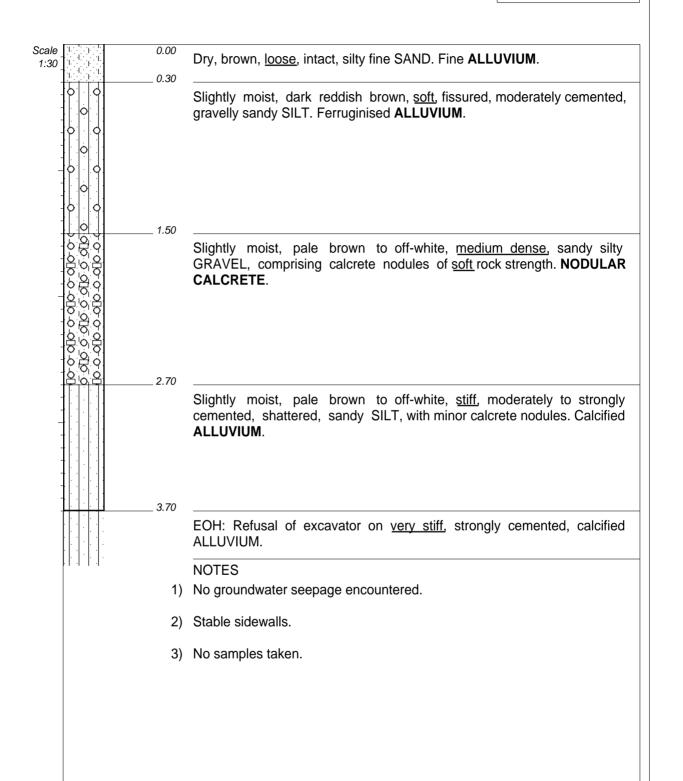
X-COORD : 803055.00 Y-COORD : 7581743.00



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP11 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM SETUP FILE: KPTP8.SET INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

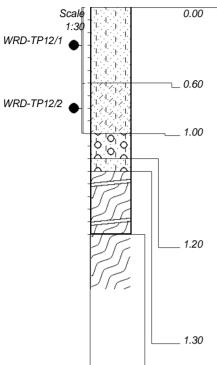
X-COORD: 803420.00 Y-COORD: 7581961.00



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP12 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, light brown, very loose, medium bedded, silty SAND. ALLUVIUM.

Note:

1. Roots present in top 200mm.

Dry to slightly moist, dark brown, medium dense, intact to root voided, clavev silty SAND. ALLUVIUM.

Dry to slightly moist, brown, slightly speckled white stained orange, medium dense to dense, matrix supported, silty sandy GRAVEL, with trace sub-angular to subrounded cobbles of quartz. Slightly ferruginous PEBBLE MARKER.

Note:

1. Occasional ferricrete nodules present.

Dry, white to grey mottled black stained orange, dense, intact, silty gravelly SAND, with trace sub-angular to subrounded quartz gravel. Reworked ferruginised RESIDUAL GNEISS.

Note:

1.80

1. Ferricrete nodules present (<10mm).

Completely to highly weathered, white speckled to streaked black stained orange, medium to coarse grained, gneissic bands, widely jointed, very soft to soft rock with pockets of medium hard rock, biotite GNEISS.

EOH: Refusal of excavator on highly to moderately weathered, medium hard rock biotite GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP12/1 taken at 0.0m--0.6m.
- 4) Disturbed sample WRD-TP12/2 taken at 0.6m--1.0m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY : FM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 26 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 0803082 Y-COORD: 7582525

HOLE NO: WRD-TP12

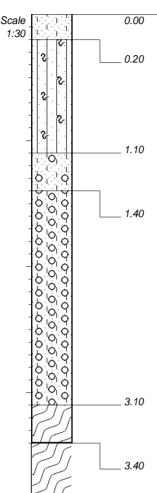
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP13
Sheet 1 of 1

JOB: 3010047806



Slightly moist, brown, loose to medium dense, intact, slightly gravelly silty SAND. **ALLUVIUM**.

Slightly moist, pale orange brown, <u>stiff</u>, shattered/blocky, fine sandy SILT, with plant roots. **ALLUVIUM**.

Slightly moist, pale brown, <u>loose</u> to <u>medium dense</u>, intact, gravelly silty SAND. Gravel comprises calcrete nodules and sub-angular to subrounded quartz gravel. Calcified **ALLUVIUM**.

Slightly moist, pale orange brown, <u>medium dense</u>, intact and bedded, clast supported, silty sandy GRAVEL, with pockets of loose gravelly SAND-SILT. Gravel comprises sub-angular to rounded quartz and gneiss gravel and small cobbles up to 60mm diameter. Coarse **ALLUVIUM**. Note:

Significant gravel increase from 2.4m.

Highly weathered, grey speckled black and white and gold stained orange along joints, very closely jointed, gneissic, <u>very soft</u> rock becoming <u>soft</u> rock granitic GNEISS with depth.

EOH: Refusal of excavator on <u>soft</u> rock tending to <u>medium hard</u> rock granitic GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Test pit sidewall depths differ.
- 4) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: B Klaas

PROFILED BY : B Klaas TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 11 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 803424.70 Y-COORD: 7582333.22



TYPE SET BY: EM

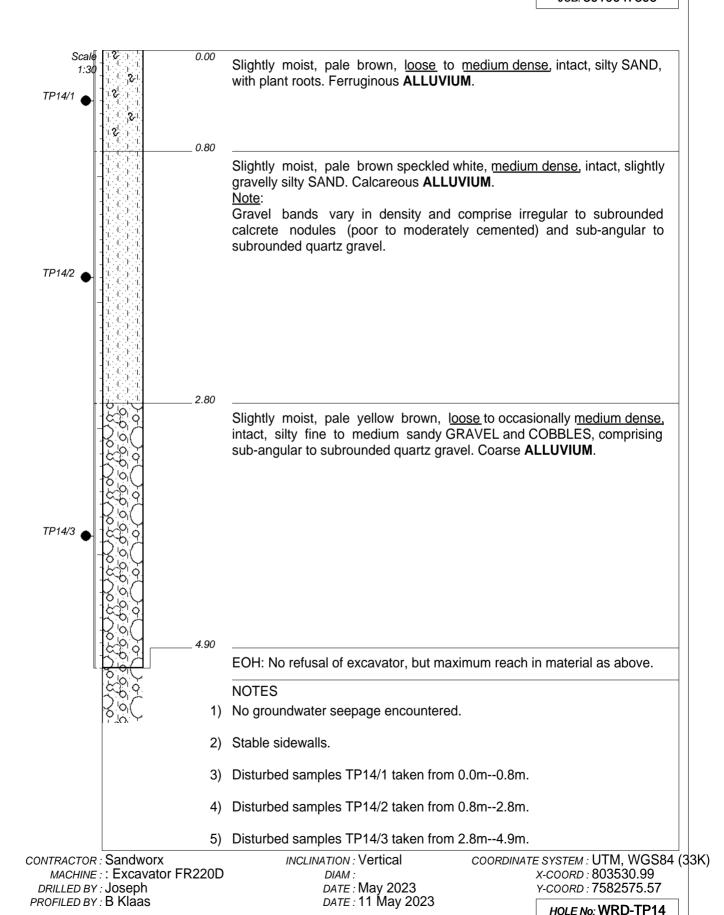
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OMITIOMIRE COPPER PROJECT

GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP14 Sheet 1 of 1

JOB: 3010047806



D079 E Mouton dotPLOT 7022 PBpH7

DATE: 07/07/2023 14:24

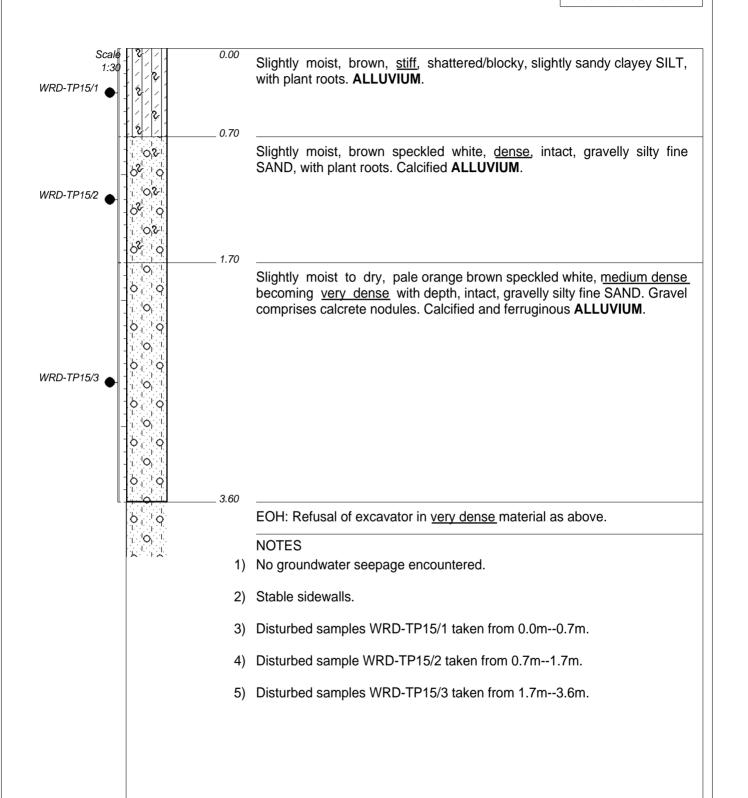
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GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP15 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY B Klaas

TYPE SET BY : EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 11 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 803953.75 Y-COORD: 7582815.05



TYPE SET BY: EM

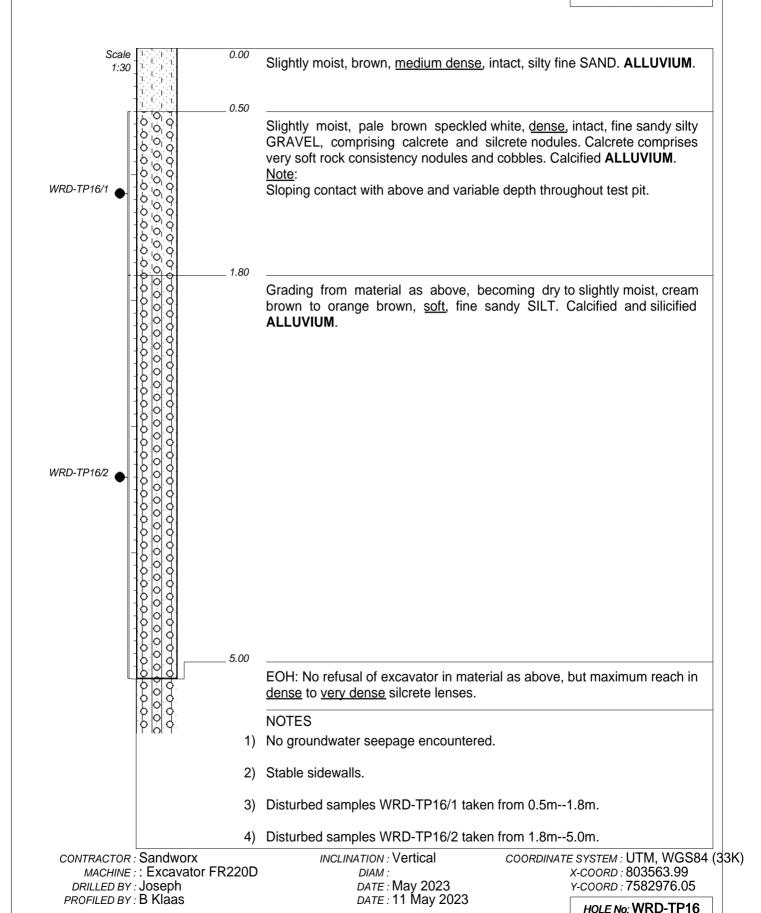
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OMITIOMIRE COPPER PROJECT

GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP16 Sheet 1 of 1

JOB: 3010047806



DATE: 07/07/2023 14:24

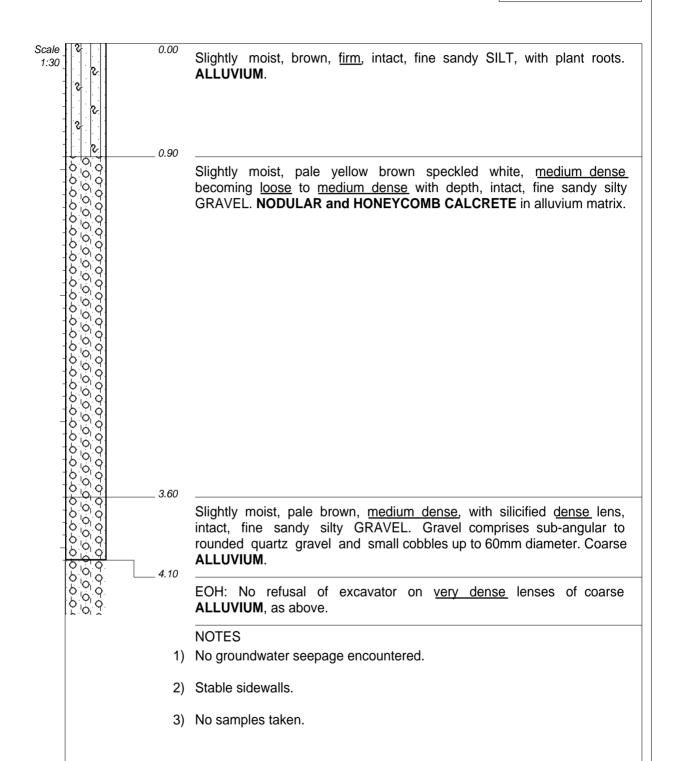
TEXT: ..51\PROFILES\PKFWRDTP.TXT



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP17 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: B Klaas

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION: Vertical**

DIAM: **DATE:** May 2023

DATE: 11 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

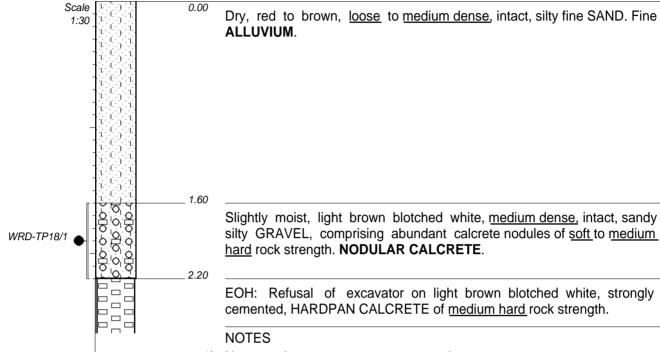
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GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP18 Sheet 1 of 1

JOB: 3010047806



- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed samples WRD-TP18/1 taken at 1.6m--2.2m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 13 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

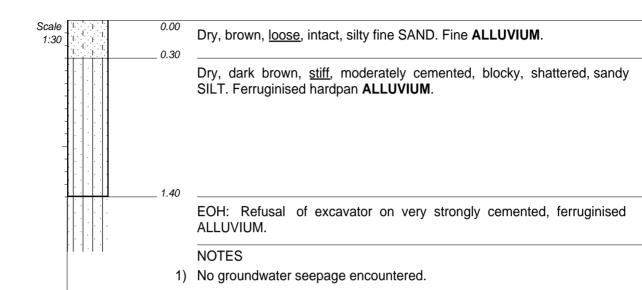
x-coord: 804076.74 Y-COORD: 7583671.36



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP19 Sheet 1 of 1

JOB: 3010047806



2) Stable sidewalls.

3) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: B Klaas

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 13 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

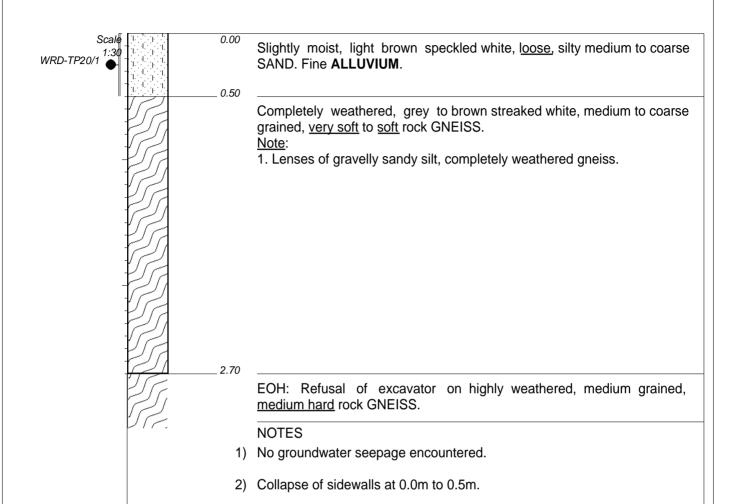
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GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP20 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: **DATE:** May 2023

3) Disturbed samples WRD-TP20/1 taken at 0.0m--0.5m.

DATE: 16 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

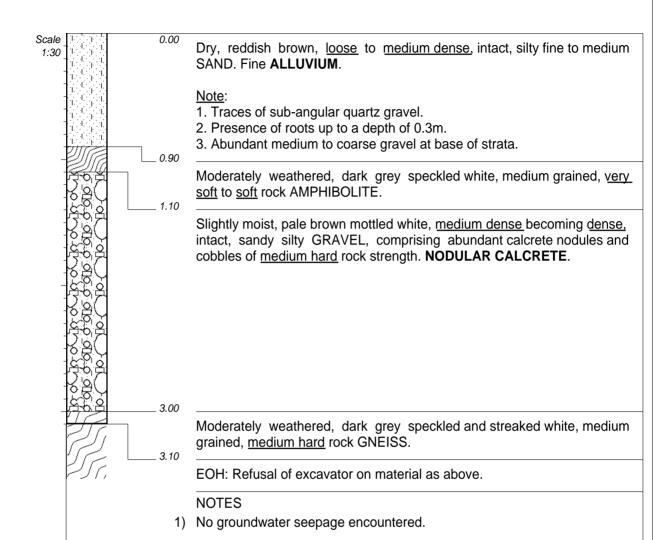
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GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP21 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

2) Stable sidewalls.

3) No samples taken.

DIAM:

DATE: May 2023

DATE: 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

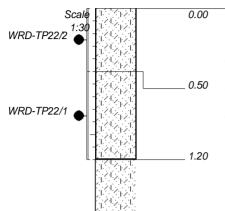
x-coord: 804187.03 Y-COORD: 7584237.04



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP22 Sheet 1 of 1

JOB: 3010047806



Dry, dark brown, $\underline{\text{medium dense}}$, intact to root voided, clayey silty SAND. Fine **ALLUVIUM**.

Note:

1. Roots present in top 100mm.

Dry, dark brown speckled white, <u>dense</u>, root voided, clayey silty SAND, with traces of coarse sand. Fine **ALLUVIUM**.

EOH: Terminated due to very slow excavation on material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP22/1 taken at 0.5m--1.2m.
- 4) Disturbed sample WRD-TP22/2 taken at 0.0m--0.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 0805033 Y-COORD: 75824029

HOLE No: WRD-TP22

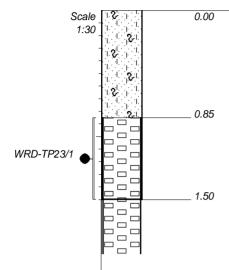
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP23 Sheet 1 of 1

JOB: 3010047806



Dry to slightly moist, dark brown, loose to $medium\ dense$, intact, silty fine SAND. Fine **ALLUVIUM**.

Notes:

1. Presence of plant roots.

Slightly moist, pale brown blotched white, moderately to strongly cemented, HARDPAN CALCRETE of <u>very soft</u> to <u>soft</u> rock strength.

EOH: Refusal of excavator on strongly to very strongly cemented, HARDPAN CALCRETE of medium hard rock strength.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed samples WRD-TP23/1 taken at 0.85m--1.50m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET **INCLINATION:** Vertical

DIAM: DATE: May 2023 DATE: 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

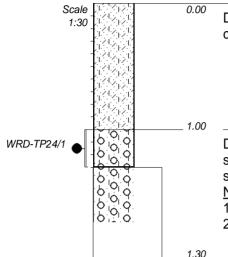
X-COORD: 804153.99 Y-COORD: 7584615.84



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP24 Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown to orange brown with depth, loose to medium dense, clayey silty coarse SAND. Fine **ALLUVIUM**.

Dry, reddish brown to brown mottled black and white, <u>dense</u>, matrix supported, silty sandy GRAVEL, with minor grey sub-angular to subrounded cobbles of quartz. Ferruginous **PEBBLE MARKER**. Notes:

- 1. Ferricrete nodules present (<30mm) with calcrete nodules (<10mm).
- 2. Some degree of nodular to honeycomb ferricrete present of weakly cementation.

EOH: Refusal of excavator on material as above, but <u>dense</u> and moderately to strongly cemented. Ferruginised **PEBBLE MARKER**.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP24/1 taken at 1.0m--1.3m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

SETUP FILE : KPTP8.SET

TYPE SET BY : EM

INCLINATION: Vertical

DIAM: DATE: May 2023

DATE: 25 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

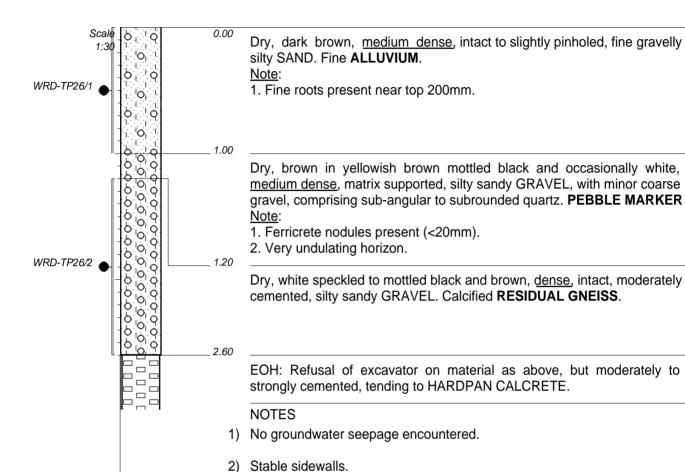
X-COORD: 0805039 Y-COORD: 7584921



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP26 Sheet 1 of 1

JOB: 3010047806



- 3) Disturbed sample WRD-TP26/1 taken at 0.0m--1.0m.
- 4) Disturbed sample WRD-TP26/2 taken at 1.2m--2.6m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 25 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

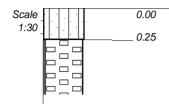
X-COORD: 0803030 Y-COORD: 7584941



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP27 Sheet 1 of 1

JOB: 3010047806



Slightly moist, dark brown, soft, shattered, sandy SILT. ALLUVIUM.

EOH: Refusal of excavator on light brown stained white, strongly cemented, HARDPAN CALCRETE, of <u>medium hard</u> rock consistency.

NOTES

- 1) No groundwater seepage encountered.
- 2) Sidewalls stable.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: May 2023 DATE: 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

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HOLE No: WRD-TP27

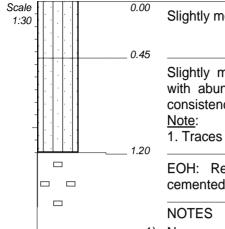
D079 E Mouton dotPLOT 7022 PBpH7



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HOLE No: WRD-TP28 Sheet 1 of 1

JOB: 3010047806



Slightly moist, dark brown, soft to firm, shattered, sandy SILT. ALLUVIUM.

Slightly moist, pale brown blotched white, sandy SILT, matrix supported, with abundant calcrete nodules and COBBLES of medium hard rock consistency. **NODULAR CALCRETE**.

1. Traces of calcrete boulders of hard rock consistency at base of strata.

EOH: Refusal of excavator on pale brown stained white, strongly cemented, HONEYCOMB CALCRETE, of medium hard rock consistency.

- 1) No groundwater seepage encountered.
- 2) Collapse of sidewalls at 0.45m to 0.8m.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY: EM
SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

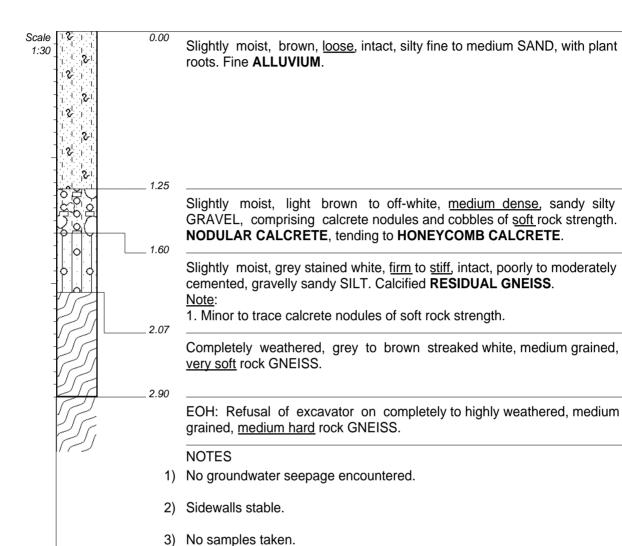
X-COORD: 802706.77 Y-COORD: 7586159.95



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP29 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

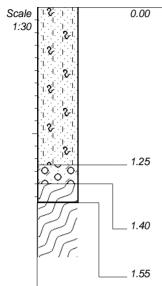
X-COORD: 803562.23 Y-COORD: 7586522.25



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP30 Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown, $\underline{\text{medium dense}}$, intact, silty fine to medium SAND, with plant roots. Fine $\underline{\text{ALLUVIUM}}$.

Note:

1. Traces of clay lenses at the base of strata.

Slightly moist, dark reddish brown speckled white, <u>dense</u>, intact, clayey sandy abundant fine to medium grained, subrounded to sub-angular quartz GRAVEL. Coarse **ALLUVIUM**.

Completely weathered, dark brown to grey streaked white, medium grained, <u>soft</u> rock GNEISS.

EOH: Refusal of excavator on completely weathered, brown to grey streaked white, medium grained, soft to medium hard rock GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY: EM

SETUP FILE : KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

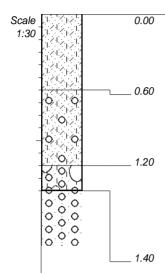
X-COORD: 803710.59 Y-COORD: 7585929.08



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP31 Sheet 1 of 1

JOB: 3010047806



Dry, brown to orange brown, \underline{loose} , intact, clayey silty SAND. Fine **ALLUVIUM**.

Note:

1. Fine roots present at top 200mm.

Dry to slightly moist, yellowish brown slightly stained orange, <u>medium</u> dense, intact, clayey silty gravelly SAND. Coarse **ALLUVIUM**.

Dry, khaki brown mottled to blotched black, <u>dense</u>, silty sandy GRAVEL, with abundant sub-angular to subrounded brown to orange cobbles of quartz. Ferruginous **PEBBLE MARKER**.

Note:

1. Traces of moderately cemented honeycomb ferricrete present.

EOH: Refusal of excavator in dry, orange brown mottled to blotched black and red, <u>dense</u>, strongly cemented, sandy GRAVEL. **HONEYCOMB** to **HARDPAN FERRICRETE** of medium hard rock consistency.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM : DATE : May 2023

DATE: 25 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

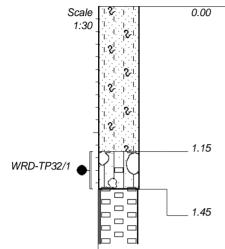
X-COORD: 0804445 Y-COORD: 7586187



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP32 Sheet 1 of 1

JOB: 3010047806



Dry, reddish brown, loose becoming medium dense, intact, silty fine SAND. Fine **ALLUVIUM**.

Note:

1. Presence of plant roots.

Slightly moist, pale brown blotched white, <u>stiff</u>, broken, moderately to strongly cemented, sandy SILT matrix, with calcrete nodules, cobble fragments of <u>medium hard</u> rock consistency. **HARDPAN CALCRETE**.

EOH: Refusal of excavator on very strongly cemented, HARDPAN CALCRETE of medium hard rock consistency.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP32/1 taken at 1.15m--1.45m.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

DRILLED BY: Joseph PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 17 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 803973.00 Y-COORD: 7585217.00

HOLE No: WRD-TP32

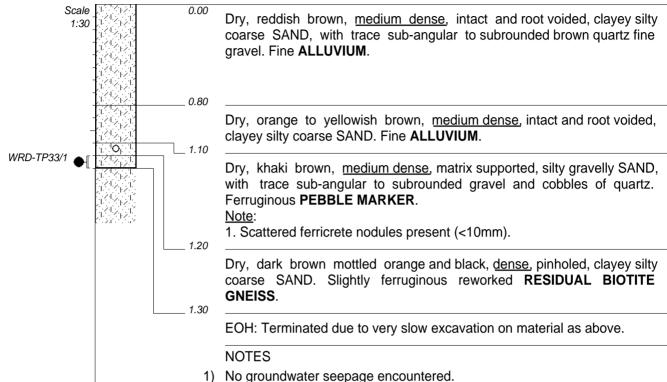
D079 E Mouton dotPLOT 7022 PBpH7



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP33 Sheet 1 of 1

JOB: 3010047806



- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP33/1 taken at 1.2m--1.3m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY: EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 25 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

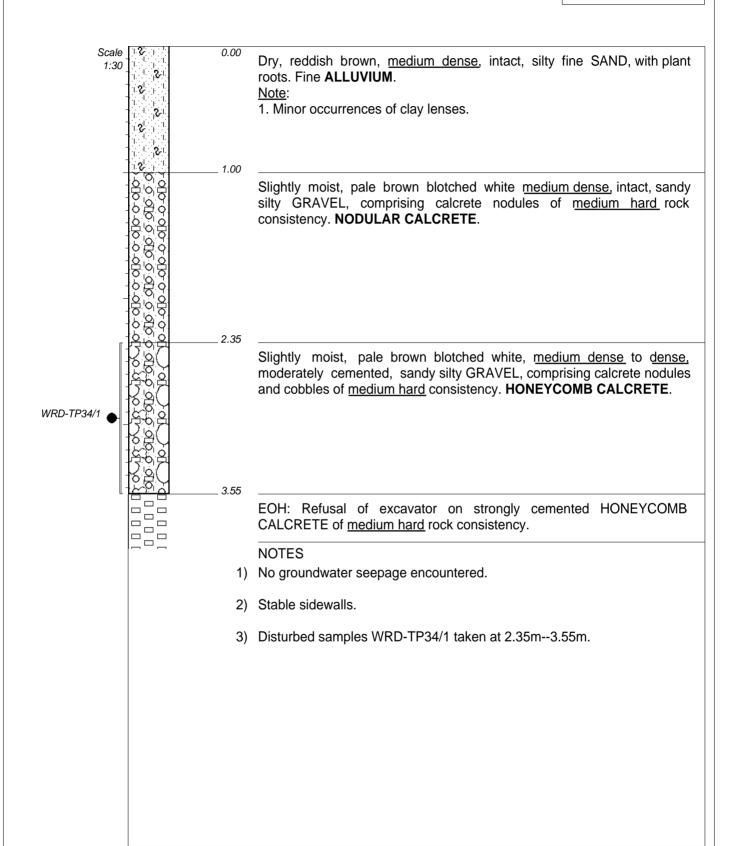
X-COORD: 0804832 Y-COORD: 7585645



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP34 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE : KPTP8.SET INCLINATION: Vertical

DIAM: DATE: May 2023 DATE: 16 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

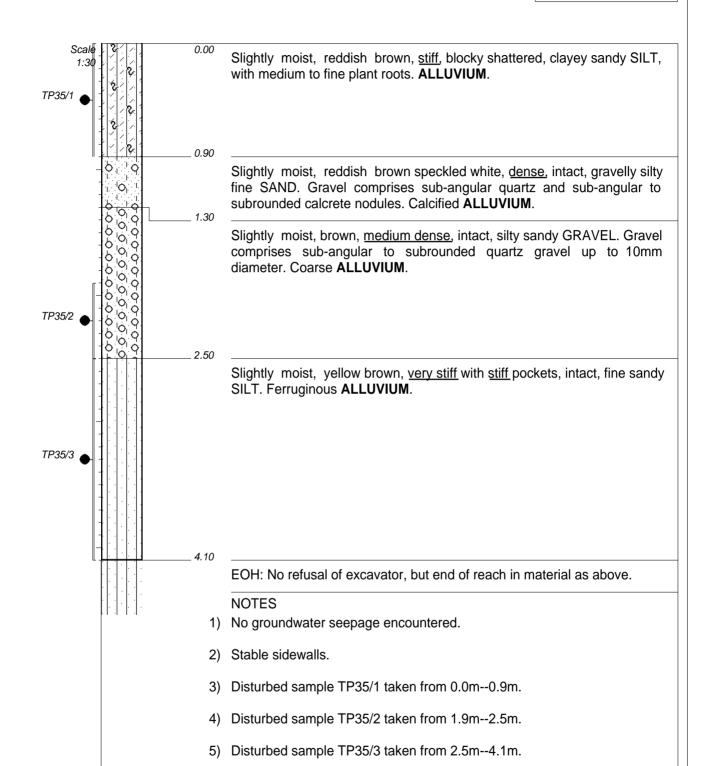
X-COORD: 804786.59 Y-COORD: 7583725.60



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP35 Sheet 1 of 1

JOB: 3010047806



CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY : B Klaas

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM :

DATE: May 2023

DATE: 10 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

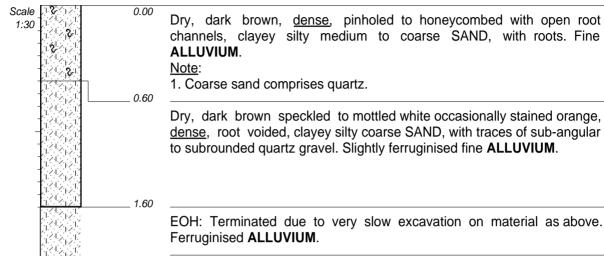
x-coord: 803149.00 y-coord: 7582266.00



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP36 Sheet 1 of 1

JOB: 3010047806



EOH: Terminated due to very slow excavation on material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No sample taken.

CONTRACTOR: Sandworx MACHINE:: Excavator FR220D DRILLED BY: Joseph PROFILED BY . C Homan

TYPE SET BY: EM SETUP FILE: KPTP8.SET **INCLINATION:** Vertical

DIAM: **DATE:** May 2023 DATE: 24 May 2023

DATE: 07/07/2023 14:24 TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K) X-COORD: 0801345

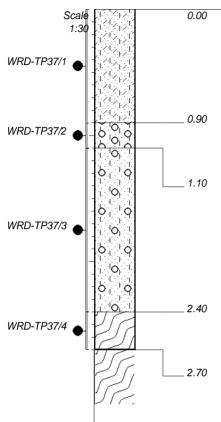
Y-COORD: 7582051



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP37 Sheet 1 of 1

JOB: 3010047806



Dry, dark brown, <u>dense</u>, blocky to slightly honeycombed, clayey silty SAND. Fine **ALLUVIUM**.

Note:

1. Scattered fine roots present.

Dry, yellowish brown mottled white and black, <u>medium dense</u>, matrix supported, silty sandy GRAVEL, comprising calcrete nodules (<20mm). Calcified **PEBBLE MARKER**.

Dry, yellowish brown occasionally mottled black and white, slightly stained orange, <u>medium dense</u>, intact, gravelly silty SAND, with trace coarse gravel, comprising subrounded gravel of quartz. Reworked **RESIDUAL GNEISS**.

Completely to highly weathered with depth, white speckled black occasional stained orange, medium to coarse grained, gneissic banding to massive, very closely jointed, very soft to soft rock with depth, GNEISS.

EOH: Refusal of excavator on material as above, but moderately weathered, <u>soft</u> to <u>medium hard</u> rock.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP37/1 taken at 0.0m--0.9m.
- 4) Disturbed sample WRD-TP37/2 taken at 0.9m--1.1m.
- 5) Disturbed sample WRD-TP37/3 taken at 1.1m--2.4m.
- 6) Disturbed sample WRD-TP37/4 taken at 2.4m--2.7m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023 DATE: 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

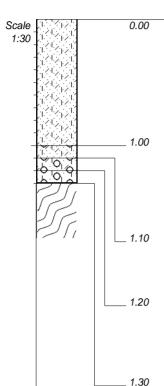
X-COORD: 0801515 Y-COORD: 7582442



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP38 Sheet 1 of 1

JOB: 3010047806



Dry, brown mottled black and red to base, <u>medium dense</u> to <u>dense</u> with depth, intact to pinholed, clayey silty SAND. Fine **ALLUVIUM**. Note:

- 1. Scattered fine roots present at top of layer.
- 2. Ferricrete nodules present at base (<10mm).

Dry, dark greyish brown mottled black blotched yellow to orange, <u>dense</u>, intact to slightly shattered and blocky, clayey gravelly silty SAND. Coarse **ALLUVIUM**.

Note:

1. Ferricrete nodules present (<25mm).

Dry, brown stained orange mottled black, <u>dense</u>, matrix supported, clayey silty sandy GRAVEL. Gravel comprises sub-angular to subrounded quartz and minor ferricrete nodules. **PEBBLE MARKER**.

Dry, grey to white occasionally brown speckled to mottled black stained yellowish orange, <u>dense</u>, intact and banded, silty sandy GRAVEL. Reworked **RESIDUAL BIOTITE GNEISS**, with pockets of highly weathered gneiss.

EOH: Excavation terminated due to very slow excavation in highly weathered, dark grey stained orange, medium to coarse grained, gneissic banded, extremely closely jointed, <u>soft</u> rock biotite GNEISS.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) No samples taken.

CONTRACTOR: Sandworx

MACHINE: Excavator FR220D

DRILLED BY : Joseph PROFILED BY : C Homan

SETUP FILE : KPTP8.SET

TYPE SET BY : FM

INCLINATION: Vertical

DIAM : DATE : May 2023 DATE : 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

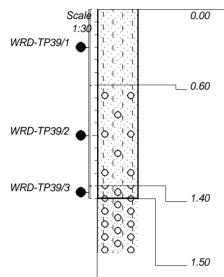
X-COORD: 0801515 Y-COORD: 7582442



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP39 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u>, intact, silty SAND to silty coarse sand. Fine **ALLUVIUM**.

Note:

1. Roots present at top 150mm.

Dry, brown to yellowish brown slightly stained orange, <u>dense</u>, intact, silty fine gravelly SAND. Ferruginous **ALLUVIUM**.

Notes:

- 1. Ferruginisation increases with depth.
- 2. Scattered ferricrete nodules present near base (<10mm).
- 3. Occasional thick roots present.

Dry, khaki stained brown to black, <u>dense</u> to <u>very dense</u>, shattered to blocky structure, silty gravelly SAND to silty sandy gravel. **HARDPAN CALCRETE / FERRICRETE**.

EOH: Refusal of excavator on material as above, but very dense.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP39/1 taken from 0.0m--0.6m.
- 4) Disturbed sample WRD-TP39/2 taken from 0.6m--1.4m.
- 5) Disturbed sample WRD-TP39/3 taken from 1.4m--1.5m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: May 2023

DATE: 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

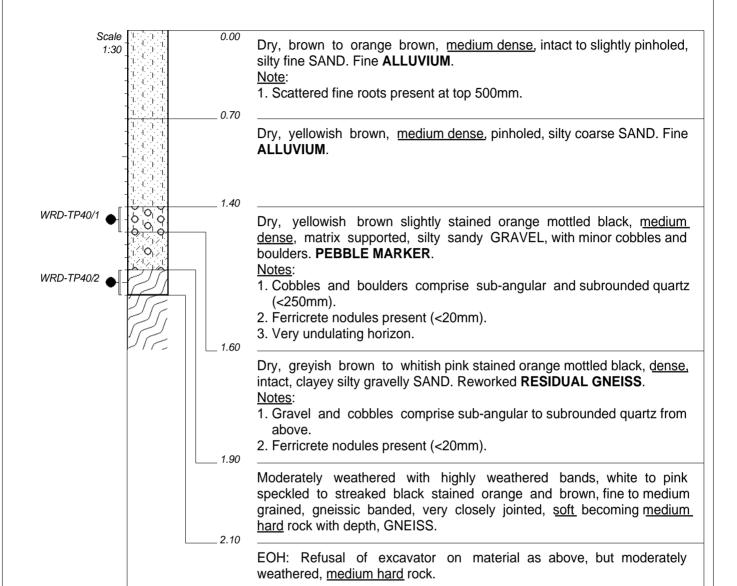
X-COORD : 0801254 Y-COORD : 7582470



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP40 Sheet 1 of 1

JOB: 3010047806



NOTES

- 1) No groundwater seepage encountered.
- Stable sidewalls.
- 3) Disturbed sample WRD-TP40/1 taken from 1.4m--1.6m.
- 4) Disturbed sample WRD-TP40/2 taken from 1.9m--2.1m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY: Joseph

PROFILED BY . C Homan

TYPE SET BY : FM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM:

DATE: May 2023

DATE: 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

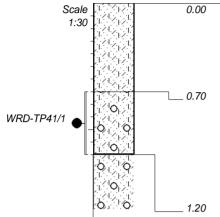
X-COORD: 0801382 Y-COORD: 7582856



GEOTECHNICAL INVESTIGATION

HOLE No: WRD-TP41 Sheet 1 of 1

JOB: 3010047806



Dry, brown, <u>medium dense</u> to <u>dense</u> with depth, intact and root voided, clayey silty SAND, with traces to minor fine gravel of quartz. Fine **ALLUVIUM**.

Note:

1. Fine gravel comprises sub-angular to subrounded guartz.

Dry, dark brown stained orange mottled black speckled to mottled white, <u>dense</u>, intact and root voided, silty clayey gravelly SAND. Coarse **ALLUVIUM**.

Notes:

- 1. Ferricrete (<7mm) and calcrete nodules (<6mm) present.
- 3. Occasional thick roots present.

EOH: Terminated due to very slow excavation on material as above.

NOTES

- 1) No groundwater seepage encountered.
- 2) Stable sidewalls.
- 3) Disturbed sample WRD-TP41/1 taken from 0.7m--1.2m.

CONTRACTOR: Sandworx

MACHINE:: Excavator FR220D

DRILLED BY : Joseph

PROFILED BY: C Homan

TYPE SET BY : EM

SETUP FILE: KPTP8.SET

INCLINATION: Vertical

DIAM: DATE: May 2023

DATE: 24 May 2023

DATE: 07/07/2023 14:24

TEXT: ..51\PROFILES\PKFWRDTP.TXT

COORDINATE SYSTEM: UTM, WGS84 (33K)

X-COORD: 0801860 Y-COORD: 7582706

Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX C2

Borehole Logs



HOLE No: HLP-BH01 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain FF -foliated CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

CG -coarse grain

JOINT SHAPE PLA-planar UND-undulating

JOINT ROUGHNESS ROCK HARDNESS

SLJ-slickensided SJ -smooth RJ -rough

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH01 Sheet 1 of 3

JOB: 3010047806

				WJ -	medium spacing wide spacing -very wide spac	g UND-undu STE-stepp cng IRR-irregu	ped										1 n 2 h 2 h 1 h		
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.71	100	0	0	NA		Scale 1:50		with the	k brown, silty clayey fine SAND, in fine scattered angular gravel at base of the layer. Fine ALLUVIUM .
										1.50	100	0	0	NA	1	-		cen	e brown blotched white, poorly nented, gravelly silty fine to dium SAND. Calcareous
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	0	NA			O O	AL I <u>Not</u>	_UVIUM . <u>e</u> :
										3.00	90	0	0	NA			0 0 1 0 1	o a 2	Chunks of calcrete nodules and obbles of soft rock consistency t 1.2m to 1.5m and 2.4m to55m.
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.13	92	0	0	NA	3	-		to	e brown mottled white, moderately strongly cemented, HARDPAN LCRETE of medium hard rock
															_ 4	-		con 4.03	sistency.
										4.68	89	0	0	NA	-				nge brown speckled black, silty ND. RESIDUAL GNEISS .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	6.18	97	0	0	NA	5			1. C	completely weathered extremely oft rock zone at 4.40m to 4.55m and 5.00m to 5.10m.
															6	_			
										6.89	100	100	61	10	-	HLP-BH01/1 ⊕ ₋ [spe	hly weathered, brown banded red ckled black streaked white, soft tending to medium hard rock
										7.68	100	100	33	14	_ 7 - - - - - -	7.4m <u>\frac{\fin}}}}}{\fighta}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}</u>		GN <u>Not</u> 1. H	EISS.
															_ 8	-		2. F	Reddish colour due to induration of ydrothermal fluid.
										9.18	100	100	71	5		HLP-BH01/2 ⊕			Possible healed shear zone.
															- 9 	-			
										10.66	100	100	49	11	10	-			
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	- DEPTH Scale 1:50	-			

HOLE No: HLP-BH01 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE FF -foliated

MF -massive FG -fine grained BF -bedded MG -medium grain CG -coarse grain

CF -cleaved

SLJ-slickensided SJ -smooth RJ -rough

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH01 Sheet 2 of 3

JOB: 3010047806

			CF -cleave SF -schiste GF -gneiss LF -lamina	ose JOIN sose VCJ- ated CJ-c MJ-1 WJ-	IT SPACING very close spa close spacing medium spacin wide spacing -very wide spa	PLA-plana	ilinear ar ulating oed	MHR-medium nara rock SR -soft rock VSR-very soft rock			-	ΓΙ			7 : <i>"!~\/\</i>	
MG	GF	IF	10-20	1 2 3	0-10 20-30 40	M1 C1-W1 AC1	RJ RJ RJ	Silt Silt Silt	1 1-2 <1	12.18	94	94	62	9	11	
										13.68	100	100	71	8	13	
										15.18	100	97	18	17	14	
										- 16.68	100	100	29	15	16	5.95 Moderately weathered, grey to brown speckled black banded white, soft rock GNEISS.
ИG	GF	VIF	10-20	1 2 3	0-10 30 50-60	VCJ NM CJ-MJ	RJ RJ RJ	Silt Silty sand FeOx	1 1-2 <1	17.36	100	88	51	10	17	Notes: 1. Weak/very soft rock zone at 16.45m to 16.51m. 2. 40mm thick quartz veins intruded
										18.18	93	93	37	11	18	along fabric.3. From 17.00m to 18.30m brecciated shear zone.
ИG	GF	IF	5	1 2	0-10 20	AC1 C1	RJ RJ	FeOx Silt	<1 <1	19.68	100	100	75	10	19	Slightly weathered, dark grey speckled black banded white, medium hard rock GNEISS becoming coarse grained and altered with depth.
										20.07	100	100	64	7	20	0.07
rain ize	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness		Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	NOTES 1) Perched water table encountered a 7.4m.

HOLE No: HLP-BH01 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded

FF -foliated CF -cleaved

SF -schistose JOINT SPACING

CG -coarse grain

SLJ-slickensided MG -medium grain SJ -smooth RJ -rough

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION HOLE No: HLP-BH01 Sheet 3 of 3

JOB: 3010047806

I I	SF -schistose GF -gneissose LF -laminated	JOINT SPACING VCJ-very close space CJ -close spacing MJ -medium spacing WJ -wide spacing VWJ-very wide space	PLA-planar g UND-undulating STE-stepped	SR -soft rock VSR-very soft rock	II	I T	1	1	1	_	
										3) NM:4) Sam 6.185) Sam	Not applicable. Not measurable. ple HLP-BH01/1 taken at m6.31m. ple HLP-BH01/2 taken at m8.60m.
Brain Rock Fabr Size Fabric Spa (mn	ic Inc Se	Joint Joint et No. Inc (Deg)	Joint Micr Spac Roug nes:	h- Filling Th	Fill Depth (kness nm)	m) Mat Ro	ck RQD v % %	Frac Freq No/m	DEPTH Scale 1:50		

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

DIAM: DATE: 20-24 April 2023 DATE: 24 April 2023

DATE: 03/07/2023 12:28

TEXT: ..51\PROFILES\PKFHLPBH.TXT

X-COORD: 801721

Y-COORD: 7585666

HOLE No: HLP-BH01

HOLE No: HLP-BH02 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

FF -foliated CF -cleaved

BF -bedded MG -medium grain CG -coarse grain

SF -schistose JOINT SPACING JOINT SHAPE GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing PLA-planar

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided SJ -smooth

EHR-extremely hard rock VHR-very hard rock RJ -rough HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH02 Sheet 1 of 3

JOB: 3010047806

				MJ -r	nose spacing medium spacing wide spacing	g UND-undu STE-stepp	lating											
		ı				cng IRR-irregu				II.			I					
										0.45	SPT	N=7			-	Scale - 1:50		Reddish brown, silty fine SAND. Fine ALLUVIUM .
										0.84	92	0	0	NA	- 1			Notes: 1. SPT indicates loose to medium
										1.50	100	0	0	NA	-	¬; -; -; -;		dense consistency. 2. From 2.75m to 3.00m abundant
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.95	SPT	N=10]; - - -		sub-angular medium grained quartz gravel and cobble fragments.
										2.00	100		0	NA :	2			nagments.
										3.00	95	0	0	NA	- - - - -			
															3	1: -1: -1:	3.20	
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.44	100	37	18	5]; ;; <u>}</u> ;		Brown to grey speckled black and white, silty SAND. RESIDUAL GNEISS.
															- 4	- 	3.85	Highly weathered, brown to grey
															- - -	1/		banded white, <u>very soft</u> rock GNEISS. <u>Notes</u> :
										6.03	100	97	75	7	_ 5			 Minor to traces of quartz veins. Fractures mainly along quartz veins.
										0.03	100	31	73		-	HLP-BH02/1 ●		
MG	GF	IF	10	1	0-10	CJ	RJ	Silt	<1						6	<u>}</u>		
															-	<u> </u>		
										7.58	100	97	74	9	7	<u> </u>		
															_ 8		7.90	Madarataly weathered brown to gray
										9.13	100	95	85	8				Moderately weathered, brown to grey speckled black banded white, soft to medium hard rock GNEISS.
															9	1 1 1		Note: 1. Completely weathered, very soft to
															-	./ - -		extremely soft rock zone at 11.15m to 11.20m.
MG-CG	GF	VIF	10	1	0-10	CJ	RJ	Silt	<1	10.67	100	100	90	4	10) 		
Grain Size	Rock Fabric	Fabric Spac	Fabric Inc	Joint Set No.	Joint Inc	Joint Spac	Micro Rough-	Joint Filling	Fill Thickness	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq	DEPTH Scale	- 1		_
0.20	TUDITO	(mm)	(deg)	001710.	(Deg)	Spac	ness	, illing	(mm)	1	.0007 78	7000V 70	/0	No/m	1:50		151	

HOLE No: HLP-BH02 Sheet 2 of 3 JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain

CG -coarse grain

SF -schistose JOINT SPACING

FF -foliated

CF -cleaved

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH02 Sheet 2 of 3

JOB: 3010047806

I	I		GF -gneiss LF -lamina	ted CJ -0 MJ - WJ -	very close spacing medium spacing wide spacing -very wide spa 	PLA-plana	ar ulating oed	VSR-very soft rock	I		-	Γ		I	1 t - ギン//	
										12.17	100	100	95	4	11.65 Slightly weathered, grey speck black banded white, medium hard	 :led d to
										13.67	100	100	91	3	hard rock granitic GNEISS. Notes: 1. From 12.60m to 12.91m hig weathered soft rock zone. 2. Thick pegmatite inclusion from 13.15m to 14.65m. 3. Highly fractured from 17.60m	ghly om
										15.17	99	99	85	5	17.85m.	
G-CG MF	F-GF	NM	NM	1 2	0-10 40	AC1 C1-W1	RJ RJ	Silt Silt	1-2 1-2	16.67	100	100	86	5	16	
										18.17	86	86	63	8	17	
										19.43	100	100	93	2	19	
										20.08	100	98	98	0	20.08 NOTES 1) NA: Not applicable.	
	abric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	g Scale	

HOLE No: HLP-BH02 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded

FF -foliated CF -cleaved

SF -schistose JOINT SPACING

MG -medium grain SJ -smooth CG -coarse grain RJ -rough

JOINT SHAPE

SLJ-slickensided

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock

MHR-medium hard rock SR -soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH02 Sheet 3 of 3

JOB: 3010047806

_			GF -gneissose LF -laminated	CJ -close MJ -mediu WJ -wide	spacing um spacing spacing	CUR-curviline PLA-planar UND-undulate STE-stepped IRR-irregular	ing I	VSR-very soft rock		. –	-		_		_			
																NM: Not measurable.		
															3)	Sample HLP-BH02/1 5.35m5.65m.	taken a	at
Grain	Rock	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint	Fill Depth (m)) Mat	Rock	RQD	Frac	DEPTH				
Size	Fabric	Spac (mm)		et No.	Inc (Deg)	Spac	Rough- ness	Filling	Thickness (mm)	recov %	recov %	CONTRA	Freq No/m	Scale 1:50	INCLINATION : Vertical	COORDINATE SYSTEM	4 · WGS84 I I	ITM 22

CONTRACTOR: MACHINE: Sullivan HD DRILLED BY:

PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET INCLINATION: Vertical

DIAM: DATE: 25-26 April 2023 DATE: 26 April 2023

DATE: 03/07/2023 12:28 TEXT: ..51\PROFILES\PKFHLPBH.TXT

COORDINATE SYSTEM: WGS84 UTM 33S X-COORD: 801240

Y-COORD: 7584810 HOLE No: HLP-BH02 HOLE No: HLP-BH03 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain FF -foliated CF -cleaved

CG -coarse grain

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

PLA-planar UND-undulating

JOINT ROUGHNESS ROCK HARDNESS

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH03 Sheet 1 of 3

	1			WJ -	medium spacin wide spacing -very wide spa	STE-stepp cng IRR-irregu	ped	T		II				T	T S	Scale - 17)
NA	NA	NA	NA	NA	NA NA	NA	NA	NA	NA	0.45	SPT 100	N=4 0	0	NA		1:50	Reddish brown, silty SAND. Fine ALLUVIUM .
INA	INA	INA	INA	INA	INA	INA	INA	IVA	INA	1.50	100	0	0	NA NA	1		Notes: 1. Traces of subrounded fine to
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.65		N=Ref			-	000	medium quartz gravel from 0.85m to 1.3m. 2. SPT indicates loose consistency.
										3.28	100	79	9	13	HLP-BH03/1	● I 1.30	Brown to off-white, silty sandy subangular to surrounded medium grained GRAVEL. Calcareous ALLUVIUM. Note: 1. SPT indicates very dense
										4.68	96	89	33	9	HLP-BH03/2	1.98	consistency. Highly weathered, brown to green speckled black streaked white, soft rock GNEISS, brecciated in places with indication of shearing. Interpreted as fault/shear zone.
MG	GF	IF	70-80	1 2 3	0-10 20 80-90	AM1 C1 AC1	RJ RJ RJ	Clayey silt Silt Calcite	1-2 1 2	6.18	100	97	47	9	5		Notes: 1. Thick quartz vein at 2.60m to 3.00m. 2. Completely weathered, very soft rock zone from 5.30m to 5.20m and 5.95m to 6.15m. 3. Near-vertical joint at 6.69m to 6.9m.
										7.62	100	97	28	17	7		4. From 2.92m to 3.28 and 7.12m to 7.623m highly fractured.
										9.16	99	94	58	8	- 8 	8.20)
										10.68	64	54	24	5	9		
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)		Rock recov %	RQD %	Frac Freq No/m	F DEPTH Scale 1:50	-	

HOLE No: HLP-BH03 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

MF -massive FG -fine grained BF -bedded MG -medium grain

FF -foliated CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

SLJ-slickensided SJ -smooth CG -coarse grain RJ -rough JOINT SHAPE

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH03 Sheet 2 of 3

			Lr -Iamina	MJ - WJ -	ciose spacing medium spacin wide spacing I-very wide spac	g UND-undul STE-steppe cng IRR-irregula	ating ed				_				
				1	0-10	CJ	RJ	FeOx & Silt	<1	11.40	100	100	38	14	Moderately weathered, brown to grey streaked white, medium hard rock GNEISS. Notes:
MG	GF	IF	80	2 3	70 80-90	MJ	RJ RJ	Clean Silt	- <1	12.73	68	45	44	3	1. Core loss of 0.64m from 9.16m to 10.68m. Presumed to be due to washing out of fines. 2. Completely weathered, friable, pitted, extremely soft rock zone at 10.40m to 10.65m.
										13.68	100	83	33	15	3. From 11.40m to 11.75m; 12.96m to 13.05m; and 13.10m to 13.24m highly broken zone. 4. Near vertical joint at 12.44m to 12.51m.
										15.18	100	98	60	9	14.80
MG	GF	IF	10	1 2 3 4	0-10 45 60 30	VCJ-CJ CJ NM VCJ	RJ RJ RJ	FeOx & Silt Silt Silty sand FeOx	<1 <1 1 <1	16.66	100	99	57	10	Slightly weathered, grey speckled black streaked and banded white, medium hard rock GNEISS, with brecciated rock in places, interpreted as shear zone.
										18.16	100	93	63	7	17.56
															Highly weathered, grey speckled black streaked white, medium hard rock GNEISS.
MG	GF	IF	80	1 2	0-10 70	M1 AC1	RJ RJ	Silt Silt	<1 <1	19.24	100	87	19	18	Note: 1. Breccia development at 17.56m to 17.65m and 19.40m to 19.52m. 2. Highly to completely weathered, very soft rock zone at 17.97m to
										20.13	100	96	17	12	18.16m; 19.10m to 19.24m and 19.95m to 20.13m.
															NOTES 1) NA: Not applicable.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)		Rock recov %	RQD %	Frac Freq No/m	Scale

HOLE No: HLP-BH03 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded

FF -foliated

CF -cleaved

SF -schistose JOINT SPACING

SLJ-slickensided MG -medium grain SJ -smooth CG -coarse grain RJ -rough

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock

MHR-medium hard rock SR -soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH03 Sheet 3 of 3

JOB: 3010047806

_			GF -gneiss LF -laminat	ted CJ -c MJ - WJ -	very close space close spacing medium spacing wide spacing -very wide space	PLA-plana UND-undu STE-stepp	nr ulating ped	VSR-very s	oft rock			_	_			-		_					_
																				2)	NM: Not measura	ble.	
																				3)	Sample HLP-E 2.27m2.41m.		aken at
																				4)	Sample HLP-E 3.73m3.93m.	3H03/2 ta	iken at
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	i	Joint Filling	Fill Thickness (mm)	Depth (m) Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEP Scal	e						
		/						1		1 \ /				CONTR	ACTOR:				ICLINATION : V	ertical	COORDINAT	F SYSTEM · W	/GS84 UTM 33

CONTRACTOR: MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

DIAM: DATE: 26-27 April 2023 DATE: 23 April 2023

DATE: 03/07/2023 12:28

TEXT: ..51\PROFILES\PKFHLPBH.TXT

COORDINATE SYSTEM: WGS84 UTM 33S X-COORD: 802101 Y-COORD: 7584148

HOLE No: HLP-BH03

HOLE No: HLP-BH04 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain

FF -foliated CF -cleaved

CG -coarse grain SF -schistose JOINT SPACING

GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

SJ -smooth RJ -rough

JOINT SHAPE

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH04 Sheet 1 of 3

				WJ -	nedium spacir vide spacing very wide spa	g UND-undu STE-stepp cng IRR-irregu	ped			П				1	1	- Soulo	FCS54	0.00	
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.45	SPT 100	N=20 0	0	NA		Scale 1:50		0.00	Dark brown, silty clayey fine SAND. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	_ 1			1.05	Light brown to yellow brown, gravelly silty CLAY. Calcareous ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.88	SPT	N=Ref		L NIA	<u> </u>				Note: 1. Gravel occurs as minor subrounded
										3.00	100 97	93	21	19	3	HLP-BH04/1 <mark>⊕</mark> [∜ ///	2.00	fine to medium quartz. Dark brown to grey, silty fine to medium SAND. RESIDUAL GNEISS .
MG	GF	VIF	0-10	1	0-10	VCJ	RJ	Sand	1-2	4.58	75	54	0	15					Highly weathered, brown to grey speckled black streaked white, soft rock GNEISS. Note: 1. From 4.05m to 4.4m completely weathered extremely soft rock zone. Interpreted as silty fine to
										6.15	100	100	41	12	5	HLP-BH04/2 ●¶		4.51	medium sand (residual gneiss). Moderately weathered, grey speckled black streaked white, medium hard rock GNEISS. Notes: 1. From 4.58m to 10.60m minor to abundant quartz veins.
										7.67	100	100	45	13	7				 Long near-vertical joint from 9.50m to 10.57m. Completely weathered extremely soft zone from 11.1m to 11.18m. Interpreted as silty sand (residual gneiss).
MG	GF	VIF	5	1 2	0-10 90	VCJ-CJ NM	RJ RJ	Silt Silty SAND	1-2 2-3	9.17	100	100	66	8	8				
										10.67	100	100	29	14	10				
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	=			

HOLE No.	HLP-B	H04
<i>ЈОВ</i> : 30	100478	306
MG	GF	IF-

BF -bedded

FF -foliated

CF -cleaved

SF -schistose

JOINT SPACING

LF -laminated CJ -close spacing

GF -gneissose VCJ-very close spacg CUR-curvilinear

JOINT SHAPE

PLA-planar

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock MG -medium grain SJ -smooth VHR-very hard rock CG -coarse grain RJ -rough HR -hard rock

MHR-medium hard rock

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION HOLE No: HLP-BH04 Sheet 2 of 3

			LF -IdIIIIII	tea СЈ-ск МЈ-т	ose spacing edium spacing	PLA-pianai UND-undul										
					ide spacing	STE-steppe										
-				VVVJ-1 	ery wide spac 	ng IRR-irregul	ar 				Т			٦	<u> </u>	-1//
															-	
															_ 11	
										40.47	400	05	00	0	-	1974 - H
										12.17	100	95	60	9	-	11.52 Clightly, weathered, grow angeled
															- _ 12	Slightly weathered, grey speckled black, streaked and banded white,
															-	medium hard rock GNEISS.
															-	Notes: 1. Pegmatite vein at 12.40m to
														_	-	12.70m and at 17.20m to 17.50m.
										13.67	100	100	55	9	_ 13	2. From 19.11m to 19.60m and
															-	19.65m to 19.70m completely weathered very soft to extremely
															-	soft rock zone.
															14	
															-	
										15.20	100	98	49	10	-	F.A
															- 45	
															<u> </u>	
															-	
				1	0-10	VCJ-CJ	RJ	Silt	1-3						-	
MG	GF	IF-VIF	10-20	2 3	20 40	CJ	RJ	Silt	1 <1	16.74	100	100	66	9	_ 16	# # # # # # # # # # # # # # # # # # #
				3	40	VWJ	RJ	FeOx	<1						- -	
															-	F.A
															17	
															- ''	
										18.23	97	95	70	10	-	
															-	
															_ 18	
															-	
															-	
										19.73	93	79	35	11	- _ 19	
										13.73	33	13	55	''	-	
															[-	
															[-	
										20.23	100	100	74	16	<u> </u>	20.23
																NOTES
Grain	Pook	Eabria	Eahria	loint	loint	loint	Micro	loint	Eill	Donth (m)	Mot	Pools	DOD.	Eroo	 DEPTH	_ 1) NA: Not applicable
Grain Size	Rock Fabric	Fabric Spac	Fabric Inc	Joint Set No.	Joint Inc	Joint Spac	Micro Rough-	Joint Filling	Thickness	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq	Scale	
		(mm)	(deg)		(Deg)		ness		(mm)					No/m	1:50	

HOLE No: HLP-BH04 Sheet 3 of 3

JOB: 3010047806

D079 E Mouton

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded

FF -foliated CF -cleaved SF -schistose

MG -medium grain CG -coarse grain

SLJ-slickensided SJ -smooth

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock RJ -rough MHR-medium hard rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: HLP-BH04 Sheet 3 of 3

JOB: 3010047806

			SF -schisto GF -gneisso LF -laminat	ose VCJ- ed CJ-c MJ-r WJ-	IT SPACING very close space close spacing medium spacing wide spacing -very wide space	PLA-plana	ilinear ar ulating oed	SR -soft rock VSR-very soft rock				- ,						
																2) NM:	Not measurable.	
																2.66	n2.86m.	aken a
																4) Sam 5.19r	ole HLP-BH04/2 t m5.35m.	aken a
n	Rock	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint	Fill	Depth (m) Mat	Rock	RQD	Frac	DEPTH			
e	Fabric	Spac (mm)	Inc (deg)	Set No.	Inc (Deg)	Spac	Rough- ness	Filling	Thicknes (mm)	3	recov %	recov %	%	Freq No/m	Scale 1:50			

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM

DIAM: DATE: 28-29 April 2023 DATE: 28-29 April 2023

DATE: 03/07/2023 12:28

X-COORD: 801731

Y-COORD: 7583536 HOLE No: HLP-BH04 HOLE No: Plant-BH01 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

FF -foliated CF -cleaved

SF -schistose JOINT SPACING

LF -laminated CJ -close spacing

MF -massive FG -fine grained BF -bedded MG -medium grain CG -coarse grain

GF -gneissose VCJ-very close spacg CUR-curvilinear

SJ -smooth RJ -rough

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock JOINT SHAPE

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH01 Sheet 1 of 3

				WJ -ı	nedium spacing vide spacing -very wide spac	g UND-undu STE-stepp ong IRR-irregu	ped											
										0.45	SPT	N=14			-	Scale - 1:50	0.00	Light brown to orange brown, silty medium SAND. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	90	0	0	NA		= 1 /1 = 11. = 1. = 1. = 1.		Note: 1. SPT indicates medium dense consistency.
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	97	0	0	NA			1.35	Light brown to orange brown, silty
NA NA	NA	NA	NA	NA	NA NA	NA NA	NA	NA NA	NA NA	2.00	SPT 88	N=Ref 0	0	NA	2			clayey fine to medium SAND.
MG	GF	NM	NM	1	0-10	VCJ	RJ	Silty sand	2-3	2.00								ALLUVIUM. Note: 1. SPT indicates very dense
MG-CG	GF-MF	NM	NM	1 2	0-10 20	CJ VCJ	RJ RJ	Silty sand Silty sand	1-2	3.50	91	91	31	15	_ 3	} 	1.75	Light brown blotched white, poorly to moderately cemented, silty sandy abundant fine to medium GRAVEL.
										4.65	97	97	63	7	- 4	 		Calcified ALLUVIUM. Note: 1. Gravel comprises fragments of hardpan calcrete of soft rock consistency with abundant sub-angular to subrounded quartz
										6.15	100	99	90	4	5	1/	2.00	gravel. Highly to completely weathered, brown speckled black, very soft to soft rock granitic GNEISS.
IG-CG	GF-MF	NM	NM	1 2	0-10 30	CJ-MJ NM	RJ RJ	FeOx & Silt Silt	1 <1	7.65	97	97	95	3	7	\ \ \ \	3.78	Moderately weathered, grey speckled black and white banded white, medium hard rock granitic GNEISS.
																7.4m <u>▼</u>		Slightly weathered, grey speckled black streaked white, medium hard rock granitic GNEISS.
										9.16	100	100	98	3	8	1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-		
MG	GF	IF	10-20	1 2	0-10 20	VCJ VCJ	RJ RJ	Silty sand Sand	1 1	10.65	100	99	19	15		1	9.14	Moderately weathered, dark grey to brown speckled black and white, soft rock GNEISS.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	- -		

HOLE No: Plant-BH01 Sheet 2 of 3 JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain

CG -coarse grain

SF -schistose JOINT SPACING

FF -foliated

CF -cleaved

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

HOLE No: Plant-BH01 Sheet 2 of 3

JOB: 3010047806

GEOTECHNICAL INVESTIGATION

		GF -gneiss LF -lamina	ted CJ -c MJ -r WJ -v	very close spa lose spacing nedium spacin wide spacing very wide spa	PLA-plana	ar ulating oed	SR-very soft rock			Т			-]	-t/1
									12.15	100	100	81	7	11	Slightly weathered, grey speckled black and white streaked white, medium hard rock granitic GNEISS. Notes: 1. Pegmatite veins from 13.00m to
									13.65	100	100	76	6	13	13.56m; 16.66m to 17.01m and 19.00m to 19.19m. 2. Highly broken zone at 17.05m to17.20m. 3. Core loss due to pulverizing spinning of loose core at end of run (19.65m to 21.15m.)
									15.15	100	100	84	7		
G-CG GF-MF	NM	NM	1 2	0-10 20-30	VCJ-MJ CJ-MJ	RJ RJ	Silt Silt	1 1	16.65	98	98	71	8		
									18.15	100	91	72	5	17 18	
									19.65	100	97	73	7		
	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint	Fill	21.15 Depth (m)	67	67 Rock	17	13 Frac		20.67

HOLE No: Plant-BH01 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE BF -bedded FF -foliated CF -cleaved SF -schistose

MF -massive FG -fine grained MG -medium grain CG -coarse grain

JOINT SPACING

GF -gneissose VCJ-very close spacg

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

CUR-curvilinear

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

DATE: 2 May 2023 DATE: 3 May 2023

DATE: 07/07/2023 14:19

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HOLE No: Plant-BH01 Sheet 3 of 3

JOB: 3010047806

				V VV3-	very wide spa	ncng IRR-irregu	lai					-			Slightly weathered wit weathered zones as closely jointed zones, good black banded white	sociated w grey speckle
				1	0-10	VCJ-CJ	RJ	Silty sand	<1	22.58	100	100	48	11	becoming medium hard Note: 1. Core loss of 0.67m 21.06m due to washin	rock GNEIS at 20.39m
MG	GF	VHF	70-80	2 3	10-20 45	CJ MJ	RJ RJ	Silt Silt	<1 1	24.08	99	99	79	7	23	
										25.05	100	100	68	7	25 25.05	
															NOTES 1) Perched water table 7.4m.	encountere
															2) NA: Not applicable.	
															3) NM: Not measurable.	
Grain	Rock	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint	Fill	Depth (m)	Mat	Rock	RQD	Frac	DEPTH	

DRILLED BY :

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

Y-COORD: 7583844

HOLE No: Plant-BH01

HOLE No: Plant-BH02 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated CF -cleaved

CG -coarse grain SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

JOINT SHAPE PLA-planar

RJ -rough

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock SJ -smooth

VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

Sheet 1 of 3 JOB: 3010047806

HOLE No: Plant-BH02

				WJ -	medium spacing wide spacing	STE-steppe	ed								
NA	NA	NA	NA	NA	-very wide spacr	ng IRR-irregula	NA	NA	NA	0.54	93	0 8	0	NA NA	Scale 1:50 Dark brown to brown, silty sandy CLAY, with scattered medium gravel at the base. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	42	42	8	Dark brown to grey mottled orange, silty fine SAND. RESIDUAL GNEISS .
MG	GF	NM	NM	1 2	0-10 30	VCJ VCJ	RJ RJ	Silt Silty sand	<1 1	2.12 2.47	92 86	N=Ref 72 86	23 37	13 10	1.52 Completely weathered, brown to grey speckled black and white, very soft rock GNEISS.
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.01	100	18	6	4	Light brown speckled white, silty fine SAND. RESIDUAL GNEISS . Note: 1. Traces of subrounded quartz cobble and coarse gravel fragments.
MG	GF	NM	NM	1	0-10	CJ	RJ	Silt	<1	4.43	76	76	0	19	Highly weathered, brown to grey, soft rock GNEISS. Note:
										5.93	100	100	73	8	1. Quartz vein from 4.74m to 4.78m. 5.25 Moderately weathered, light brown to grey speckled black and streaked
										7.50	99	97	49	12	white, medium hard rock granitic GNEISS. Notes: 1. Highly fractured from 6.25m to 6.42m and 8.55m to 8.65m. 2. 45 degrees quartz vein at 6.95m. 3. From 11.46m to 11.56m highly weathered soft rock zone.
MG-CG	GF-MF	NM	NM	1	0-10	AC1-C1	RJ	Silty sand	1-2	9.03	97	93	75	8	9
										10.40	100	100	64	11	10
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold HOLE No: Plant-BH02 **OMITIOMIRE COPPER PROJECT** HOLE No: Plant-BH02 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock **PLANT SITE** Sheet 2 of 3 Sheet 2 of 3 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 **GEOTECHNICAL INVESTIGATION** JOB: 3010047806 CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock GF -gneissose CUR-curvilinear VSR-very soft rock VCJ-very close spacg LF -laminated CJ -close spacing PLA-planar MJ -medium spacing UND-undulating WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular 11 11.98 97 97 41 12 11.95 12 Slightly weathered to unweathered, light grey to light brown speckled white and pink streaked black, hard to 13.50 100 100 80 6 very hard rock granitic GNEISS, 13 interpreted as a partially healed shear Notes: 1. Highly broken zone at 16.31m to 16.49m. 14 14.60 97 97 75 9 2. Rock mass improves with depth. 15.27 100 100 90 6 15 16.49 100 91 53 6 16 17 0-10 CJ RJ Silty & clean <1 17.99 5 100 100 81 RJ 2 30-40 MJ Calcite 1-2 CG MF-GF NM NM RJ1-2 3 50-60 CJ-MJ Calcite 70 NM RJ FeOx <1 18 19.49 100 100 97 19 20 20.73 99 99 78 4 Fill Depth (m) RQD DEPTH Grain Rock Fabric Fabric Joint Joint Joint Micro Joint Mat Rock Frac Filling Size Fabric Spac Inc Set No. Inc Rough-Thickness recov % recov % Freq Scale Spac 1:50 (mm) (deg) (Deg) ness (mm) No/m

HOLE No: Plant-BH02 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded FF -foliated

CF -cleaved

SF -schistose

MG -medium grain CG -coarse grain GF -gneissose LF -laminated

JOINT SPACING VCJ-very close spacg CJ -close spacing MJ -medium spacing

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

JOINT SHAPE

CUR-curvilinear

UND-undulating

PLA-planar

EHR-extremely hard rock VHR-very hard rock SJ -smooth RJ -rough HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

DATE: 4-6 May 2023 DATE: 5-6 May 2023

DATE: 07/07/2023 14:19

TEXT: ..\PROFILES\PKFPLANTBH.TXT

HOLE No: Plant-BH02 Sheet 3 of 3

JOB: 3010047806

-	I	1 1		WJ -	wide spacing -very wide spac 	STE-stepp cng IRR-irregu	ped	1	I	20.00	100	400	100	T-0	1 ∟	-₩ <i>へ</i> ノI	7
										22.27		100	93	2	21		
										23.10	100	78	78	2	_ 23	22.92	Moderately weathered, light brown to
CG	MF-GF	NM	NM	1 2	0-10 30	AC1 AC1	RJ RJ	Calcite & FeOx Calcite & FeOx	1 1	24.59	85	62	27	5	_ 24		light grey speckled pink, white and black, medium hard rock granitic GNEISS. Note: 1. Breccia zone from 22.95m to 23.10m and 23.85m to 24.28m.
										25.16	100	100	56	9	_ 25	1) 2)	NOTES No groundwater seepage encountered. NA: Not applicable. NM: Not measurable.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD % CONTRACTO MACHIN	Frac Freq No/m DR: RA Lone	DEPTH Scale 1:50 gstaff	INCLINATION : Vertical DIAM :	COORDINATE SYSTEM: WGS 84 UTM 3 X-COORD: 801481.43 Y-COORD: 7583235,41

DRILLED BY:

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

Y-COORD: 7583235.41

HOLE No: Plant-BH02

HOLE No: Plant-BH03 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

MF -massive FG -fine grained BF -bedded MG -medium grain

FF -foliated

CF -cleaved

CG -coarse grain

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

SJ -smooth RJ -rough

JOINT SHAPE

PLA-planar

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH03 Sheet 1 of 3

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	-			
MG	GF	VIF-IF	10-20	1 2 3	0-10 40 70-80	AM1 AC1-C1	RJ RJ RJ	FeOx & silt Silt Clean	1-2 <1 -	10.63	100	99	45	12	10				
										9.13	100	97	39	15	- 8 - 8 - 9	-			medium hard rock GNEISS. Notes: 1. From 8.67m to 8.90m and 9.61m to 9.71m highly weathered very soft rock zone. 2. Significant pyrite mineralization along foliation.
										7.61	100	100	75	9	7			6.35	2. Completely weathered, extremely soft rock at 4.46m to 4.63m. Interpreted as silty sandy gravel (residual gneiss). Moderately weathered, dark grey to green speckled black streaked white,
G	GF	VIF	10-20	2	20-30	MJ	RJ	Silty sand	<1	6.13	71	68	11	13	5	5.70m <u>∇</u> -		3.24	Highly weathered, brown streaked white stained red, soft rock GNEISS. Note: 1. Highly fractured zone from 3.51m to 4.63m.
	0.5	VIE	40.20	1	0-10	VCJ	RJ	FeOx & Silt	1-2	4.63	100	80	0	22	4	-			Completely weathered becoming highly weathered with depth, yellow brown speckled black, soft rock GNEISS. Note: 1. Highly friable and pitted.
-CG	GF	VIF	10-20	1 2	0-10 20	CJ CJ	RJ RJ	Silty sand Silty sand	1-2	3.51	99	95	62	7	3	- - - - -		1.78	Notes: 1. Scattered traces of subrounded cobble fragments. 2. SPT indicates very dense consistency.
										2.00	100	100	68	5	_ 2	- - -			clayey fine to medium, sub-angular to subrounded quartz GRAVEL. ALLUVIUM .
Α	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100 SPT	0 N=Ref	0	NA	- - - -	- - -	0 0 0 0 0 0	1.27	Reddish brown to orange brown, silty
A	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	100	0	0	NA	_ 1	- - - -			Note: 1. SPT indicates loose consistency.
										0.45	SPT	N=8			-	Scale - 1:50 <u>-</u>		0.00	Reddish brown, silty fine to medium SAND. ALLUVIUM .

HOLE No: Plant-BH03 Sheet 2 of 3 JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain SJ -smooth FF -foliated CG -coarse grain

CF -cleaved

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock RJ -rough HR -hard rock MHR-medium hard rock



OMITIOMIRE COPPER PROJECT PLANT SITE

HOLE No: Plant-BH03 Sheet 2 of 3

JOB: 3010047806

GEOTECHNICAL INVESTIGATION

				3	70-80	VWJ	RJ	Silty sand	1	16.63	100	87	15	17	16	1. Highly fractured from 15.37m to 16.21m. 2. Silt zone at 15.30m to 15.37m.
MG	GF	VIF	10	1 2 3	0-10 20 70-80	AM1 C1 AC1	RJ RJ RJ	Silt Silt Silty sand	1 1-2 1	16.63	100	87	15	17	16	soft rock GNEISS. Notes: 1. Highly fractured from 15.37m to 16.21m.
MG	GF	VIF	10	1 2	0-10 70	VCJ-MJ	RJ RJ	Clean FeOx	<1	15.13	100	98	71	8	14	14.90 Highly to completely weathered, grey to brown banded white, <u>very soft</u> to
										13.63	97	97	31	14	12	12.60 Slightly weathered to unweathered, dark grey speckled black streaked white, <u>hard</u> rock GNEISS.
			GF -gneiss LF -lamina	ted CJ -c MJ -ı WJ -ı	-very close spa close spacing medium spacin wide spacing I-very wide spa	PLA-planar	ating d	/SR-very soft rock		12.13	100	99	35	16	11	

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold **OMITIOMIRE COPPER PROJECT** HOLE No: Plant-BH03 HOLE No: Plant-BH03 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock **PLANT SITE** Sheet 3 of 3 Sheet 3 of 3 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 **GEOTECHNICAL INVESTIGATION** JOB: 3010047806 CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock GF -gneissose CUR-curvilinear VSR-very soft rock VCJ-very close spacg LF -laminated CJ -close spacing PLA-planar UND-undulating MJ -medium spacing WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular MG GF IF 80 RJ3 CJ Silt 20 <1 21 1-2 70 NM RJCalcite 22.63 100 100 75 5 22 23 24.13 100 100 70 5 24 25.00 97 97 97 2 25 25.00 **NOTES** 1) Perched groundwater table encountered at 5.70m. 2) NA: Not applicable. 3) NM: Not measurable. Grain Fabric Fabric Micro Depth (m) Mat Rock RQD Frac DEPTH Rock Joint Joint Joint Joint Fabric Filling Thickness Size Spac Inc Set No. Inc Roughrecov % recov % Scale (mm) (deg) (Deg) (mm) No/m 1:50 ness COORDINATE SYSTEM: WGS 84 UTM 33S CONTRACTOR: RA Longstaff INCLINATION: Vertical MACHINE: Sullivan HD X-COORD: 802365.16 DIAM:

DRILLED BY

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

Y-COORD: 7583468.49

HOLE No: Plant-BH03

DATE: 4-6 May 2023 DATE: 5-6 May 2023

DATE: 07/07/2023 14:19

TEXT: ..\PROFILES\PKFPLANTBH.TXT

HOLE No: Plant-BH04 Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain FF -foliated CF -cleaved

CG -coarse grain

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

JOINT SHAPE

PLA-planar

EHR-extremely hard rock SJ -smooth RJ -rough

VHR-very hard rock HR -hard rock MHR-medium hard rock SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH04 Sheet 1 of 2

			Lr -Iamina	MJ -I WJ -	nose spacing medium spacing wide spacing -very wide spac	STE-stepp	lating ed											
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	92.00	0	-	NA		Scale 1:50 1:50 1:50	0.00	Brown, clayey silty SAND. Fine ALLUVIUM. Notes:
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100.00		-	NA	1		0.80	Scattered fine roots present. Minor core loss due to washing out of soft soils.
										2.00	100.00	N=Ref \(\)	0	23	2			Orange brown mottled light brown and black, clayey silty SAND, with traces of sub-angular fine gravel to
MG-CG	GF	IF	20-40	1	10-20	CJ	RJ	Chl + FeOx	<1	3.54	88.00	88	55	10	_ 3	PLT-BH04/1 ♠∏	1.70	subrounded cobbles of quartz. Ferruginised fine ALLUVIUM . Note: 1. Ferricrete nodules present (<15mm).
MG-CG	Gr	IF	20-40	2	30-40	MJ	RJ	FeOx	<1	4.78	98.00	98	63	8	_ 4			Highly to moderately weathered in places, white to light brown banded greenish brown speckled black and slightly stained orange to light green, closely to moderately jointed, soft to medium hard rock biotite GNEISS.
MG-CG	MF-GF	IF-VIF	10-20	1 2	10-20 40-60	VCJ-CJ	SJ RJ	CI.Silt & FeOx FeOx	<2 <1	6.28	100.00	100	59	22	5	PLT-BH04/2 ●Ⅱ	5.10	Note: 1. Completely to moderately weathered zone near upper 300mm. 2. Minor core loss due to washing out of finer material between joints. Moderately weathered, white streaked
										7.78	100	100	67	17	7	7.4m <u>▼</u>	6.30	to banded yellowish light brown, very closely to closely jointed, hard to very hard rock GNEISS. Moderately to highly weathered in places, white to grey banded greenish
MG-CG	GF	IF-VIF	10-30	1 2	20-30 50-60	CJ MJ-WJ	RJ RJ	Chl + FeOx FeOx	<1 <1	9.28	87	87	64	9	- 8 - 8 - 9			brown speckled to streaked black and maroon, closely jointed, <u>soft</u> to <u>medium hard</u> rock with <u>hard</u> in places, biotite GNEISS. Notes: 1. Brecciated zones present at 6.3m to 6.4m and 7.7m to 8.0m. 2. Green to brown staining on joints.
										10.09	94	94	36	19	10		10.09	
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m _j	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	-		_

HOLE No: Plant-BH04 Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded FF -foliated

CF -cleaved SF -schistose

GF -gneissose

JOINT SPACING VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

SLJ-slickensided MG -medium grain SJ -smooth CG -coarse grain RJ -rough

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT PLANT SITE

GEOTECHNICAL INVESTIGATION

HOLE No: Plant-BH04 Sheet 2 of 2

JOB: 3010047806

			LF -lamina	ted CJ -c MJ -ı WJ -ı	very close spacing medium spacing wide spacing -very wide spac	PLA-plana	r Ilating ed	van-very so.	it rook	ı	II.		-	1	-	1		_		٦
																			NOTES	od st
																		1)	Perched water table encountered 7.4m.	ed at
																		2)	NA: Not applicable.	
																		3)	NM: Not measurable.	
																		4)	Sample PLT-BH04/1 taken 2.61m2.78m.	at
																		5)	Sample PLT-BH04/2 taken 5.21m5.35m.	at
Grain	Pook	Eabria	Eahria	loint	loint	loint	Mioro	,	loint	E:II	Donth /m) Mot	Pools	POD	Frac	DEPT	·u			
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Fi	loint illing	Fill Thickness (mm)	Depth (m) Mat recov %	Rock recov %	RQD %	Freq No/m	Scale 1:50	9			
														CONTRA	CTOR : RA	Longs	staff	INCLINATION : Vertical DIAM :	COORDINATE SYSTEM: UTM,V X-COORD: 802119	VGS84(33

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: Chris Homan

TYPE SET BY : EM SETUP FILE: KPBHC008.SET DIAM:

DATE:

DATE: 1 June 2023

DATE: 07/07/2023 14:19 TEXT: ..\PROFILES\PKFPLANTBH.TXT

x-coord : 802119

Y-COORD: 7583643 HOLE No: Plant-BH04 HOLE No: SWD-BH01 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

MF -massive FG -fine grained BF -bedded MG -medium grain

CF -cleaved

FF -foliated

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

SLJ-slickensided SJ -smooth CG -coarse grain RJ -rough

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock JOINT SHAPE

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH01 Sheet 1 of 3

MG	GF	IF	80	1 2	0-10 40-50	M1 AC1-C1	RJ RJ	Sand Silt	1-2	8.09	99	99	44	11	7	7.4m_ <u>\</u>		 From 7.00m to 7.50m highly broken zone. Thick pegmatite vein from 7.50m to 7.72m.
										6.88	100	100	80	10	- - - - - - - -		3.30 5	Highly weathered, pale brown speckled black and white, very soft to soft rock granitic GNEISS. Notes:
										5.11	96 87	96 87	86 51	9	- - - - - - -		5.50	4.08m to 4.09m.
G-CG	MF	NM	NM	1 2 3	0-10 20-30 40-50	M1 C1	RJ RJ RJ	Silty sand Silty sand Silt	2-5 1-2 2	4.32	100	100	78	7	- - - - - - - - - - - - - - - - - - -	SWD-BH01/4		Moderately weathered, light brown speckled black and grey mottled white, medium hard rock GNEISS. Note: 1. Thin 10mm zone of silty sand at
										3.50	100	75	40	10	- - - - -	SWD-BH01/3	2.80	Extremely soft rock zone at 2.22m to 2.37m. Interpreted as silty sand (residual Gneiss)
G-CG	MF	NM	NM	1 2	0-10 20	CJ CJ-CJ	RJ RJ	Silt Silty sand	1-2 1-2	3.00	90	75	75	11	2	SWD-BH01/1 ● SWD-BH01/2 ● [Highly weathered, dark brown with red iron oxide staining, very soft rock granitic GNEISS. Note:
A	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	92 SPT	0 N=Ref	0	NA	_ 1 - - - - - - - - - - -	-0.10 10.1 11.0 11.0 11.0 11.0	0 0 1 0 1.85	medium to coarse quartz with cobble fragments.
										0.72	97	0	0	NA		1:50]	0.00	Dark brown, gravelly silty fine to medium SAND. ALLUVIUM . Note: 1. Gravel comprises subrounded

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold HOLE No: SWD-BH01 **OMITIOMIRE COPPER PROJECT** HOLE No: SWD-BH01 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock Sheet 2 of 3 Sheet 2 of 3 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock **GEOTECHNICAL INVESTIGATION** FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 JOB: 3010047806 CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock VSR-very soft rock CUR-curvilinear GF -gneissose VCJ-very close spacg LF -laminated CJ -close spacing PLA-planar MJ -medium spacing UND-undulating WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular 11.06 | 100 - 100 52 11 Highly weathered, brown speckled black banded white, soft rock GNEISS. 12.50 100 100 61 8 12 VCJ-CJ 0-10 RJ Silty sand 20-30 VCJ-CJ RJ 2 13 2 MG GF IF 10-30 3 40-50 CJ RJ Stained & Silt 1-3 14.03 99 99 54 9 NM RJ Stained 70 14 95 15.50 95 71 8 14.85 15 Slightly weathered to unweathered, grey speckled black mottled white, hard to very hard rock granitic GNEISS. Note: 16 1. Moderately fractured from 23.00m 16.76 100 100 83 6 to 23.70m. 2. Completely weathered zone at 22.85m to 22.92m. 17 18.60 100 100 84 4 18 19 0-10 CJ-MJ RJ Clean 20.09 100 100 89 3 2 30-40 VWJ RJ 2-3 Silt MG-CG MF-GF NM NM 3 40-50 WJ RJ Silt & stained 1 80-90 WJ RJ Silt 1 20 Fill Depth (m) Mat RQD DEPTH Grain Rock Fabric Fabric Joint Joint Joint Micro Joint Rock Frac Size Fabric Spac Inc Set No. Inc Rough-Filling Thickness recov % recov % Freq Scale Spac (mm) (deg) (Deg) ness (mm) No/m 1:50

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold **OMITIOMIRE COPPER PROJECT** HOLE No: SWD-BH01 MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock Sheet 3 of 3 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock **GEOTECHNICAL INVESTIGATION** FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock VSR-very soft rock CUR-curvilinear GF -gneissose VCJ-very close spacg LF -laminated CJ -close spacing PLA-planar MJ -medium spacing UND-undulating WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular 21.39 | 100 100 100 2 21 22 22.82 100 100 100 0 23 88 88.00 24.27 55 8 24 24.27 **NOTES** 1) Perched water table encountered at 7.4m. 2) NA: Not applicable. 3) NM: Not measurable. 4) Sample SWD-BH01/1 2.04m--2.26m. 5) Sample SWD-BH01/2 2.39m--2.51m. 6) Sample SWD-BH01/3 3.14m--3.48m. 7) Sample SWD-BH01/4 3.36m--3.95m. Grain Fabric Micro Depth (m) Mat Rock RQD Frac DEPTH Rock Fabric Joint Joint Joint Joint

> No/m CONTRACTOR: RA Longstaff MACHINE: Sullivan HD

Scale

1:50

DRILLED BY PROFILED BY: S YaFrance

Freq

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

DIAM: DATE: 13-15 May 2023 DATE: 14-15 May 2023

INCLINATION: Vertical

DATE: 07/07/2023 14:21

COORDINATE SYSTEM: WGS84 UTM 33S X-COORD: 801384 Y-COORD: 7581911

HOLE No: SWD-BH01

taken at

taken at

taken at

taken at

HOLE No: SWD-BH01

Sheet 3 of 3

JOB: 3010047806

TEXT: ..51\PROFILES\PKFSWDBH.TXT

Fabric

Spac

(mm)

Inc

(deg)

Set No.

Inc

(Deg)

Rough-

ness

Filling

Thickness

recov %

recov %

Size

HOLE No: SWD-BH02 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain CG -coarse grain

FF -foliated CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

JOINT SHAPE

PLA-planar

EHR-extremely hard rock SJ -smooth VHR-very hard rock RJ -rough HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02 Sheet 1 of 3

				WJ -	medium spacin wide spacing	STE-steppe	ed											
NA	NA	NA	NA	NA	-very wide spa	cng IRR-irregul	nA NA	NA	NA	0.74	64	0	0	NA	-	Scale	0.00	Dark brown, clayey silty SAND, displaying shrinkage cracks. ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	67	0	0	NA	1		1.05	Dark brown to red, clayey sandy GRAVEL. Coarse ALLUVIUM .
1471			1471	1471	147	10.0	147.	10.0	10/1	1.87		SPT=Ref	<u> </u>	NA ,		10, 7.01		Notes: 1. Gravel is subrounded to
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	_ 100 _	0		, <u>INA</u>	, 			sub-angular fine to medium grained
										3.00	92	84	27	10	_ 3	SWD-BH02/1 ●[quartz. 2. SPT refusal possibly caused by presence of gravel. 3. Core loss of 0.25m from 1.05m to
										4.54	99	84	50	11	_ 4		2.08	1.50m. Presumed to be due to washing out of fines. Dark brown to grey speckled black and white, silty SAND. RESIDUAL
MG	GF	IF	10-20	1 2	5-10 40-50	MJ VCJ-CJ	RJ RJ	Silt Silt	1 1-2						5			GNEISS.Note:1. On desiccation material increases in strength to very soft to soft rock, described as completely to highly
				۷	40-30	IVIJ	NJ	Siit	1-2	6.07	98	98	59	10	6		2.34	weathered, grey speckled black and white, medium grained, very closely jointed, very soft to soft rock gneiss. Highly to moderately weathered,
										7.60	100	97	63	9	7			brown to grey speckled black banded white, <u>soft</u> to <u>medium hard</u> rock GNEISS. Notes: 1. Quartz vein from 3.35m to 3.39m.
																		2. Completely weathered, extremely soft rock zone at 3.35m to 3.50m;
										8.81	94	92	52	14	_ 8 _ _	SWD-BH02/2 SWD-BH02/3	7.00	4.80m to 4.88m; and 5.51m to 5.54m, tending to residual gneiss. 3. Thick pegmatite vein from 7.30m to 7.50m.
MG	GF	IF	10-20	1	0-10	AC1-C1	RJ	Silt	1-3						- - - - 9		7.90	Moderately weathered, grey to brown streaked yellow and white, soft rock GNEISS.
										10.34	100	100	36	12	10			
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	-	10.34	

HOLE No: SWD-BH02 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated

CF -cleaved

SF -schistose JOINT SPACING LF -laminated CJ -close spacing

CG -coarse grain RJ -rough JOINT SHAPE GF -gneissose VCJ-very close spacg CUR-curvilinear PLA-planar

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided SJ -smooth

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02 Sheet 2 of 3

				WJ -	medium spacing wide spacing -very wide spacr	STE-steppe	ed				_				
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	11.88	27	6	0	12	Dark brown speckled white, gravelly sandy silt. RESIDUAL GNEISS .
										12.64	100	100	26	8	12.40
										13.61	96	93	43	12	1. Completely weathered, extremely
										15.11	87	83	15	17	soft rock zone from 14.55m to 14.82m; 16.44m to 16.52m; and 16.98m to 17.03m, with core loss attributed to washing out of residual soil in these zones.
MG	GF	IF	20-30	1	5-10	VCJ	RJ	Silt	<1	16.61	100	100	43	11	
										18.11	96	93	40	15	17
				1	0-10	VCJ	RJ	Stained	<1	18.54	100	100	26	19	18.20
MG	MF-GF	NM	NM	2 3	20-30 80-90	CJ NM	RJ RJ	Stained Clean	<1 -	19.61	100	98	87	6	GNEISS. Note: 1. Near vertical joint from 18.29m to
										21.11	100	97	97	2	20 18.40m. Unweathered, grey speckled black and white, very hard rock granitic GNEISS.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	g Scale

HOLE No: SWD-BH02 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded FF -foliated

CF -cleaved

MG -medium grain CG -coarse grain

SLJ-slickensided SJ -smooth RJ -rough

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH02 Sheet 3 of 3

JOB: 3010047806

	I	SF -schist GF -gneis LF -lamina	ose JOIN sose VCJ- ated CJ-c MJ-1 WJ-	IT SPACING very close spaciose spacing medium spacing wide spacing -very wide spa	PLA-plana	APE Silinear \ nr ulating ped	NHR-medium hard rock SR -soft rock VSR-very soft rock	ı		-	Г			.	-WとノI				
-CG MF-GF	NM	NM	1	0-10	MJ-WJ	RJ	Stained	<1	22.53	100	100	98	1	21					
									24.11	100	100	100	1	_ 23 _ _ _ 24					
									25.11	100	100	100	0	25		25.11 NOTES 1) NA: Not applic	cable.		
																2) NM: Not meas3) Sample SV 2.84m2.98m4) Sample SV	ND-BH02/1 ı.	taken taken	
																7.96m8.16m 5) Sample SV 8.16m8.35m	n. ND-BH02/3	taken	
ain Rock ze Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50					

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET DIAM:

DATE: 17 April 2023 DATE: 18 April 2023

DATE: 07/07/2023 14:21 TEXT: ..51\PROFILES\PKFSWDBH.TXT

X-COORD: 801681

Y-COORD: 7581852 HOLE No: SWD-BH02 HOLE No: SWD-BH03 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

FF -foliated

CF -cleaved

BF -bedded MG -medium grain

CG -coarse grain

SF -schistose JOINT SPACING

GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

JOINT SHAPE PLA-planar UND-undulating

SJ -smooth

RJ -rough

HR -hard rock

SR -soft rock

VSR-very soft rock

MHR-medium hard rock

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH03 Sheet 1 of 3

				WJ -	medium spacir wide spacing -very wide spa	ng UND-undu STE-steppe acng IRR-irregul	ed								
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.74	100	0	0	NA	16.1.6
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	Orange brown, sandy clayey SILT. ALLUVIUM.
ИG	GF	-	-	1	0-10	VCJ-CJ	RJ	Silt	1	2.05	100	100	55	9	Completely weathered, brown to grey banded white, soft rock GNEISS.
										3.54	100	97	67	11	Highly to moderately weathered, brown to grey streaked white, medium hard rock with zones of soft rock GNEISS. Notes: 1. Highly broken zone from 6.20m to
										4.79	93	93	70	8	6.97m. 2. Core loss from 6.30m to 7.35m due to washing out of fines.
ИG	GF	VIF	10-20	1 2	0-10 70	NM VCJ-CJ	RJ RJ	Silt + Sand FeOx & Calcite	1 2	6.30	96	92	58	11	6
										7.35	84	70	10	13	7
										8.85	100	98	41	13	7.83 Completely weathered, dark grey to
ИG	GF	VIF	10-20	1 2	0-10 60	CJ NM	RJ RJ	Silt FeOx	1 <1	0.00	100	30	71		dark brown with mottled orange joints, very soft to soft rock GNEISS. Note:
				_											1. Clay zone at 7.83m to 7.85m.
										10.40	98	98	45	14	Slightly weathered gray speckled
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	Scale

HOLE No: SWD-BH03

Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated

CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

SJ -smooth CG -coarse grain RJ -rough JOINT SHAPE

PLA-planar

UND-undulating

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock HR -hard rock

MHR-medium hard rock SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH03 Sheet 2 of 3

				WJ -	medium spacing wide spacing -very wide spa	STE-stepp acng IRR-irregu	ped			П		_	ı		
MG	GF	VIF-IF	10-20	1 2	0-10 40	VCJ-CJ NM	RJ RJ	Pyrite FeOx & Calcite	<1 <1	11.95	100	100	41	14	
										13.50	100	100	83	8	Slightly weathered to unweathered, dark grey speckled black mottled white and pink streaked and banded white, very hard granitic GNEISS. Note:
										15.05	100	99	85	6	1. Abundant pegmatite inclusions of varying sizes. 14.6m ▼ 15
1G-CG	GF	IF	10-20	1 2 3	0-10 30 65	CJ VCJ CJ-MJ	RJ RJ RJ	Silt & Clean FeOx Clean	<1 <1 -	16.55	100	100	99	5	
										18.10	99	99	85	5	17
										18.40	100	100	33	7	
										19.75	100	100	73	7	19
										20.23	100	100	85	4	20.23
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	C DEPTH q Scale

HOLE No: SWD-BH03 Sheet 3 of 3 JOB: 3010047806

BF -bedded FF -foliated CF -cleaved

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

MG -medium grain SJ -smooth CG -coarse grain RJ -rough

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH03 Sheet 3 of 3

JOB: 3010047806

			SF -schistose GF -gneissose LF -laminated	JOINT SPACING VCJ-very close space CJ -close spacing MJ -medium spacing WJ -wide spacing VWJ-very wide space	PLA-planar UND-undulating STE-stepped	SR -soft rock VSR-very soft rock								
				vivo voiy wide spaci	ng navinogulai						_		NOTES 1) Perched water to 14.6m.	able encountered at
													2) NA: Not applicable	
													3) NM: Not measurab	le.
													4) Sample SWD-B 3.82m4.13m.	the at the second secon
Grain	Rock	Fabric	Fabric J	loint Joint	Joint Micr	o Joint	Fill Depth (n	n) Mat	Rock	RQD	Frac	DEPTH		
Size	Fabric	Spac (mm)	Inc Se (deg)	et No. Inc (Deg)	Spac Roug nes		Thickness (mm)	recov %	recov %	% 	Freq No/m	Scale 1:50 A Longstaff ullivan HD	INCLINATION : 90 deg COORDINATI	E SYSTEM : WGS84 UTM 33S

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

DIAM: DATE: 9-10 May 2023 DATE: 10-11 May 2023

DATE: 07/07/2023 14:21

TEXT: ..51\PROFILES\PKFSWDBH.TXT

X-COORD: 801768

Y-COORD: 7581835 HOLE No: SWD-BH03

HOLE No: SWD-BH04 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain

FF -foliated CF -cleaved

CG -coarse grain

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

SLJ-slickensided SJ -smooth

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock RJ -rough HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH04 Sheet 1 of 3

				VWJ	-very wide spad	cng IRR-irregu	lar			0.04	400				-	Scale - 1:50]	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	Brown, slity fine to medium grained
										0.84	100	0	0	NA				SAND. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	41	0	0	NA	_ 1	_ - - -		
										1.95	SPT	N=9	0	NA NA		- - -		
										2.00	100 96	0	0	NA NA	,	-	2.2	Brown to grey, silty fine to medium
NΑ	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.06	100	29	29	0	-			grained SAND. RESIDUAL GNEISS . Note:
										3.00	100	29		0	_ 3			1. Cobble sized fragments at 2.44m to 2.58m.
IG	GF	IF	10-30	1 2	5-10 40	M1 AC1	RJ RJ	FeOx Silty sand	<1 1-2	4.63	100	83	27	13		SWD-BH04/1 ⊕[2.9	Moderately weathered, grey speckled black and white, soft to medium hard
				3	50-60	CJ	RJ	FeOx	<1						_ 4	_ - - -		rock GNEISS. Note:
															-	- - - -		1. Highly broken zone from 3.06m to 3.20m.
										5.94	65	0	0	0	5		4.5	Grey to brown, silty fine to medium SAND. RESIDUAL GNEISS .
A	NA	NA	NA	NA	NA	NA	NA	NA	NA						-	- - - - -		Notes: 1. From 4.72m to 4.84m and 5.38m to
										6.63	100	0	0	0	- 6	-		5.52m alternating zones of extremely soft to very soft rock
																-		gneiss. 2. Core loss of 0.46m from 4.63m to
										7.63	100	100	31	11	_ 7	<u>-</u>		5.94m, due to washing out of fines.3. Near vertical joint from 8.34m to 8.84m.
															-	- - - -	6.6	Moderately weathered, grey speckled
															_ 8	SWD-BH04/2		black streaked white, soft to medium hard rock GNEISS.
G	GF	IF	0-20	1	0-10	CJ	RJ	Sand	1	9.03	100	100	23	19		0112 2110 112 →		Notes: 1. From 6.90m to 7.94m; 8.13m to 8.20m; 8.75m to 9.09m and 9.90m
-	- .		0	2	80-90	CJ	RJ	FeOx & Silt	2-3						9	- - - -		to 10.74m alternating zones of completely weathered extremely
															-	- - -		soft to very soft rock gneiss. 2. Pegmatite vein from 9.09m to
										10.54	100	100	40	12	_ 10	- - - -		9.90m.
rain	Rock	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint	Fill	Depth (m)	Mat	Rock	RQD	Frac				

HOLE No: SWD-BH04 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated

CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

CG -coarse grain

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

SJ -smooth RJ -rough

JOINT SHAPE

UND-undulating

PLA-planar

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH04 Sheet 2 of 3

-				WJ -	meaium spacir wide spacing -very wide spa	STE-steppe scng IRR-irregul	ed								
										12.01	100	100	78	8	medium hard rock granitic GNEISS. Notes: 1. Near vertical joint at 12.85m to
MG	GF	IF	20-30	1 2 3	0-10 70 80-90	VCJ-CJ NM VWJ	RJ SJ RJ	Silt Clayey silt FeOx & Silt	1 1 2-3	13.56	96	96	30	15	13.20m. 2. Highly broken zone at 13.30m to 13.40m and 14.55m to 14.60m.
										15.05	100	100	56	10	14.6m \(\frac{\nabla}{\sqrt{\sq}}\sqrt{\sq}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}
										15.56	100	100	54	10	0
										16.57	100	100	100	1	grey speckled black banded white, hard to very hard rock GNEISS. Notes:
				1	0-10	VCJ-CJ	RJ	Silt	1	17.98	97	95	67	9	1. From 16.57m to 16.60m completely weathered extremely soft rock zone. Interpreted as silty sand (residual gneiss). 2. Near vertical joint at 16.70m to 17.00m; 18.15m to 18.49m and 20.12m to 20.35m. 3. Highly broken zone from 19.36m to
MG	GF	IF	10-20	2	70-80	VWJ	SJ	Calcite	2-3	19.53	100	100	34	13	3 19.53m.
										21.03		100	68	7	
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)		Rock recov %	RQD %	Frac Freq No/m	eq Scale

HOLE No: SWD-BH04 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

FF -foliated

CF -cleaved

BF -bedded

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear

MG -medium grain SJ -smooth CG -coarse grain RJ -rough JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

DATE: 12 May 2023 DATE: 12 May 2023

DATE: 07/07/2023 14:21

TEXT: ..51\PROFILES\PKFSWDBH.TXT

HOLE No: SWD-BH04 Sheet 3 of 3

JOB: 3010047806

		ı	LF -lamina	MJ -r WJ -ı	lose spacing nedium spacing wide spacing very wide spac	STE-stepp	ılating ned		ı	11		1	,	-		-1/	
															_ 21	21.03	
																1	NOTES
																1) [Perched water table encountered a 4.6m.
																2) 1	NA: Not applicable.
																3) 1	NM: Not measurable.
																4) 5	Sample SWD-BH04/1 taken a 3.71m3.88m.
																5) \$	Sample SWD-BH04/2 taken a 3.23m8.39m.
ain ze	Rock Fabric	Fabric Spac	Fabric Inc	Joint Set No.	Joint Inc	Joint Spac	Micro Rough-	Joint Filling	Fill Thickness	Depth (m)	Mat recov % I	Rock recov %	RQD %	Frac Freq	DEPTH Scale		
		(mm)	(deg)		(Deg)		ness		(mm)					No/m	1:50	INCLINATION : Vertical	COORDINATE SYSTEM: WGS84 U
													MA	CHINE : S	A Longstaff ullivan HD	DIAM:	<i>x-coord :</i> 801678

DRILLED BY:

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

Y-COORD: 7581770

HOLE No: SWD-BH04

HOLE No: SWD-BH05 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain

FF -foliated CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing MJ -medium spacing

CG -coarse grain

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

SJ -smooth RJ -rough

JOINT SHAPE

PLA-planar

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH05 Sheet 1 of 3

			Lr -lamina	MJ -r WJ -ı	nose spacing medium spacing wide spacing -very wide spac	PLA-piana UND-undu STE-stepp ng IRR-irregu	lating ed											
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	100	0	0	NA	-	1:50	0.0 0.0 0.0 0.0	Brown speckled white, gravelly silty medium to coarse SAND. Fine ALLUVIUM .
										1.50	98 SPT	0 N=7	0	NA	2		1.0	Brown to dark brown, silty sandy CLAY. Fine ALLUVIUM . Notes: 1. SPT indicates soft consistency.
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	100 F	0	0	NA NA	3	-		Scattered traces of subrounded to rounded medium grained quartz gravel.
										3.45 3.65 4.14	SPT SPT 96	N=8 N=Ref 0	0	NA	_ 3	-	3.9	5
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.64	98	0	0	NA	_ 7	-		Brown to dark grey, sandy SILT. RESIDUAL GNEISS . <u>Note</u> : 1. Quartz veins at 4.78m to 4.80m.
-G-MG	GF	IF	0-10	1	0-10	CJ	RJ	Silt	<1	6.14	100	88	82	NA 5	6	SWD-BH05/1 <mark>⊕</mark> [-	5.3	Highly to completely weathered, dark grey to dark brown speckled black, very soft rock GNEISS.
										7.44	100	100	48	12	7		6.3	Moderately weathered, grey to brown speckled black streaked white, medium hard rock GNEISS. Notes: 1. Near vertical joint at 11.24m to
										9.03	94	91	31	11	8	8.05m <u> </u>		11.50m. 2. Highly broken zone from 15.30m to 15.42m. 3. Pegmatite veins at 12.96 to 13.21m.
															9			
Grain	Pools	Enhrin	Fabric	loint	loint	loint	Mioro	loint	Fill	10.53	100	100	55 RQD	5 Frac		SWD-BH05/2 ● [[]		
Grain Size	Rock Fabric	Fabric Spac (mm)	lnc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	KQD %	Frac Freq No/m	Scale 1:50			

Г			
	HOLE No: Shee	SWD-BH et 2 of 3	105
	<i>JOB</i> : 301	1004780)6
	MG	GF	V
	MG	GF	V

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain

FF -foliated

SF -schistose JOINT SPACING

CF -cleaved

SLJ-slickensided SJ -smooth CG -coarse grain RJ -rough

JOINT SHAPE

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH05 Sheet 2 of 3

r	ı	ı	GF -gneiss	ose VCJ- ted CJ-c MJ- WJ-	very close spa close spacing medium spacin wide spacing I-very wide spa	PLA-plana	linear r lating ed	VSR-very soft rock	ı	II		г	ı	ı	7 L -1/1
MG	GF	VIF	10-20	1 2 3	0-10 60 80-90	AM1 C1	RJ RJ RJ	Silty sand FeOx & Calcrete FeOx	1 <1 <1	12.08	100	100	50	10	11
										13.61	100	100	29	13	13
										15.14	92	92	65	10	14
										16.64	100	88	56	11	Slightly weathered, grey to brown speckled black streaked white, medium hard rock GNEISS. Notes: 1. Completely weathered extremely
MG	GF	VIF	10-20	1 2	0-10 20	ACT-CT CT	RJ RJ	Silt Silt	1 1	18.14	95	85	60	10	soft rock zones at 16.64m to 16.70m; 17.00m to 17.08m and 1.89m to 18.93m. 2. Interpreted as residual gneiss, comprising dark brown silty sand.
										19.64	100	92	83	5	19
										20.01	100	100	100	5	20.01 NOTES 1) Perched water table encountered at 8.05m.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)		Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale

HOLE No: SWD-BH05 Sheet 3 of 3 JOB: 3010047806

D079 E Mouton

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained MG -medium grain BF -bedded FF -foliated CG -coarse grain

CF -cleaved

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided SJ -smooth RJ -rough

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

Knight Piésold

OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION HOLE No: SWD-BH05 Sheet 3 of 3

JOB: 3010047806

Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Si Inc Si (deg)	Joint Join et No. Inc (Deg	Spa		h- Filling	Fill Depth (n Thickness (mm)	n) Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50			
														5) Sa 9.6	ample SWD-BH05/2 59m9.93m.	taken at
														3) NM 4) Sa	A: Not applicable. M: Not measurable. Imple SWD-BH05/1 38m6.02m.	taken at
	1	I	SF -schistose GF -gneissose LF -laminated	JOINT SPACI. VCJ-very close CJ -close space MJ -medium s WJ -wide space VWJ-very wide	e spacg CU cing PL pacing UN cing STI	NT SHAPE R-curvilinear A-planar D-undulating E-stepped R-irregular	SR -soft rock VSR-very soft rock	1 11	, -	Г	ı	ı -	1 1	_		

MACHINE: Sullivan HD

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM

DIAM: DATE: 20-22 May 2023 DATE: 21-22 May 2023

DATE: 07/07/2023 14:21

X-COORD: 801740

Y-COORD: 7581876 HOLE No: SWD-BH05 HOLE No: SWD-BH06 Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded

FF -foliated

CF -cleaved

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated C.L -close spacing

CG -coarse grain

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided MG -medium grain

SJ -smooth RJ -rough

JOINT SHAPE

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH06 Sheet 1 of 2

			GF -gneiss LF -lamina	nted CJ -c MJ -ı WJ -ı	very close spa close spacing medium spacin wide spacing -very wide spa	PLA-plana	ar ulating ped	/SK-Very soft rock									
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	86	0	-	NA	Sca 1::	50 1 1 2 2 1 1 2 2 1 1 2 2 1 1 2 2 1 1 2 2 1 2 2 1 2 2 1 2	Dark brown, clayey silty coarse SAND. Fine ALLUVIUM . Note:
										1.50	100	0	-	NA	1	0.80	Slight core loss due to washing out of fines.
 NA	NA	NA	NA	NA	NA NA	NA	NA	NA	NA	1.95	SPT1	N=37		N ALA		10002 10001 10001 10001	Orange brown to brown with depth, clayey silty SAND to clayey silty coarse SAND, with traces of
		1.0.								3.00	100 } 64	0	-	NA NA] - 2 		sub-angular to subrounded, fine grained gravel of quartz. Fine ALLUVIUM .
NA NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.19	SPT2	N=Ref			3	3.00	Red to brown, silty coarse SAND.
	INA	INA	INA	INA	INA	INA	INA	INA	INA	3.75	100	0	-	NA	-		ALLUVIUM. Note: 1. Material is predominantly coarse
										4.05	100	0		NA NA	_		sand of quartz of is coarser than layers above.
										4.70	100	U	-	INA	5	3.70	Red to brown, clayey silty coarse SAND, with minor sub-angular to
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA		400						subrounded gravel and cobbles of quartz. Coarse ALLUVIUM .
										6.26	100	11	0	NM	6		Notes: 1. Dark red, clayey silty sand with minor sub-angular quartz gravel,
															- - - - - - -	6 20	alluvium band between 3.90m and 4.10m. 2. Material very intact in core box.
NA NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	7.76	75	17	0	NM	7	6.30	Light red to brown, clayey silty SAND, with traces of subrounded white gravel
															-		and cobbles of quartz with depth. Coarse ALLUVIUM . Notes:
										8.29	100	100	28	28	_ 8		Material slumps after placing in core tray.
										9.26	100	100	15	19	9	7.60	Gravel and cobbles stained dark red on surfaces.
															- [
										10.76	100	100	70	13	10		
Grain	Rock	Fabric	Fabric	Joint	Joint	Joint	Micro	Joint		Depth (m)	Mat	Rock	RQD	Frac	DEPTH	-	
Size	Fabric	Spac (mm)	Inc (deg)	Set No.	Inc (Deg)	Spac	Rough- ness	Filling	Thicknes (mm)		recov %	recov %	%	Freq No/m	Scale 1:50	VA	

HOLE No: SWD-BH06 Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded

FF -foliated CF -cleaved

SF -schistose

CG -coarse grain GF -gneissose VCJ-very close spacg LF -laminated CJ -close spacing

JOINT SPACING

MG -medium grain

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

JOINT SHAPE

CUR-curvilinear

PLA-planar

EHR-extremely hard rock VHR-very hard rock SJ -smooth RJ -rough HR -hard rock

MHR-medium hard rock SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

DATE: 1 June 2023

DATE: 07/07/2023 14:21

TEXT: ..51\PROFILES\PKFSWDBH.TXT

HOLE No: SWD-BH06 Sheet 2 of 2

JOB: 3010047806

		WJ -wide spacing VWJ-very wide spacn	STE-stepped g IRR-irregula						-		-	der der	hly to moderately weathered with oth, grey banded greenish brown to
CG GF IF	30-50	1 20-30 2 40-50	M1 C1	RJ RJ	FeOx & Chl FeOx	<1 <1	12.26	100	100	80	9	sta mo har ver No	k grey speckled black to green and ned orange on joints, closely to derately jointed, soft to medium do rock with depth, with pockets on y soft rock biotite GNEISS. es: Completely weathered zone
							13.76	100	100	56	17	13	present throughout zone at 8.10m to 8.30m; 12.80m to 13.00m; 4.10m to 14.20m; 14.57m to 4.67m. Tends to migmatite in places, from the below 13.20m with distinct
							15.27	99	99	85	10	_ 14	otygmatic folding.
													TES : Not applicable.
												2) NN	: Not measurable.
n Rock Fabri e Fabric Spa		Joint Joint et No. Inc	Joint Spac	Micro Rough-	Joint Filling	Fill Thickness	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq	DEPTH Scale	

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

HOLE No: SWD-BH06

HOLE No: SWD-BH07 Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive BF -bedded FF -foliated

CF -cleaved SF -schistose

GF -gneissose LF -laminated

JOINT SPACING VCJ-very close spacg CJ -close spacing MJ -medium spacing

FG -fine grained

CG -coarse grain

SLJ-slickensided MG -medium grain

SJ -smooth RJ -rough

JOINT SHAPE

CUR-curvilinear

UND-undulating

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock

MHR-medium hard rock SR -soft rock VSR-very soft rock

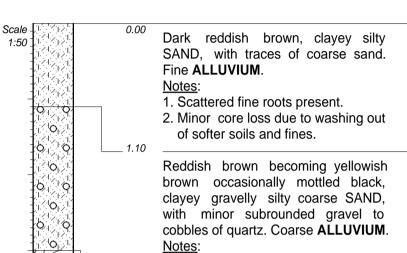


OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH07 Sheet 1 of 2

JOB: 3010047806

						cng IRR-irregu				0.84	81	0	_	NA	-
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.04	01	0	-	INA	1
										1.50	100	0	-	NA	-
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	2.00	100	0	-	NA	_ 2
										3.00	97	0	-	NA	3
										4.50	83	69	14	13	_ 4
										6.05	100	100	54	14	5
										7.55	100	100	71	15	7
				4	20.20	VCLCL	61	E00v % Chl % ~:		9.05	100	100	75	13	8
G-CG	GF	IF-VIF	20-30	1 2	20-30 40-50	MJ VCJ-CJ	SJ RJ	FeOx & ChI & mica FeOx & ChI & mica		10.60	100	100	81	12	10
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	↓ DEPTH Scale 1:50



1. Zone is strongly ferruginised.

- 2. Black and orange staining on cobble surfaces.
- 3. Minor core loss due to washing out of fines.

Moderately weathered becoming slightly weathered from 12.50m, white banded grey and light brown speckled and streaked black with orange and light green staining on joints, closely to moderately jointed, medium hard to hard rock with depth, biotite GNEISS. Notes:

- 1. Rock breaks mostly along shallow dipping very distinct gneissic bands.
- 2. Quartz gravel present at top of layer. Interpreted as contamination from above.

HOLE No: SWD-BH07 Sheet 2 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive

BF -bedded FF -foliated CF -cleaved SF -schistose

GF -gneissose LF -laminated

MJ -medium spacing WJ -wide spacing

CG -coarse grain JOINT SPACING JOINT SHAPE VCJ-very close spacg CUR-curvilinear CJ -close spacing PLA-planar UND-undulating

VWJ-very wide spacng IRR-irregular

FG -fine grained

MG -medium grain

SLJ-slickensided SJ -smooth RJ -rough

STE-stepped

Micro

Rough-

ness

Joint

Joint

Filling

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

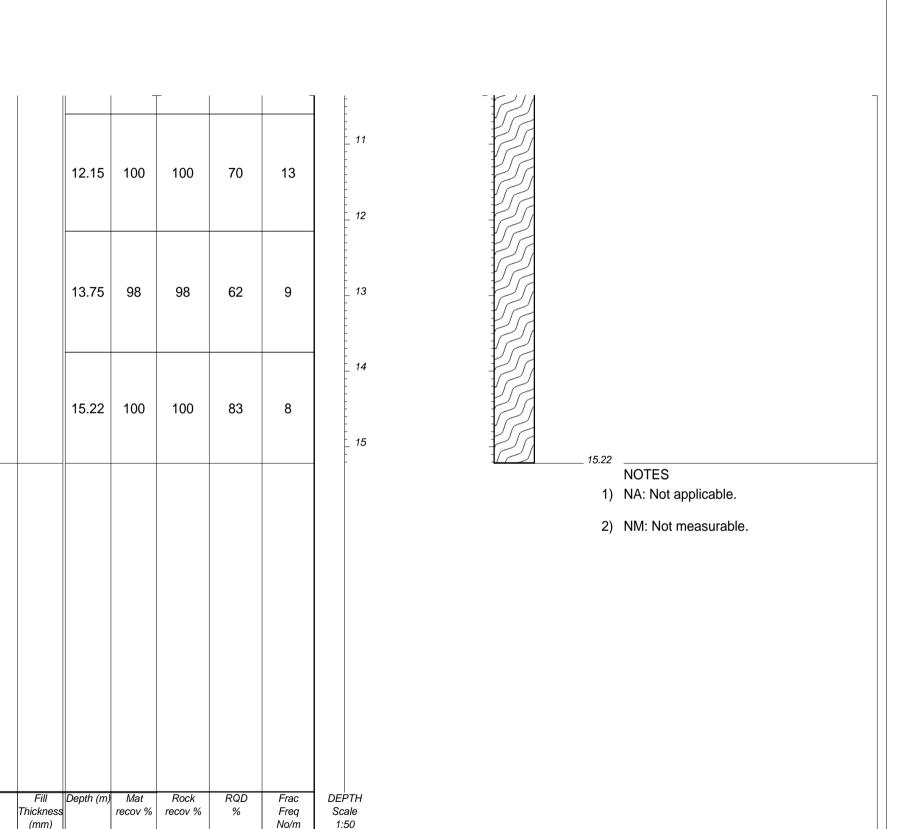
SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH07 Sheet 2 of 2

JOB: 3010047806



CONTRACTOR: RA Longstaff MACHINE: Sullivan HD

DRILLED BY PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET INCLINATION: Vertical DIAM: DATE: 2023

DATE: 1 June 2023

DATE: 07/07/2023 14:21 TEXT: ..51\PROFILES\PKFSWDBH.TXT

COORDINATE SYSTEM: WGS84 UTM 33S X-COORD: 804077

Y-COORD: 7582410 HOLE No: SWD-BH07

Rock

Fabric

Fabric

Spac

(mm)

Fabric

Inc

(deg)

Joint

Set No.

Joint

Inc

(Deg)

Grain

Size

HOLE No: SWD-BH08 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain

FF -foliated CF -cleaved

CG -coarse grain

SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear LF -laminated CJ -close spacing

SLJ-slickensided

SJ -smooth RJ -rough

JOINT SHAPE

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH08 Sheet 1 of 3

			LF -lamma	MJ -ı WJ -	nose spacing medium spacin wide spacing -very wide spa	ng UND-undu STE-steppi cng IRR-irregul	lating ed											
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	71	0	0	NA	-	Scale - T. 50 1:50	0.00	Light brown speckled white, silty medium to coarse SAND. Fine ALLUVIUM .
										1.50	100	0	0	NA	_ 1		1.20	Dark brown, silty clayey medium to
										1.95	SPT	N=13	0	NA R	2		400000 400000 400000 400000	coarse SAND. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	100	0	0	NA	-		(%) (%) (%) (%) (%) (%) (%)	
										3.45	SPT	N=10			_ 3		2.90	Brown, clayey silty medium to coarse SAND. Fine ALLUVIUM .
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	4.19	91	0	0	NA				Note: 1. Traces of coarse gravel and cobble fragments at 3.99m to 4.25m.
										4.78	100	0	0	NA			4.60	
										5.50	65	65	27	14	5	SWD-BH08/1		Highly weathered, grey to brown speckled black streaked white, soft rock GNEISS. Notes:
	0-		40.00					011		6.24	99	99	19	23	6			Completely weathered extremely soft rock zone from 6.94m to 7.09m. Interpreted as silty sand
MG	GF	IF	10-20	1	0-10	VCJ-CJ	RJ	Silt	1-2	7.77	92	76	39	10	7			(residual gneiss).2. Clay zone (completely weathered mafic zone) at 7.31m to 7.34m.
										9.30	100	100	59	12	- 8 		7.97	Moderately weathered, grey speckled black streaked white, medium hard rock GNEISS. Notes: 1. From 12.59m to 12.97m thick
															- [-			pegmatite inclusion. 2. Highly weathered very soft rock zone at 13.55m to 13.73m.
										10.80	96	93	59	11	_ 10	1		
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	- 1		

HOLE No: SWD-BH08 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated

CF -cleaved

CG -coarse grain

SLJ-slickensided SJ -smooth

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock RJ -rough HR -hard rock MHR-medium hard rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH08 Sheet 2 of 3

		I	CF -cleave SF -schisto GF -gneiss LF -lamina	ose JOIN cose VCJ- ted CJ-c MJ WJ-	IT SPACING Every close space close spacing medium spacing wide spacing -very wide space	PLA-plana UND-undu STE-stepp	ilinear ar ulating oed	MHR-medium hard rock SR -soft rock VSR-very soft rock	ı	II	-	г			7 ·
MG	GF	IF	80	1 2 3	0-10 20 60-70	M1 C1 C1	RJ RJ RJ	Sand Silt FeOx	1 <1 <1	12.30	97	97	51	11	
										13.80	97	93	47	11	13.70
G-VCG	MF	NM	NM	1	30-40	CJ	RJ	Silt	3	14.55	100	85 51	13	20 NM	Slightly weathered, white mottled pi hard rock PEGMATITE, w brecciation in places resembli shearing. Note: 1. Highly fractured.
										16.80	97	97	68	8	15 1. Highly fractured. Slightly weathered to unweather grey speckled black streaked a banded white, hard rock GNEISS. Note: 1. Thick pegmatite band from 16.15 to 16.55m.
MG	GF	IF	10-20	1 2 3	0-10 20 80	C1 C1 C1	RJ RJ RJ	Clean Silt FeOx	- <1 <1	18.30	100	99	64	10	17
										19.80	100	100	65	8	19
										20.23	100	100	28	16	
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint - Filling	Fill Thicknes (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	Scale

HOLE No: SWD-BH08 Sheet 3 of 3

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained SLJ-slickensided MG -medium grain BF -bedded SJ -smooth FF -foliated CG -coarse grain RJ -rough

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION HOLE No: SWD-BH08 Sheet 3 of 3

OB: 3010047806	FF -foliated CF -cleaved SF -schistose GF -gneissose LF -laminated	JOINT SPACING VCJ-very close spacg CJ -close spacing MJ -medium spacing WJ -wide spacing VWJ-very wide space	PLA-planar UND-undulating STE-stepped	HR -hard rock MHR-medium hard rock SR -soft rock VSR-very soft rock	I II	ı T	- 1		ı -		OB: 3010047806
										2) NM: Not measurable.	
										3) Sample SWD-BH08/ 5.07m5.21m.	1 taken at
rain Rock Fabrio	Fabric J	oint Joint	Joint Micr	ro Joint	Fill Depth (m)	Mat	Rock	RQD	Frac	DEPTH	

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET DATE: 15-16 May 2023 DATE: 15-16 May 2023

DATE: 07/07/2023 14:21

TEXT: ..51\PROFILES\PKFSWDBH.TXT

X-COORD: 804591 Y-COORD: 7583229

HOLE No: SWD-BH08

HOLE No: SWD-BH09 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE

MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated CF -cleaved

CG -coarse grain SF -schistose JOINT SPACING LF -laminated CJ -close spacing

RJ -rough JOINT SHAPE GF -gneissose VCJ-very close spacg CUR-curvilinear PLA-planar

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock VHR-very hard rock SJ -smooth HR -hard rock

MHR-medium hard rock SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH09 Sheet 1 of 3

			LF -lamina	MJ -ı WJ -	nedium spacing wide spacing	PLA-pianai ng UND-undui STE-steppi ncng IRR-irregul	lating ed											
NA	NA	NA	NA	NA	NA	NA NA	NA	NA	NA	0.84	100	0	0	NA		Scale 1:50 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.00	Light brown, silty medium to coarse SAND. Fine ALLUVIUM . Note: 1. Coarse subrounded gravel.
										1.50	100	0	0	NA			1.35	Cobble fragments at base of strata.
										1.95	SPT	N=8			- - - - - - - - - - - - - - - - - - -	124221 124221 124221		Brown, clayey silty fine to medium SAND. Fine ALLUVIUM .
										2.09	71 96	0	0	NA NA				Note: 1. Traces of subrounded fine to medium quartz gravel.
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.34	68	0	0	NA	- - - - 3			
										3.79	SPT	N=13						
										4.30	90	0	0	NA				
										4.84	100	0	0	NA			4.70	
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA						5			Dark brown speckled black and white, gravelly silty coarse SAND. RESIDUAL GNEISS.
										6.30	88	73	1	4	_ 6		5.17	Highly weathered, dark brown speckled black and white, very soft rock GNEISS.
MG	GF	NM	NM	1 2	0-10 80	CJ NM	RJ RJ	Silt Silt	1 1	7.80	83	62	15	8	7			Note: 1. From 6.20m to 6.30m and 7.60m to 7.80m extremely soft rock zone, recovered as silty sand (residual gneiss).
															- 8		7.80	
										9.24	99	99	77	2				Highly weathered, brown to grey speckled black streaked white, soft rock GNEISS. Note:
															- E 9			1. Very soft rock zone from 10.47m to 10.80m.
MG	GF	IF	0-20	1 2	10 30	CJ ACJ-CJ	RJ RJ	Silt Silt	1 1-2	10.80	100	100	62	10	_ 10			
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes: (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	-		

HOLE No: SWD-BH09 Sheet 2 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained BF -bedded MG -medium grain FF -foliated

CF -cleaved

SF -schistose JOINT SPACING

LF -laminated CJ -close spacing MJ -medium spacing

CG -coarse grain

JOINT SHAPE GF -gneissose VCJ-very close spacg CUR-curvilinear PLA-planar UND-undulating

RJ -rough

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided EHR-extremely hard rock SJ -smooth

VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH09 Sheet 2 of 3

					wide spacing -very wide spa	STE-stepp acng IRR-irregu									-1-1/1
				1	10	VCJ-CJ	RJ	FeOx	<1	12.30	100	98	73	10	11.76 Slightly weathered, grey streaked banded white, medium hard r
MG	GF	IF	0-20	2	30	CJ CJ	RJ RJ	Clean	-	13.03	100	100	59	14	GNEISS 1. Pyrite mineralisation at 12.10r 12.20m.
										13.80	99	99	99	4	Unweathered, dark grey speciblack streaked and banded where to very hard rock GNEISS.
										15.30	99	99	99	3	Note: 1. Quartz vein at 13.90 to 14.10m 16.65m to 16.84m.
MG-CG	GF	IF	10-20	1 2	10 20	CJ-MJ	RJ RJ	Clean Clean		16.80	100	100	95	7	
										18.30	100	100	93	5	
										19.80	100	100	97	4	19
										20.23	100	100	100	1	20.23
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes (mm)	Depth (m,	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	NOTES 1) NA: Not applicable. DEPTH Scale 1:50

HOLE No: SWD-BH09 Sheet 3 of 3 JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained SLJ-slickensided BF -bedded MG -medium grain SJ -smooth FF -foliated CG -coarse grain RJ -rough

GF -gneissose VCJ-very close spacg CUR-curvilinear

CF -cleaved

SF -schistose JOINT SPACING

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

JOINT SHAPE



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION HOLE No: SWD-BH09 Sheet 3 of 3

JOB: 3010047806

	MJ -medium spacing WJ -wide spacing VWJ-very wide spacing	STE-stepped	1		ı	II							
					1	II							
	VWJ-very wide spacno	g IRR-irregular				Ш							
						11				_		_	
			1										
												2) NM: N	lot measurable.
												–,	
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Fabric Jo	oint Joint	Joint	Micro	Joint	Fill	Depth (m)	Mat	Rock	RQD	Frac	DEPTH		
Inc Set	t No. Inc	Spac		Filling	Thicknes	s	recov %	recov %	%	Freq	Scale		
(ueg)	(Deg)		11692		(111111)	II			OCNITO	100/111	1.00 Λ L opgoto#	INOLINATION Vortical	COORDINATE SYSTEM: WGS84 UT X-COORD: 804688 Y-COORD: 7583254
	Fabric J Inc Se (deg)	Inc Set No. Inc	Inc Set No. Inc Spac	Inc Set No. Inc Spac Rough-	Inc Set No. Inc Spac Rough- Filling	Inc Set No. Inc Spac Rough- Filling Thicknes	Inc Set No. Inc Spac Rough- Filling Thickness	Inc Set No. Inc Spac Rough- Filling Thickness recov %	Inc Set No. Inc Spac Rough- Filling Thickness recov % recov %	Inc Set No. Inc Spac Rough- ness Filling Thickness (mm) recov % recov % %	Inc Set No. Inc Spac Rough-ness Filling Thickness (mm) recov % recov % % Freq No/m	Inc Set No. Inc Spac Rough- Filling Thickness recov % recov % % Freq Scale	Inc Set No. Inc Spac Rough- Filling Thickness recov % recov % Freq Scale (deg) (Deg) ness (mm) 1:50

DRILLED BY: PROFILED BY: S YaFrance

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

DIAM: DATE: 13-15 May 2023 DATE: 15 May 2023

DATE: 07/07/2023 14:21 TEXT: ..51\PROFILES\PKFSWDBH.TXT

Y-COORD: 7583254

HOLE No: SWD-BH09

HOLE No: SWD-BH10 Sheet 1 of 2

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive BF -bedded

FF -foliated CF -cleaved SF -schistose

CG -coarse grain GF -gneissose

JOINT SPACING VCJ-very close spacg LF -laminated

CUR-curvilinear CJ -close spacing MJ -medium spacing UND-undulating

FG -fine grained

MG -medium grain

JOINT ROUGHNESS ROCK HARDNESS

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

PLA-planar

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

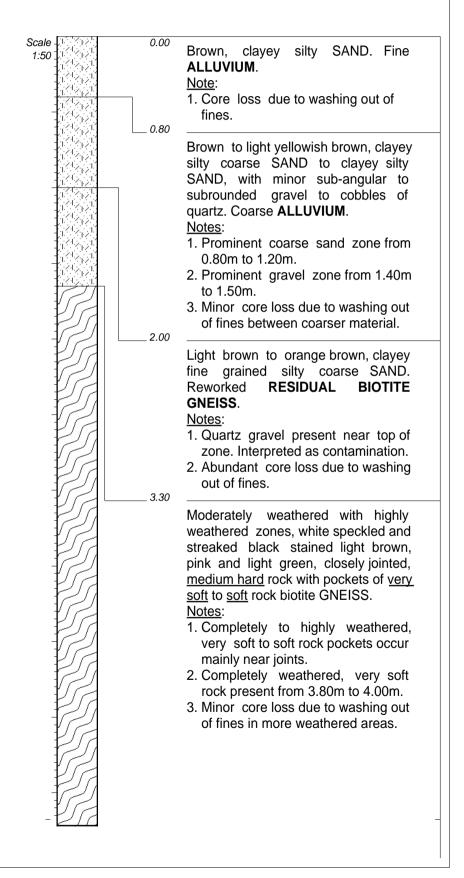
SR -soft rock VSR-very soft rock



OMITIOMIRE COPPER PROJECT GEOTECHNICAL INVESTIGATION

HOLE No: SWD-BH10 Sheet 1 of 2

				WJ -	wide spacing -very wide spac	STE-steppe ng IRR-irregul	ed								_
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	71	0	-	NA	-
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	71	12	0	NM	1
										1.95	SPT1	N=Ref	J - 1	NA	_ 2
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.00	100 j	0	-	NA	-
										3.15	SPT2	N=Ref		NA.	3
										4.34	92	67	0	24	- 4
										5.88	94	94	27	19	5
MG-CG	GF	IF-VIF	0-10	1 2 3	10-30 40-50 80-90	MJ CJ-MJ	SJ RJ RJ	Silty sand + mica FeOx & Chl & mica FeOx & mica & Chl		7.34	100	100	45	14	- 6 - 7
										8.89	99	99	19	22	8
										10.44	99	99	29	23	9
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thickness (mm)	Depth (m)	Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50



ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold HOLE No: SWD-BH10 **OMITIOMIRE COPPER PROJECT** MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock Sheet 2 of 2 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock **GEOTECHNICAL INVESTIGATION** FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock LF -laminated CJ -close spacing PLA-planar MJ -medium spacing UND-undulating WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular NOTES 1) NA: Not applicable. 2) NM: Not measurable.

> CONTRACTOR: RA Longstaff MACHINE: Sullivan HD DRILLED BY

DEPTH

Scale

1:50

PROFILED BY: S YaFrance

Frac

Freq

No/m

TYPE SET BY : EM SETUP FILE: KPBHC008.SET

RQD

INCLINATION: Vertical DIAM:

DATE: June 2023 **DATE:** 1 June 2023

DATE: 07/07/2023 14:21

COORDINATE SYSTEM: WGS84 UTM 33S X-COORD: 801538

Y-COORD: 7581868 HOLE No: SWD-BH10

HOLE No: SWD-BH10

Sheet 2 of 2

JOB: 3010047806

TEXT: ..51\PROFILES\PKFSWDBH.TXT

Rock

Fabric

Fabric

Spac

(mm)

Fabric

Inc

(deg)

Joint

Set No.

Joint

Inc

(Deg)

Micro

Rough-

ness

Joint

Joint

Filling

Depth (m)

Thickness

(mm)

Mat

recov %

Rock

recov %

Grain

Size

HOLE No: WRD-BH01 Sheet 1 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE MF -massive FG -fine grained

BF -bedded MG -medium grain FF -foliated CF -cleaved

CG -coarse grain SF -schistose JOINT SPACING GF -gneissose VCJ-very close spacg CUR-curvilinear

LF -laminated CJ -close spacing MJ -medium spacing

SLJ-slickensided SJ -smooth RJ -rough

JOINT SHAPE

UND-undulating

PLA-planar

JOINT ROUGHNESS ROCK HARDNESS EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

VSR-very soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT WASTE ROCK DUMP

GEOTECHNICAL INVESTIGATION

HOLE No: WRD-BH01 Sheet 1 of 3

					vide spacing verv wide spa	STE-stepp cng IRR-irregu										
NA	NA _	NA	NA	NA	NA	NA	NA _	NA	_ NA						Scale 0.00 Dark brown, silty fine	SAND Fine
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	0.84	98	0	0	NA	1.50 ALLUVIUM. 0.19	SAND. FINE
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	1.50	100	0	0	NA	Pale brown to off-white POWDERY CALCRETE Note:	, sandy SILT.
												_	_		1. Traces of calcrete nod	ules.
										2.00	98	0	0	NA	Pale brown blotcl moderately cemented, CALCRETE of soft rock	HARDPAN
NA	NA	NA	NA	NA	NA	NA	NA	NA	NA	3.49	77	0	0	NA	Brown mottled white, grate to medium SAND. ALLUVIUM.	
															Note: 1. Traces of scattered	d calcrete
MG	GF	IF	20	1	0-10	CJ	RJ	Silty sand	<1	4.75	100	0	0	NA	nodules. 2. Hardpan calcrete zor to 3.27m and 3.45m to	
															Grey to brown speckly white silty mediu	ed black and m SAND.
										5.47	100	76	49	7	WRD-BH01/2 • RESIDUAL GNEISS. Note:	
										6.25	100	0	0	NA	1. Calcified residual gne 4.25m to 4.40m.	
MG	NM	NM	NM	NM	NM	NM	NM	NM	NM	6.97	99	28	19	3	Generally completely brown to grey speckled by white, very soft rock GN recovered as fine silt	lack streaked IEISS, mostly y sand with
										7.75	99	0	0	NA	scattered angular grave residual soil.	l, resembling
										8.23	100	100	27	10	WRD-BH01/3 Moderately weathered,	
															grey streaked and ba <u>medium hard</u> rock GNEI <u>soft</u> rock bands.	SS, with <u>very</u>
										9.25	100	100	71	9	WRD-BH01/4 Note: 1. Completely weathere rock zone at 9.80n	d very soft
															10.10m to 10.22m a 10.70m.	nd 10.60m to
										10.75	100	91	48	9	2. Highly broken at 13.90	m to 14.01m.
Grain Size	Rock Fabric	Fabric Spac (mm)	Fabric Inc (deg)	Joint Set No.	Joint Inc (Deg)	Joint Spac	Micro Rough- ness	Joint Filling	Fill Thicknes: (mm)		Mat recov %	Rock recov %	RQD %	Frac Freq No/m	DEPTH Scale 1:50	

ROCK FABRIC GRAIN SIZE JOINT ROUGHNESS ROCK HARDNESS Knight Piésold HOLE No: WRD-BH01 **OMITIOMIRE COPPER PROJECT** MF -massive FG -fine grained SLJ-slickensided EHR-extremely hard rock **WASTE ROCK DUMP** Sheet 2 of 3 BF -bedded MG -medium grain SJ -smooth VHR-very hard rock FF -foliated CG -coarse grain RJ -rough HR -hard rock JOB: 3010047806 **GEOTECHNICAL INVESTIGATION** CF -cleaved MHR-medium hard rock SF -schistose JOINT SPACING JOINT SHAPE SR -soft rock GF -gneissose VCJ-very close spacg CUR-curvilinear VSR-very soft rock LF -laminated CJ -close spacing PLA-planar MJ -medium spacing UND-undulating WJ -wide spacing STE-stepped VWJ-very wide spacng IRR-irregular 0-10 VCJ-CJ RJ Sand 1-2 11 10-20 VIF-IF MG GF 2 VCJ-CJ RJ 2-3 10-20 20 Sand 12.25 99 99 7 80 12 13 13.75 63 11 14 14.35 15.25 100 92 9 53 Slightly weathered to unweathered, dark grey speckled black streaked and 15 banded white, hard to very hard rock GNEISS. 16 16.75 97 97 84 5 17 10-20 0-10 CJ-MJ RJ Clean VIF-IF GF MG 10-20 2 30 CJ-MJ RJ Stained <1 18.25 100 100 97 5 18 19 19.75 | 100 100 85 6 20 20.28 91 91 91 0 20.28 Fabric Fill Depth (m) Mat RQD DEPTH Grain Rock Fabric Joint Joint Joint Micro Joint Rock Frac Rough-Filling Freq Size Fabric Spac Inc Set No. Inc Thickness recov % recov % Scale Spac

(mm)

No/m

1:50

(mm)

(deg)

(Deg)

ness

HOLE No: WRD-BH01

Sheet 2 of 3

HOLE No: WRD-BH01 Sheet 3 of 3

JOB: 3010047806

ROCK FABRIC GRAIN SIZE BF -bedded FF -foliated

MF -massive FG -fine grained CF -cleaved

GF -aneissose VCJ-very close spaca CUR-curvilinear

MG -medium grain CG -coarse grain SF -schistose JOINT SPACING

SJ -smooth RJ -rough

JOINT ROUGHNESS ROCK HARDNESS SLJ-slickensided

JOINT SHAPE

EHR-extremely hard rock VHR-very hard rock HR -hard rock MHR-medium hard rock

SR -soft rock

Knight Piésold

OMITIOMIRE COPPER PROJECT WASTE ROCK DUMP

GEOTECHNICAL INVESTIGATION

DATE: 8-9 May 2023 DATE: 10 May 2023

DATE: 03/07/2023 12:26

TEXT: ..51\PROFILES\PKFWRDBH.TXT

HOLE No: WRD-BH01 Sheet 3 of 3

JOB: 3010047806

		WJ-very wide spacn										
										NOTES 1) Perched 7.4m.	water table enco	untered
										2) NA: Not	applicable.	
										3) NM: Not	measurable.	
										4) Sample 1.12m		taken a
										5) Sample 5.13m5		taken
										6) Sample 7.75m7		taken
										7) Sample 8.53m8	WRD-BH01/4 t 3.93m.	taken
Fabric Spac	Fabric Joint Inc Set N (deg)		Joint Micr Spac Roug nes	h- Filling	Fill Depth (mg	Rock recov %	RQD %	Frac Freq No/m	 DEPTH Scale 1:50			

DRILLED BY:

TYPE SET BY : EM

PROFILED BY: S YaFrance

SETUP FILE: KPBHC008.SET

Y-COORD: 7585025

HOLE No: WRD-BH01

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX D

Photographs of Test Pits and Borehole Cores

Appendix D1

Test Pit Photographs

Appendix D2

Boreholes Core Photographs



Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX D1

Test Pit Photographs



STORMWATER DIVERSION CHANNEL



Plate 1: Typical soil profile at SWD-TP04.



Plate 2: Shallow excavator refusal at SWD-TP08 on ferrugenized alluvium of soft rock strength.





Plate 3: Northerly view of excavations underway at SWD-TP11.



Plate 4: Zoomed in view of ferrugenized soils at SWD-TP11.





Plate 5: Shallow refusals encountered on soft rock gneiss at SWD-TP25.



Plate 6: Rehabilitation of SWD-TP26 within low-lying clay pans.



HEAP LEACH PAD



Plate 7: Refusal at HLP-TP06 on very dense residual gneiss.

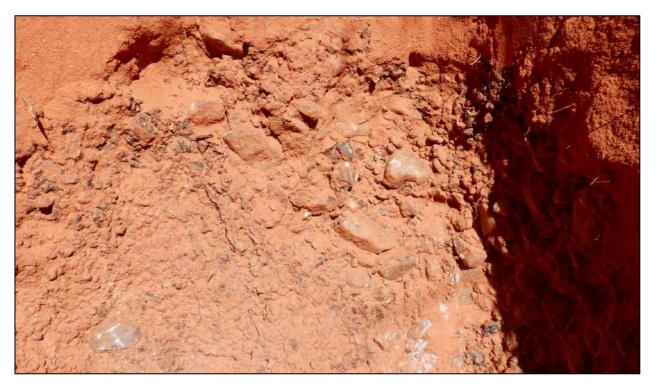


Plate 8: Matrix supported pebble marker with ferricrete nodules at HLP-TP09.





Plate 9: Thin undulating pebble marker observed at HLP-TP13.



Plate 10: Typical soil profile at HLP-TP24.





Plate 11: Typical soil profile at HLP-TP26.



Plate 12: Shallow excavator refusal on ferrugenized alluvium at HLP-TP28.



WASTE ROCK DUMP (WRD)



Plate 13: View of soil profile and shallow water table at WRD-TP01.



Plate 14: Macro-instability of test pit sidewalls at WRD-TP01.





Plate 15: Typical pedogenic profile with refusal on medium hard rock gneiss.



Plate 16: Shallow refusal on soft rock gneiss WRD-TP20.





Plate 17: Presence of folding at WRD-TP20.



Plate 18: Shallow refusal on hardpan calcrete of soft rock strength at WRD-TP23.



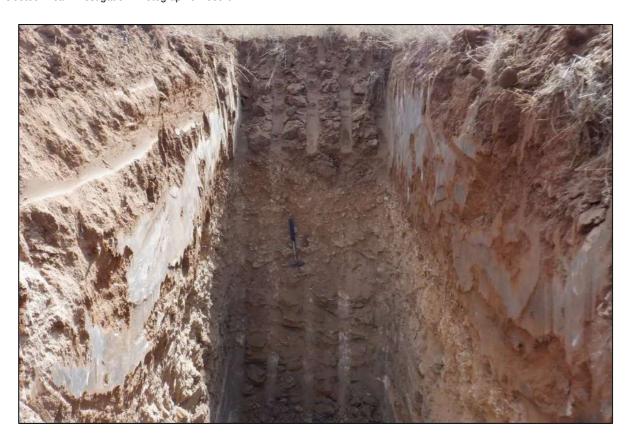


Plate 19: Typical soil profile at WRD-TP29.



Plate 20: WRD-TP34 exposing well developed nodular calcrete horizon.





Plate 21: Zoomed in view of well-developed calcrete nodules of soft rock strength.



Plate 22: Ongoing sampling activities at WRD-TP34.



NEW ROAD



Plate 23: Existing farm roads in the vicinity of RD-TP01.



Plate 24: Fine alluvium at RD-TP01.





Plate 25: Rehabilitated RD-TP02.

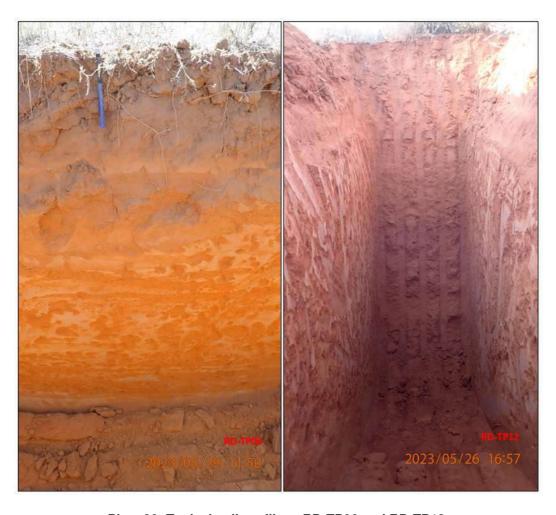


Plate 26: Typical soil profile at RD-TP06 and RD-TP12.





Plate 27: Typical soil profile at RD-TP16 and RD-TP20.



POTENTIAL BORROW SOURCES



Plate 28: Excavations underway at AM-TP01.



Plate 29: Steeply dipping gneiss outcrops.





Plate 30: Access roads observed within the vicinity of the identified borrow area.



Plate 30: Typical profile west of the potential borrow area.





Plate 31: Clayey silty sand material recovered from AM-TP06B.



Plate 32: Typical fine alluvial soils overlying ferruginous alluvium.



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Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX D2

Borehole Core Photographs



STORMWATER DIVERSION CHANNEL

BH01-SWD

0 - 24.27m









BH02-SWD

0 - 25.11m









BH03-SWD

0 - 20.23m









BH04-SWD

0 - 21.03m











BH05-SWD

0 - 20.01m









BH06-SWD

0 - 15.26m









BH07-SWD

0m - 15.22m









BH08-SWD

0m - 20.01m









BH09-SWD

0 - 20.23m









BH10-SWD

0 - 20.09m









HEAP LEACH PAD

BH01-HLP

0 - 20.07m









BH02-HLP

0 - 20.08m









BH03-HLP

0 - 20.13m









BH04-HLP

0.- 20.23m









PROCESSING PLANT

BH01-Plant

0 - 25.05m









BH02-Plant

0 - 25.16m













BH03-Plant

0-25.00m













BH04 - Plant

0 - 10.09m

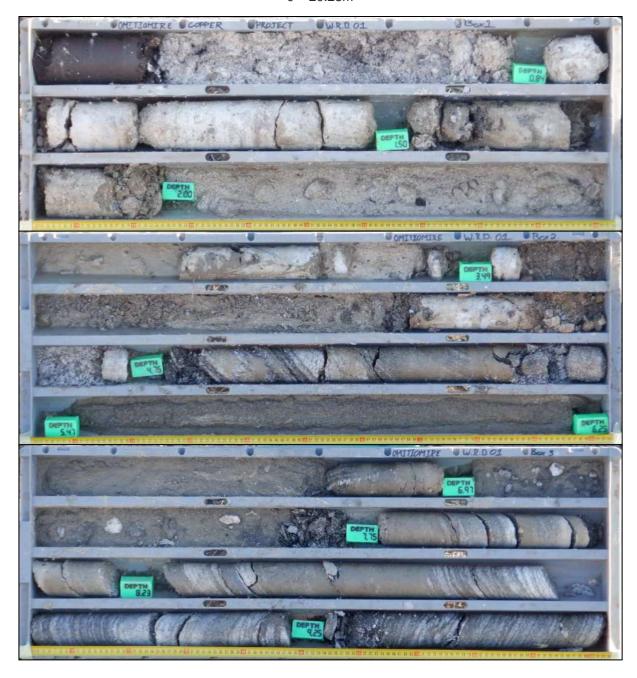




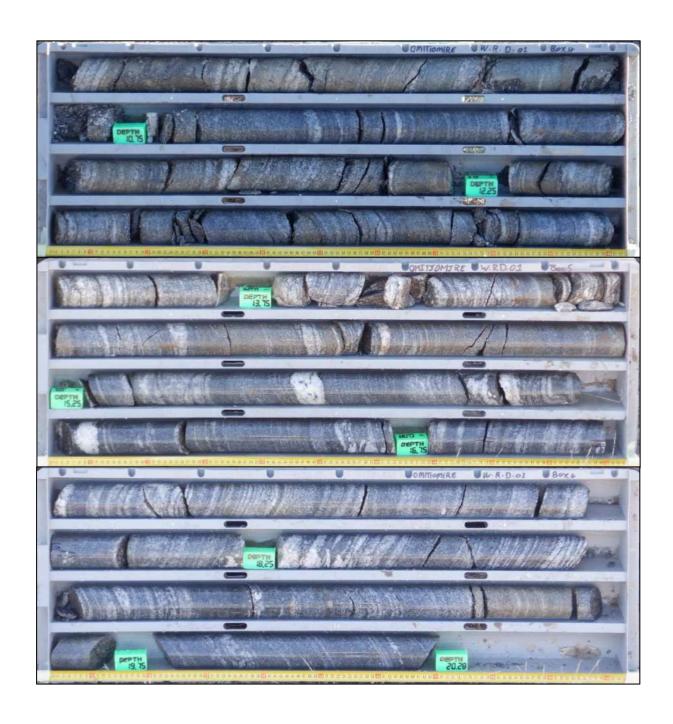
WASTE ROCK DUMP (WRD)

BH01-WRD

0 - 20.28m









Gration Mining and Exploration (Pty) Ltd	
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Feasibility Level Geotechnical Investigation - Factual and Interpretive Report	
Todasam, 2000 Good and modern and morpholic report	

APPENDIX E

Terminologies and Conventions used in Soil and Rock Logging





CONSULTING									
LIST OF BASIC	SOIL DESCRIPTOR	RS							
CHARACTERISTIC	COLOUR PATTERN / FE	ATURE							
Term	Description								
Speckled	Very small patches of	colour, < 2mm.							
Mottled	Irregular patches of colour, 2 - 6mm.								
Blotched	Large irregular patches of colour, 6 - 20mm.								
Banded	Approximately parallel	bands of varying colour, describe thickness / spa	cing.						
Streaked	Randomly oriented str	eaks of colour, describe thickness / spacing.							
Stained	Local colour variations	associated with discontinuity surfaces.							
CONSISTENCY OF	GRANULAR SOILS								
Consistency	Description Typical Dry Density (kg/m³)				Saturated SPT Blow Counts, N	·			
Very loose	Crumbles easily when scraped with geological pick.				< 4	,			
Loose	Small resistance to penetration by sharp end of geological pick. 1450 - 1600				4 - 10				
Medium dense	Considerable resistance to penetration by sharp end of geological pick. 1600 - 1750			10 - 30					
Dense	Very high resistance to penetration of sharp end of geological pick, requires many blows of pick for excavation. 1750 - 1925				30 - 50 > 50				
Very dense	Very dense High resistance to repeated blows of geological pick, requires power tools for excavation. > 1925								
CONSISTENCY OF	COHESIVE SOILS								
Consistency	Description				UCS	Saturated SPT	Blow Counts, N		
Consistency	·					Sensitive soils	Insensitive soils		
Very soft		e pushed in to the shaft of handle, easily moulde			< 50 kPa	< 2	< 5		
Soft		numb, sharp end of pick can be pushed in 30 to 4			50 - 125 kPa 125 - 250 kPa	2 - 4	5 - 10		
Firm		ted by thumb with effort, sharp end of pick pushed in up to 10mm, very difficult to mould with fingers, can just be penetrated with ordinary hand spade.				4 - 8	10 - 25		
Stiff Very stiff	Penetrated by thumb nail, slight indentation produced by pushing pick point into soil, cannot be moulded by fingers, requires hand pick for excavation. Indented by thumb nail with difficulty, slight indentation produced by blow of pick point, requires power tools for excavation.				250 - 500 kPa 0.5 - 1 MPa	8 - 15 15 - 20	25 - 50 50 - 80		
	indented by thamb ha	Twitt difficulty, slight indentation produced by bio	w of pick point, requires power tools for excavation.		U.S - I IVIF a	13 - 20	30 - 80		
STRUCTURE		T a							
	erm	Description							
	ntact	Structureless, no structure identified.							
	sured		en or closed, stained or unstained, and of variable origin.						
	ensided		continuity surfaces which are smooth or glossy and possible striated.						
Sha	attered	Very closely to extremely closely spaced disc	ontinuities resulting in gravel sized soil fragments which are usually stiff to	very stiff and difficult to b	reak down through mou	lding by fingers.			
	shattered	As above, but sand-sized fragments.							
Stratified / laminated / foliated / etc These and other accepted geological terms may be used to describe sedimentary structures in transposed soils as well as relict structures in residual soils.									
	Pinhole Pinhole-sized voids or pores (up to say 2mm) which may require a hand lens to identify.								
Honeycombed Similar to pinhole but voids and pores > 2mm, pore size may be specified in mm.									
	supported	Clasts supported by matrix.							
Clast-s	supported	Clasts touching, matrix may or may not be pre	esent.						
	ASSES (MIT CLASSIFICA	ATION)							
Grain size	01 17 11	5, 111, 10, 11							
(mm)	Classification	Field Identification	t						
< 0.002 0.002 - 0.06	Clay Feels sticky or soapy, soils hands, shiny when wet. Silt Chalky feel on teeth, when dry rubs off hands, dilatent.								
0.002 - 0.06	Fine sand	Gritty feel on teeth.	, unatorit.						
0.2 - 0.6	Medium sand	Observed with naked eye.							
0.6 - 2	Coarse sand	Observed with naked eye.							
2 - 6	Fine gravel	Observed with naked eye.							
6 - 20	Medium gravel	Observed with naked eye.							
20 - 60	Coarse gravel	Observed with naked eye.							
60 - 200	Cobbles	Observed with naked eye.							
> 200	Boulders	Observed with naked eye.							
ORIGINS OF TRANS	SPORTED SOILS IN SOU	THERN AFRICA							
	Prigin	Agency of Transportation	Problems that can be Expected				l		
	obile dune sands	Waves, current and tides	Collapsible fabric; instability of dredged marine deposit excavations; hi	•	ariable carbonate ceme	ntation.	l		
Estuarine and deltaic Talus (coarse colluvium)		Tidal rivers depositing into saline water	Compressibility; variability; sensitivity; quick sand; high soluble salt cor	tent.			l		
		Gravity	Slope instability.				l		
	wash (fine colluvium)	Sheetwash Wind	Expansive characteristics; compressibility; dispersive characteristics. Collapsible fabric; mobile (dunes); poor compaction characteristics.				ł		
Aeolian deposits Sandy soils of mixed origin		Sheetwash, wind, termites	Collapsible fabric; dispersive characteristics; compressibility, subject to	flooding			l		
	uvium	Streams	Expansive characteristics; dispersive characteristics; compressibility, subject to				l		
	ustrine	Streams depositing in lakes, pans or vleis	Compressibility; expansive characteristics; high soluble salt content.	,50t to50mig.			l		
Eac	222 119								



LIST OF BASIC ROCK DESCRIPTORS

CHARACTERISTIC COLOUR PATTERN / FEATURE

Term	Description
Speckled	Very small patches of colour, < 2mm.
Mottled	Irregular patches of colour, 2 - 6mm.
Blotched	Large irregular patches of colour, 6 - 20mm.
Banded	Approximately parallel bands of varying colour, describe thickness / spacing.
Streaked	Randomly oriented streaks of colour, describe thickness / spacing.
Stained	Local colour variations associated with discontinuity surfaces.

GRAIN SIZE

·········			
Abbreviation	Description	Size (mm)	Explanatory notes
vFG	Very fine grained	< 0.2	Individual grains cannot be seen with a hand lens (x10 magnification).
FG	Fine grained	0.2 - 0.6	Grains not visible to the naked eye, visible as individual grains under hand lens.
MG	Medium grained	0.6 - 2.0	Grains clearly visible under hand lens, just visible to the naked eye.
CG	Coarse grained	2.0 - 6.0	Grains clearly visible to the naked eye.
vCG	Very coarse grained	> 6.0	Grains measurable.

FARRIC TYPE

FABRIC LIFE	
Abbreviation	Description
BAF	Banded
BF	Bedded
CF	Cleaved
FBF	Flow banded
FF	Foliated
GF	Gneissic
LF	Laminated
MF	Massive
SF	Schistose

FABRIC SPACING

Abbreviation	Description	Spacing	
VTF	Very thickly	> 2m	
TF	Thickly	0.6 - 2m	
MEF	Medium	0.2 - 0.6m	
HF	Thinly	60mm - 0.2m	
VHF	Very thinly	20 - 60mm	
IF	Intensely	6 - 20mm	
VIF	Very intensely	< 6mm	

DEGREE OF WEATHERING

Abbreviation	Description	Extent of Intact Rock Discoloration	Fracture Condition	Fracture Surface Characteristics	Original Texture	Grain Boundary Condition
UW	Unweathered / fresh	None	Closed or discoloured	Unchanged or may be stained	Preserved	Tight
sw	Slightly weathered	< 20% of fracture spacing on both sides of fracture	May contain thin filling of altered material	Stained or partial discoloration	Preserved	Tight
MW	Moderately weathered	> 20% of fracture spacing on both sides of fracture	May contain filling of altered material	Partial to complete discoloration, not friable except in poorly cemented rocks	Preserved	Partial opening
HW	Highly weathered	Throughout	-	Friable and usually pitted	Mainly preserved	Partial separation
CW	Completely weathered	Throughout	-	Resembles a soil	Partly preserved	Complete separation

ROCK HARDNESS FIELD CLASSIFICATION

Abbreviation	Description	Range of UCS (MPa)	Explanatory notes
Soil	-	< 1	Soil description in terms of $\underline{\underline{M}}$ oisture / $\underline{\underline{C}}$ olour / $\underline{\underline{C}}$ onsistency / $\underline{\underline{S}}$ tructure / $\underline{\underline{S}}$ oil Type / $\underline{\underline{O}}$ rigin / $\underline{\underline{O}}$ ther Features / $\underline{\underline{W}}$ ater.
ESR	Extremely soft		Indented by thumbnail (should also be described as a soil on log sheet).
VSR	Very soft	1 - 3	Crumbles under firm blows with sharp end of geological pick, can be pared/peeled with a knife.
SR	Soft	3 - 10	Shallow indentations of 2 to 4mm under firm blows with sharp end of geological pick, can just be scraped with a knife.
MHR	Medium hard	10 - 25	Handheld specimen breaks / fractures with firm blow of geological pick, specimen cannot be scraped or pared with a knife.
HR	Hard	25 - 70	Handheld specimen requires more than one blow with geological pick before breaking / fracturing.
VHR	Very hard	70 - 200	Specimen requires many blows with geological pick before breaking / fracturing.
EHR	Extremely hard	> 200	Specimen can only be chipped under firm blows of geological hammer.

JOINT SPACING

	Description for	
	Joints, Faults or	
Abbreviation	Other Fractures	Spacing
EWJ	Extremely widely	> 6m
VWJ	Very widely	> 2m / 2 - 6m
MJ	Widely	0.6 - 2m
MJ	Moderately	0.2 - 0.6m
CJ	Closely	60mm - 0.2m
VCJ	Very closely	20 - 60mm
ECJ	Extremely closely	< 20mm
NFILL THICKNESS		

INFILL THICKNESS			
Abbreviation	Description	Infill Thickness	
CT	Closed	0mm	
VT	Very tight	0 - 0.1mm	
T	Tight	0.1 - 0.25mm	
PO	Partly open	0.25 - 0.5mm	
0	Open	0.5 - 2.5mm	
MW	Moderately wide	2.5 - 10mm	
W	Wide	10 - 25mm	
VW	Very wide	25mm - 10cm	
EW	Extremely wide	10cm - 1m	
С	Cavernous	> 1m	

Notes :

If more than 25mm, give actual thickness.

If more than 25mm, append photo(s).

JOINT MICRO-ROUGHNESS

CHAT MICHO-1100 CHINEGO					
Abbreviation	Description				
SLJ	Slickensided				
SJ	Smooth				
RJ	Rough				

JOINT MICRO-SHAPE

 (i.t.o. surface expression over < 100mm length)</th>

 Abbreviation
 Description

 CUR
 Curvilinear

 PLA
 Planar

 UND
 Undulating

 STE
 Stepped

 IRR
 Irregular

INFILL TYPE

Abbreviation	Description	Explanatory notes
NF	None	
STF	Staining only	
Non-softening sheared		
material, e.g. calcite		
NFF	Fine	
NMF	Medium	
NCF	Coarse	
Soft sheared material,		
e.g. talc		
SFF	Fine	
SMF	Medium	
SCF	Coarse	
Gouge description (if		
TG	Thick gouge	Gouge thickness < amplitude
VTG	Very thick gouge	Gouge thickness > amplitude of irregularities

Unified Soil Classification System Descriptors (Hazelton & Murphy, 2007)

USCS Group	Group Descriptor
GW	Well-graded gravels, gravel-sand mixtures, little or no fines.
GP	Poorly graded gravels, gravel-sand mixtures, little or no fines.
GM	Silty gravels, poorly graded gravel-sand-silt mixtures.
GC	Clayey gravels, poorly graded gravel-sand-clay mixtures.
SW	Well-graded sands, gravelly sands, little or no fines.
SP	Poorly graded sands, gravelly sands, little or no fines.
SM	Silty sands, poorly graded sand-silt mixtures.
SC	Clayey sands, poorly graded sand-clay mixtures.
ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands with slight plasticity.
CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
OL	Organic silts and organic silt-clays of low plasticity.
MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.
СН	Inorganic clays of high plasticity, fat clays.
ОН	Organic clays of medium to high plasticity.
PT	Peat and other highly organic soils.

AASHTO Classification System Descriptors (ASTM D3282-15, 2019)

AASH TO Group	General Material Classific ation	% passing 2.00 mm	% passing 0.425 mm	% passing 0.075 mm	Liqui d Limit (LL)	Plasti city Index (PI)	Significant Constituent s	Rating as Subgrad e
A-1-a	Granular	≤ 50%	≤ 30%	≤ 15%	-	≤ 6	Stone fragments, gravel and sand	Excellent to good
A-1-b	Granular	-	≤ 50%	≤ 25%	-	≤ 6	Stone fragments, gravel and sand	Excellent to good
A-3	Granular	-	≥ 51%	≤ 10%	-	N.P.	Fine sand	Excellent to good
A-2-4	Granular	-	-	≤ 35%	≤ 40	≤ 10	Silty or clayey gravel and sand	Excellent to good
A-2-5	Granular	-	-	≤ 35%	≥ 41	≤ 10	Silty or clayey gravel and sand	Excellent to good
A-2-6	Granular	-	-	≤ 35%	≤ 40	≥ 11	Silty or clayey gravel and sand	Excellent to good
A-2-7	Granular	-	ı	≤ 35%	≥ 41	≥ 11	Silty or clayey gravel and sand	Excellent to good
A-4	Silt-Clay	-	-	≥ 36%	≤ 40	≤ 10	Silty soils	Fair to poor
A-5	Silt-Clay	-	-	≥ 36%	≥ 41	≤ 10	Silty soils	Fair to poor
A-6	Silt-Clay	-	-	≥ 36%	≤ 40	≥ 11	Clayey soils	Fair to poor
A-7-5	Silt-Clay	-	-	≥ 36%	≥ 41	≥ 11	Clayey soils	Fair to poor
A-7-6	Silt-Clay	-	-	≥ 36%	≥ 41	≥ 11	Clayey soils	Fair to poor

Notes:

- 1. PI of subgroup A-7-5 is less or equal than the LL 30.
- 2. PI of subgroup A-7-6 is greater than the LL 30.
- 3. N.P. Non-plastic.

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APPENDIX F

Geophysical Testing Report



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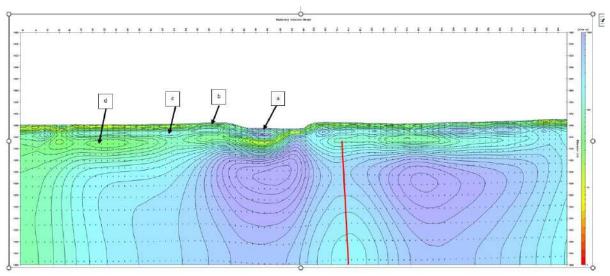
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e-mail: gsymons@geonamibia.net

Field and Interpretational Report: Electrical Resistivity Tomography Survey undertaken for Knight Piésold Consulting.



Electrical Resistivity Tomography (ERT) results from Line 9 at Omitiomire. The overburden can be divided into a, a sandy horizon in channel, b, a clayey horizon in floodplain and c, a sandy horizon in floodplain. Horizon d is interpreted as weathered basement. Vertical faulting is marked by the red line which appears to be underneath the horizontal Kalahari overburden layers within the basement.

Report Prepared for Knight Piésold Consulting.
Report Prepared by: D-J Adams and G Symons
Gregory Symons Geophysics
March 2023

Table of Contents

1.	INTRODUCTION	1
2.	ELECTRICAL RESISTIVITY TOMOGRAPHY METHOD	2
3.	EQUIPMENT AND PERSONNEL	3
3.1	EQUIPMENT	3
3.2	PERSONNEL	3
4.	SURVEYS SPECIFICATIONS, DATA REDUCTION AND PROCESSING	3
5.	INTERPRETATION	4
6.	CONCLUSIONS AND RECOMMENDATIONS	19
7.	ACKNOWI FDGFMENTS	. 20

Appendix 1: Electrical Resistivity Tomography 2D Inversion Sections.

Appendix 2: Data Archive

1. Introduction

This report summarizes details of the Electrical Resistivity Tomography (ERT) survey conducted for Knight Piésold Consulting by Gregory Symons Geophysics (GSG) at Omitiomire, northwest of Gobabis, in Omaheke region, Namibia.

The client requested 13 lines for the survey, which we completed over 14 days. Using the above-mentioned geophysical method, we surveyed the 13 lines as indicated on the location map (Figure 1) below. The survey started on the 3rd of March 2023, and we completed it on the 14th of March, 2023. In the report folder with this report is a comprehensive time sheet in Excel.

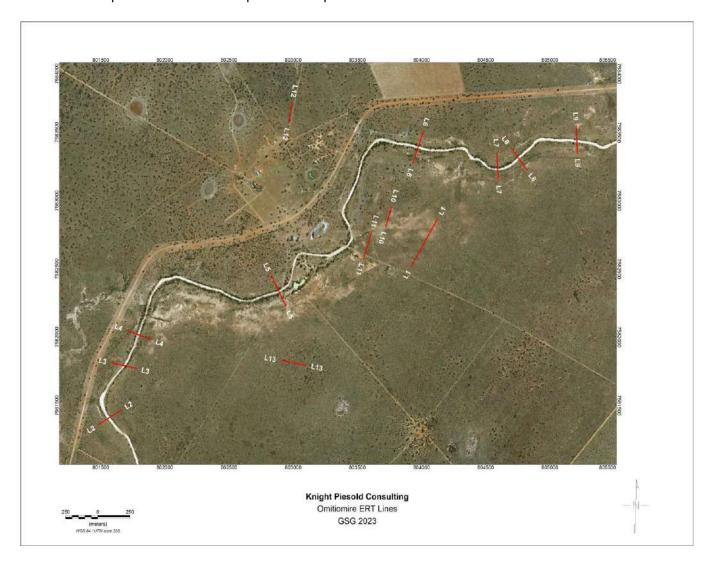


Figure 1: A map showing the thirteen ERT lines surveyed at Omitiomire.

2. Electrical Resistivity Tomography Method

In the Electrical Resistivity Tomography (ERT) method, we inject a current (AB) into the ground and measure a voltage (MN). From this, we can derive a resistivity at a certain point below the surface. In Figure 2 below we show the relationship between AB and MN and the plotting point for the resistivity below the surface.

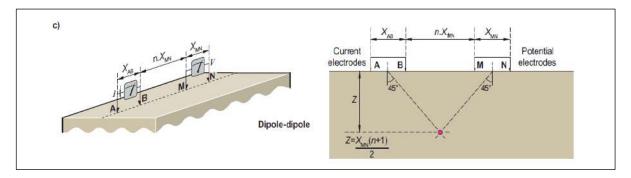


Figure 2: Diagram showing AB injection electrode, MN measurement electrodes and the plotting point.

By arranging 96 electrodes spaced regularly along a line, we can select different combinations of AB and MN to generate numerous subsurface plotting points, as indicated in Figure 3 below. In this example, we have the 96 electrodes spaced 5 m apart, generating 1031 subsurface resistivity measurements.

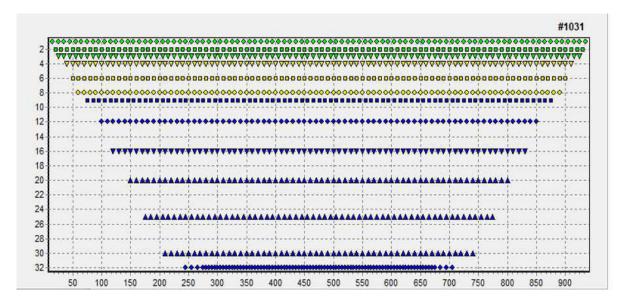


Figure 3: Diagram showing the multiplicity of ERT resistivity measurements below a 1km (960m) line section using 96 electrodes and 5 m station spacing.

Traditional dipole-dipole measurements would typically yield only about 200 subsurface measurements. With the ERT and the additional 5-fold increase in subsurface data, we can:

- Model overburden and shallow features well.
- Detect vertical features below overburden such as faults and fractures.

 Have a better depth of investigation than traditional dipole-dipole measurements due large AB/MN offsets.

3. Equipment and Personnel

3.1 Equipment

- DGPS Trimble R2 System,
- Syscal R2 resistivity system, comprising the R2 Syscal receiver, battery powered transmitter, multiplexer box and all related wires and electrodes.
- Two double cab bakkies and a land cruiser.

3.2 Personnel

- David-John Adams, Phillip Nauyoma, Hilton Stern, and Cecil Murangi with field assistants for data acquisition,
- Gregory Symons, Neville Brown, and David-John Adams (Geophysicists) for data processing, reporting, client liaison and QC.

4. Surveys Specifications, Data Reduction and Processing

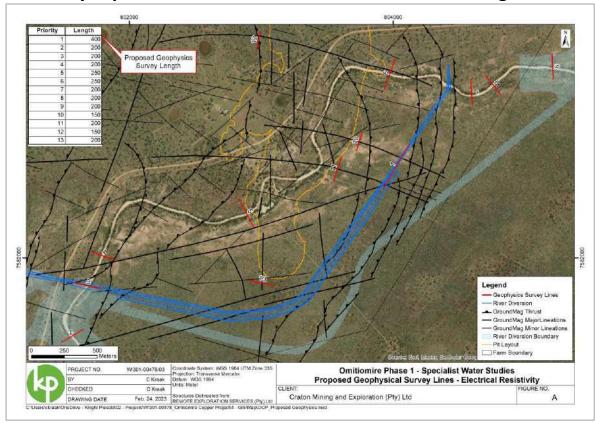


Figure 4: Map Showing position of Lines relative to pit outline, faulting derived from Aeromagnetic Interpretation, and position of current and possible future channel/s to the river.

The survey was collected using the following parameters:

- Station Spacings: 2m on all lines except for Lines 5 and 6 where 3 m was used. We also collected repeat data on Line 1 at a 4 m dipole spacing.
- Line lengths: 400, 250, 200, 150 m.
- Number of Electrodes: 96 electrodes.
- Number of Multi-node boxes: 6 of which each had 16 electrodes connected to it and one connecter cable that connects each box to the resistivity meter.
- The data was processed with the Iris Software (ProsysII) to examine the pseudo section in the field and to convert the data in a suitable format (*.dat).
- The data was then imported to software provided by ZONGE where invalid data was pruned, DGPS data coordinates and elevations were merged with the resistivity data, and a resistivity depth inversion section produced. In Appendix 1, the resistivity depth inversions are presented with the measured and calculated pseudo-sections.

5. Interpretation

The inverted resistivity depth section shows the following features:

- A maximum of 3 overburden layers are observed. This stratigraphy is defined as (a) a top sandy resistive layer, followed by (b), a conductive clay bearing layer and (c) a deeper resistive sandy layer.
- Below this overburn we consistently appear to find weathered conductive basement which changes at depth to resistive fresh rock.
- In places we see vertical to sub-vertical conductive features in the weathered and fresh basement indicative of vertical faulting. The vertical faulting appears to not penetrate the overburden. Clear faulting is indicated in red while more tentative faulting is indicated in black.
- Figure 4 above shows a structural interpretation of the area based on an aeromagnetic interpretation. Thrust faults are purported to be bringing in the Cu mineralization and are typically "old" (late Damaran 480-500 My). We would not expect to see these faults in the resistivity data and in general this is the case with the current resistivity data set. Vertical faulting in a N/S and E/W direction is related to Cretaceous and late Karroo tectonics which appears to be detected by the resistivity as conductive water bearing faults and fractures. Overburden is Kalahari aged with no or little evidence of faulting. The resistive layers may be related to windblown Kalahari. The conductive overburden layer appears to be associated with the Nossob River and could be related to flood events where the Nossob River has overflown its banks and deposited more clay like lithologies on the flood plain.

In Figures 5 to 17 below, the above interpretation scheme is implemented onto each of the resistivity lines.

Interpretation of Individual Lines

Resistivity Depth Sections are presented below. The following features are highlighted:

- Overburden stratigraphy as defined into units belonging to the Kalahari a) sandy, b) clayey, c) sandy.
- Weathered basement d).
- Faulting divided into clear faulting (red) and tentative faulting (black).

Line1, spacing = 4m, length = 400m

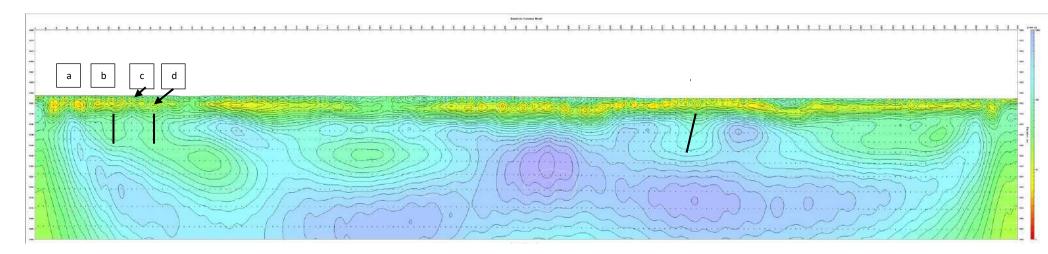


Figure 5: Showing

Faulting at stations: 32 (1), 48 (1), and 256 (1)

Weathered and Overburden units (d) and (c) present.

Line2, spacing = 2m, length = 200m

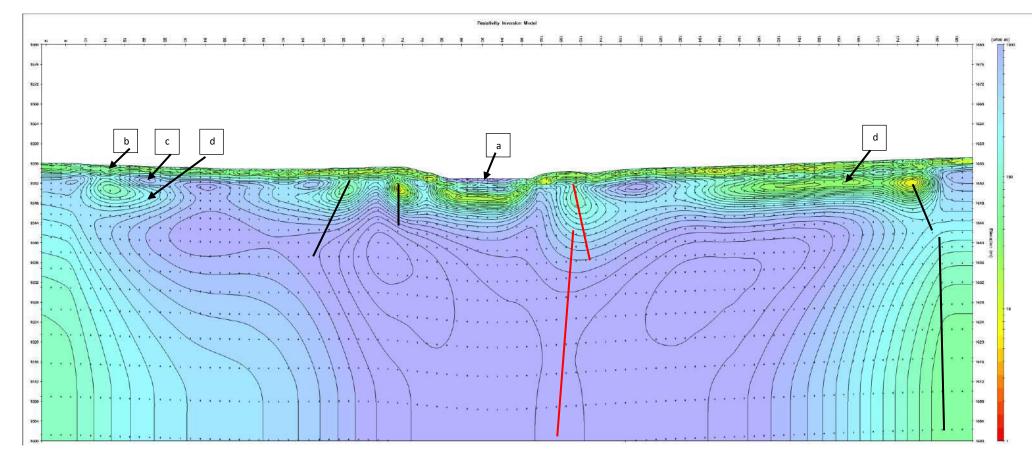


Figure 6: Showing

Faulting at stations: 62(1), 72(1), 110(2), 178(1)

Line3, spacing = 2m, length = 200m

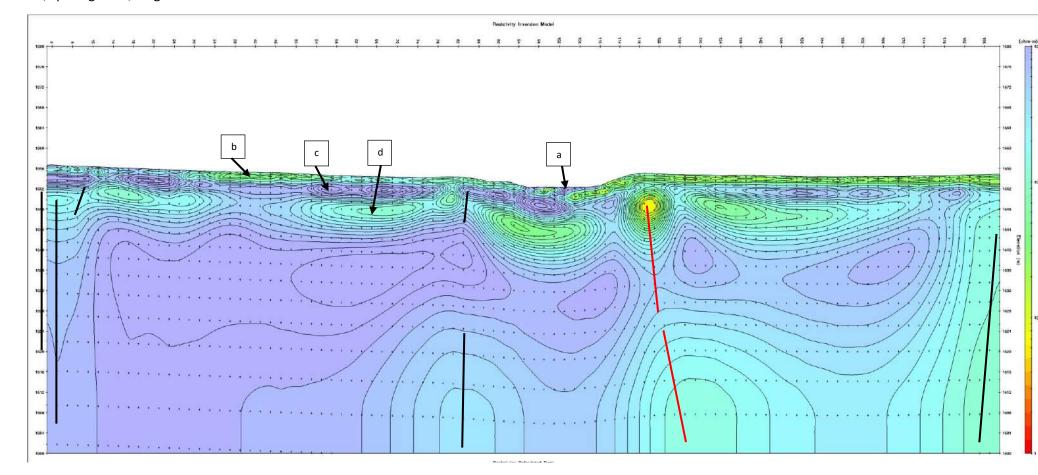


Figure 7: Showing

Faulting at stations: 6(1), 82(1), 120(2), 188(1)

Line4, spacing = 2m, length = 200m

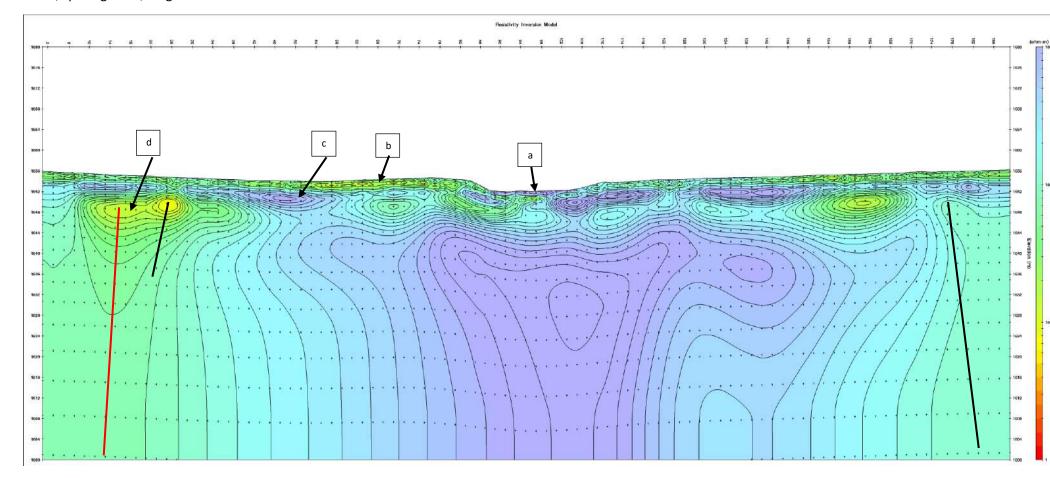


Figure 8: Showing

Faulting at stations: 14(2), 22(1), 178(1)

Line5, spacing = 3m, length = 250m

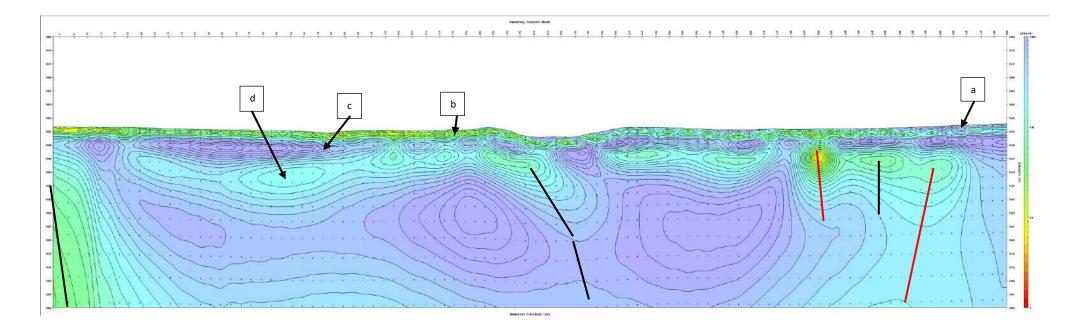


Figure 9: Showing

Faulting at stations: 5(1), 143(1), 229(2), 247(1), 262(2)

Line6, spacing = 3m, length = 250m

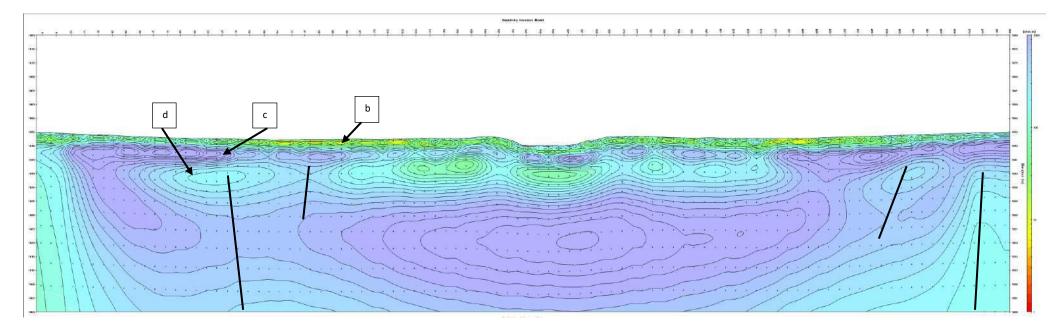


Figure 10: Showing

Faulting at stations: 57(1), 81(1), 253(1), 277(1)

Line7, spacing = 2m, length = 200m

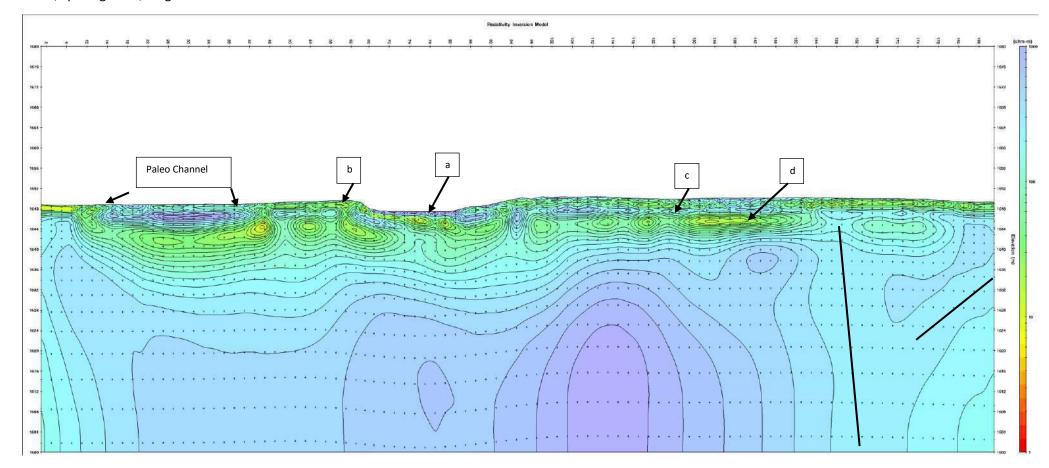


Figure 11: Showing

Faulting at stations: 157 (1)

Line8, spacing = 2m, length = 200m

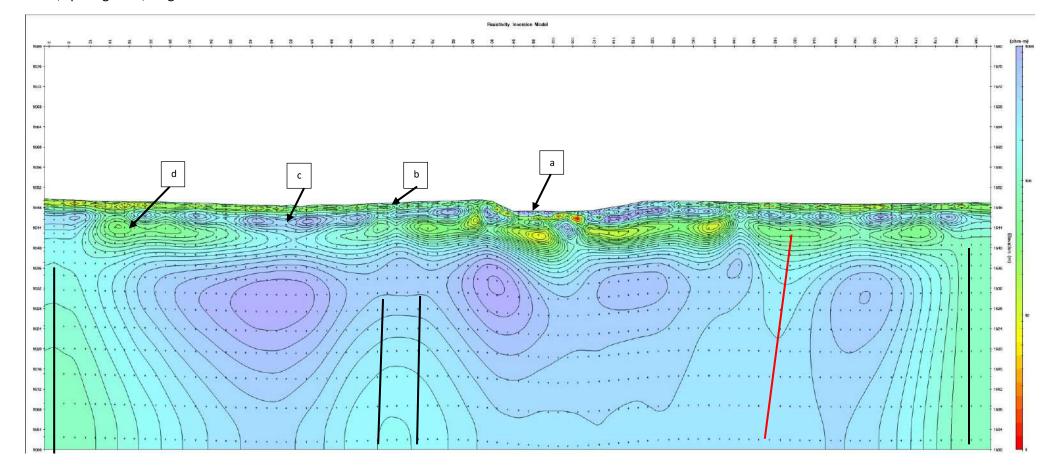


Figure 12: Showing

Faulting at stations: 3 (1), 68 (1), 74 (1), 148 (2), 184(1)

Line9, spacing = 2m, length = 200m

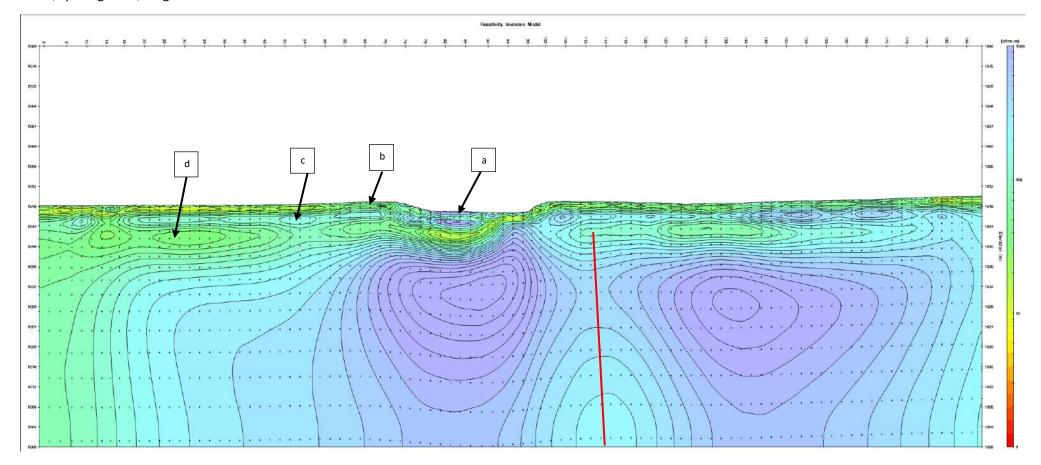


Figure 13: Showing

Faulting at stations: 110(2)

Line10, spacing = 2m, length = 150m

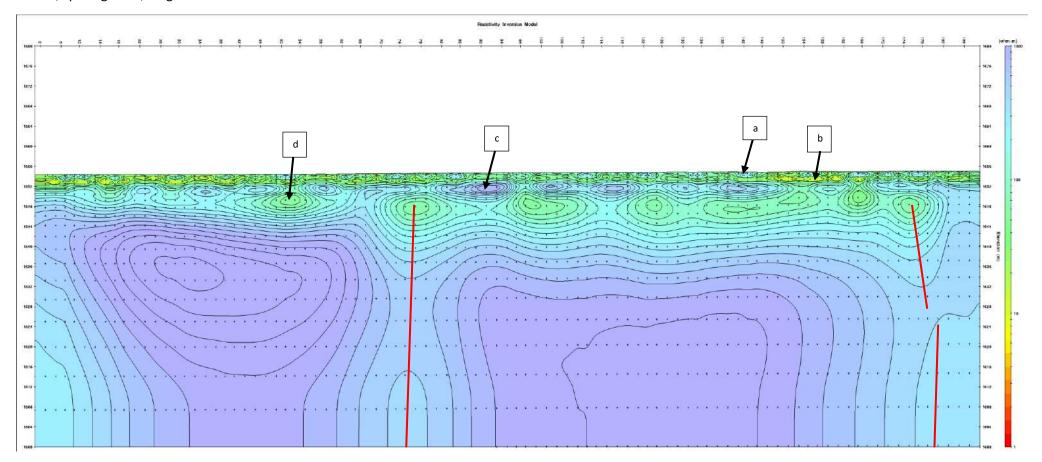


Figure 14: Showing

Faulting at stations: 77(2), 176(2)

Line11, spacing = 2m, length = 200m

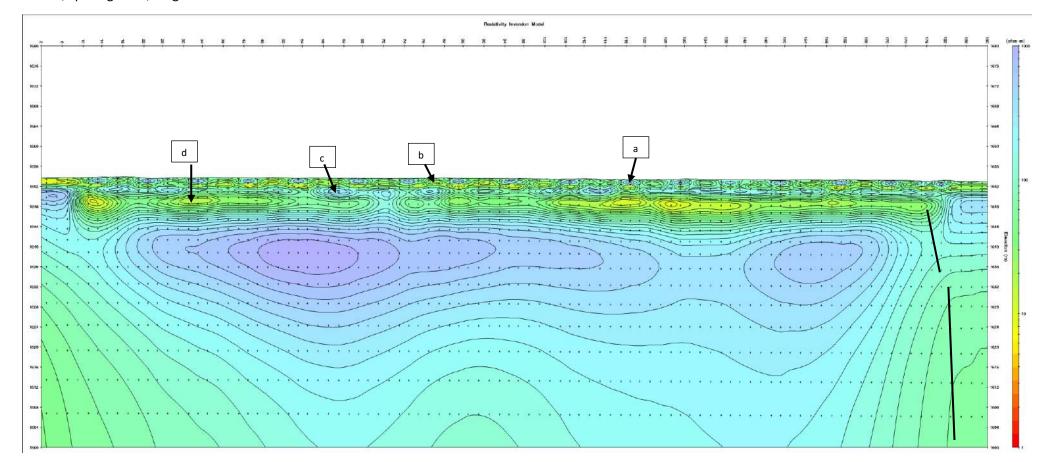


Figure 15: Showing

Faulting at stations: 178(1)

Line12, spacing = 2m, length = 150m

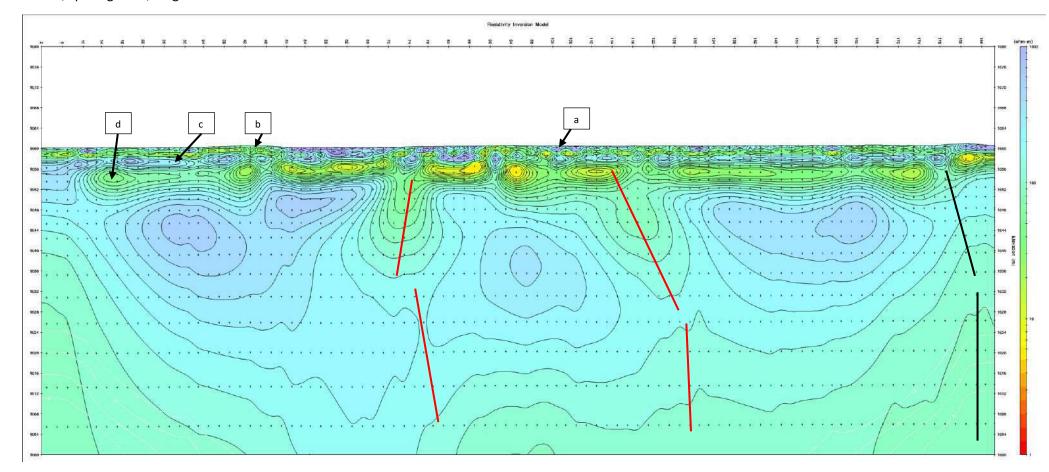


Figure 16: Showing

Faulting at stations: 74(2), 114(2), 178(1)

Line13, spacing = 2m, length = 200m

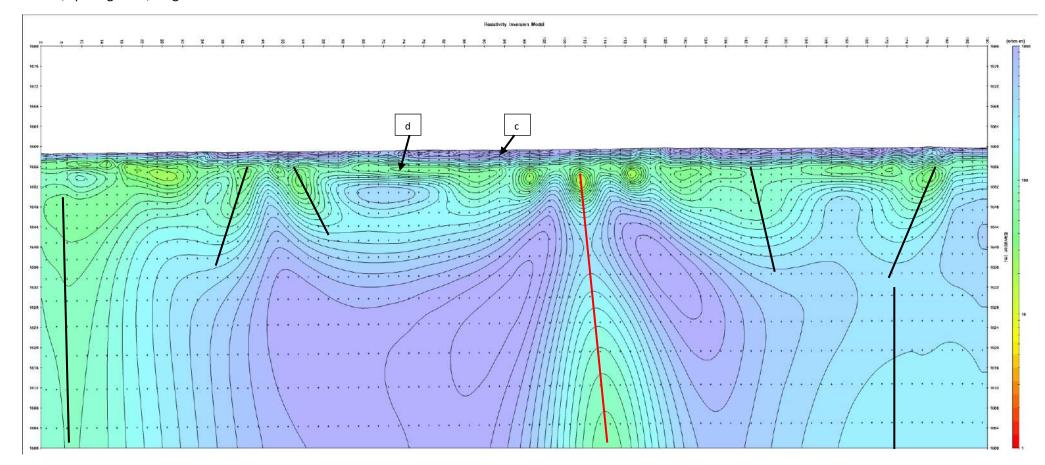


Figure 17: Showing

Faulting at stations: 6(1), 42(1), 52(1), 108(2), 142(1), 180(1)

Weathered and Overburden units (d) and (c) present.

In Figure 18 below we show the interpreted faults from the resistivity data overlain onto the aeromagnetic fault interpretation.

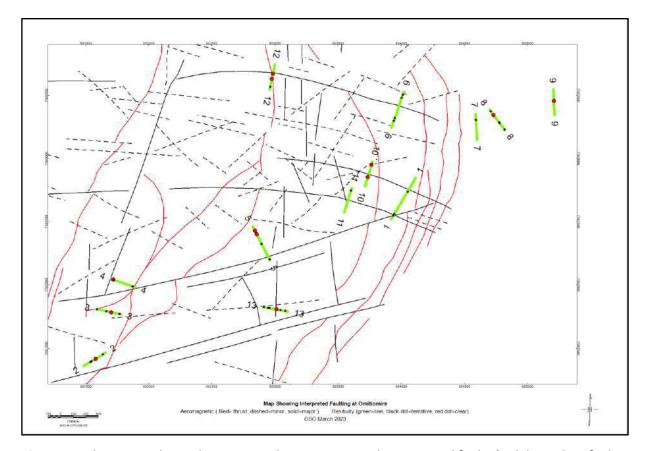


Figure 18: This Figure shows the resistivity lines in green with interpreted faults (red dot = clear fault, black dot = tentative fault) overlain onto aeromagnetic lineaments and faults.

Table 1 below details the top position of all faults interpreted above.

Table1 Giving Position and Classification of Faults (2= clear, 1= tentative)

Table1 Giving Fosition and Classification of Faults (2- clear, 1- tentative)							
Line	Line	Station	Easting	Northing	Elevation	Mark	
L1	1	32	803935.714	7582548.374	1654.881	1	
L1	1	48	803943.655	7582562.251	1654.811	1	
L1	1	256	804051.586	7582740.021	1653.922	1	
L2	2	62	801537.401	7581392.809	1655.018	1	
L2	2	72	801545.994	7581398.061	1655.332	1	
L2	2	110	801578.469	7581417.581	1654.217	2	
L2	2	178	801636.920	7581452.683	1656.891	1	
L3	3	6	801590.365	7581809.515	1656.473	1	
L3	3	82	801664.492	7581793.448	1654.292	1	
L3	3	120	801701.212	7581785.456	1655.011	2	
L3	3	188	801767.768	7581771.843	1654.851	1	
L4	4	14	801718.221	7582043.958	1655.256	2	
L4	4	22	801725.668	7582041.312	1655.007	1	
L4	4	178	801871.949	7581988.970	1655.831	1	
L5	5	6	802956.323	7582203.160	1653.391	1	

Line	Line	Station	Easting	Northing	Elevation	Mark
L5	5	144	802892.140	7582325.361	1650.425	1
L5	5	228	802855.418	7582400.767	1652.807	2
L5	5	246	802847.442	7582416.943	1652.960	1
L5	5	261	802840.451	7582430.191	1653.488	2
L7	7	156	804590.057	7583306.515	1650.193	1
L8	8	4	804817.748	7583230.576	1649.570	1
L8	8	68	804778.943	7583281.525	1648.801	1
L8	8	74	804775.455	7583286.338	1649.106	1
L8	8	148	804730.517	7583344.823	1648.917	2
L8	8	184	804708.645	7583373.431	1648.977	1
L9	9	110	805209.109	7583456.612	1648.842	2
L10	10	76	803735.145	7582856.130	1654.578	2
L10	10	176	803765.165	7582951.791	1655.019	2
L11	11	178	803601.863	7582746.873	1653.049	1
L13	13	6	802909.977	7581827.520	1658.585	1
L13	13	42	802945.444	7581821.024	1658.838	1
L13	13	52	802955.332	7581819.224	1658.942	1
L13	13	108	803010.498	7581809.196	1659.442	2
L13	13	142	803043.997	7581803.238	1659.598	1
L13	13	180	803081.411	7581796.278	1659.881	1

6. Conclusions and recommendations

Conclusions

- Faults interpreted form the ERT resistivity sections indicated in red are confidently interpretated as faults or fractures that may contain or transport water.
- In the top 0-8m we see a clear Kalahari stratigraphy consisting of windblown or reiver sediment. The stratigraphic succession is an upper resistive sandy layer, a middle conductive clayey layer, and a lower resistive sandy layer.
- Basement rocks below the overlying Kalahari appear to be weathered in places and more conductive.
- The Kalahari succession in this area appears to be influenced by episodic flooding of the Nossob River into its flood plain.

Recommendations

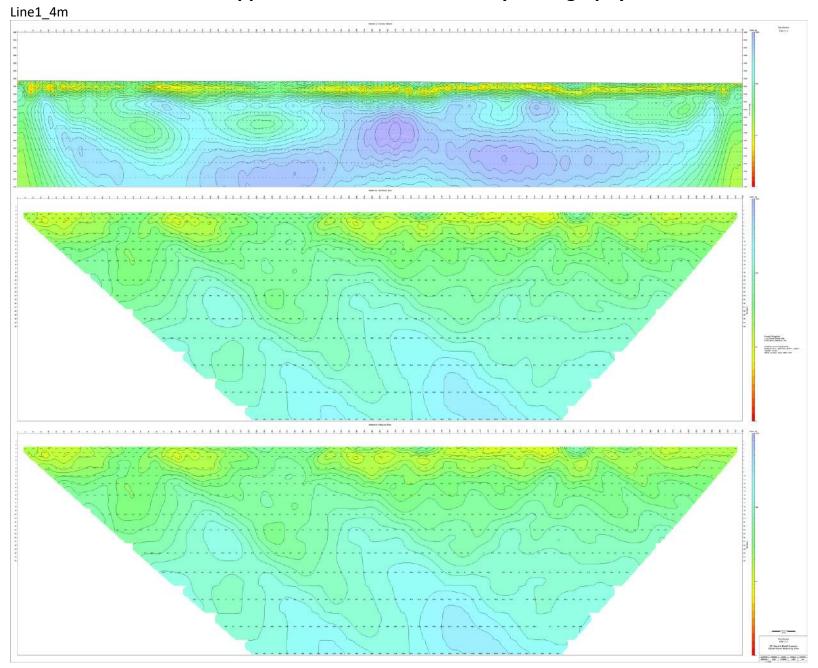
- Faults marked in red are recommended as possible candidates that may carry or conduct ground water.
- When planning a pit, shallow Kalahari conductive and more clayey sediments should be borne in mind as a potential mining hazard. As this is a very shallow unit, earth moving machinery may get bogged down during rainy periods where these sediments are present.
 Secondly this material although shallow may show a tendency to collapse into the pit during rainy periods. Care is advised in pit and pit access road planning.

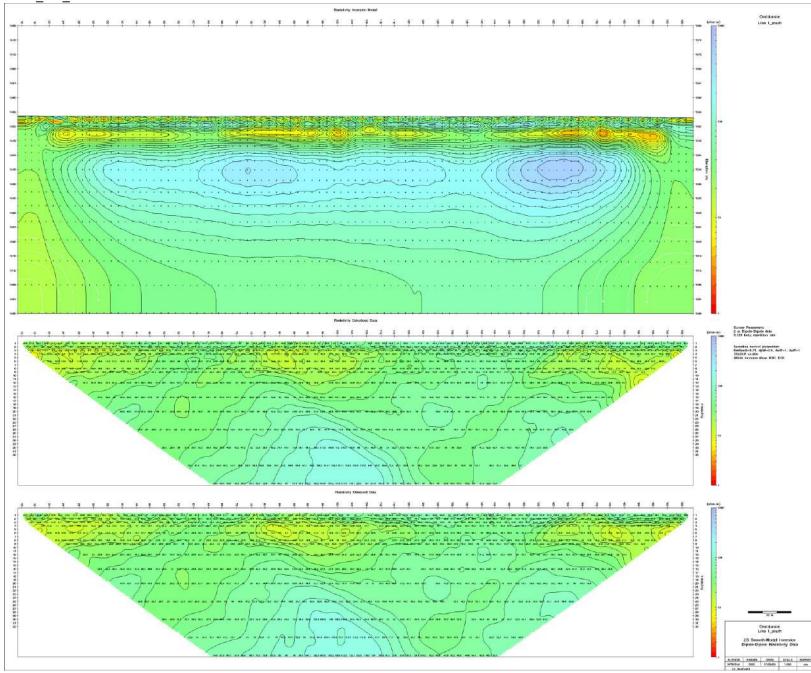
7. Acknowledgements

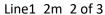
We thank Knight Piésold Consulting and all related parties for all the support rendered to us during this survey. Craton Mining and exploration is thanked for the camping accommodation.

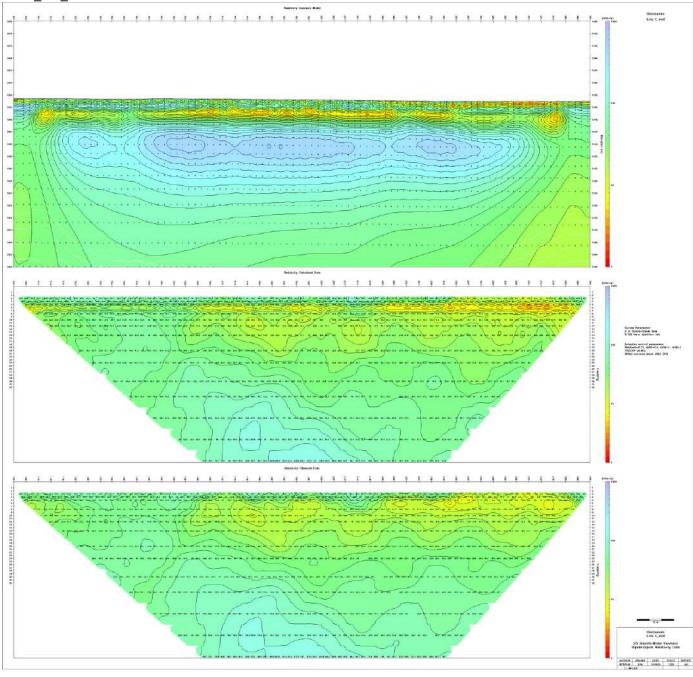
G Symons, D-J Adams (Geophysicists) March 2023.

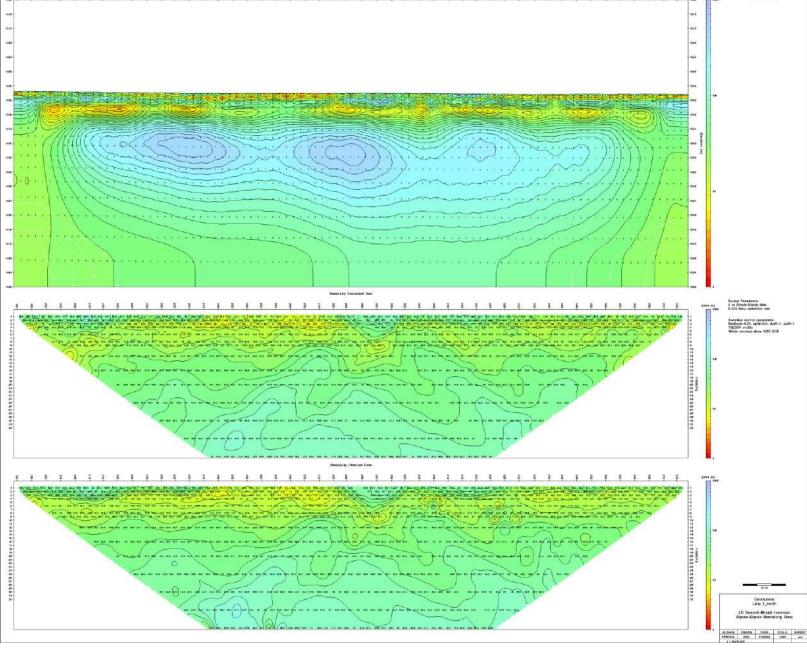
Appendix 1: Electrical Resistivity Tomography 2D Inversion Sections

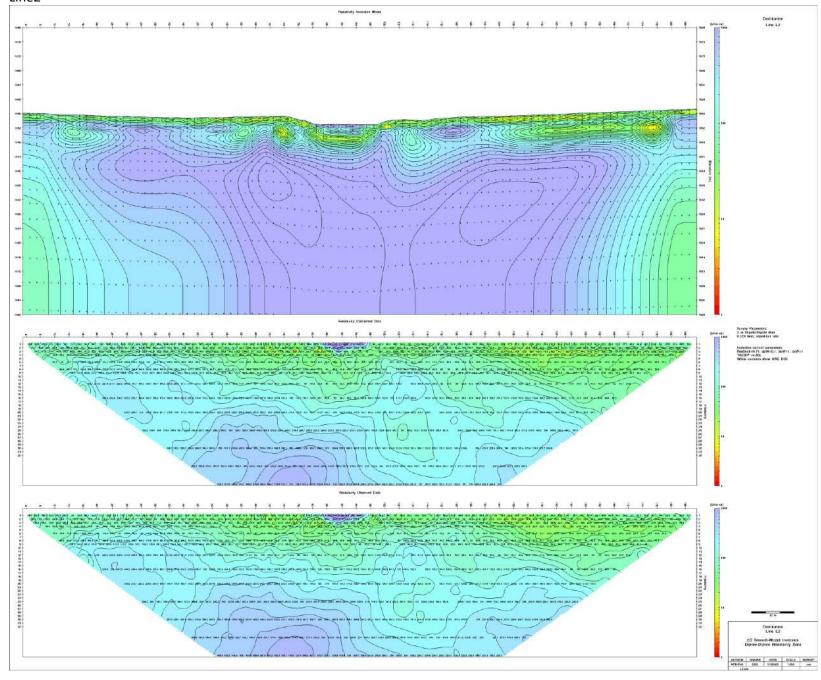




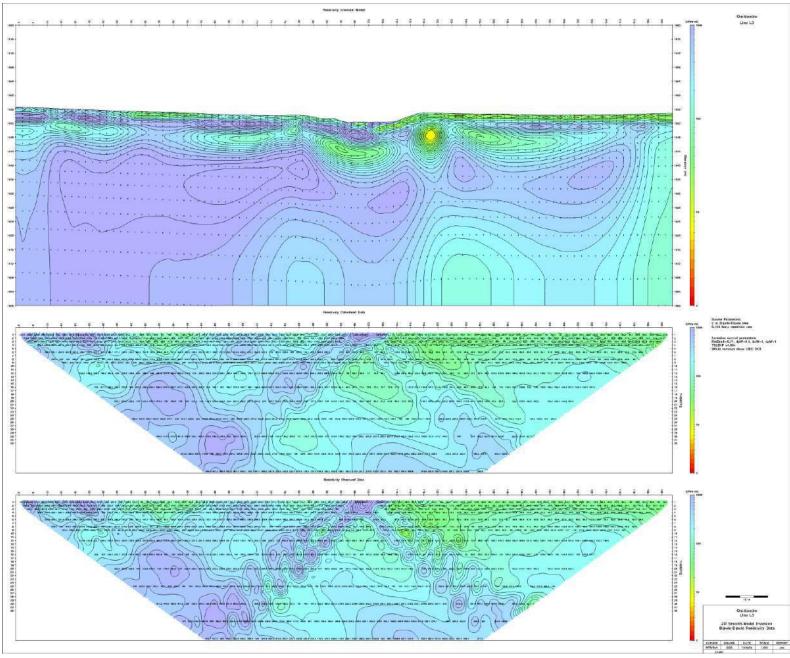


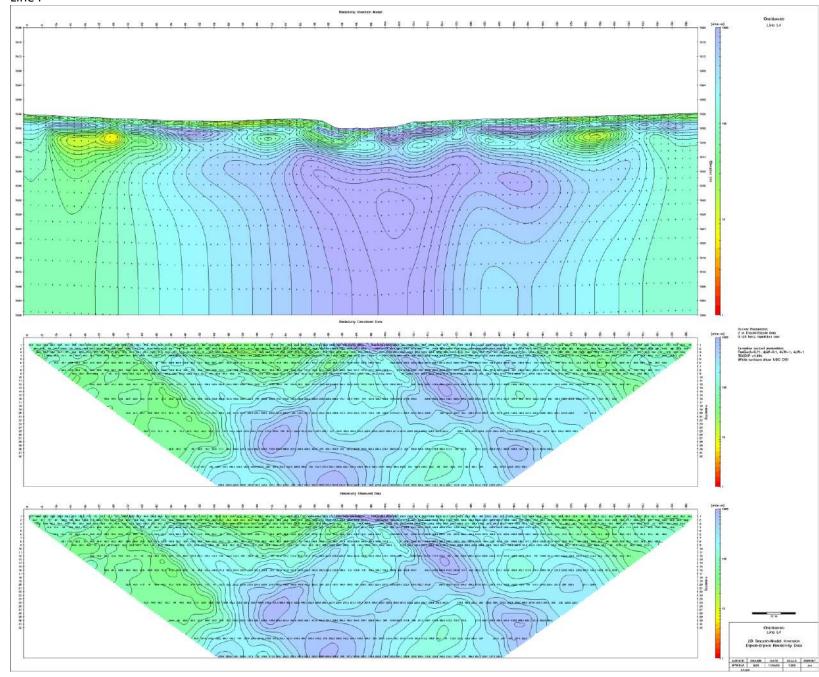




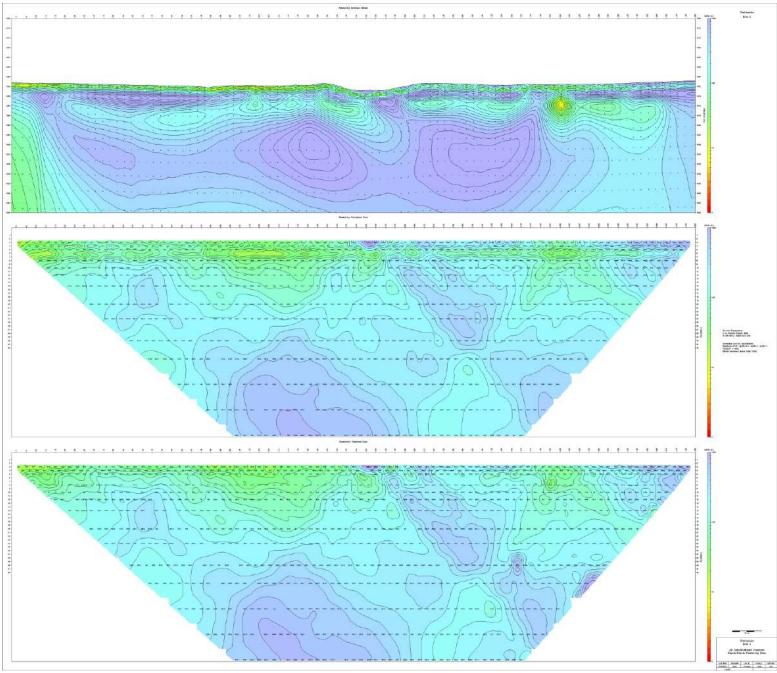




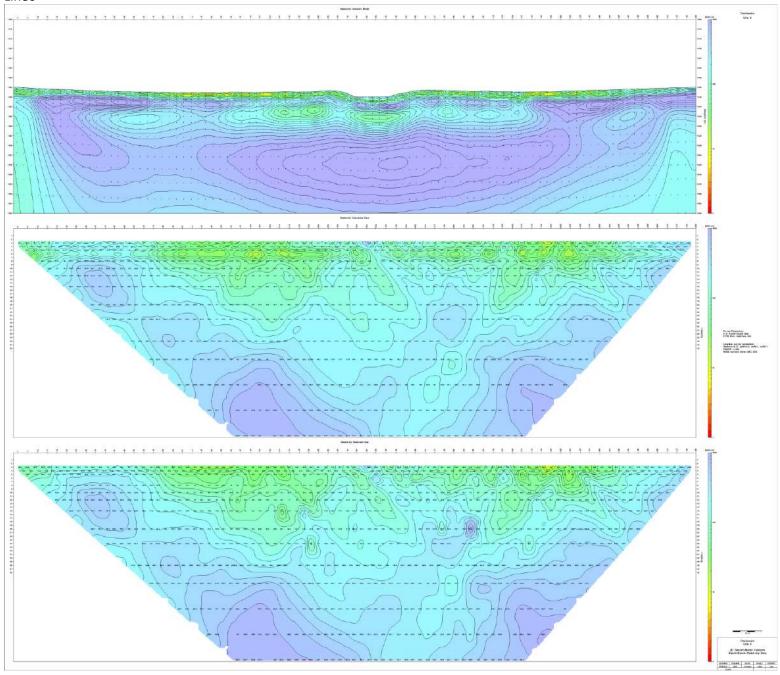


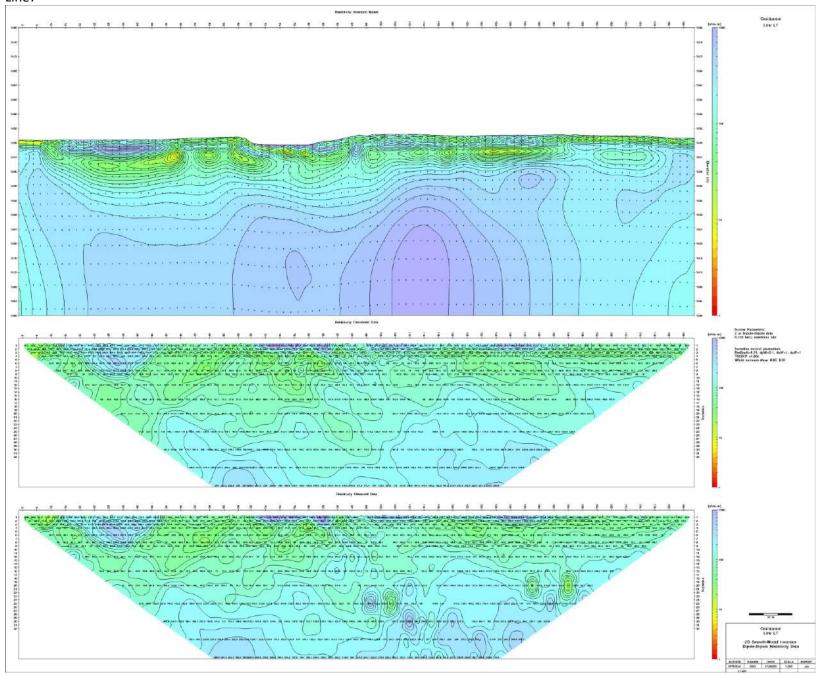


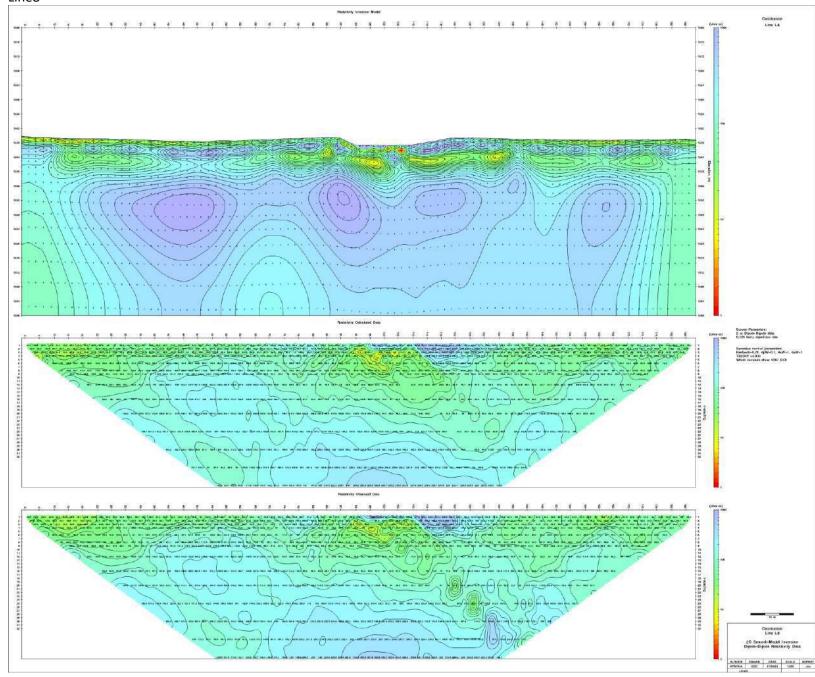


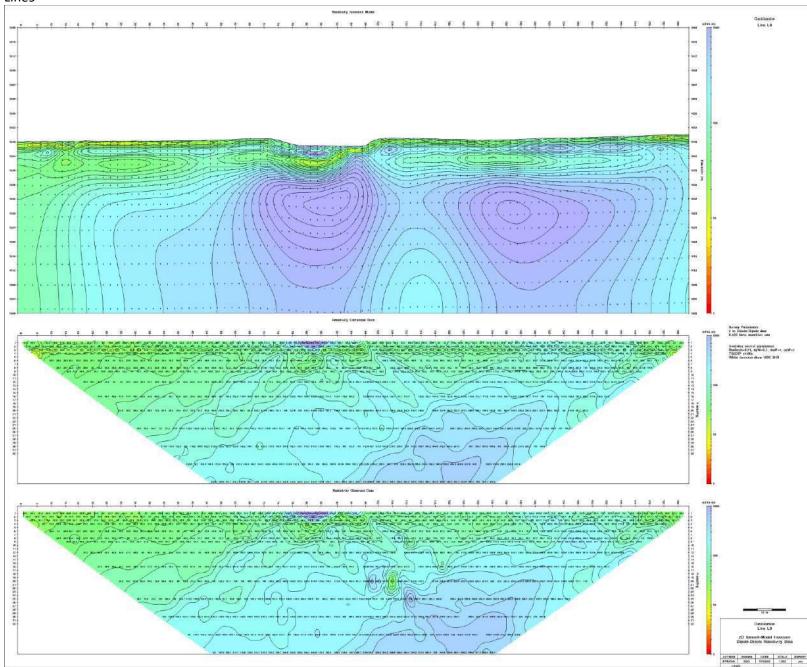


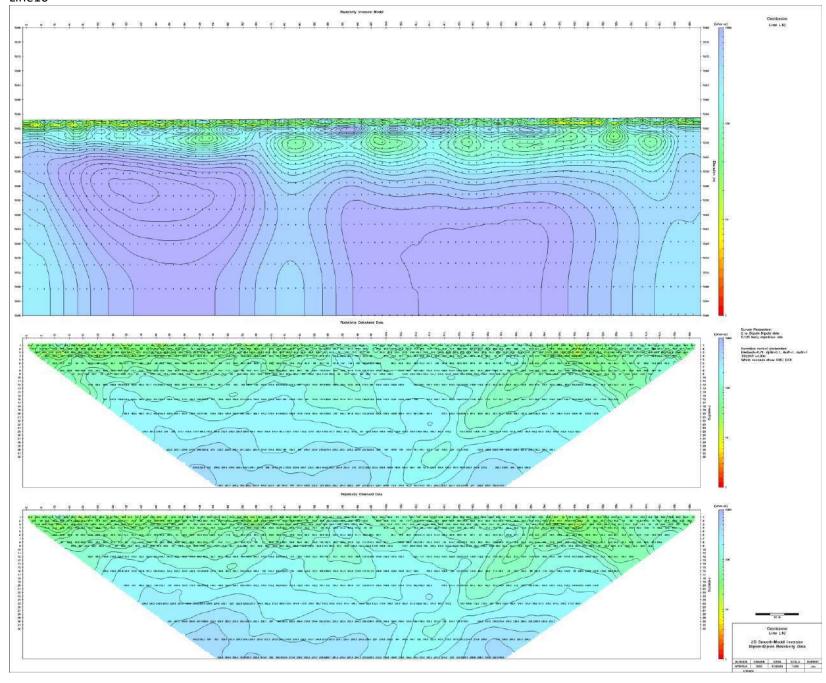




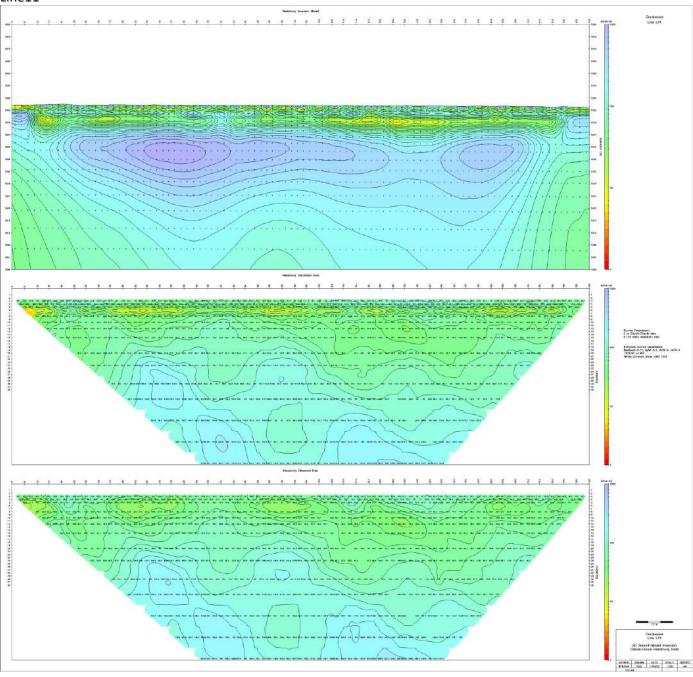




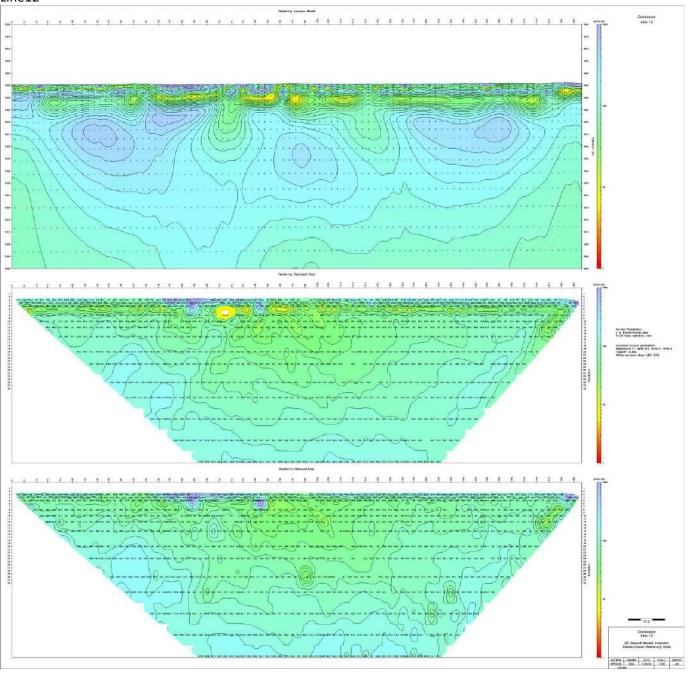


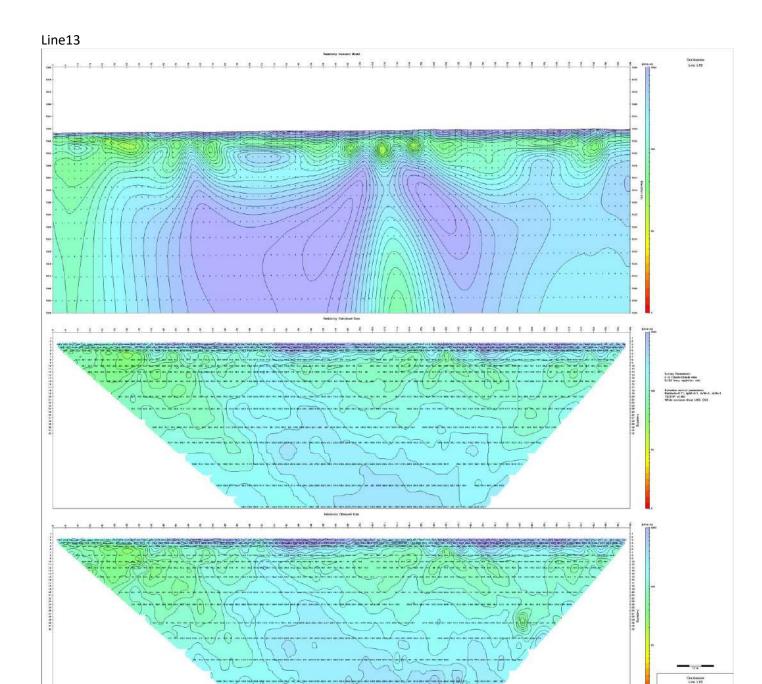












Appendix 2: Data Archive

GSG organized an archive of all data as follows:

- Report: A folder with this report as a word document, a timesheet Excel document, and a PNGs folder with all the Maps for the three Surveys,
- Resistivity: A folder with all raw and processed Electrical Resistivity Tomography Data. Each line is presented, the most important files being the PNG, map and Geosoft *.grd files which give the inverted resistivity results.

Craton Mining and Exploration (Pty) Ltd

Omitiomire Copper Project: Heap leach earthworks and foundation design, Phase 2 Ground Water and Surface Water Studies
Feasibility Level Geotechnical Investigation - Factual and Interpretive Report

APPENDIX G

Laboratory Test Results







SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0 Project: Omitiomire Sampling date: 23-Jun-23

Section: Potential Borrow Sources Stabilizing agent : From chainage: Layer: To chainage:

Report no : SG23/1612 Page no: 1

	Report no: SG23/1612 Page no: 1					1	
	Sample no.	S/10394	S/10395	S/10396	S/10399	S/10402	S/10405
	Chainage						
sample detail	Position (from LHS)	AM-TP01	AM-TP02	AM-TP05	AM-TP06	AM-TP08-B	AM-TP04
	Depth sampled (mm)	400-600	0-700	0-600	400-1000	500-700	300-1100
	Sample description	Brown stained orange brown mottled black, clayey silty sand, slightly ferruginised fine alluvium	Reddish brown, clayey silty sand with traces of subangular to subrounded fine gravel, fine alluvium	Brown, clayey silty sand, fine alluvium	Reddish brown, clayey silty sand with traces of subangular to subrounded gravel to fine gravel, fine alluvium	Dark brown with orange staining, clayey silty sand, fine alluvium	Reddish brown, clayey silty sandy gravel, calcareous alluvium
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
ify	Dry density (kg/m ³)						
ens	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fie	O.M.C. (%)						
	Compaction (%)						
	63,0 mm						100
lg)	53,0 mm						88
ssir	37,5 mm						80
sieve analysis (% passing)	26,5 mm						64
%)	19,0 mm		100				50
<u>si</u>	13,2 mm		99			100	41
aly	4,75 mm	100	99	100	100	100	25
an	2,00 mm	100	97	100	99	99	21
eve	0,425 mm	83	83	92	83	84	18
<u>s</u>	0,075 mm	48	46	41	43	48	4
	Grading Modulus (GM)	0.69	0.74	0.67	0.75	0.69	2.57
erg S	Liquid Limit (LL)	26	26	19	22	25	
Atterberg Limits	Plasticity Index (PI)	10	9	5	7	9	1
Att	Linear Shrinkage (LS)	4.0	3.0	2.5	2.5	3.5	
	CBR @ 100 %				29		147
& CBR	CBR @ 98 %				18		129
8	CBR @ 95 %				9		105
ဝ န	CBR @ 93 %				5		92
ASH valt	CBR @ 90 %				<3		75
ξ	Swell @ 100 %				0.0		0.0
Mod AASHT value	Mod AASHTO (kg/m ³)				2079		2149
	O.M.C. (%)				9.2		6.5
ility 2004(E)	% Compaction @ Moisture				90.0% Mod AASHTO @ OMC		90.0% Mod AASHTO @ OMC
eabi	Permeabilty cm/s				1.29 E-05		1.01 E-04
Permeability	% Compaction @ Moisture						
esi So	Permeabilty cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. WS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

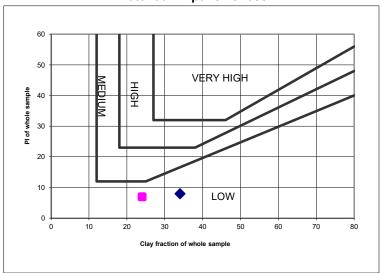
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

Client Knight Piesold (Pty) Ltd (Namibia) Project No. Project Omitiomire Sampling Date 23-Jun-23

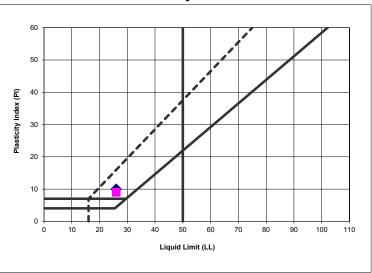
	Official
6/40204	S/10395
	0-700
AM-TP01	AM-TP02
Clayey sand	Clayey sand
2.528	2.697
(% passing)
	100
	99
100	99
100	97
83	83
48	46
sis (% passi	ing)
43	36
40	32
38	29
35	28
34	24
34	24
14	22
52	53
0	1
g Limits	
26	26
10	9
4.0	3.0
8	7
0.69	0.74
	2.528 (% passing 100 100 83 48 sis (% passi 43 40 38 35 34 14 52 0 g Limits 26 10 4.0 8

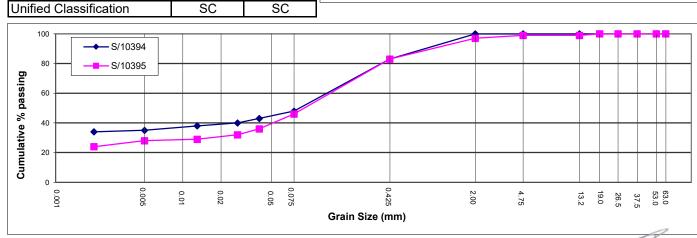
TRB Classification

Potential Expansiveness



Plasticity Chart





A-4 (0)

SC

A-4 (0)

SC



SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

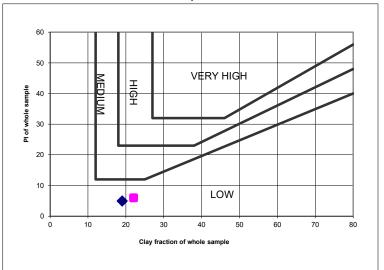
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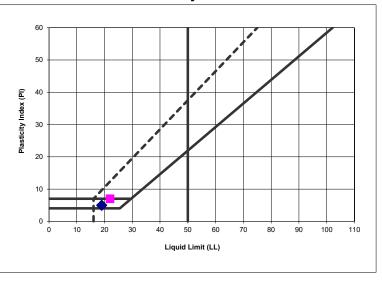
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10396	S/10399	
Report No.	SG23/1612		
Depth (mm)	0-600	400-1000	
Position	AM-TP05	AM-TP06	
Material description	Silty, clayey sand	Clayey sand	
Moisture (%)			
SG	2.741	2.652	
Sieve Analysis	(% passing)	
63.0 mm	<u> </u>	,	
53.0 mm			
37.5 mm			
26.5 mm			
19.0 mm			
13.2 mm			
4.75 mm	100	100	
2.00 mm	100	99	
0.425 mm	92	83	
0.075 mm	41	43	
Hydrometer Analys	sis (% passi	ng)	
0.040 mm	32	35	
0.027 mm	28	33	
0.013 mm	25	30	
0.005 mm	22	26	
0.002 mm	19	22	
% Clay (<0.002 mm)	19	22	
% Silt (<0.075, >0.002 mm)	22	21	
% Sand (<4.75, >0.075 mm)	59	57	
% Gravel (> 4.75 mm)	0	0	
Atterberg	Limits		
Liquid Limit (-0.425)	19	22	
Plasticity Index (-0.425)	5	7	
Linear Shrinkage (%)	2.5	2.5	
PI of whole sample	5	6	
Grading Modulus	0.67	0.75	
TRB Classification	A-4 (0)	A-4 (0)	
Unified Classification	SC-SM	SC	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

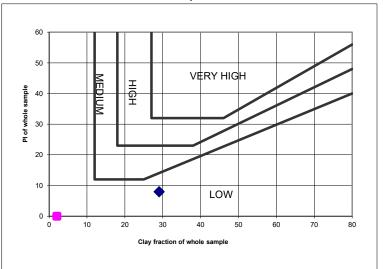
Client Knight Piesold (Pty) Ltd (Namibia) Project No. Project Sampling Date Omitiomire 23-Jun-23

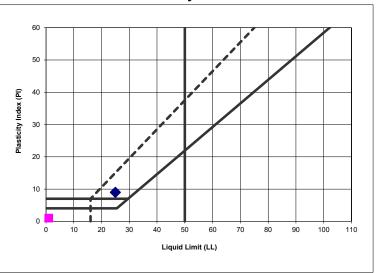
•		
Sample No.	S/10402	S/10405
Report No.	SG23	3/1612
Depth (mm)	500-700	300-1100
Position	AM-TP08-B	AM-TP04
Material description	Clayey sand	Poorly graded gravel with sand
Moisture (%)		
SG	2.620	2.624
Sieve Analysis	(% passing)
63.0 mm		100
53.0 mm		88
37.5 mm		80
26.5 mm		64
19.0 mm		50
13.2 mm	100	41
4.75 mm	100	25
2.00 mm	99	21
0.425 mm	84	18
0.075 mm	48	4
Hydrometer Analy	ysis (% passi	ing)
0.040 mm	40	4
0.027 mm	38	3
0.013 mm	36	3
0.005 mm	33	3
0.002 mm	29	2
% Clay (<0.002 mm)	29	2
% Silt (<0.075, >0.002 mm)	19	2
% Sand (<4.75, >0.075 mm)	52	21
% Gravel (> 4.75 mm)	0	75
Atterber	g Limits	
Liquid Limit (-0.425)	25	
Plasticity Index (-0.425)	9	1
Linear Shrinkage (%)	3.5	0.0
PI of whole sample	8	0
Grading Modulus	0.69	2.57
TRB Classification	A-4 (0)	A-1-a (0)
11 10 101 10 1		0.0

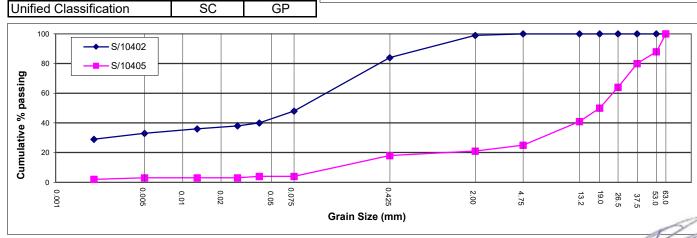
SC

GP

Potential Expansiveness









Report no: SG23/1612



Page no: 5

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0 **Project:** Omitiomire Sampling date: 23-Jun-23 Section: Potential Borrow Sources Stabilizing agent:

To chainage: Layer: From chainage:

	Sample no.	S/10406	S/10407	S/10408	l ago no i c
	Chainage	G/ 10 100	0,10101	0/10/100	
tail	Position (from LHS)	AM-TP02	AM-TP04	AM-TP03	
de de	Depth sampled (mm)	900-1100	1200-1500	700-1000	
sample detail		Completely weathered,	Completely to highly	Reddish brown, clayey	
san	Sample description	dark grey streaked white to reddish brown, soft to	weathered, white speckled to streaked	silty sandy gravel with cobbles, ferruginised /	
	Method of Preparation				
	used	Scalping	Scalping	Scalping	
	Testing depth (mm)				
sity	Dry density (kg/m ³)				
lens	Moisture (%)				
field density	Mod AASHTO (kg/m ³)				
fie	O.M.C. (%)				
	Compaction (%)				
	63,0 mm				
(gu	53,0 mm			100	
ssir	37,5 mm	100		96	
pas	26,5 mm	95		87	
%)	19,0 mm	90		77	
<u>:S</u>	13,2 mm	84	100	71	
sieve analysis (% passing)	4,75 mm	60	98	51	
ans	2,00 mm	45	87	34	
Ve	0,425 mm	34	40	28	
sie	0,075 mm	20	21	15	
	Grading Modulus (GM)	2.01	1.52	2.23	
rg ,	Liquid Limit (LL)	32		23	
Atterberg Limits	Plasticity Index (PI)	12	1	8	
Atte	Linear Shrinkage (LS)	6.0		3.5	
	CBR @ 100 %		76		
BR	CBR @ 98 %		51		
AASHTO & CBR values	CBR @ 95 %		28		
0 S	CBR @ 93 %		18		
ASHTO values	CBR @ 90 %		10		
₹ ^	Swell @ 100 %		0.0		
Mod	Mod AASHTO (kg/m³)		2020		
2	O.M.C. (%)		8.8		
(E)	% Compaction @ Moisture		0.0		
oilit	Permeabilty cm/s				
1 eak 892-1					
Permeability	% Compaction @ Moisture				
. SO/	Permeabilty cm/s				

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

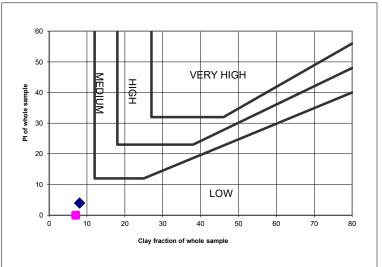
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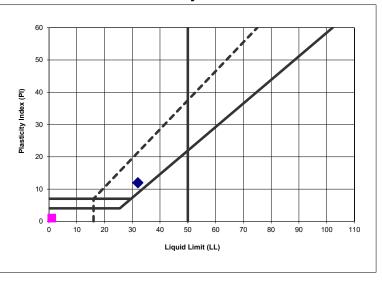
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

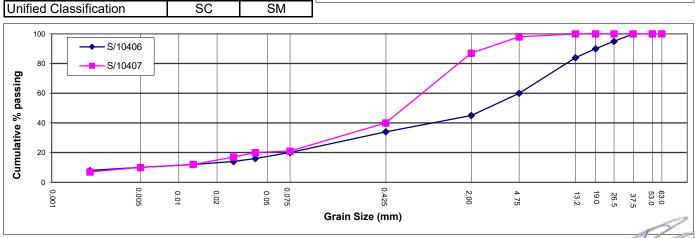
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10406	S/10407
Report No.	SG23	/1612
Depth (mm)	900-1100	1200-1500
Position	AM-TP02	AM-TP04
	Clayey sand with	Silty sand
Material description	gravel	omy dama
Moisture (%)		
SG	2.629	2.571
Sieve Analysis	s (% passing)
63.0 mm		
53.0 mm		
37.5 mm	100	
26.5 mm	95	
19.0 mm	90	
13.2 mm	84	100
4.75 mm	60	98
2.00 mm	45	87
0.425 mm	34	40
0.075 mm	20	21
Hydrometer Anal	ysis (% passi	ng)
0.040 mm	16	20
0.027 mm	14	17
0.013 mm	12	12
0.005 mm	10	10
0.002 mm	8	7
% Clay (<0.002 mm)	8	7
% Silt (<0.075, >0.002 mm)	12	14
% Sand (<4.75, >0.075 mm)	40	77
% Gravel (> 4.75 mm)	40	2
Atterber	g Limits	
Liquid Limit (-0.425)	32	
Plasticity Index (-0.425)	12	1
Linear Shrinkage (%)	6.0	0.0
PI of whole sample	4	0
Grading Modulus	2.01	1.52
TRB Classification	A-4 (1)	A-1-b (0)
		- (-)

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

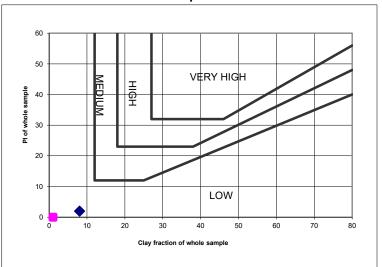
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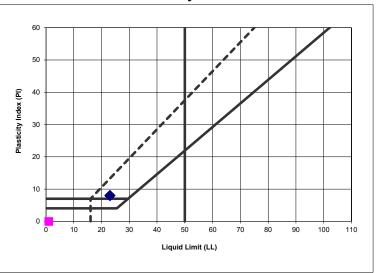
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

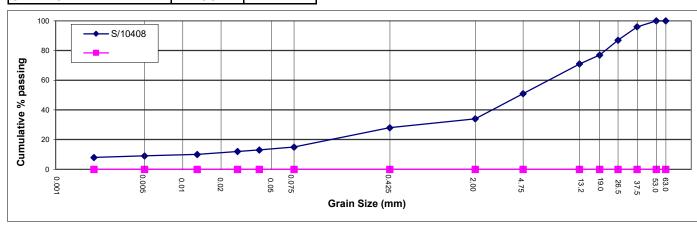
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Fioject		Official
Sample No.	S/10408	
Report No.	SG23	/1612
Depth (mm)	700-1000	
Position	AM-TP03	
	Clayey gravel with	
Material description	sand	
Moisture (%)		
SG	2.658	
Sieve Analysis	s (% passing)
63.0 mm		
53.0 mm	100	
37.5 mm	96	
26.5 mm	87	
19.0 mm	77	
13.2 mm	71	
4.75 mm	51	
2.00 mm	34	
0.425 mm	28	
0.075 mm	15	
Hydrometer Anal	ysis (% passi	ng)
0.040 mm	13	
0.027 mm	12	
0.013 mm	10	
0.005 mm	9	
0.002 mm	8	
% Clay (<0.002 mm)	8	
% Silt (<0.075, >0.002 mm)	7	
% Sand (<4.75, >0.075 mm)	36	
% Gravel (> 4.75 mm)	49	
Atterbei	g Limits	
Liquid Limit (-0.425)	23	
Plasticity Index (-0.425)	8	
Linear Shrinkage (%)	3.5	
PI of whole sample	2	
Grading Modulus	2.23	
TRB Classification	A-2-4 (0)	
Unified Classification	GC	

Potential Expansiveness











SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0 Project: Omitiomire Sampling date: 23-Jun-23

Section: Heap Leach Pad (HLP) Stabilizing agent : From chainage: Layer: To chainage:

Report no: SG23/1616 Page no: 1

	Report no: SG23/1616 Page no: 1					1	
	Sample no.	S/10409	S/10412	S/10414	S/10418	S/10419	S/10421
	Chainage						
sample detail	Position (from LHS)	HLP-TP03	HLP-TP08	HLP-TP24	HLP-TP13	HLP-TP24	HLP-TP20
	Depth sampled (mm)	800-2300	650-1350	1200-1750	300-2250	800-1200	1600-1800
	Sample description	Yellowish brown to khaki, silty sandy gravel, calcified coarse alluvium	Brown, clayey sandy silt. Fine Alluvium	Brown, sandy silty gravel. Coarse Alluvium.	Light brown to brown, gravelly silty fine sand. Calcareous Alluvium.	Brown, gravelly sandy silt. Calcareous Alluvium.	Completely weathered, white speckled to streaked black stained khaki to orange, very soft rock, gneiss
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
iŧ	Dry density (kg/m ³)						
ens	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fie	O.M.C. (%)						
	Compaction (%)						
	63,0 mm	89				100	
sieve analysis (% passing)	53,0 mm	66		100		88	
ssii	37,5 mm	66		83		85	
pa	26,5 mm	57		73		80	
%)	19,0 mm	48	100	68		79	100
sis	13,2 mm	45	98	63		79	97
laly	4,75 mm	38	98	50	100	76	77
ar	2,00 mm	33	95	43	99	74	60
өле	0,425 mm	23	77	28	74	53	25
.is	0,075 mm	13	60	9	27	19	11
	Grading Modulus (GM)	2.31	0.68	2.20	1.00	1.54	2.04
Atterberg Limits	Liquid Limit (LL)		56				28
tterberg Limits	Plasticity Index (PI)		22	1	1	1	8
At	Linear Shrinkage (LS)	6.0	10.0				2.5
	CBR @ 100 %		4	191	81		
& CBR	CBR @ 98 %		<3	107	60		
8	CBR @ 95 %		<3	45	38		
ASHTO values	CBR @ 93 %		<3	25	28		
Mod AASHTO values	CBR @ 90 %		<3	11	18		
φ	Swell @ 100 %		0.1	0.1	0.0		
Ψ	Mod AASHTO (kg/m ³)	1858	1517	2138	2054		
	O.M.C. (%)		23.3	6.5	7.3		
ility 2004(E)	% Compaction @ Moisture		90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC		93.0% Proctor @ OMC	
eab 92-11:	Permeabilty cm/s		9.26 E-07	2.19 E-04		1.51 E-04	
Permeability	% Compaction @ Moisture						
- OSI	Permeabilty cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0

Project no: 0795.0

Sempling data 1.23 Jun 3

Project : OmitiomireSampling date : 23-Jun-23Section : Heap Leach Pad (HLP)Stabilizing agent :

Layer: From chainage: To chainage:

Report no: SG23/1616 Page no: 2

	Report no: SG23/1616		 	 Page no :	
	Sample no.	S/10419			
	Chainage				
tail	Position (from LHS)	HLP-TP24			
e de	Depth sampled (mm)	800-1200			
sample detail	Sample description	Brown, gravelly sandy silt. Calcareous Alluvium.			
	Method of Preparation				
	used				
	Testing depth (mm)				
field density	Dry density (kg/m³)				
gen	Moisture (%)				
P	Mod AASHTO (kg/m ³)				
fie	O.M.C. (%)				
	Compaction (%)				
	63,0 mm	100			
ing	53,0 mm	88			
188	37,5 mm	85			
bg o	26,5 mm	80			
%	19,0 mm	79			
/sis	13,2 mm	79			
sieve analysis (% passing)	4,75 mm	76			
ar	2,00 mm	74			
eve	0,425 mm	53			
Si	0,075 mm	19			
	Grading Modulus (GM)				
erg ts	Liquid Limit (LL)				
erb	Plasticity Index (PI)				
Atterberg Limits	Linear Shrinkage (LS)	0.0			
	CBR @ 100 %				
Mod AASHTO & CBR values	CBR @ 98 %				
8	CBR @ 95 %				
TO	CBR @ 93 %				
ASHTO values	CBR @ 90 %				
¥	Swell @ 100 %				
Moc	Mod AASHTO (kg/m ³)				
	O.M.C. (%)				
cto	Proctor MDD (kg/m³)	2085			
Chem Procto	O.M.C. (%)	6.7			
. em	pH				
់	Conductivity (Sm ⁻¹)				

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

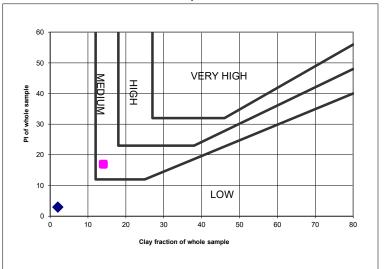
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

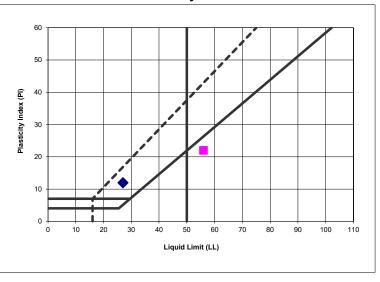
 Client
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 Project No.
 10795

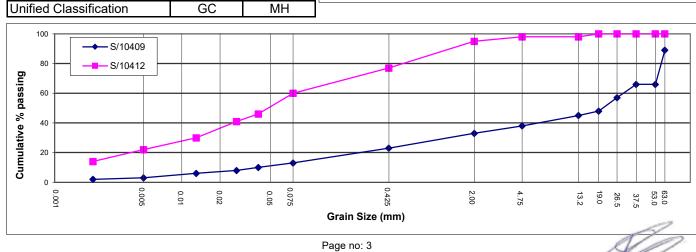
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10409	S/10412
Report No.	SG23	/1616
Depth (mm)	800-2300	650-1350
Position	HLP-TP03	HLP-TP08
Material description	Clayey gravel with sand	Sandy elastic silt
Moisture (%)		
SG	2.608	2.492
Sieve Analysi	s (% passing)
63.0 mm	89	
53.0 mm	66	
37.5 mm	66	
26.5 mm	57	
19.0 mm	48	100
13.2 mm	45	98
4.75 mm	38	98
2.00 mm	33	95
0.425 mm	23	77
0.075 mm	13	60
Hydrometer Ana	lysis (% passi	ng)
0.040 mm	10	46
0.027 mm	8	41
0.013 mm	6	30
0.005 mm	3	22
0.002 mm	2	14
% Clay (<0.002 mm)	2	14
% Silt (<0.075, >0.002 mm)	11	46
% Sand (<4.75, >0.075 mm)	25	38
% Gravel (> 4.75 mm)	62	2
Atterbe	rg Limits	
Liquid Limit (-0.425)	27	56
Plasticity Index (-0.425)	12	22
Linear Shrinkage (%)	6.0	10.0
PI of whole sample	3	17
Grading Modulus	2.31	0.68
TRB Classification	A-2-6 (0)	A-7-5 (12)
Unifical Classification	00	NAL I

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

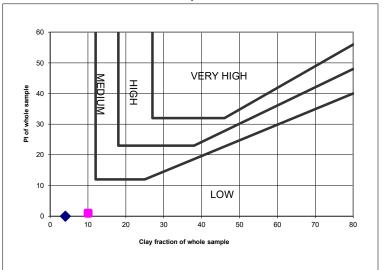
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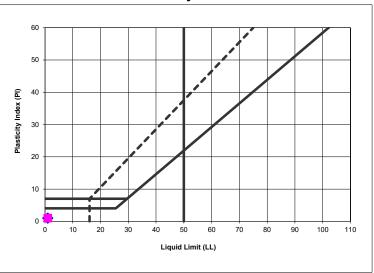
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 Project No.
 10795

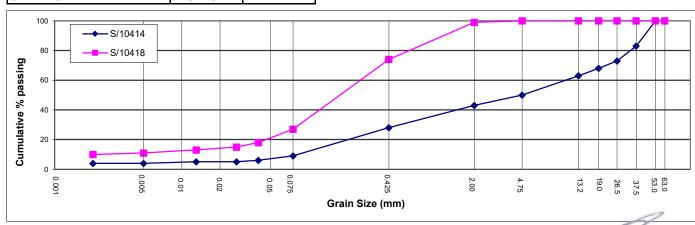
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10414	S/10418	
Report No.	SG23/1616		
Depth (mm)	1200-1750	300-2250	
Position	HLP-TP24	HLP-TP13	
Material description	Poorly graded gravel with silt and sand	Silty sand	
Moisture (%)			
SG	2.564	2.620	
Sieve Analysis	(% passing)	
63.0 mm			
53.0 mm	100		
37.5 mm	83		
26.5 mm	73		
19.0 mm	68		
13.2 mm	63		
4.75 mm	50	100	
2.00 mm	43	99	
0.425 mm	28	74	
0.075 mm	9	27	
Hydrometer Analys	sis (% passi	ng)	
0.040 mm	6	18	
0.027 mm	5	15	
0.013 mm	5	13	
0.005 mm	4	11	
0.002 mm	4	10	
% Clay (<0.002 mm)	4	10	
% Silt (<0.075, >0.002 mm)	5	17	
% Sand (<4.75, >0.075 mm)	41	73	
% Gravel (> 4.75 mm)	50	0	
Atterberg	Limits		
Liquid Limit (-0.425)			
Plasticity Index (-0.425)	1	1	
Linear Shrinkage (%)	0.0	0.0	
PI of whole sample	0	1	
Grading Modulus	2.20	1.00	
TRB Classification	A-1-a (0)	A-2-4 (0)	
Unified Classification	GP-GM	SM	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

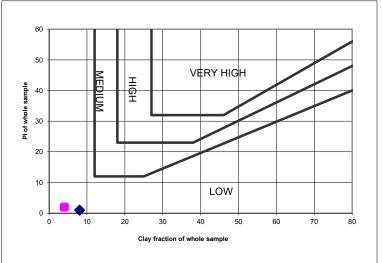
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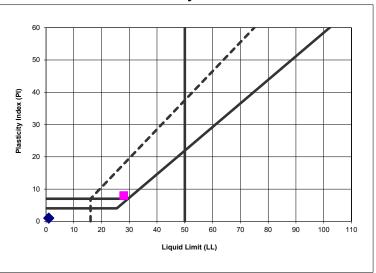
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 Project No.
 10795

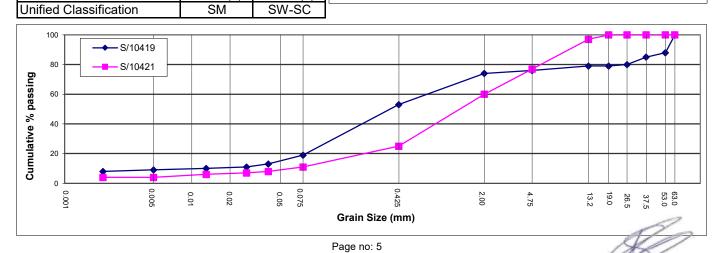
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

•			
Sample No.	S/10419	S/10421	
Report No.	SG23/1616		
Depth (mm)	800-1200	1600-1800	
Position	HLP-TP24	HLP-TP20	
	Silty sand with	Well graded sand	
Material description	gravel	with clay and gravel	
Moisture (%)			
SG	2.609	2.566	
Sieve Analysis	(% passing)	
63.0 mm	100		
53.0 mm	88		
37.5 mm	85		
26.5 mm	80		
19.0 mm	79	100	
13.2 mm	79	97	
4.75 mm	76	77	
2.00 mm	74	60	
0.425 mm	53	25	
0.075 mm	19	11	
Hydrometer Analy	sis (% passi	ing)	
0.040 mm	13	8	
0.027 mm	11	7	
0.013 mm	10	6	
0.005 mm	9	4	
0.002 mm	8	4	
% Clay (<0.002 mm)	8	4	
% Silt (<0.075, >0.002 mm)	11	7	
% Sand (<4.75, >0.075 mm)	57	66	
% Gravel (> 4.75 mm)	24	23	
Atterberg	g Limits		
Liquid Limit (-0.425)		28	
Plasticity Index (-0.425)	1	8	
Linear Shrinkage (%)	0.0	2.5	
PI of whole sample	1	2	
Grading Modulus	1.54	2.04	
TRB Classification	A-2-4 (0)	A-1-b (0)	
11 16 101 16 11	014	0144 000	

Potential Expansiveness











Project no: 10795.0

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project : Omitiomire Sampling date : 23-Jun-23

Section : Heap Leach Pad (HLP)

Layer : From chainage : To chainage :

Report no: SG23/1616 Page no: 6

	Report no: SG23/1616				Page no: 6
	Sample no.	S/10423	S/10424	S/10426	
	Chainage				
I≡	Position (from LHS)	HLP-TP04	HL/PA-TP29	HLP-TP03	
deta	Depth sampled (mm)	1100-1350	1700-1900	2300-3100	
sample detail	Sample description	Light brown blotched wihite, sandy silty gravel. Nodular Calcrete.	Yellowish brown to khaki, silty sandy gravel, ferruginised pebble marker	Khaki to white streaked to banded black, silty sandy gravel to silty gravelly sand, calcified residual gneiss	
	Method of Preparation used	Scalping	Scalping	Scalping	
	Testing depth (mm)				
iŧy	Dry density (kg/m ³)				
ens	Moisture (%)				
field density	Mod AASHTO (kg/m ³)				
fie	O.M.C. (%)				
	Compaction (%)				
	63,0 mm	85	100	90	
βι	53,0 mm	64	92	70	
ssir	37,5 mm	55	85	68	
pa	26,5 mm	51	76	63	
sieve analysis (% passing)	19,0 mm	47	67	60	
<u>si</u>	13,2 mm	45	61	58	
aly	4,75 mm	40	50	51	
an	2,00 mm	35	45	46	
eve	0,425 mm	27	29	24	
<u>.</u>	0,075 mm	16	14	11	
	Grading Modulus (GM)		2.12	2.19	
Atterberg Limits	Liquid Limit (LL)		19	25	
tterber Limits	Plasticity Index (PI)	11	6	5	
Att	Linear Shrinkage (LS)	4.0	2.0	2.5	
	CBR @ 100 %		87	44	
& CBR	CBR @ 98 %	29	64	33	
∞	CBR @ 95 %	16	40	21	
Mod AASHTO values	CBR @ 93 %		29	16	
ASP val	CBR @ 90 %		18	10	
φ	Swell @ 100 %		0.0	0.1	
ĕ	Mod AASHTO (kg/m ³)	1994	2204	1974	
	O.M.C. (%)	10.7	6.3	11.0	
ility :2004(E)	% Compaction @ Moisture	90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC	90.0% Mod AASHTO @ OMC	
eab 92-11:	Permeabilty cm/s		7.78 E-04	3.92 E-05	
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture				
081	Permeabilty cm/s				

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

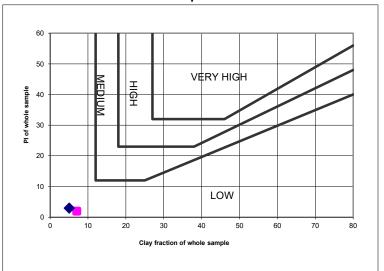
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

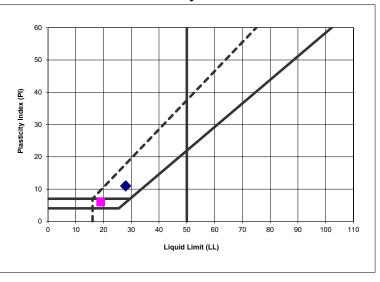
 Client
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 Project No.
 10795

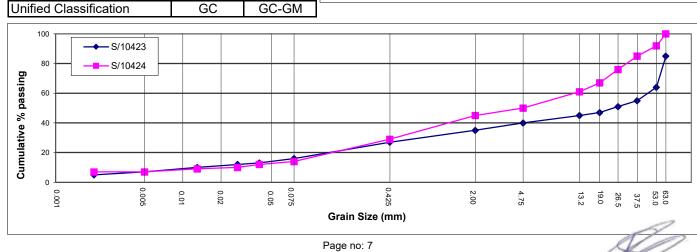
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10423	S/10424
Report No.		/1616
Depth (mm)	1100-1350	1700-1900
Position	HLP-TP04	HL/PA-TP29
Material description	Clayey gravel with sand	Silty, clayey gravel with sand
Moisture (%)		
SG	2.617	2.619
Sieve Analysi	s (% passing)
63.0 mm	85	100
53.0 mm	64	92
37.5 mm	55	85
26.5 mm	51	76
19.0 mm	47	67
13.2 mm	45	61
4.75 mm	40	50
2.00 mm	35	45
0.425 mm	27	29
0.075 mm	16	14
Hydrometer Ana	lysis (% passi	ng)
0.040 mm	13	12
0.027 mm	12	10
0.013 mm	10	9
0.005 mm	7	7
0.002 mm	5	7
% Clay (<0.002 mm)	5	7
% Silt (<0.075, >0.002 mm)	11	7
% Sand (<4.75, >0.075 mm)	24	36
% Gravel (> 4.75 mm)	60	50
Atterbe	rg Limits	
Liquid Limit (-0.425)	28	19
Plasticity Index (-0.425)	11	6
Linear Shrinkage (%)	4.0	2.0
PI of whole sample	3	2
Grading Modulus	2.22	2.12
TRB Classification	A-2-6 (1)	A-1-a (0)
11 'C 101 'C C	00,	00 014

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

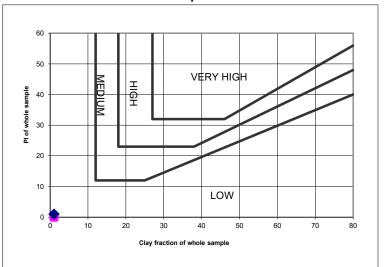
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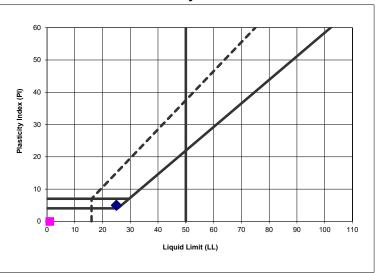
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

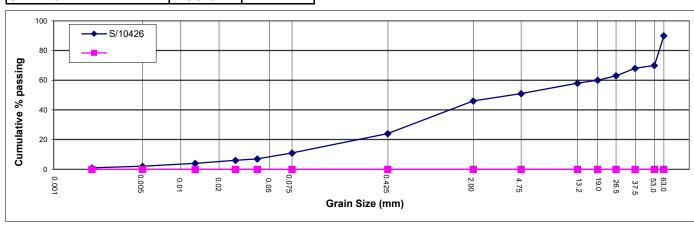
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

1 10,000	-	Omition
	0440400	1
Sample No.	S/10426	
Report No.	SG23	/1616
Depth (mm)	2300-3100	
Position	HLP-TP03	
	Silty, clayey gravel	
Material description	with sand	
Moisture (%)		
SG	2.601	
Sieve Analysi	is (% passing)
63.0 mm	90	
53.0 mm	70	
37.5 mm	68	
26.5 mm	63	
19.0 mm	60	
13.2 mm	58	
4.75 mm	51	
2.00 mm	46	
0.425 mm	24	
0.075 mm	11	
Hydrometer Ana	lysis (% passi	ng)
0.040 mm	7	
0.027 mm	6	
0.013 mm	4	
0.005 mm	2	
0.002 mm	1	
% Clay (<0.002 mm)	1	
% Silt (<0.075, >0.002 mm)	10	
% Sand (<4.75, >0.075 mm)	40	
% Gravel (> 4.75 mm)	49	
	erg Limits	
Liquid Limit (-0.425)	25	
Plasticity Index (-0.425)	5	
Linear Shrinkage (%)	2.5	
PI of whole sample	1	
Grading Modulus	2.19	
TRB Classification	A-1-a (0)	
Unified Classification	GC-GM	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

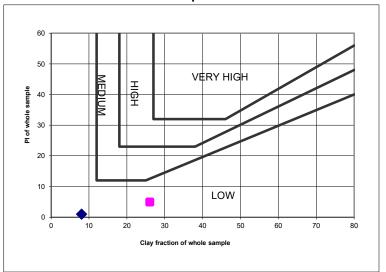
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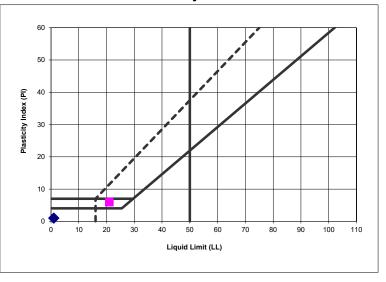
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 Project No.
 10795

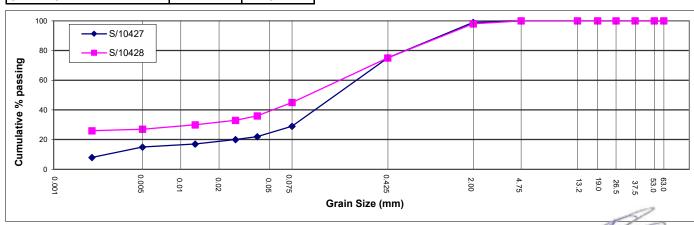
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

· -		
Sample No.	S/10427	S/10428
Report No.	SG23	/1617
Depth (mm)	0-700	700-1000
Position	Plant-TP32	Plant-TP32
Material description	Silty sand	Silty clayey sand
Moisture (%)		
SG	2.609	2.515
Sieve Analysis (% passing)
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		
13.2 mm		100
4.75 mm	100	100
2.00 mm	99	98
0.425 mm	75	75
0.075 mm	29	45
Hydrometer Analys	is (% passi	ng)
0.040 mm	22	36
0.027 mm	20	33
0.013 mm	17	30
0.005 mm	15	27
0.002 mm	8	26
% Clay (<0.002 mm)	8	26
% Silt (<0.075, >0.002 mm)	21	19
% Sand (<4.75, >0.075 mm)	71	55
% Gravel (> 4.75 mm)	0	0
Atterberg	Limits	
Liquid Limit (-0.425)		21
Plasticity Index (-0.425)	1	6
Linear Shrinkage (%)	0.0	2.0
PI of whole sample	1	5
Grading Modulus	0.97	0.82
TRB Classification	A-2-4 (0)	A-4 (2)

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

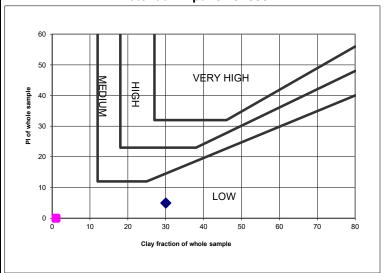
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

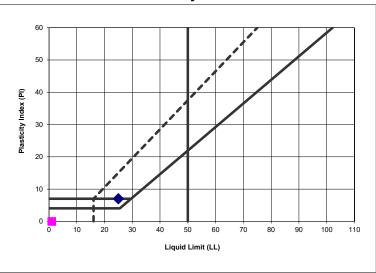
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

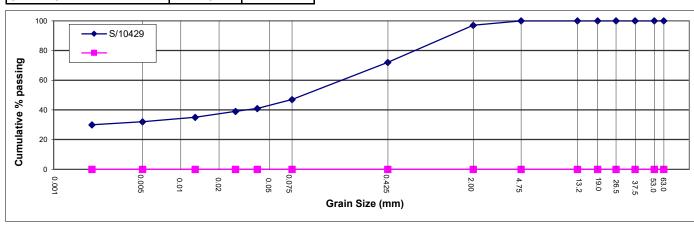
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Project		Omition
Sample No.	S/10429	
Report No.		3/1617
Depth (mm)		7 10 17
Depth (mm)	1000-1700	
Position	Plant-TP32	
Material description	Clayey sand	
Moisture (%)		
SG	2.538	
Sieve Analysi	s (% passing)
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		
13.2 mm	100	
4.75 mm	100	
2.00 mm	97	
0.425 mm	72	
0.075 mm	47	
Hydrometer Ana	lysis (% passi	ing)
0.040 mm	41	
0.027 mm	39	
0.013 mm	35	
0.005 mm	32	
0.002 mm	30	
% Clay (<0.002 mm)	30	
% Silt (<0.075, >0.002 mm)	17	
% Sand (<4.75, >0.075 mm)	53	
% Gravel (> 4.75 mm)	0	
	rg Limits	
Liquid Limit (-0.425)	25	
Plasticity Index (-0.425)	7	
Linear Shrinkage (%)	2.5	
PI of whole sample	5	
Grading Modulus	0.84	
TRB Classification	A-4 (3)	
Unified Classification	SC	

Potential Expansiveness









SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0

Project : OmitiomireSampling date : 23-Jun-23Section : New RoadStabilizing agent :

Layer: From chainage: To chainage: Report no: SG23/1618 Page no: 1

	Sample no.	S/10432	S/10437	S/10443	S/10450	S/10452	S/10454
	Chainage		5/1045/	G/10443	G/10430	G/ 10432	G/ 10434
	Position (from LHS)		RD-TP04	RD-TP12	RD-TP08	RD-TP02	RD-TP14
tail	Depth sampled (mm)	1200-2800	600-2900	1100-4300	1000-3200	3900-4300	1400-2600
sample detail	Deptil sampled (min)		600-2900		1000-3200	3900-4300	
	Sample description	Brown to greyish brown, silty clayey sand with minor cobbles to boulders, coarse alluvium	Reddish brown, clayey silty coarse sand with traces of gravel, fine alluvium	Reddish brown slightly stained brown, clayey silty gravelly sand with minor fine gravel, fine alluvium	White to yellowish brown, silty gravelly sand, calcified alluvium	Completely weathered, white streaked black and gold, very soft rock, biotite gneiss	Completely weathered, dark grey streaked white stained orange, very soft rock, ferruginised biotite gneiss
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
Ē	Dry density (kg/m ³)						
field density	Moisture (%)						
o o	Mod AASHTO (kg/m ³)						
fiel	O.M.C. (%)						
	Compaction (%)						
	63,0 mm	89				100	
g)	53,0 mm	89				85	100
ssin	37,5 mm	86				81	91
pas	26,5 mm	82				73	86
%)	19,0 mm	79			100	72	81
<u></u>	13,2 mm	79			98	71	75
sieve analysis (% passing)	4,75 mm	73	100	100	92	69	61
an	2,00 mm	58	99	97	88	62	52
) ve	0,425 mm	21	84	77	71	26	40
Sie	0,075 mm	13	34	30	46	16	25
	Grading Modulus (GM)	2.08	0.83	0.96	0.95	1.96	1.83
erg s	Liquid Limit (LL)	28			48	29	34
Atterberg Limits	Plasticity Index (PI)	11	1	1	20	7	9
Atte	Linear Shrinkage (LS)	3.5			7.5	2.5	3.5
	CBR @ 100 %		118	60	5	29	23
BR	CBR @ 98 %		81	46	5	20	20
HTO & CBR lues	CBR @ 95 %		46	30	4	11	17
HTO	CBR @ 93 %		32	23	4	8	15
ASH valu	CBR @ 90 %		18	16	3	5	12
₹	Swell @ 100 %		0.1	-0.1	0.0	0.0	0.0
Mod AASI val	Mod AASHTO (kg/m ³)		2041	2119	1640	1940	2016
	O.M.C. (%)		8.1	7.4	20.0	9.2	12.9
lity 2004(E)	% Compaction @ Moisture						
2-11:2	Permeabilty cm/s						
Permeability ISO/TS 17892-11:2004(E)	% Compaction @ Moisture						
Р SO/	Permeabilty cm/s						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.





SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client : Knight Piesold (Pty) Ltd (Namibia)Project no : 10795.0Project : OmitiomireSampling date : 23-Jun-23

Section: New Road Stabilizing agent:

Layer: From chainage: To chainage:

Report no: SG23/1618 Page no: 2

	Report no: SG23/1618			Page no :	2
	Sample no.	S/10455			
	Chainage				
I≡	Position (from LHS)	RD-TP13			
deta	Depth sampled (mm)	2600-4000			
sample detail		Yellowish brown with scattered red, white and			
amp	Sample description	black mottles, clayey gravelly silty fine sand			
S		with minor gravels,			
	Method of Preparation	slightly			
	used	Scalping			
	Testing depth (mm)				
sity	Dry density (kg/m ³)				
field density	Moisture (%)				
o pl	Mod AASHTO (kg/m ³)				
fie	O.M.C. (%)				
	Compaction (%)				
	63,0 mm				
lg)	53,0 mm				
ssir	37,5 mm				
pa	26,5 mm				
%)	19,0 mm	100			
sis	13,2 mm	99			
sieve analysis (% passing)	4,75 mm	95			
an	2,00 mm	90			
eve	0,425 mm	77			
<u>si</u>	0,075 mm	38			
	Grading Modulus (GM)				
erg s	Liquid Limit (LL)	27			
Atterberg Limits	Plasticity Index (PI)	9			
Att	Linear Shrinkage (LS)	4.5			
	CBR @ 100 %				
& CBR	CBR @ 98 %				
ತ ೧	CBR @ 95 %				
ASHTO values	CBR @ 93 %				
\SH valt	CBR @ 90 %				
Mod AASHTO values	Swell @ 100 %				
Мос	Mod AASHTO (kg/m ³)				
	O.M.C. (%)				
ty)4(E)	% Compaction @ Moisture				
Permeability	Permeabilty cm/s				
mea 7892-	% Compaction @ Moisture				
Peri					
	Permeabilty cm/s				

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

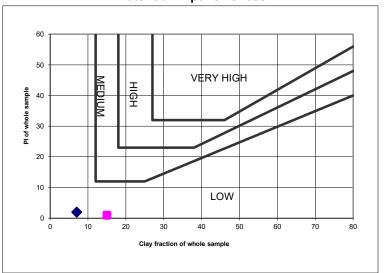
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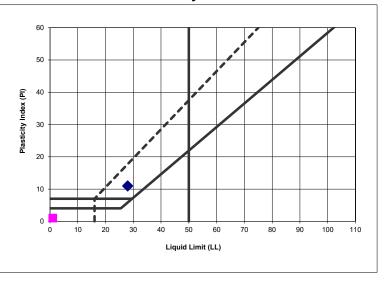
 Client
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 Project No.
 10795

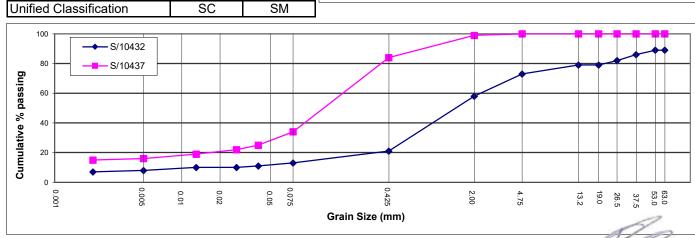
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10432	S/10437
Report No.	SG23	/1618
Depth (mm)	1200-2800	600-2900
Position	RD-TP02	RD-TP04
Material description	Clayey sand with gravel	Silty sand
Moisture (%)		
SG	2.609	2.637
Sieve Analys	is (% passing)
63.0 mm	89	
53.0 mm	89	
37.5 mm	86	
26.5 mm	82	
19.0 mm	79	
13.2 mm	79	
4.75 mm	73	100
2.00 mm	58	99
0.425 mm	21	84
0.075 mm	13	34
Hydrometer Ana	lysis (% passi	ng)
0.040 mm	11	25
0.027 mm	10	22
0.013 mm	10	19
0.005 mm	8	16
0.002 mm	7	15
% Clay (<0.002 mm)	7	15
% Silt (<0.075, >0.002 mm)	6	19
% Sand (<4.75, >0.075 mm)	60	66
% Gravel (> 4.75 mm)	27	0
Atterbe	erg Limits	
Liquid Limit (-0.425)	28	
Plasticity Index (-0.425)	11	1
Linear Shrinkage (%)	3.5	0.0
PI of whole sample	2	1
Grading Modulus	2.08	0.83
TRB Classification	A-2-6 (0)	A-2-4 (0)
Unifical Classification	00	014

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

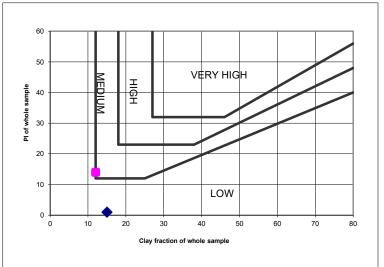
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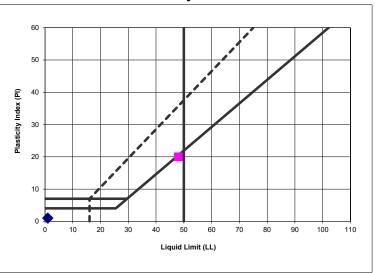
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 Project No.
 10795

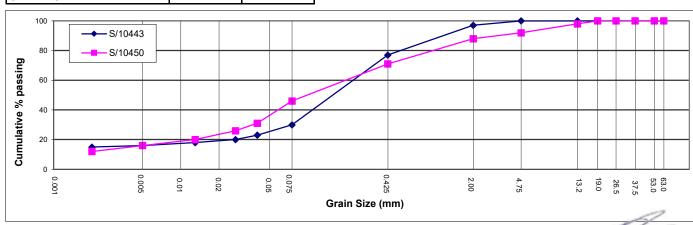
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Floject		Offilial
Sample No.	S/10443	S/10450
Report No.	SG23	/1618
Depth (mm)	1100-4300	1000-3200
Position	RD-TP12	RD-TP08
Material description	Silty sand	Silty sand
Moisture (%)	0.040	0.444
SG	2.648	2.411
Sieve Analysis	(% passing)
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm		100
13.2 mm		98
4.75 mm	100	92
2.00 mm	97	88
0.425 mm	77	71
0.075 mm	30	46
Hydrometer Analys	sis (% passi	ng)
0.040 mm	23	31
0.027 mm	20	26
0.013 mm	18	20
0.005 mm	16	16
0.002 mm	15	12
% Clay (<0.002 mm)	15	12
% Silt (<0.075, >0.002 mm)	15	34
% Sand (<4.75, >0.075 mm)	70	46
% Gravel (> 4.75 mm)	0	8
Atterberg	Limits	
Liquid Limit (-0.425)		48
Plasticity Index (-0.425)	1	20
Linear Shrinkage (%)	0.0	7.5
PI of whole sample	1	14
Grading Modulus	0.96	0.95
TRB Classification	A-2-4 (0)	A-7-5 (5)
Unified Classification	SM	SM

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

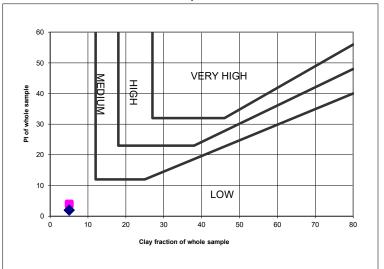
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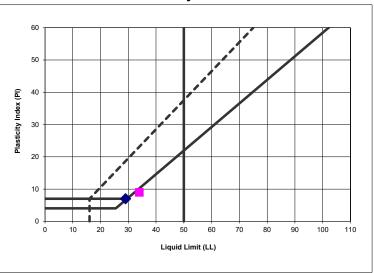
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 Project No.
 10795

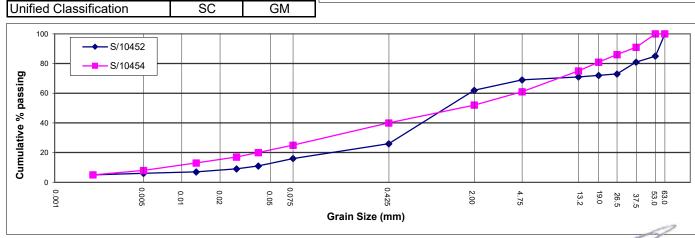
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10452	S/10454
Report No.	SG23	/1618
Depth (mm)	3900-4300	1400-2600
Position	RD-TP02	RD-TP14
Material description	Clayey sand with gravel	Silty gravel with sand
Moisture (%)		
SG	2.673	2.604
Sieve Analys	is (% passing)
63.0 mm	100	
53.0 mm	85	100
37.5 mm	81	91
26.5 mm	73	86
19.0 mm	72	81
13.2 mm	71	75
4.75 mm	69	61
2.00 mm	62	52
0.425 mm	26	40
0.075 mm	16	25
Hydrometer Ana	lysis (% passi	ng)
0.040 mm	11	20
0.027 mm	9	17
0.013 mm	7	13
0.005 mm	6	8
0.002 mm	5	5
% Clay (<0.002 mm)	5	5
% Silt (<0.075, >0.002 mm)	11	20
% Sand (<4.75, >0.075 mm)	53	36
% Gravel (> 4.75 mm)	31	39
Atterbe	erg Limits	
Liquid Limit (-0.425)	29	34
Plasticity Index (-0.425)	7	9
Linear Shrinkage (%)	2.5	3.5
PI of whole sample	2	4
Grading Modulus	1.96	1.83
TRB Classification	A-2-4 (0)	A-2-4 (0)
Haifing Olamaifingsina	CC	CM

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

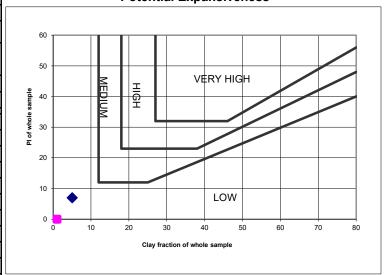
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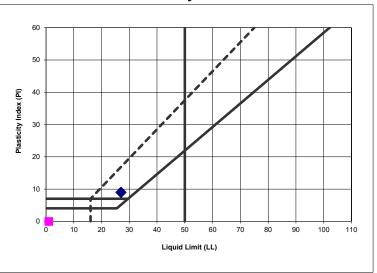
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 Project No.
 10795

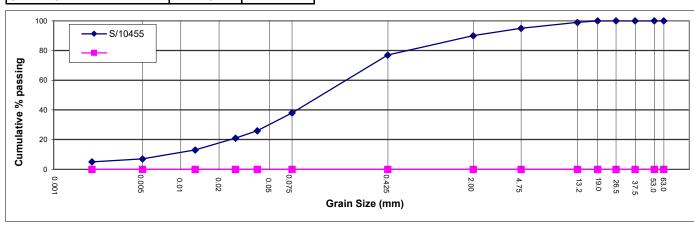
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Project		Omition
Sample No.	S/10455	
Report No.		3/1618
Depth (mm)		7 10 10
Deptir (min)	2600-4000	
Position	RD-TP13	
Material description	Clayey sand	
Moisture (%)		
SG	2.669	
Sieve Analysis	s (% passing)
63.0 mm		
53.0 mm		
37.5 mm		
26.5 mm		
19.0 mm	100	
13.2 mm	99	
4.75 mm	95	
2.00 mm	90	
0.425 mm	77	
0.075 mm	38	
Hydrometer Anal	ysis (% passi	ing)
0.040 mm	26	
0.027 mm	21	
0.013 mm	13	
0.005 mm	7	
0.002 mm	5	
% Clay (<0.002 mm)	5	
% Silt (<0.075, >0.002 mm)	33	
% Sand (<4.75, >0.075 mm)	57	
% Gravel (> 4.75 mm)	5	
	rg Limits	
Liquid Limit (-0.425)	27	
Plasticity Index (-0.425)	9	
Linear Shrinkage (%)	4.5	
PI of whole sample	7	
Grading Modulus	0.95	
TRB Classification	A-4 (1)	
Unified Classification	SC	

Potential Expansiveness









Layer:



SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0 Project: Omitiomire Sampling date: 23-Jun-23

Section: Stormwater Diversion Channel (SWD)

Stabilizing agent : To chainage: From chainage:

Report no: SG23/1619 Page no: 1

_	Report no : 0023/1013					r age no .	
	Sample no.	S/10456	S/10459	S/10463	S/10465	S/10466	S/10468
	Chainage						
I ≡ I	Position (from LHS)	SWD-TP02	SWD-TP15	SWD-TP23	SWD-TP17	SWD-TP08	SWD-TP19
Jeta	Depth sampled (mm)	0-700	500-2500	1200-2500	1300-2200	1000-1400	1400-2400
sample detail	Sample description	Brown mottled black , gravelly silty sand, fine alluvium	Orange brown stained red, clayey silty fine gravelly sand, fine alluvium	Dark reddish brown, sandy silty clay. Fine Alluvium	Olive brown mottled white stained orange, gravelly silty fine sand. Calcified Alluvium.	Dark reddish brown speckled white, silty sandy gravel. Ferruginized Alluvium.	Yellowish brown mottled black stained red, clayey silty gravelly sand with trace gravels, ferruginised alluvium
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
īţ	Dry density (kg/m³)						
ensi	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fiel	O.M.C. (%)						
	Compaction (%)						
	63,0 mm						
g)	53,0 mm						
sin	37,5 mm				100		
sieve analysis (% passing)	26,5 mm				95	100	
1%	19,0 mm				92	95	
sis (13,2 mm			100	92	94	100
slys	4,75 mm	100	100	99	81	84	99
ans	2,00 mm	99	98	98	71	67	97
Ve	0,425 mm	74	74	86	45	42	85
sie	0,075 mm	32	40	53	23	19	46
	Grading Modulus (GM)	0.95	0.88	0.63	1.61	1.72	0.72
rg ,	Liquid Limit (LL)		20	23	38	31	25
Atterberg Limits	Plasticity Index (PI)	3	8	8	16	15	8
Atte	Linear Shrinkage (LS)	1.0	4.0	2.5	5.5	5.0	2.0
	CBR @ 100 %		12			6	
BR	CBR @ 98 %		12			4	
& CBR	CBR @ 95 %		11			<3	
HTO ues	CBR @ 93 %		11			<3	
ASH valt	CBR @ 90 %		10			<3	
₹	Swell @ 100 %		0.0			0.3	
Mod AASHTO values	Mod AASHTO (kg/m³)		2055			2053	
	O.M.C. (%)		9.7			7.3	
lity 2004(E)	% Compaction @ Moisture		90.0% Mod AASHTO @ OMC			90.0% Mod AASHTO @ OMC	
eabi	Permeabilty cm/s		3.57 E-05			4.59 E-05	
Permeability	% Compaction @ Moisture						
es lso	Permeabilty cm/s						

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

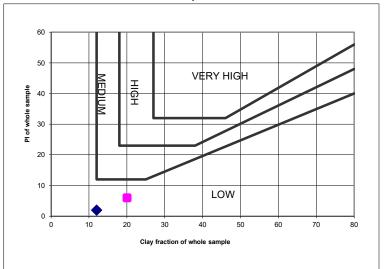
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

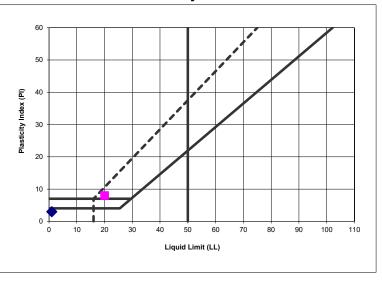
 Client
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 Project No.
 10795

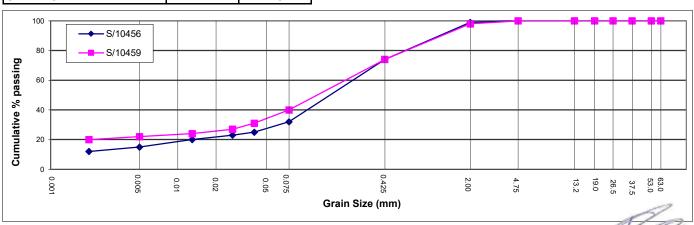
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10456	S/10459	
Report No.	SG23/1619		
Depth (mm)	0-700	500-2500	
Position	SWD-TP02	SWD-TP15	
Matarial decarintian	Silty sand	Clayey sand	
Material description			
Moisture (%)	0.500	2.000	
SG	2.586	2.628	
Sieve Analysis	(% passing)	
63.0 mm			
53.0 mm			
37.5 mm			
26.5 mm			
19.0 mm			
13.2 mm			
4.75 mm	100	100	
2.00 mm	99	98	
0.425 mm	74	74	
0.075 mm	32	40	
Hydrometer Analysis (% passing)			
0.040 mm	25	31	
0.027 mm	23	27	
0.013 mm	20	24	
0.005 mm	15	22	
0.002 mm	12	20	
% Clay (<0.002 mm)	12	20	
% Silt (<0.075, >0.002 mm)	20	20	
% Sand (<4.75, >0.075 mm)	68	60	
% Gravel (> 4.75 mm)	0	0	
Atterberg Limits			
Liquid Limit (-0.425)		20	
Plasticity Index (-0.425)	3	8	
Linear Shrinkage (%)	1.0	4.0	
PI of whole sample	2	6	
Grading Modulus	0.95	0.88	
TRB Classification	A-2-4 (0)	A-4 (0)	
Unified Classification	SM	SC	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

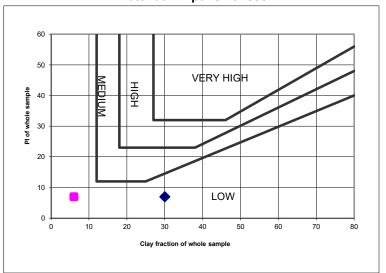
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

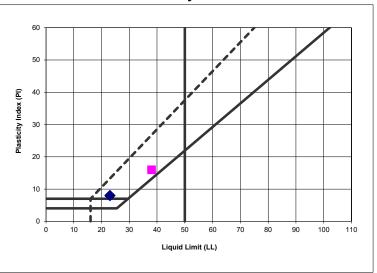
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

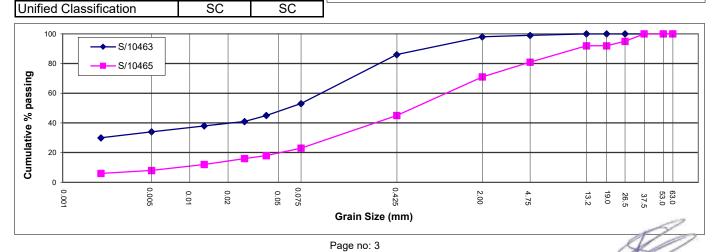
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

•	-			
Sample No.	S/10463	S/10465		
Report No.	SG23	SG23/1619		
Depth (mm)	1200-2500	1300-2200		
Position	SWD-TP23	SWD-TP17		
Material description	Clayey sand	Clayey sand with gravel		
Moisture (%)				
SG	2.654	2.615		
Sieve Analysis (% passing)				
63.0 mm				
53.0 mm				
37.5 mm		100		
26.5 mm		95		
19.0 mm		92		
13.2 mm	100	92		
4.75 mm	99	81		
2.00 mm	98	71		
0.425 mm	86	45		
0.075 mm	53	23		
Hydrometer Analysis (% passing)				
0.040 mm	45	18		
0.027 mm	41	16		
0.013 mm	38	12		
0.005 mm	34	8		
0.002 mm	30	6		
% Clay (<0.002 mm)	30	6		
% Silt (<0.075, >0.002 mm)	23	17		
% Sand (<4.75, >0.075 mm)	46	58		
% Gravel (> 4.75 mm)	1	19		
Atterbei	rg Limits			
Liquid Limit (-0.425)	23	38		
Plasticity Index (-0.425)	8	16		
Linear Shrinkage (%)	2.5	5.5		
PI of whole sample	7	7		
Grading Modulus	0.63	1.61		
TRB Classification	A-4 (4)	A-2-6 (1)		

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

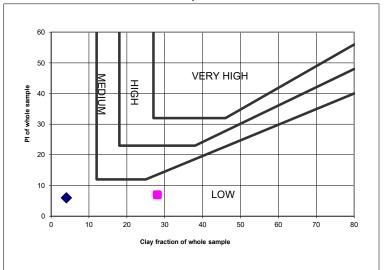
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

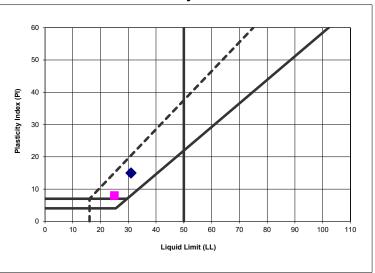
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

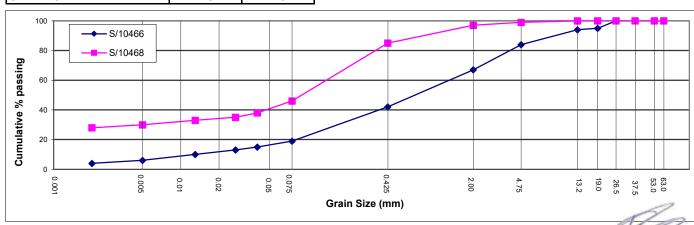
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10466	S/10468		
Report No.	SG23/1619			
Depth (mm)	1000-1400	1400-2400		
Position	SWD-TP08	SWD-TP19		
	Clayey sand with	Clayey sand		
Material description	gravel			
Moisture (%)				
SG	2.557	2.645		
Sieve Analysis	(% passing)		
63.0 mm				
53.0 mm				
37.5 mm				
26.5 mm	100			
19.0 mm	95			
13.2 mm	94	100		
4.75 mm	84	99		
2.00 mm	67	97		
0.425 mm	42	85		
0.075 mm	19	46		
Hydrometer Analysis (% passing)				
0.040 mm	15	38		
0.027 mm	13	35		
0.013 mm	10	33		
0.005 mm	6	30		
0.002 mm	4	28		
% Clay (<0.002 mm)	4	28		
% Silt (<0.075, >0.002 mm)	15	18		
% Sand (<4.75, >0.075 mm)	65	53		
% Gravel (> 4.75 mm)	16	1		
Atterberg	Limits			
Liquid Limit (-0.425)	31	25		
Plasticity Index (-0.425)	15	8		
Linear Shrinkage (%)	5.0	2.0		
PI of whole sample	6	7		
Grading Modulus	1.72	0.72		
TRB Classification	A-2-6 (1)	A-4 (1)		
Unified Classification	SC	SC		

Potential Expansiveness











Project no: 10795.0

SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Sampling date: 23-Jun-23 Project: Omitiomire

Section: Stormwater Diversion Channel (SWD) Stabilizing agent: From chainage: To chainage: Layer:

Report no: SG23/1619 Page no: 5 Sample no S/10470 S/10471 Chainage Position (from LHS) SWD-TP07 SWD-TP10 sample detail Depth sampled (mm) 1400-1900 1200-2100 Completely weathered, green speckled to Reddish brown mottled Sample description streaked black, white white, silty sandy gravel and gold, very soft rock Nodular Ferricrete. biotite gneiss Method of Preparation Scalping Scalping used Testing depth (mm) field density Dry density (kg/m³) Moisture (%) Mod AASHTO (kg/m³) O.M.C. (%) Compaction (%) 63,0 mm 100 53,0 mm 86 sieve analysis (% passing) 37.5 mm 100 80 74 26,5 mm 95 92 66 19,0 mm 13,2 mm 91 61 4.75 mm 75 59 2,00 mm 63 57 0,425 mm 40 49 0,075 mm 20 32 Grading Modulus (GM) 1.77 1.62 Liquid Limit (LL) 32 29 Atterberg Limits Plasticity Index (PI) 12 12 Linear Shrinkage (LS) 3.5 6.0 CBR @ 100 % 65 8 Mod AASHTO & CBR CBR @ 98 % 40 6 CBR @ 95 % 19 4 CBR @ 93 % 12 3 CBR @ 90 % <3 6 Swell @ 100 % 0.0 0.0 Mod AASHTO (kg/m3) 2174 1875 O.M.C. (%) 7.2 14.9 % Compaction @ Moisture SO/TS 17892-11:2004(E Permeability

Remarks:

Permeabilty cm/s

% Compaction @ Moisture Permeabilty cm/s

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

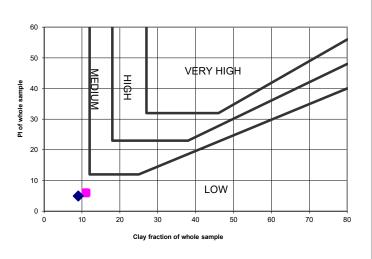
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

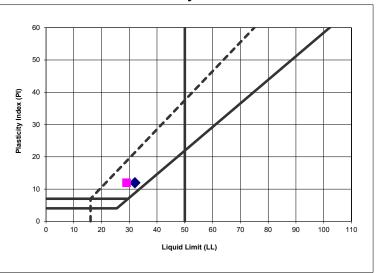
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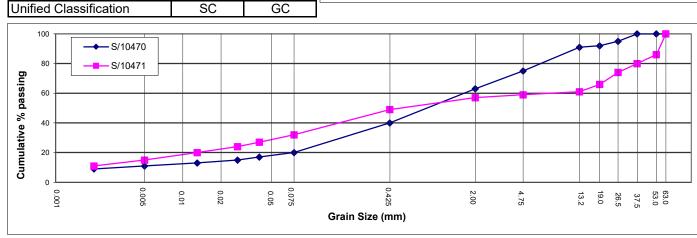
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

	_		
Sample No.	S/10470 S/1047		
Report No.	SG23	/1619	
Depth (mm)	1400-1900	1200-2100	
Position	SWD-TP07	SWD-TP10	
	Clayey sand with	Clayey gravel with	
Material description	gravel	sand	
Moisture (%)			
SG	2.736	2.629	
Sieve Analysis	(% passing)	
63.0 mm		100	
53.0 mm		86	
37.5 mm	100	80	
26.5 mm	95	74	
19.0 mm	92	66	
13.2 mm	91	61	
4.75 mm	75	59	
2.00 mm	63	57	
0.425 mm	40	49	
0.075 mm	20	32	
Hydrometer Analy	sis (% passi	ng)	
0.040 mm	17	27	
0.027 mm	15	24	
0.013 mm	13	20	
0.005 mm	11	15	
0.002 mm	9	11	
% Clay (<0.002 mm)	9	11	
% Silt (<0.075, >0.002 mm)	11	21	
% Sand (<4.75, >0.075 mm)	55	27	
% Gravel (> 4.75 mm)	25	41	
Atterberg	g Limits		
Liquid Limit (-0.425)	32	29	
Plasticity Index (-0.425)	12	12	
Linear Shrinkage (%)	3.5	6.0	
PI of whole sample	5	6	
Grading Modulus	1.77	1.62	
TRB Classification	A-2-6 (1)	A-2-6 (1)	
11 'C 101 'C C	00,	00,	

Potential Expansiveness











SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0 Project: Omitiomire Sampling date: 23-Jun-23

Section: Waste Rock Dump Stabilizing agent : From chainage: To chainage: Layer:

Report no: SG23/1620 Page no: 1

	Report no : SG23/1620					Page no :	
	Sample no.	S/10472	S/10474	S/10478	S/10479	S/10488	S/10494
	Chainage						
ië.	Position (from LHS)		WRD-TP04	WRD-TP14	WRD-TP15	WRD-TP04	WRD-TP26
deta	Depth sampled (mm)	1400-3500	1500-2400	2800-4900	0-700	400-1500	1200-2600
sample detail	Sample description	Dark reddish brown, silty sandy clay. Fine Alluvium.	Orange brown, gravelly silty sand. Alluvium.	Pale yellow brown, silty sandy gravel and cobbles. Coarse Alluvium.	Brown, sandy clayey silt. Fine Alluvium.	Pale brown mottled white, gravelly sandy silt. Calcified Alluvium.	White speckled to mottled black and brown, silty sandy gravel, hardpan calcrete
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
ity	Dry density (kg/m ³)						
ens	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fie	O.M.C. (%)						
	Compaction (%)						
	63,0 mm						82
(gı	53,0 mm			100			76
ssir	37,5 mm		100	85			71
pas	26,5 mm		98	75			63
%)	19,0 mm		98	71			60
sis	13,2 mm	100	97	68	100	100	53
sieve analysis (% passing)	4,75 mm	100	96	63	97	98	42
an	2,00 mm	98	94	52	95	92	34
eve	0,425 mm	81	79	32	86	76	22
sie	0,075 mm	40	43	18	62	28	20
	Grading Modulus (GM)	0.81	0.84	1.98	0.57	1.04	2.24
erg S	Liquid Limit (LL)	23	29	29	33	25	31
Atterberg Limits	Plasticity Index (PI)	9	12	11	16	6	9
Att	Linear Shrinkage (LS)	2.5	2.5	4.0	7.0	2.5	3.0
	CBR @ 100 %	9	8	9		8	
& CBR	CBR @ 98 %	5	7	8		6	
& ⊘	CBR @ 95 %	<3	5	8		3	
TO es	CBR @ 93 %	<3	5	7		<3	
ASH ⁻ valu	CBR @ 90 %	<3	4	7		<3	
Mod AASH valu	Swell @ 100 %	0.0	0.1	0.0		0.1	
Moc	Mod AASHTO (kg/m ³)	2035	1963	2071		1975	
	O.M.C. (%)	9.3	12.0	9.5		11.0	
ty 04(E)	% Compaction @ Moisture			90.0% Mod AASHTO @ OMC		90.0% Mod AASHTO @ OMC	
11:20	Permeabilty cm/s			7.44 E-05		1.19 E-05	
Permeability	% Compaction @ Moisture						
Per o/TS	· -						
<u>ğ</u> Rema	Permeabilty cm/s						20

Remarks:

Note 1: In the case of G6 and G7 material with a large coarse fraction, Maximum Pi = 3GM+10

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SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia) Project no: 10795.0 **Project:** Omitiomire Sampling date: 23-Jun-23

Section: Waste Rock Dump Stabilizing agent : From chainage:

To chainage : Layer: SC22/4620

	Report no: SG23/1620			Page no :	2
	Sample no.	S/10479			
	Chainage				
tail	Position (from LHS)	WRD-TP15			
e det	Depth sampled (mm)	0-700			
sample detail	Sample description	Brown, sandy clayey silt. Fine Alluvium.			
	Method of Preparation				
	used Testing depth (mm)				
_					
ısit	Dry density (kg/m³) Moisture (%)				
der					
field density	Mod AASHTO (kg/m ³) O.M.C. (%)				
-	Compaction (%)				
	63,0 mm				
a)	53,0 mm				
sin	37,5 mm				
oas	26,5 mm				
%	19,0 mm				
sieve analysis (% passing)	13,2 mm	100			
alys	4,75 mm	97			
ans	2,00 mm	95			
ve Ve	0,425 mm	86			
sie	0,075 mm	62			
	Grading Modulus (GM)	0.57			
ərg s	Liquid Limit (LL)	33			
erbe	Plasticity Index (PI)	16			
Atterberg Limits	Linear Shrinkage (LS)	7.0			
	CBR @ 100 %				
Mod AASHTO & CBR values	CBR @ 98 %				
ಪ	CBR @ 95 %				
ASHTO values	CBR @ 93 %				
4SF valı	CBR @ 90 %				
Α̈́	Swell @ 100 %				
Mo	Mod AASHTO (kg/m ³)				
	O.M.C. (%)				
cto	Proctor MDD (kg/m³)	1628			
Pro	O.M.C. (%)	14.0			
Chem Procto	рН				
ភ	Conductivity (Sm ⁻¹)				

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SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

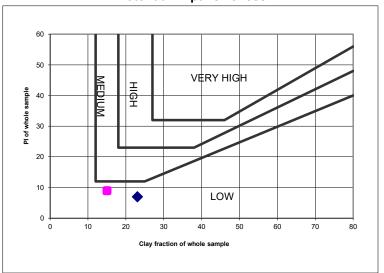
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

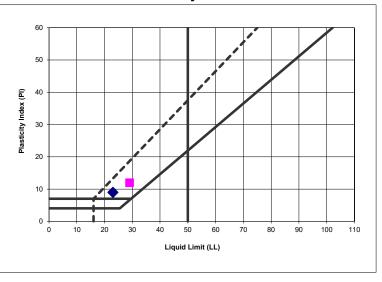
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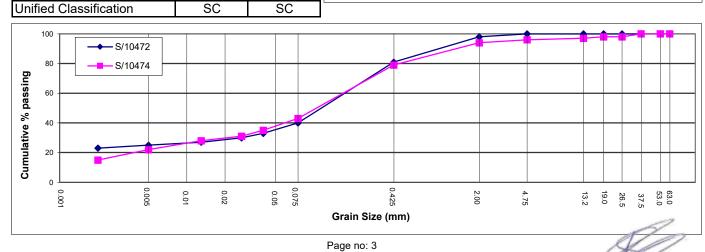
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Report No. SG23/1620 Depth (mm) 1400-3500 1500-2400 Position WRD-TP01 WRD-TP04 Material description Clayey sand Clayey sand Moisture (%) 2.624 2.641 Sieve Analysis (% passing) 63.0 mm 53.0 mm 37.5 mm 100 26.5 mm 98 98 19.0 mm 98 94 13.2 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Silt (<0.075, >0.002 mm) 17 28 % Sand (<4.75, >0.075 mm) 60 53 % Gravel (>4.75 mm) 0 4 Atterberg Limits	•					
Depth (mm)	Sample No.	S/10472	S/10474			
Material description	Report No.	SG23/1620				
Material description Clayey sand Clayey sand Moisture (%) 2.624 2.641 Sieve Analysis (% passing) 63.0 mm 37.5 mm 100 26.5 mm 98 190 mm 19.0 mm 98 94 19.0 mm 98 94 2.00 mm 98 94 0.425 mm 100 96 2.00 mm 98 94 0.425 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	Depth (mm)	1400-3500	1500-2400			
Moisture (%) SG	Position	WRD-TP01	WRD-TP04			
Sieve Analysis (% passing)	Material description	Clayey sand	Clayey sand			
Sieve Analysis (% passing) 63.0 mm 53.0 mm 37.5 mm 100 26.5 mm 98 19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)						
63.0 mm 100 53.0 mm 100 26.5 mm 98 19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	, ,	2.624	2.641			
63.0 mm 100 53.0 mm 100 26.5 mm 98 19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	Sieve Analysis (% passing)					
100 26.5 mm 98 98 19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 43 44 43 45 45 45 45		Ì				
26.5 mm 98 19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	53.0 mm					
19.0 mm 98 13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Silt (<0.075, >0.002 mm) 17 28 % Sand (<4.75, >0.075 mm) 60 53 % Gravel (>4.75 mm) 0 4 Atterberg Limits Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 Pl of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	37.5 mm		100			
13.2 mm 100 97 4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	26.5 mm		98			
4.75 mm 100 96 2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	19.0 mm		98			
2.00 mm 98 94 0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	13.2 mm	100	97			
0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	4.75 mm	100	96			
0.425 mm 81 79 0.075 mm 40 43 Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	2.00 mm	98	94			
Hydrometer Analysis (% passing) 0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	0.425 mm	81	79			
0.040 mm 33 35 0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	0.075 mm	40	43			
0.027 mm 30 31 0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	Hydrometer Analy	sis (% passi	ng)			
0.013 mm 27 28 0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	0.040 mm	33	35			
0.005 mm 25 22 0.002 mm 23 15 % Clay (<0.002 mm)	0.027 mm	30	31			
0.002 mm 23 15 % Clay (<0.002 mm)	0.013 mm	27	28			
% Clay (<0.002 mm)	0.005 mm	25	22			
% Silt (<0.075, >0.002 mm) 17 28 % Sand (<4.75, >0.075 mm) 60 53 % Gravel (>4.75 mm) 0 4 Atterberg Limits Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 Pl of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	0.002 mm	23	15			
% Sand (<4.75, >0.075 mm) 60 53 % Gravel (>4.75 mm) 0 4 Atterberg Limits Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 PI of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	% Clay (<0.002 mm)	23	15			
% Sand (<4.75, >0.075 mm) 60 53 % Gravel (>4.75 mm) 0 4 Atterberg Limits Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 PI of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	% Silt (<0.075, >0.002 mm)	17	28			
Atterberg Limits Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 Pl of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	% Sand (<4.75, >0.075 mm)	60	53			
Liquid Limit (-0.425) 23 29 Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 Pl of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	% Gravel (> 4.75 mm)	0	4			
Plasticity Index (-0.425) 9 12 Linear Shrinkage (%) 2.5 2.5 PI of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	Atterberg	Limits				
Linear Shrinkage (%) 2.5 2.5 PI of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	Liquid Limit (-0.425)	23				
PI of whole sample 7 9 Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	Plasticity Index (-0.425)	9				
Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	Linear Shrinkage (%)	2.5	2.5			
Grading Modulus 0.81 0.84 TRB Classification A-4 (1) A-6 (3)	PI of whole sample	7	9			
TRB Classification A-4 (1) A-6 (3)	Grading Modulus	0.81	0.84			
	<u> </u>	A-4 (1)	A-6 (3)			
	Unified Classification	SC	SC			

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

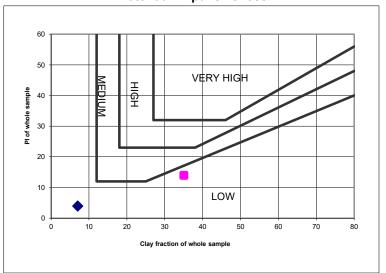
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

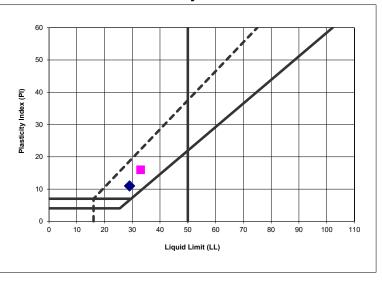
 Client
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 Project No.
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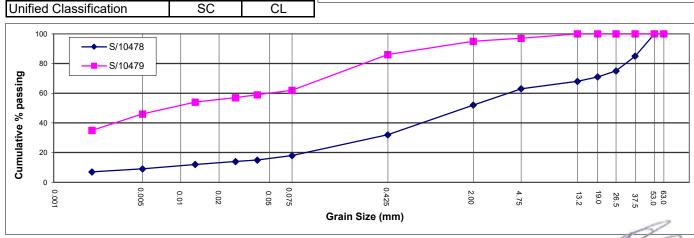
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

•			
Sample No.	S/10478	S/10479	
Report No.	SG23/1620		
Depth (mm)	2800-4900	0-700	
Position	WRD-TP14	WRD-TP15	
	Clayey sand with	Sandy lean clay	
Material description	gravel	Garidy Ican clay	
Moisture (%)			
SG	2.616	2.619	
Sieve Analysis	(% passing)	
63.0 mm			
53.0 mm	100		
37.5 mm	85		
26.5 mm	75		
19.0 mm	71		
13.2 mm	68	100	
4.75 mm	63	97	
2.00 mm	52	95	
0.425 mm	32	86	
0.075 mm	18	62	
Hydrometer Analy	sis (% passi	ng)	
0.040 mm	15	59	
0.027 mm	14	57	
0.013 mm	12	54	
0.005 mm	9	46	
0.002 mm	7	35	
% Clay (<0.002 mm)	7	35	
% Silt (<0.075, >0.002 mm)	11	27	
% Sand (<4.75, >0.075 mm)	45	35	
% Gravel (> 4.75 mm)	37	3	
Atterberg	Limits		
Liquid Limit (-0.425)	29	33	
Plasticity Index (-0.425)	11	16	
Linear Shrinkage (%)	4.0	7.0	
PI of whole sample	4	14	
Grading Modulus	1.98	0.57	
TRB Classification	A-2-6 (0)	A-6 (7)	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

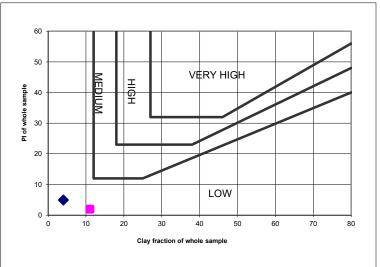
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

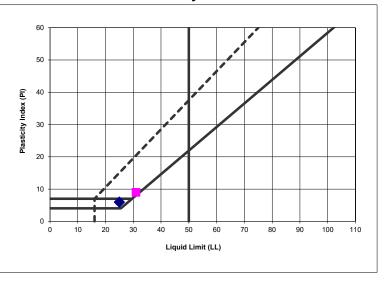
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10488	S/10494
Report No.		3/1620
Depth (mm)	400-1500	1200-2600
Position	WRD-TP04	WRD-TP26
Position	WKB II 04	
Material description	Silty, clayey sand	Clayey gravel with sand
Moisture (%)		
SG	2.502	2.588
Sieve Analysis	(% passing)
63.0 mm		82
53.0 mm		76
37.5 mm		71
26.5 mm		63
19.0 mm		60
13.2 mm	100	53
4.75 mm	98	42
2.00 mm	92	34
0.425 mm	76	22
0.075 mm	28	20
Hydrometer Analy	sis (% passi	ng)
0.040 mm	15	19
0.027 mm	12	18
0.013 mm	8	17
0.005 mm	5	13
0.002 mm	4	11
% Clay (<0.002 mm)	4	11
% Silt (<0.075, >0.002 mm)	24	9
% Sand (<4.75, >0.075 mm)	70	22
% Gravel (> 4.75 mm)	2	58
Atterberg	g Limits	
Liquid Limit (-0.425)	25	31
Plasticity Index (-0.425)	6	9
Linear Shrinkage (%)	2.5	3.0
PI of whole sample	5	2
Grading Modulus	1.04	2.24
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	SC-SM	GC

Potential Expansiveness











SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no : 10795.0 Project: Omitiomire Sampling date: 23-Jun-23

Section: Waste Rock Dump Stabilizing agent : From chainage: To chainage: Layer:

Report no: SG23/1620 Page no: 6

	Report no: SG23/1620					Page no :	6
	Sample no.	S/10496	S/10498	S/10500	S/10502	S/10504	S/10507
	Chainage						
l _	Position (from LHS)	WRD-TP35	WRD-TP39	WRD-TP37	WRD-TP32	WRD-TP16	WRD-TP37
stail	Depth sampled (mm)	2500-4100	1400-1500	2400-2700	1150-1450	500-1800	900-1100
sample detail	Sample description	Yellow brown, sandy silt. Ferruginous Alluvium.	Khaki stained brown to black, silty gravelly sand to silty sandy gravel, ferruginised alluvium	Completely to highly weathered, white speckled black stained orange, gneiss	Pale brown blotched white, sandy silt with abundant calcrete nodules and cobbles. Honeycomb Calcrete.	Pale brown speckled white, sandly silty gravel. Nodular Calcrete.	Yellowish brown mottled white and black, silty sandy gravel, calcified pebble marker
	Method of Preparation used	Scalping	Scalping	Scalping	Scalping	Scalping	Scalping
	Testing depth (mm)						
īţ	Dry density (kg/m³)						
ensi	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fiel	O.M.C. (%)						
	Compaction (%)						
	63,0 mm				88	88	
βι	53,0 mm			100	83	73	
sieve analysis (% passing)	37,5 mm			84	76	64	
pas	26,5 mm			79	62	47	100
%)	19,0 mm		100	76	56	36	99
sis	13,2 mm	100	99	71	51	33	98
aly	4,75 mm	97	78	61	44	26	90
au	2,00 mm	92	65	52	39	23	81
eve	0,425 mm	75	39	31	29	20	63
S	0,075 mm	41	17	12	17	10	42
	Grading Modulus (GM)		1.79	2.05	2.15	2.47	1.14
Atterberg Limits	Liquid Limit (LL)		36	23	24	30	37
tterber Limits	Plasticity Index (PI)		10	8	8	11	13
At	Linear Shrinkage (LS)	6.0	3.5	4.0	3.0	4.0	6.5
~	CBR @ 100 %			27		27	
CBR	CBR @ 98 %	3		21		24	
	CBR @ 95 %			14		19	
ASHTO values	CBR @ 93 %			11		17	
AS	CBR @ 90 %			7		14	
Mod AASHTO 8 values	Swell @ 100 %		-	0.1		0.2	
Ĭ	Mod AASHTO (kg/m³)	1946	Insufficient	2010		1869	
	O.M.C. (%)		material	10.8		14.5	
ility 2004(E	% Compaction @ Moisture		Insufficient material				
eab 92-11:	Permeabilty cm/s						
Permeability ISOTS 17892-11:2004(E)	% Compaction @ Moisture						
- ISC	Permeabilty cm/s						-

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS of its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

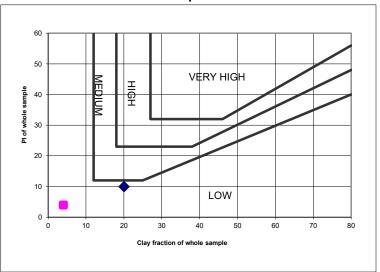
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

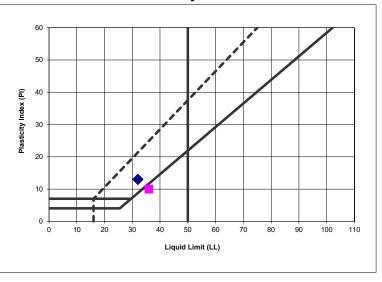
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

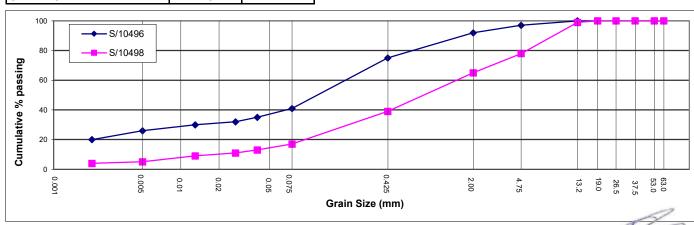
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10496	S/10498	
Report No.	SG23/1620		
Depth (mm)	2500-4100	1400-1500	
Position	WRD-TP35	WRD-TP39	
Material description	Clayey sand	Silty sand with gravel	
Moisture (%)			
SG	2.647	2.596	
Sieve Analysis	(% passing)	
63.0 mm	Ì		
53.0 mm			
37.5 mm			
26.5 mm			
19.0 mm		100	
13.2 mm	100	99	
4.75 mm	97	78	
2.00 mm	92	65	
0.425 mm	75	39	
0.075 mm	41	17	
Hydrometer Analys	sis (% passi	ng)	
0.040 mm	35	13	
0.027 mm	32	11	
0.013 mm	30	9	
0.005 mm	26	5	
0.002 mm	20	4	
% Clay (<0.002 mm)	20	4	
% Silt (<0.075, >0.002 mm)	21	13	
% Sand (<4.75, >0.075 mm)	56	61	
% Gravel (> 4.75 mm)	3	22	
Atterberg	Limits		
Liquid Limit (-0.425)	32	36	
Plasticity Index (-0.425)	13	10	
Linear Shrinkage (%)	6.0	3.5	
PI of whole sample	10	4	
Grading Modulus	0.92	1.79	
TRB Classification	A-6 (2)	A-2-4 (0)	
Unified Classification	SC	SM	

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

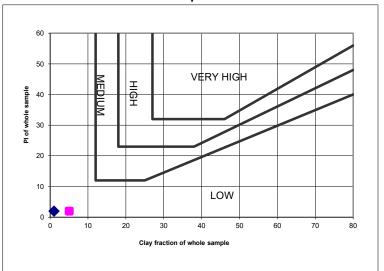
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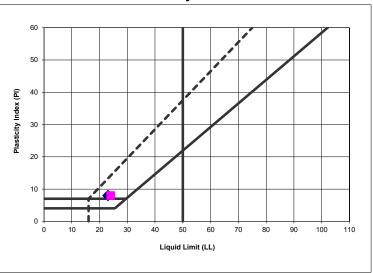
 Client
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 Project No.
 10795

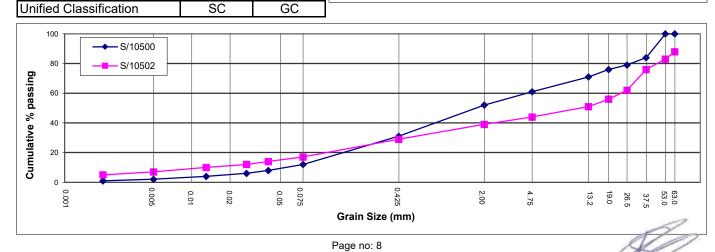
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10500	S/10502
Report No.	SG23	/1620
Depth (mm)	2400-2700	1150-1450
Position	WRD-TP37	WRD-TP32
Material description	Clayey sand with gravel	Clayey gravel with sand
Moisture (%)		
SG	2.655	2.602
Sieve Analysi	s (% passing	•
63.0 mm		88
53.0 mm	100	83
37.5 mm	84	76
26.5 mm	79	62
19.0 mm	76	56
13.2 mm	71	51
4.75 mm	61	44
2.00 mm	52	39
0.425 mm	31	29
0.075 mm	12	17
Hydrometer Anal	lysis (% passi	ing)
0.040 mm	8	14
0.027 mm	6	12
0.013 mm	4	10
0.005 mm	2	7
0.002 mm	1	5
% Clay (<0.002 mm)	1	5
% Silt (<0.075, >0.002 mm)	11	12
% Sand (<4.75, >0.075 mm)	49	27
% Gravel (> 4.75 mm)	39	56
	rg Limits	
Liquid Limit (-0.425)	23	24
Plasticity Index (-0.425)	8	8
Linear Shrinkage (%)	4.0	3.0
PI of whole sample	2	2
Grading Modulus	2.05	2.15
TRB Classification	A-2-4 (0)	A-2-4 (0)
Unified Classification	90	CC

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

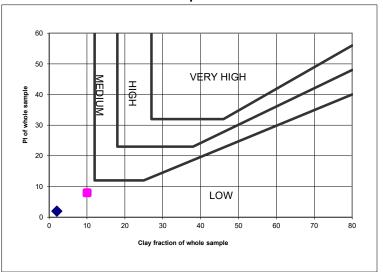
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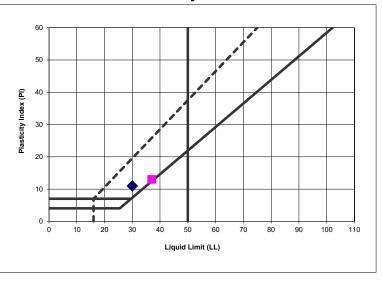
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

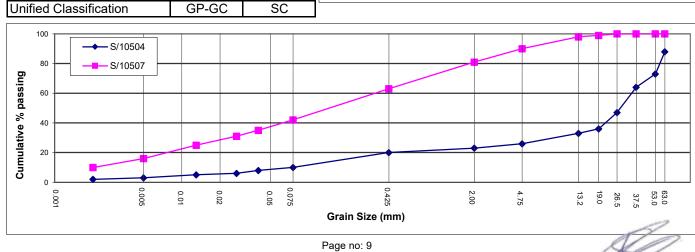
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10504	S/10507
Report No.	SG23	
Depth (mm)	500-1800	900-1100
Position	WRD-TP16	WRD-TP37
Material description	Poorly graded gravel with clay and sand	Clayey sand
Moisture (%)		
SG	2.614	2.518
Sieve Analysis	s (% passing)	
63.0 mm	88	
53.0 mm	73	
37.5 mm	64	
26.5 mm	47	100
19.0 mm	36	99
13.2 mm	33	98
4.75 mm	26	90
2.00 mm	23	81
0.425 mm	20	63
0.075 mm	10	42
Hydrometer Anal	ysis (% passi	ng)
0.040 mm	8	35
0.027 mm	6	31
0.013 mm	5	25
0.005 mm	3	16
0.002 mm	2	10
% Clay (<0.002 mm)	2	10
% Silt (<0.075, >0.002 mm)	8	32
% Sand (<4.75, >0.075 mm)	16	48
% Gravel (> 4.75 mm)	74	10
Atterber	g Limits	
Liquid Limit (-0.425)	30	37
Plasticity Index (-0.425)	11	13
Linear Shrinkage (%)	4.0	6.5
PI of whole sample	2	8
Grading Modulus	2.47	1.14
TRB Classification	A-2-6 (0)	A-4 (2)
Unified Classification	GP-GC	SC

Potential Expansiveness











SUMMARY OF TEST RESULTS - EARTHWORKS

Testing performed in accordance with the applicable methods under SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795.0

Project : Omitiomire Sampling date : 23-Jun-23

Section : Waste Rock Dump

Layer : From chainage : To chainage :

	Report no : 5G23/1620					Page no :	10
	Sample no.	S/10509	S/10510	S/10511	S/10512		
	Chainage						
	Position (from LHS)	WRD-TP02	WRD-TP33	WRD-TP37	WRD-TP23		
tail	Depth sampled (mm)	1400-1600	1200-1300	1100-2400	850-1500		
sample detail	Sample description	White to grey mottled black stained orange, silty gravelly sand, calcified reworked residual gneiss	Dark brown mottled orange and black, clayey silty coarse sand, slightly ferruginised reworked residual biotite gneiss	Yellowish brown mottled black and white, gravelly silty sand, reworked residual gneiss	Pale brown blotched white, hardpan calcrete.		
	Method of Preparation	Scalping	Scalping	Soolning	Saalning		
	used	Scalping	Scalping	Scalping	Scalping		
	Testing depth (mm)						
sity	Dry density (kg/m ³)						
den	Moisture (%)						
field density	Mod AASHTO (kg/m ³)						
fie	O.M.C. (%)						
	Compaction (%)						
	63,0 mm				75		
ng)	53,0 mm	100			62		
ssi	37,5 mm	91			58		
pa	26,5 mm	80			58		
sieve analysis (% passing)	19,0 mm	71		100	58		
Sis	13,2 mm	66	100	100	51		
aly	4,75 mm	42	100	92	37		
an	2,00 mm	28	96	86	31		
eve	0,425 mm	18	70	59	20		
<u>s</u>	0,075 mm	5	34	24	8		
	Grading Modulus (GM)	2.49	1.00	1.31	2.41		
erg :s	Liquid Limit (LL)		22	24	35		
Atterberg Limits	Plasticity Index (PI)	1	8	6	9		
Att	Linear Shrinkage (LS)		3.0	3.0	4.0		
	CBR @ 100 %				64		
& CBR	CBR @ 98 %				54		
۵ د	CBR @ 95 %				42		
ASHTO values	CBR @ 93 %				36		
SH	CBR @ 90 %				28		
₹	Swell @ 100 %				0.0		
Mod AASHTO values	Mod AASHTO (kg/m ³)				1651		
_	O.M.C. (%)				19.9		
y 4(E)	% Compaction @ Moisture						
bilit	Permeabilty cm/s						
nea '892-1	-						
Permeability ISOTS 17892-11:2004(E)	% Compaction @ Moisture						
Rema	Permeabilty cm/s						

Remarks:

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correcty. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

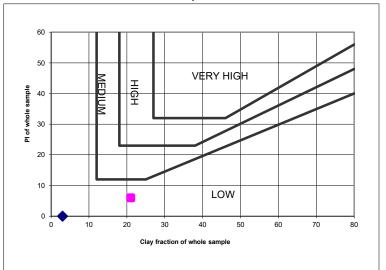
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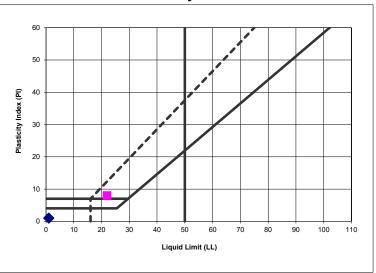
 Client
 Knight Piesold (Pty) Ltd (Namibia)
 Project No.
 10795

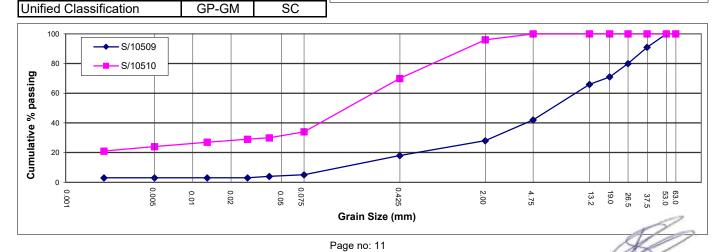
 Project
 Omitiomire
 Sampling Date
 23-Jun-23

Sample No.	S/10509	S/10510
Report No.	SG23	
·		
Depth (mm)	1400-1600	1200-1300
Position	WRD-TP02	WRD-TP33
Material description	Poorly graded gravel with silt and sand	Clayey sand
Moisture (%)		
SG	2.635	2.634
Sieve Analysi	s (% passing)
63.0 mm	Ì	
53.0 mm	100	
37.5 mm	91	
26.5 mm	80	
19.0 mm	71	
13.2 mm	66	100
4.75 mm	42	100
2.00 mm	28	96
0.425 mm	18	70
0.075 mm	5	34
Hydrometer Anal	lysis (% passi	ng)
0.040 mm	4	30
0.027 mm	3	29
0.013 mm	3	27
0.005 mm	3	24
0.002 mm	3	21
% Clay (<0.002 mm)	3	21
% Silt (<0.075, >0.002 mm)	2	13
% Sand (<4.75, >0.075 mm)	37	66
% Gravel (> 4.75 mm)	58	0
Atterbe	rg Limits	
Liquid Limit (-0.425)		22
Plasticity Index (-0.425)	1	8
Linear Shrinkage (%)	0.0	3.0
PI of whole sample	0	6
Grading Modulus	2.49	1
TRB Classification	A-1-a (0)	A-2-4 (0)

Potential Expansiveness









SUMMARY OF INDICATOR / HYDROMETER TEST RESULTS

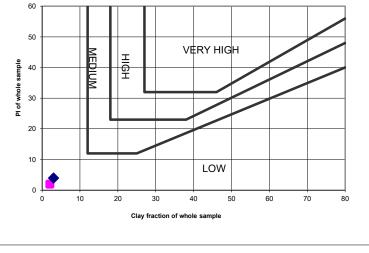
Tests done according to: TMH 1 Methods A1a + A1a + A2 + A3 + A5, ASTM D422 - 63 (2002), ASTM D2487 - 06

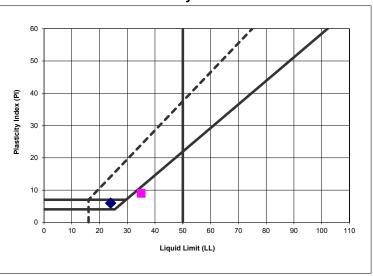
Client Knight Piesold (Pty) Ltd (Namibia) Project No. Project Omitiomire Sampling Date 23-Jun-23

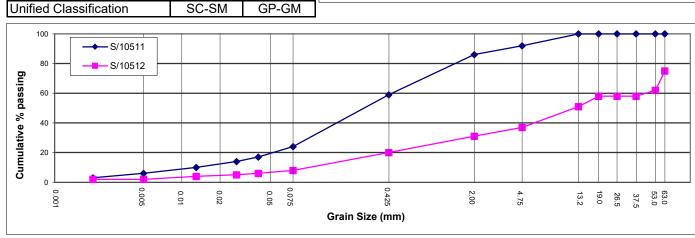
Sample No.	S/10511	S/10512			
Report No.	SG23	/1620			
Depth (mm)	1100-2400	850-1500			
Position	WRD-TP37	WRD-TP23			
Material description	Silty, clayey sand	Poorly graded gravel with silt and sand			
Moisture (%)					
SG	2.648	2.543			
Sieve Analysis	(% passing)			
63.0 mm	Ì	, 75			
53.0 mm		62			
37.5 mm		58			
26.5 mm		58			
19.0 mm	100	58			
13.2 mm	100	51			
4.75 mm	92	37			
2.00 mm	86	31			
0.425 mm	59	20			
0.075 mm	24	8			
Hydrometer Analys	sis (% passi	ng)			
0.040 mm	17	6			
0.027 mm	14	5			
0.013 mm	10	4			
0.005 mm	6	2			
0.002 mm	3	2			
% Clay (<0.002 mm)	3	2			
% Silt (<0.075, >0.002 mm)	21	6			
% Sand (<4.75, >0.075 mm)	68	29			
% Gravel (> 4.75 mm)	8	63			
Atterberg Limits					
Liquid Limit (-0.425)	24	35			
Plasticity Index (-0.425)	6	9			
Linear Shrinkage (%)	3.0	4.0			
PI of whole sample	4	2			
Grading Modulus	1.31	2.41			
TRB Classification	A-2-4 (0)	A-2-4 (0)			
Hariffer of Olean Street and	00 014	0001			

VERY HIGH

Potential Expansiveness









SUMMARY OF AGGREGATE TEST RESULTS

Tests done according to SANS 3001

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10795

Project: OmitiomireDate: 31-Aug-23Source: PLT-BH01: 2.80 - 9.00mReport no: AG23/336

Order no: Page no: 1

Order 110.				Paye 110.	I
	Sample no.	S/10595			
	Description	PLT-BH01 : 2.80 - 9.00m			
	75.0 mm				
	63.0 mm				
	53.0 mm				
	37.5 mm				
	26.5 mm				
	19.0 mm				
	13.2 mm				
Sieve Analysis	9.5 mm				
nal	6.7 mm				
e A	4.75 mm				
iev	2.36 mm				
0	1.18 mm				
	0.600 mm				
	0.425 mm				
	0.300 mm				
	0.150 mm				
	0.075 mm				
	Fineness Modulus				
	Density (kg/m³)				
	d Bulk Density (kg/m³)				
	e Density (kg/m³)				
Apparent Re	elative Density (kg/m³)				
Water absor					
Sand Equiva					
Organic Imp	urities				
Aggregate C	rushing Value (dry)	19.3			
Aggregate C	Crushing Value (wet)	36.2			
10% FACT (
10% FACT (
Flakiness Inc					
Riedel & We					
	ast Dimension				
	Abrasion (LAA)				
PI on LAA fir					
Mill Abrasion (MA)					
Deleterious Clay Content (MBT)					
Presence of Sugar					
Water Soluble Sulphates (% m/m)					
Water Soluble Salts (% m/m)					
	ntent (% m/m CI)				
Domarke:			 ·	 	

Remarks:

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Project no: 10795.0

Namibia Technical Services cc

SUMMARY OF TEST RESULTS BASSON INDEX & BRE

Client: Knight Piesold (Pty) Ltd (Namibia)

PO Box 86062 Report date: 29-Aug-23 Eros, Windhoek ALS Reference: I231538

Namibia Sample ID :

ANALYCIC	Unit	Sample Reference / description			on	
ANALYSIS	Unit	S/10410	S/10418			
Sample position		HLP-TP05	HLP-TP13			
Depth from to (m)		0.0-1.1	0.3-2.25			
рН	1 / log H	6.9	6.6			
Temp at which above pH performed	°C	25.1	24.6			
pH Value at 25 °C	1 / log H	6.90	6.60			
Calcium carbonate-saturated pH (Calc) at 20 °C	1 / log H	10.90	10.70			
Electrical Conductivity at 25 °C	mS/m	1.6	2			
Total dissolved solids * (Calc)	mg/l	11	13			
Total Alkalinity as CaCO ₄	mg/l	5	5			
Total Hardness as CaCO ₃ (Calc)	mg/l	4	5			
Calcium Hardness as CaCO ₃ (Calc)	mg/l	2	3			
Calcium as Ca	mg/l	1	1.1			
Magnesium Hardness CaCO3 (Calc)	mg/l	2	2			
Magnesium ion (as Mg)	mg/l	0.6	0.6			
Total ammonium ion (as NH ₄)	mg/l	0.03	0.11			
Total Sulphate ion (as SO ₄)	mg/l	6	49			
Chloride ion (as Cl) (see note 1 below)	mg/l	5	5			
Total water soluble salts	mg/l	140	290			
Langelier Index at 20 °C (Calc)		-4.0	-4.2			
Ryznar Index at 20 °C (Calc)		15.0	15.0			
Corrosivity Ratio (Calc)		2.7	11.7			
Indices for standard local conditi	Indices for standard local conditions (ie under conditions of laminar flow at 20°C)					
Leaching corrosion sub-index	LSCI	3201	3305			
Spalling corrosion sub-index	SCSI	1	5			
Aggressiveness index	Nc	3202	3310			

* TDS Calculated EC x 6.7

2:1 Distilled Water: Soil Extract

Important notes:

- 1. The above aggressiveness index is only applicable for conditions of laminar flow at a mean annual temperature of 20 °C.
- 2. For stagnant / turbulent conditions the aggressiveness index must be corrected.
- 3. For wet / dry cycling conditions (for example tidal zones) the aggressiveness index must be corrected.
- 4. For mean annual temperatures lower / higher than 20 °C the aggressiveness index must be corrected.

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Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, or any erroneous statement or opinion. If a test report is published or reproduced by the client, it will be done in full, without any omittance.



Project no: 10799.0

Namibia Technical Services cc

SUMMARY OF TEST RESULTS BASSON INDEX & BRE

Client: Knight Piesold (Pty) Ltd (Namibia)

PO Box 86062 Report date: 29-Aug-23 Eros, Windhoek ALS Reference: 1231538

Namibia Sample ID:

Report no: 23BI/09 Page no: 2 of 2

To correct for:	Multiply	By: (see Notes 2 to 5 below)
Turbulence	LSCI	1.75
Stagnance	LSCI	0.5
Temperature	LSCI,SCSI,N7	$(1+(0.05 \times (T-20)))$ where T = mean annual temperature
Wet-dry cycles	SCSI	$0.23 \times 10^{-6} \times Cl^{-} \times DTF \times CPA$ where DTF = dry time fraction and
		CPA = wet-dry cycles per annum

Note 1: Only if the concrete contains embedded steel

Note 2: To preserve the correct logical relationships when dealing with negative sub-indices (ie LSCI or SCSI

having minus values) they should be multiplied by the reciprocal of the relevant factor indicated in

this column.

Note 3: If more than one correction is required, multiply by the product of the individual correction factors

Note 4: Use subscript c to indicate that the index has been corrected, eg for turbulent conditions LSCIc = LSCI

x 1.75

Note 5: Round off corrected indices to the nearest 100

GUIDELINES FOR ASSESSING OVERALL AGGRESSIVENSS (Nc):

Nc	Aggressiveness	
Not greater than 300	None to mild	
400 - 700	Mild to moderate	
800 - 1000	High	
= or > 1100	Very High	

Aggressiveness Towards Concrete and Fibre Cement Pipes

Index	Aggressive	Neutral	Non-Aggressive
a) Stability pH (pHs)	> pH	= pH	< pH
b) Langelier Index	Neg. Value	Zero	Pos. Value
c) Ryznar Index	> 7.5	6 - 7	< 6

Corrosiveness Towards Metals				
Corrosivity Ratio > 0.2				

Sample Reference / description	Corrosivity Indices	Basson Index
S/10410	Corrosive	Very High
S/10418	Corrosive	Very High

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Project no: 10795.0

Namibia Technical Services cc

SUMMARY OF TEST RESULTS BASSON INDEX & BRE

Client: Knight Piesold (Pty) Ltd (Namibia)

PO Box 86062 Report date: 29-Aug-23 Eros, Windhoek ALS Reference: 1231538

Namibia Sample ID :

ANALVSIS	Unit	Sample Reference / description			on	
ANALYSIS	Unit	S/10468	S/10470			
Sample position		SWD-TP19	SWD-TP07			
Depth from to (m)		1.4-2.4	1.4-1.9			
рН	1 / log H	7.8	8.1			
Temp at which above pH performed	°C	25.1	25.2			
pH Value at 25 °C	1 / log H	7.80	8.10			
Calcium carbonate-saturated pH (Calc) at 20 °C	1 / log H	8.70	9.00			
Electrical Conductivity at 25 °C	mS/m	12.1	13			
Total dissolved solids * (Calc)	mg/l	81	87			
Total Alkalinity as CaCO ₄	mg/l	55	57.5			
Total Hardness as CaCO ₃ (Calc)	mg/l	63	29			
Calcium Hardness as CaCO ₃ (Calc)	mg/l	35	18			
Calcium as Ca	mg/l	13.9	7.1			
Magnesium Hardness CaCO3 (Calc)	mg/l	28	11			
Magnesium ion (as Mg)	mg/l	6.9	2.7			
Total ammonium ion (as NH ₄)	mg/l	0.15	0.01			
Total Sulphate ion (as SO ₄)	mg/l	47	12			
Chloride ion (as Cl) (see note 1 below)	mg/l	5	12			
Total water soluble salts	mg/l	240	170			
Langelier Index at 20 °C (Calc)		-0.9	-0.8			
Ryznar Index at 20 °C (Calc)		9.7	9.9			
Corrosivity Ratio (Calc)		1.0	0.5			
Indices for standard local conditi	Indices for standard local conditions (ie under conditions of laminar flow at 20°C)					
Leaching corrosion sub-index	LSCI	1040	1068			
Spalling corrosion sub-index	SCSI	6	3			
Aggressiveness index	Nc	1046	1071			

* TDS Calculated EC x 6.7

2:1 Distilled Water: Soil Extract

Important notes:

- 1. The above aggressiveness index is only applicable for conditions of laminar flow at a mean annual temperature of 20 °C.
- 2. For stagnant / turbulent conditions the aggressiveness index must be corrected.
- 3. For wet / dry cycling conditions (for example tidal zones) the aggressiveness index must be corrected.
- 4. For mean annual temperatures lower / higher than 20 $^{\rm o}{\rm C}$ the aggressiveness index must be corrected.

8

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SUMMARY OF TEST RESULTS BASSON INDEX & BRE

Client: Knight Piesold (Pty) Ltd (Namibia)

Project no: 10799.0 PO Box 86062 Report date: 29-Aug-23 Eros, Windhoek ALS Reference: 1231538

Namibia Sample ID:

Report no: 23BI/09 **Page no**: 2 of 2

To correct for:	Multiply	By: (see Notes 2 to 5 below)
Turbulence	LSCI	1.75
Stagnance	LSCI	0.5
Temperature	LSCI,SCSI,N7	$(1+(0.05 \times (T-20)))$ where T = mean annual temperature
Wet-dry cycles	SCSI	$0.23 \times 10^{-6} \times Cl^{-} \times DTF \times CPA$ where DTF = dry time fraction and
		CPA = wet-dry cycles per annum

Only if the concrete contains embedded steel Note 1:

Note 2: To preserve the correct logical relationships when dealing with negative sub-indices (ie LSCI or SCSI

having minus values) they should be multiplied by the reciprocal of the relevant factor indicated in

this column.

Note 3: If more than one correction is required, multiply by the product of the individual correction factors

Note 4: Use subscript c to indicate that the index has been corrected, eg for turbulent conditions LSCIc = LSCI

x 1.75

Round off corrected indices to the nearest 100 Note 5:

GUIDELINES FOR ASSESSING OVERALL AGGRESSIVENSS (Nc):

Nc	Aggressiveness	
Not greater than 300	None to mild	
400 - 700	Mild to moderate	
800 - 1000	High	
= or > 1100	Very High	

Aggressiveness Towards Concrete and Fibre Cement Pipes

66			
Index	Aggressive	Neutral	Non-Aggressive
a) Stability pH (pHs)	> pH	= pH	< pH
b) Langelier Index	Neg. Value	Zero	Pos. Value
c) Ryznar Index	> 7.5	6 - 7	< 6

Corrosiveness Towards Metals				
Corrosivity Ratio	> 0.2			

Sample Reference / description	Corrosivity Indices	Basson Index		
S/10468	Corrosive	High to Very High		
S/10470	Corrosive	High to Very High		

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			4P res	sistance (R)			
	Electrode spacing (m)	3	6	12	24	36	54
	Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
	N-S	4.27	2.34	1.01	0.56	0.44	0.36
	E-W	4.30	2.29	1.02	0.58	0.44	0.37
	v	Venner Resistiv	vity (p) calcul	ated from ab	ove resitance v	alues	
	Electrode spacing (m)	3	6	12	24	36	54
1	Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	80	88	76	84	100	122
E-W	81	86	77	87	100	126

N-S	80	88	76	84	100	122
E-W	81	86	77	87	100	126
v	Venner Resistiv	vity (p) as tes	ted with mac	hine (Direct rea	ading)	
Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivi Ω.m
N-S	80	88	76	84	100	122
E-W	81	88	76	83	100	126

3	6	12	24	36	54
Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
4.13	1.76	0.83	0.61	0.43	0.31
4.21	1.89	0.90	0.62	0.40	0.29
	Ω 4.13	Resistance Ω Resistance Ω 4.13 1.76	Resistance Ω Resistance Ω Ω Resistance Ω Ω	Resistance Ω Resistance Ω Resistance Ω 4.131.760.830.61	Resistance Ω Resistance Ω Resistance Ω Resistance Ω 4.131.760.830.610.43

Wenner Resistivity (p) calculated from above resitance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	78	66	63	92	97	105
E-W	79	71	68	93	90	98

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	78	66	62	92	98	107
E-W	77	71	68	92	90	98

			10 1011110 (11)			
Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.71	1.09	0.75	0.37	0.45	0.71
E-W	1.72	1.10	0.75	0.38	0.46	0.56

Wenner Resistivity (p) calculated from above resitance values

Electrode spacing (m)	3	O	12	24	30	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	32	41	57	56	102	241
E-W	32	41	57	57	104	190

3

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	32	41	56	56	101	241
E-W	33	43	56	56	101	190

			47 165	istance (R)			
	Electrode spacing (m)	3	6	12	24	36	54
	Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
	N-S	1.44	0.71	0.44	0.44	0.38	0.38
	E-W	1.32	0.72	0.53	0.33	0.36	0.33
					ove resitance v		
	Electrode spacing (m)	3	6	12	24	36	54
4	Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
	N-S	27	27	33	66	86	129
	E-W	25	27	40	50	81	112
		/enner Resistiv	vity (p) as tes	ted with macl	nine (Direct rea	ading)	
	Electrode spacing (m)	3	6	12	24	36	54
	Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m

N-S E-W

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.30	0.75	0.47	0.33	0.32	0.44
E-W	1.26	0.73	0.44	0.26	0.28	0.36

Wenner Resistivity (p) calculated from above resitance values

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	25	28	35	50	72	149
E-W	24	28	33	39	63	122

5

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	24	28	35	49	70	149
E-W	24	28	33	39	63	122

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.56	1.06	0.69	0.56	0.62	0.52
E-W	1.45	1.01	0.66	0.50	0.56	0.47

Wenner Resistivity (p) calculated from above resitance values

Electrode spacing (m)	3	б	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	29	40	52	84	140	176
E-W	27	38	50	75	127	159

6

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	29	40	52	84	140	176
E-W	27	38	50	74	126	159

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	1.45	1.02	0.63	0.45	0.54	0.47
E-W	1.45	1.01	0.65	0.47	0.50	0.45

Electrode spacing (m)	3	В	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	27	38	48	68	122	159
E-W	27	38	49	71	113	153

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	27	38	47	69	119	159
E-W	27	38	48	71	113	153

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω	Resistance Ω
N-S	4.52	2.12	0.96	0.68	0.47	0.65
E-W	4.46	2.05	0.88	0.66	0.47	0.62

Wenner Resistivity (p) calculated from above resitance values

Electrode spacing (m)	3	O	12	24	30	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	85	80	72	103	106	221
E-W	84	77	66	100	106	210

Wenner Resistivity (p) as tested with machine (Direct reading)

Electrode spacing (m)	3	6	12	24	36	54
Test Orientation	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m	Resistivity Ω.m
N-S	85	80	72	103	106	221
E-W	84	77	66	99	103	210

8



Point Load Index Testing ASTM D5731

Client: Knight Piesold (Pty) Ltd (Namibia) **Job No**: 10795

Drilling/delivery date: 28/Jun/23 Project: WI301-00478/06: OMITIOMIRE

Core Size

// = parrallel to planes of weekness;

HQ

Testing date: 28/Jul/23 Report no: PL / 14 Page no: 1 Sample Preparation Specimens cut with Diamond Table saw from Rock Core Samples

	Jumpic i repu		- р-с	is cut with Dia						COI C 312C		ΠQ	03	
Lab No	No.	Depth	Туре	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s (50)} (MPa)	Estimated UCS Value (MPa)	С
S/10576	SWD-BH01	2.39 - 2.51	а	р	28.7	60.8	1.10	2222.88	47.15	0.49	0.97	0.48	12.01	24.91
S/10576	SWD-BH01	2.39 - 2.51	d	р	27.9		1.70		60.80		1.09	0.50	+	24.91
3, 203, 0	3112 31101	2.03 2.31	<u> </u>	2	27.13	00.0	1.70	3030.01	00.00	0.10	1.05	0.50	12.51	2 113 1
S/10577	SWD-BH01	3.14 - 3.48	a	р	59.4	60.3	4.10	4562.83	67.55	0.90	1.14	1.03	25.53	24.82
S/10577	SWD-BH01	3.14 - 3.48	d	p	29.2	60.3	2.80	3636.09	60.30	0.77	1.09			24.82
				·										
S/10578	SWD-BH01	3.36 - 3.95	а	р	59.3	60.1	6.25	4540.04	67.38	1.38	1.14	1.57	39.02	24.78
S/10578	SWD-BH01	3.36 - 3.95	d	р	30.7	60.1	3.60	3612.01	60.10	1.00	1.09	1.08	26.83	24.78
S/10579	SWD-BH02	2.84 - 2.98	а	р	39.1	59.9	2.65	2983.55	54.62	0.89	1.04	0.92	22.87	24.74
S/10579	SWD-BH02	2.84 - 2.98	d	р	29.7	59.9	1.55	3588.01	59.90	0.43	1.08	0.47	11.59	24.74
S/10581	SWD-BH02	8.16 - 8.35	а	р	40.1	60.6	3.40	3095.62	55.64	1.10	1.05	1.15	28.67	24.87
S/10581	SWD-BH02	8.16 - 8.35	d	р	28	60.6	3.40	3672.36	60.60	0.93	1.09	1.01	25.11	24.87
S/10582	SWD-BH03	3.82 - 4.13	a	р	38.4	60.1	2.50	2939.92	54.22	0.85	1.04	0.88	21.86	24.78
S/10582	SWD-BH03	3.82 - 4.13	d	р	72.1	60.1	1.60	3612.01	60.10	0.44	1.09	0.48	11.92	24.78

p = perpendicular



Point Load Index Testing ASTM D5731

Client: Knight Piesold (Pty) Ltd (Namibia)

Job No: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no : PL / 14Page no : 2Testing date : 28/Jul/23Sample PreparationSpecimens cut with Diamond Table saw from Rock Core SamplesCore SizeHQ63

Lab No	No.	Depth	Туре	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s (50)} (MPa)	Estimated UCS Value (MPa)	C
S/10583	SWD-BH04	3.71 - 3.88	а	р	56.3	59.2	18.50	4245.81	65.16	4.36	1.13	4.91	120.83	24.62
S/10583	SWD-BH04	3.71 - 3.88	d	р	28.9	59.2	0.60	3504.64	59.20	0.17	1.08	0.18	4.55	24.62
S/10584	SWD-BH04	8.23 - 8.39	а	р	42.2	60.1	12.40	3230.85	56.84	3.84	1.06	4.07	100.76	24.78
S/10584	SWD-BH04	8.23 - 8.39	d	р	22.6	60.1	5.45	3612.01	60.10	1.51	1.09	1.64	40.62	24.78
S/10585	SWD-BH05	5.88 - 6.02	a	р	43.6	60.8	1.55	3376.92	58.11	0.46	1.07	0.49	12.23	24.91
S/10585	SWD-BH05	5.88 - 6.02	d	р	28.7	60.8	0.35	3696.64	60.80	0.09	1.09	0.10	2.58	24.91
S/10586	SWD-BH05	9.69 - 9.93	a	р	27.6	60.6	3.85	2130.65	46.16	1.81	0.96	1.74	43.36	24.87
S/10586	SWD-BH05	9.69 - 9.93	d	р	23.1	60.6	2.05	3672.36	60.60	0.56	1.09	0.61	15.14	24.87
S/10587	SWD-BH08	5.07 - 5.21	a	р	39.1	58.2	2.05	2898.88	53.84	0.71	1.03	0.73	17.86	24.43
S/10587	SWD-BH08	5.07 - 5.21	d	р	30.2	58.2	0.60	3387.24	58.20	0.18	1.07	0.19	4.63	24.43



// = parrallel to planes of weekness;



Point Load Index Testing ASTM D5731

Client: Knight Piesold (Pty) Ltd (Namibia) **Job No**: 10795

Drilling/delivery date: 28/Jun/23 Project: WI301-00478/06: OMITIOMIRE

Report no: PL / 14 Page no: 3 Testing date: 28/Jul/23

	Sample Prepa	ration	Specimer	ns cut with Dia	mond Table	saw from R	ock Core Sa	amples		Core Size		HQ	63	
Lab No	No.	Depth	Туре	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s (50)} (MPa)	Estimated UCS Value (MPa)	С
S/10588	HLP-BH01	6.18 - 6.31	а	р	27.2	60.9	0.85	2110.17	45.94	0.40	0.96	0.39	9.67	24.93
S/10588	HLP-BH01	6.18 - 6.31	d	р	22.8	60.9	0.50	3708.81	60.90	0.13	1.09	0.15	3.67	24.93
S/10589	HLP-BH01	8.35 - 8.60	a	р	38.3	60.3	1.75	2942.03	54.24	0.59	1.04	0.62	15.31	24.82
S/10589	HLP-BH01	8.35 - 8.60	а	р	46.5	60.3	1.60	3571.91	59.77	0.45	1.08	0.49	12.05	24.82
S/10590	HLP-BH02	5.35 - 5.65	а	р	35.9	60.6	0.95	2771.39	52.64	0.34	1.02	0.35	8.73	24.87
S/10590	HLP-BH02	5.35 - 5.65	d	р	29.8	60.6	0.65	3672.36	60.60	0.18	1.09	0.19	4.80	24.87
S/10591	HLP-BH03	2.27 - 2.41	a	р	26.1	60.1	0.40	1998.23	44.70	0.20	0.95	0.19	4.72	24.78
S/10591	HLP-BH03	2.27 - 2.41	d	р	22.1	60.1	0.35	3612.01	60.10	0.10	1.09	0.11	2.61	24.78
S/10592	HLP-BH03	3.73 - 3.93	a	р	51.3	60.3	1.45	3940.62	62.77	0.37	1.11	0.41	10.12	24.82
S/10592	HLP-BH03	3.73 - 3.93	d	р	31.9	60.3	1.00		60.30		1.09			24.82
S/10594	HLP-BH04	5.19 - 5.35	a	р	38.5	60.6	1.95	2972.10	54.52	0.66	1.04	0.68	16.97	24.87
S/10594	HLP-BH04	5.19 - 5.35	d	р	30.1	60.6	1.05				1.09			24.87





Sample Preparation

Point Load Index Testing ASTM D5731

Client: Knight Piesold (Pty) Ltd (Namibia) **Job No**: 10795

Drilling/delivery date: 28/Jun/23 Project: WI301-00478/06: OMITIOMIRE

Core Size

// = parrallel to planes of weekness;

HO

Testing date: 28/Jul/23 Report no: PL / 14 Page no: 4 Specimens cut with Diamond Table saw from Rock Core Samples

	Sumple Prepu	il attoll	эрссинсі	is cut with Dia	mona rabic	3aW IIOIII I	OCK COTE 3	ampics		Core Size		пц	03	
Lab No	No.	Depth	Туре	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s (50)} (MPa)	Estimated UCS Value (MPa)	C
S/10595	PLT-BH01	2.80 - 9.00	а	р	58.6	60.1	6.55	4486.45	66.98	1.46	1.14	1.67	41.27	24.78
S/10595	PLT-BH01	2.80 - 9.00	d	р	30.1	60.1	5.25	3612.01	60.10	1.45	1.09	1.58	39.13	24.78
S/10596	PLT-BH02	4.73 - 4.94	а	р	40.5	60.3	1.60	3111.02	55.78	0.51	1.05	0.54	13.41	24.82
S/10596	PLT-BH02	4.73 - 4.94	а	р	45.1	60.3	0.90	3464.37	58.86	0.26	1.08	0.28	6.94	24.82
S/10597	PLT-BH02	8.09 - 8.37	a	р	58.5	60.8	16.00	4530.96	67.31	3.53	1.14	4.04	100.56	24.91
S/10598	PLT-BH03	1.78 - 1.98	а	р	32.3	84.4	1.55	3472.76	58.93	0.45	1.08	0.48	14.06	29.26
S/10598	PLT-BH03	1.78 - 1.98	d	р	41.2	84.4	3.25	7123.36	84.40	0.46	1.27	0.58	16.89	29.26
S/10599	PLT-BH03	6.33 - 6.55	a	р	30.4	60.2	3.90	2331.31	48.28	1.67	0.98	1.65	40.84	24.80
S/10599	PLT-BH03	6.33 - 6.55	d	//	28.1	60.2	2.70	3624.04	60.20	0.75	1.09	0.81	20.09	24.80





Point Load Index Testing ASTM D5731

Client: Knight Piesold (Pty) Ltd (Namibia) **Job No**: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no: PL / 14 Page no: 5 Testing date: 28/Jul/23

	Sample Prepo	ration	Specimer	ns cut with Dia	mond Table	saw from R	ock Core Sa	amples		Core Size		HQ	63	
Lab No	No.	Depth	Туре	Orientation	W/L (mm)	D(mm)	P(kN)	D _e ² (mm ²)	D _e (mm)	I _s (MPa)	F	I _{s (50)} (MPa)	Estimated UCS Value (MPa)	С
S/10601	WRD-BH01	5.13 - 5.30	а	р	28.6	60.5	0.50	2204.20	46.95	0.23	0.97	0.22	5.48	24.86
S/10601	WRD-BH01	5.13 - 5.30	d	р	27.5	60.5	0.91	3660.25	60.50	0.25	1.09	0.27	6.73	24.86
S/10602	WRD-BH01	7.75 - 7.88	a	р	42.3	60.4	0.40	3254.68	57.05	0.12	1.06	0.13	3.24	24.84



Dr Sabine Verryn

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e: sabine.verryn@xrd.co.za

XRD Analytical and Consulting cc 75 Kafue Street, Lynnwood Glen, 0081, South Africa

CLIENT: STL

DATE: 28 August 2023

SAMPLES: 1 Sample (PO T 035)

ANALYSIS: Petrographic analysis & XRD

REPORT ON OMITIOMIRE - NTS-05

1. SERVICE REQUESTED

Petrographic description & XRD

2. **SAMPLE DESCRIPTION**

Rock fragment

3. TESTS CONDUCTED

1 x Thin Section, 1 x XRD

4. **DESCRIPTION& ANALYSIS**

A thin section was prepared from a rock fragment set in epoxy and the remainder of the sample was split and milled for XRD analysis.

NOTE:

Quantities of minerals shown in the petrographic description are largely based on amounts calculated from XRD analysis as this reflects a more representative composition.

In case the results do not correspond to the results of other analytical techniques, please let me know for further fine-tuning of XRD results.

XRD

The material was prepared for XRD analysis using a back-loading preparation method. It was analysed with a PANalytical Aeris diffractometer with a PIXcel detector and fixed slits with Fe-filtered Co-Kα radiation. The phases were identified using X'PertHighscore plus software.

The relative phase amounts (weight %) were estimated using the Rietveld method. Mineral names may not reflect the actual compositions of minerals identified, but rather the mineral group. Due to crystallite size and preferred orientation effects, results may not be as accurate as shown. Mineral quantities observed in the overall sample in the thin section are in agreement with those calculated from XRD.

XRD Results:

Project Name	Omitiomire
Project no	NTS-05

Client No	Depth (m)	Our sample No	Test(s)
PLT-BH01/S/10595	2.80 - 9.0	NTS-05-44	Petrography and ASR comment

	Quartz	Microcline	Plagioclase	Biotite
S/10595	36.0	28.8	33.5	1.7

PETROGRAPHIC DESCRIPTIONS

PLT-BH01/S/10595

Sample type: Granite

Macroscopic description: The sample is composed of a rock fragment of a light grey colour with a

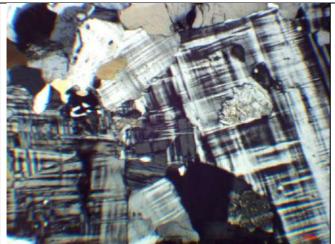
small to medium grain size.

Microscopic description: The sample investigated in the thin section overall comprises quartz crystals (30-40%), plagioclase (25-35%), microcline (25-35%), and

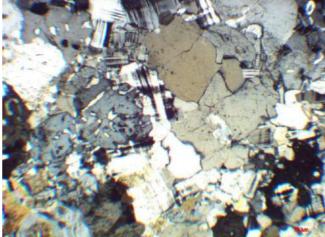
biotite (~2%).

The rock may represent granite where the largest portion is composed of roughly subhedral plagioclase (albite) with faint albite twinning. Microcline appears as anhedral crystals with cross-hatch twinning. Quartz occurs as anhedral crystals with a high degree of undulatory extinction. Isolated biotite flakes occur on grain boundaries and as small clusters.

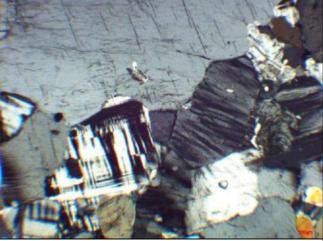
Grain size data: 100 – 1500 microns



Photomicrograph: Cross-polarized light. 40x magnification
A large cluster of microcline

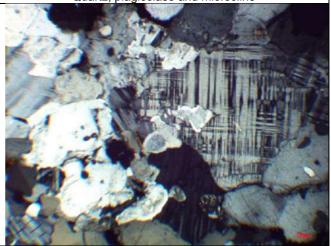


Photomicrograph: Cross-polarized light. 40x magnification. Quartz, plagioclase and microcline



Photomicrograph: Cross-polarized light. 40x magnification.

Quartz, feldspar, and minor biotite



Photomicrograph: Cross-polarized light. 40x magnification.

Quartz, plagioclase and microcline

5. COMMENTS

Rock types are based on mineralogy and texture rather than stratigraphic position.

Depending on the intended use of the material the following comments can be made:

- Due to the presence of highly strained quartz and quartz with interlobate grain boundaries in the sample, a reaction with alkalis in cement can occur unless otherwise shown by additional tests.
- The mica content consists of about 2% biotite.
- The sample may represent granite.
- It must be pointed out that the composition of rocks can vary over short distances in the field.

Analyst: Wiebke Grote

Authorized: Dr. Sabine Verryn (Pr.Sci.Nat)



UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client: Knight Piesold (Pty) Ltd (Namibia)

Job No: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no: RC / 14 Page no: 1 Testing date: 28/Jul/23

Report no : 1007 14		i ago no .	•		rooting dato .	20/041/20
Testing detail		Core no.	Core no.	Core no.	Core no.	Core no.
l esting detail		C / 10575	C / 10577	C / 10580	C / 10582	C / 10586
Rock type		Highly to completely weathered, dark brown with red iron oxide staining, very soft rock, granitic gneiss	Moderately to slightly weathered, light brown speckled black and grey mottled white, medium hard rock, gneiss	Moderately weathered, grey to brown streaked yellow and white, soft rock, gneiss	Moderately weathered, grey speckled black and white, soft to medium hard rock, gneiss	Highly to moderately weathered, grey to brown speckled black streaked white, medium hard rock, gneiss
Core marking		SWD-BH01	SWD-BH01	SWD-BH02	SWD-BH03	SWD-BH05
Core depth		2.04 - 2.26	3.14 - 3.48	7.96 - 8.16	3.82 - 4.13	9.69 - 9.93
Trimming method		cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade	cutting table with diamond blade
Method of end preparation		grinding	grinding	grinding	grinding	grinding
Core length trimmed	(mm)	135	135	126	139	124
Core length capped / ground	(mm)	132	132	121	135	120
Core diameter	(mm)	59	60	61	60	60
Core length / diameter ratio	(2.0)	2.29	2.25	2.08	2.31	2.05
Failing load	(kN)	22	66	96	34	73
Unconfined Compressive Strength	(MPa)	8.0	23.5	33.5	12.0	25.5
Notes on failure		normal	normal	normal	normal	normal

Everything possible is being done to ensure that tests are representative and are performed accurately, and that reports and conclusions are quoted correctly. NTS or its officials can in no way be held liable for consequential damage or loss due to any error made in carrying out the tests, nor for any erroneous statement or opinion contained in a report based on such tests. If a test report is published or reproduced by the client, it will be done in full, without any omittance.





Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client: Knight Piesold (Pty) Ltd (Namibia)

Job No: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no: RC / 14 Page no: 2 Testing date: 28/Jul/23

			_	10009 0		
Testing detail		Core no. C / 10590	Core no. C / 10593			
Rock type		Highly to completely weathered, brown to grey, banded white, very soft rock gneiss	Highly weathered, brown to grey speckled black streaked white, soft rock, gneiss			
Core marking		HLP-BH02	HLP-BH04			
Core depth		5.35 - 5.65	2.66 - 2.86			
Trimming method		cutting table with diamond blade	cutting table with diamond blade			
Method of end preparation		grinding	grinding			
Core length trimmed	(mm)	128	132			
Core length capped / ground	(mm)	122	128			
Core diameter	(mm)	60	61			
Core length / diameter ratio	(2.0)	2.13	2.17			
Failing load	(kN)	22	21			
Unconfined Compressive Strength	(MPa)	8.0	7.0			
Notes on failure		normal	normal			
e a: 21:1: 1 (a ()			() () (NTO	

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Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client: Knight Piesold (Pty) Ltd (Namibia)

Job No: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no: RC / 14 Page no: 3 Testing date: 28/Jul/23

1		•	_	•	
Testing detail		Core no.	Core no.		
resung detan		C / 10597	C / 10599		
Rock type		Moderately weathered, light brown to grey speckled black and streaked white, medium hard rock, granitic gneiss	Moderately weathered, dark grey to green speckled black streaked whit, medium hard rock, gneiss		
Core marking		PLT-BH02	PLT-BH03		
Core depth		8.09 - 8.37	6.33 - 6.55		
Trimming method		cutting table with diamond blade	cutting table with diamond blade		
Method of end preparation		grinding	grinding		
Core length trimmed	(mm)	126	90		
Core length capped / ground	(mm)	120	84		
Core diameter	(mm)	60	61		
Core length / diameter ratio	(2.0)	2.10	1.48		
Failing load	(kN)	119	30		
Unconfined Compressive Strength	(MPa)	42.0	10.5		
Notes on failure		normal	normal	 	

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Namibia Technical Services cc

UNCONFINED COMPRESSIVE STRENGTH OF ROCK CORES

Client: Knight Piesold (Pty) Ltd (Namibia)

Job No: 10795

Project: WI301-00478/06: OMITIOMIRE Drilling/delivery date: 28/Jun/23

Report no: RC / 14 Page no: 4 Testing date: 28/Jul/23

			•			
Testing detail		Core no. C / 10600				
Rock type		Pale brown blotched white, hardpan calcrete of soft rock strength				
Core marking		WRD-BH01				
Core depth		1.12 - 1.35				
Trimming method		cutting table with diamond blade				
Method of end preparation		grinding				
Core length trimmed	(mm)	173				
Core length capped / ground	(mm)	171				
Core diameter	(mm)	85				
Core length / diameter ratio	(2.0)	2.03				
Failing load	(kN)	29				
Unconfined Compressive Strength	(MPa)	5.0				
Notes on failure		normal				
- " " " " " " " " " " " " " " " " " " "						

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Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: HLP-TP26 Depth: (m) 0.0 - 1.1

Job Number: NTS-05 Lab Number: NTS-05-37 Date: 08/11/2023

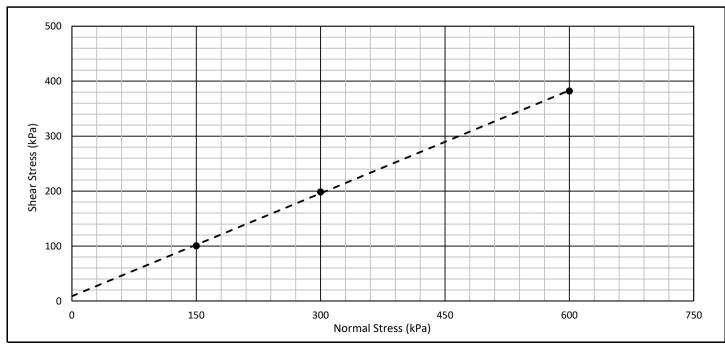
General Test Data				
Type of Test:	Slow Drained, saturated			
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 2030kg/m ³ at 7.8%			
Normal Stresses (kPa):	150, 300, 600			
Rate of Shear (mm/min):	0.011			
Comments:	-			

Initial Specimen Details						
		Specimen 1	Specimen 2	Specimen 3		
Moisture Content	%	8.2	7.9	8.0		
Dry Density	g/cm³	1.818	1.822	1.821		
Void Ratio	-	0.603	0.600	0.601		
Degree of Saturation	%	39.5	38.6	38.9		
Particle Density (SG)	-		2.915 - Determined			

Final Specimen Details				
Moisture Content	%	14.2	16.6	15.2

Shear Details					
Maximum Shear Stress:	kPa	100.6	198.6	382.2	
Strain at Maximum Shear Stress:	mm	9.11	9.33	10.00	

Shear Results				
φ'	Deg.	32		
C'	kPa	9		



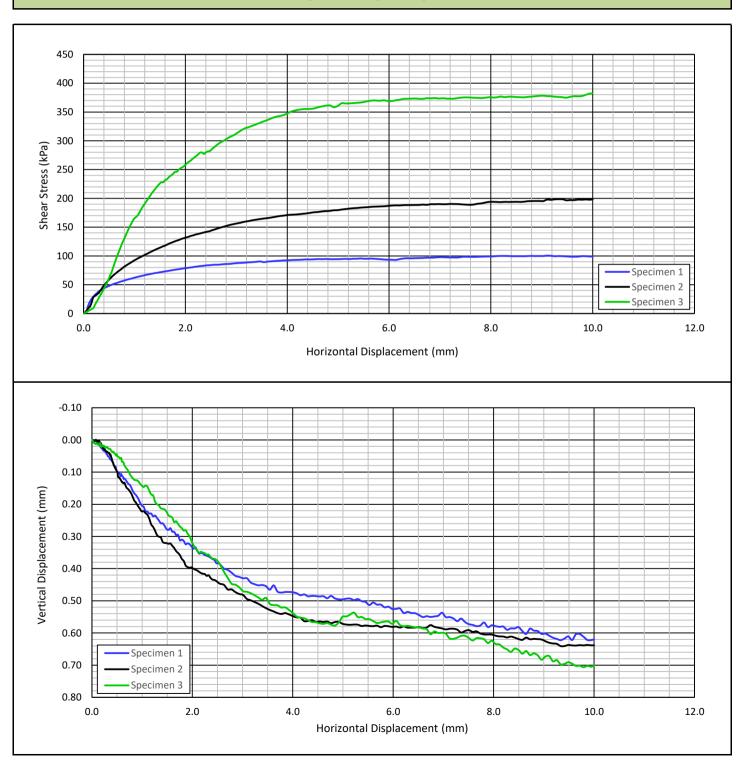




Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: HLP-TP26
Depth: (m) 0.0 - 1.1

Job Number:NTS-05Lab Number:NTS-05-37Date:08/11/2023







Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: HLP-TP03
Depth: (m) 2.3 - 3.1

 Job Number:
 NTS-05

 Lab Number:
 NTS-05-38

 Date:
 08/11/2023

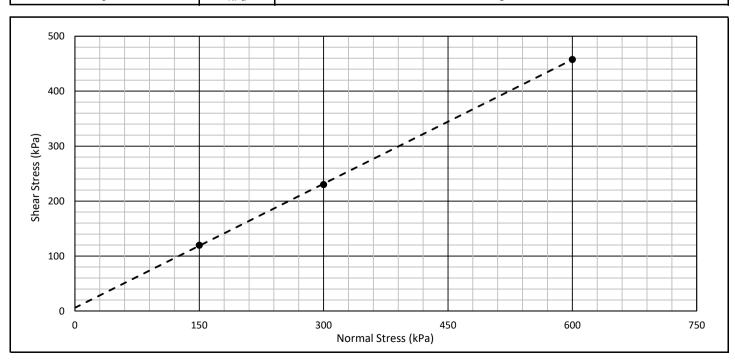
General Test Data				
Type of Test:	Slow Drained, saturated			
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1974kg/m³ at 11.0%			
Normal Stresses (kPa):	150, 300, 600			
Rate of Shear (mm/min):	0.025			
Comments:	-			

Initial Specimen Details						
		Specimen 1	Specimen 2	Specimen 3		
Moisture Content	%	11.1	11.5	11.1		
Dry Density	g/cm³	1.779	1.771	1.777		
Void Ratio	-	0.658	0.665	0.659		
Degree of Saturation	%	49.5	51.1	49.9		
Particle Density (SG)	-		2.949 - Determined			

Final Specimen Details				
Moisture Content	%	22.2	20.5	20.1

Shear Details				
Maximum Shear Stress:	kPa	119.7	230.2	457.8
Strain at Maximum Shear Stress:	mm	9.95	7.51	7.36

Shear Results		
φ'	Deg.	37
c'	kPa	6



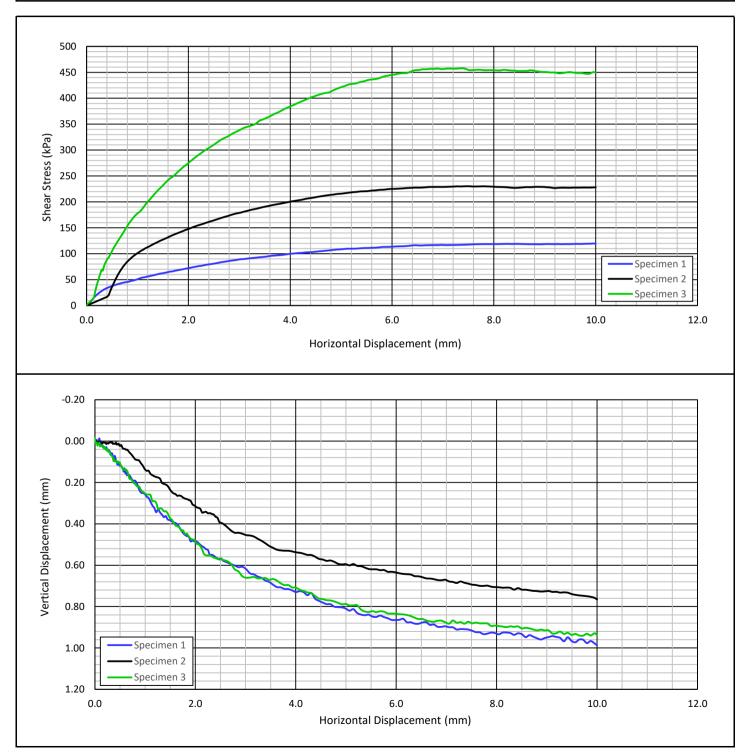




Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: HLP-TP03
Depth: (m) 2.3 - 3.1

Job Number:NTS-05Lab Number:NTS-05-38Date:08/11/2023







Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: SWD-TP15
Depth: (m) 0.5 - 2.5

 Job Number:
 NTS-05

 Lab Number:
 NTS-05-39

 Date:
 08/11/2023

SHEARBOX TEST

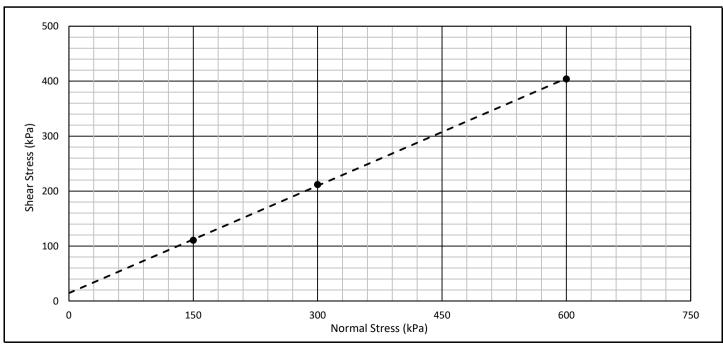
General Test Data			
Type of Test:	Slow Drained, saturated		
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 2055kg/m³ at 9.7%		
Normal Stresses (kPa):	150, 300, 600		
Rate of Shear (mm/min):	0.011		
Comments:	-		

		Initial Specimen Deta	ils	
		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	10.0	10.1	9.9
Dry Density	g/cm³	1.847	1.845	1.848
Void Ratio	-	0.594	0.595	0.593
Degree of Saturation	%	49.5	49.8	49.2
Particle Density (SG)	-		2.944 - Determined	

Final Specimen Details				
Moisture Content	%	18.5	17.0	17.4

		Shear Details		
Maximum Shear Stress:	kPa	110.6	212.0	404.3
Strain at Maximum Shear Stress:	mm	10.00	10.00	10.00

Shear Results		
φ'	Deg.	33
C'	kPa	14



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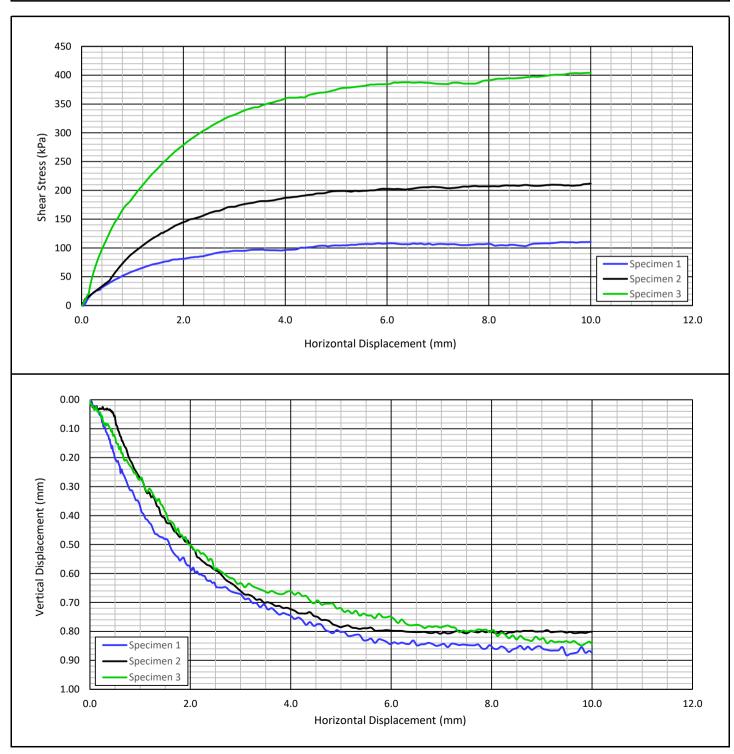




Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: SWD-TP15
Depth: (m) 0.5 - 2.5

Job Number: NTS-05 Lab Number: NTS-05-39 Date: 08/11/2023







Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: WRD-TP04
Depth: (m) 0.4 - 1.5

 Job Number:
 NTS-05

 Lab Number:
 NTS-05-42

 Date:
 08/11/2023

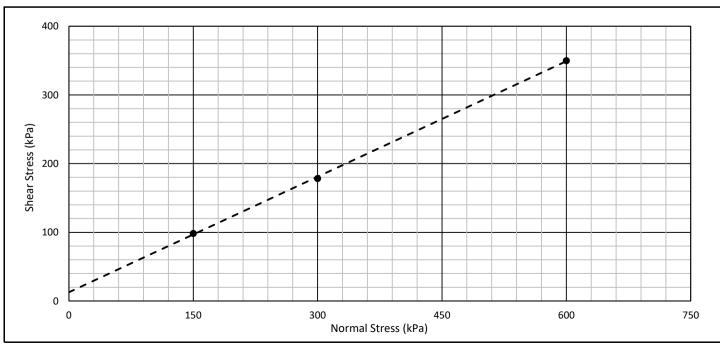
General Test Data			
Type of Test:	Slow Drained, saturated		
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1975kg/m³ at 11.0%		
Normal Stresses (kPa):	150, 300, 600		
Rate of Shear (mm/min):	0.025		
Comments:	-		

Initial Specimen Details				
		Specimen 1	Specimen 2	Specimen 3
Moisture Content	%	11.3	11.5	11.1
Dry Density	g/cm³	1.810	1.805	1.812
Void Ratio	-	0.501	0.505	0.500
Degree of Saturation	%	61.0	62.1	60.6
Particle Density (SG)	-		2.718 - Determined	

Final Specimen Details				
Moisture Content	%	22.2	20.7	26.1

		Shear Details		
Maximum Shear Stress:	kPa	98.5	178.4	349.9
Strain at Maximum Shear Stress:	mm	9.87	10.00	9.95

Shear Results		
φ'	Deg.	29
C'	kPa	13



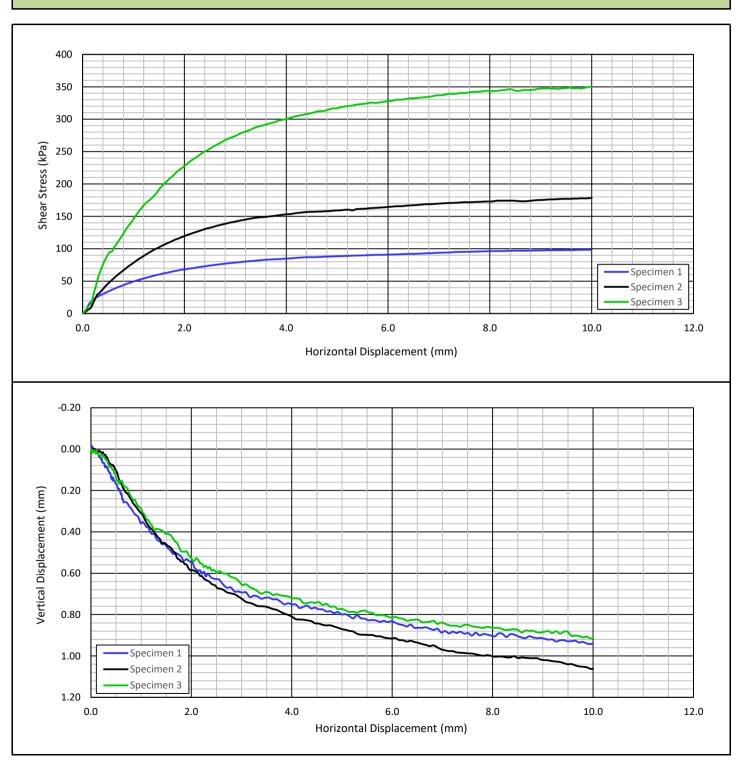




Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: WRD-TP04
Depth: (m) 0.4 - 1.5

Job Number:NTS-05Lab Number:NTS-05-42Date:08/11/2023







Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: WRD-TP35
Depth: (m) 2.1 - 4.1

 Job Number:
 NTS-05

 Lab Number:
 NTS-05-43

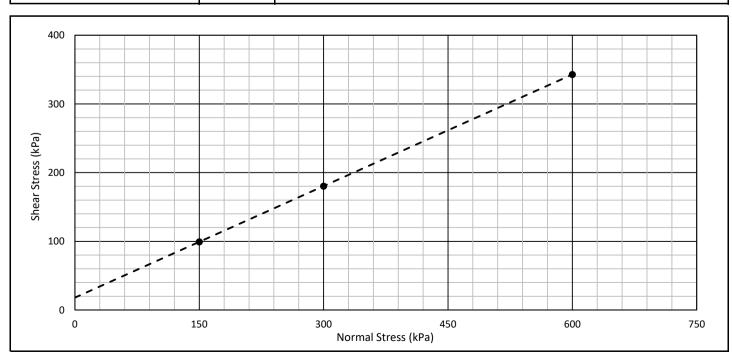
 Date:
 10/11/2023

General Test Data			
Type of Test:	Slow Drained, saturated		
Type of Sample:	Remoulded to 90% of supplied MDD (MOD AASHTO) - 1946kg/m³ at 11.2%		
Normal Stresses (kPa):	150, 300, 600		
Rate of Shear (mm/min):	0.011		
Comments:	-		

Initial Specimen Details						
		Specimen 1	Specimen 2	Specimen 3		
Moisture Content	%	11.4	11.3	11.5		
Dry Density	g/cm³	1.751	1.753	1.749		
Void Ratio	-	0.705	0.704	0.707		
Degree of Saturation	%	48.1	47.8	48.5		
Particle Density (SG)	-		2.987 - Determined			

Final Specimen Details						
Moisture Content	%	23.2	20.4	20.4		
		Shear Details				
Maximum Shear Stress:	kPa	99.3	180.2	342.9		
Strain at Maximum Shear Stress:	mm	8.34	10.00	10.00		

Shear Results				
φ'	Deg.	28		
C'	kPa	18		







Client Name: Namibia Technical Services

Project Name: Omitiomire Sample: WRD-TP35
Depth: (m) 2.1 - 4.1

 Job Number:
 NTS-05

 Lab Number:
 NTS-05-43

 Date:
 10/11/2023

