DPSH REPORT

New Development on Erf 4747, Swakopmund, Namibia

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Photograph Courtesy of Lighthouse Property Investment Trust

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Report review history:

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Authors & Reviewers Qualifications and Affiliations:

Dennis McDonald holds a national diploma in Civil Engineering and has been trained as a Civil Engineering technician covering project management, civil and structural design, contracts management, survey, laboratory management, investigations and testing, geotechnical investigations and report writing.

He has 42 years' combined experience, with 22 years managing his own civil SANAS Accredited engineering laboratories and geotechnical consultancy in the Southern Cape and Eastern Cape of South Africa. Dennis McDonald is registered with the Engineering Council of South Africa (ECSA) as a Registered Engineering technician # 2000 400 58, the South African Institute of Civil Engineers (SAICE), the Institute of Municipal Engineers of South Africa (IMESA), SABITA and SAT.

Burger Fourie holds an Honors Bachelor of Science (Geology) degree. He is currently undergoing in-service training as a Natural Scientist practicing Engineering Geology. He has 3 years' experience in Engineering Geology. Burger Fourie is registered with the South African Council for Natural Scientific Professions (SACNASP) as a Candidate Natural Scientist, Registration number: 115062.

<u>Declaration of Independence:</u>

The authors of this report are independent professional consultants with no vested interest in the project, other than remuneration for work associated with the compilation of this report.

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Appendix 1: DPSH Test Results

1. Introduction

Omamanya Geotechnical Consultants was appointed by Mr Nik Moroff on behalf of Jimmey Construction to conduct Dynamic Probe Super Heavy (DPSH) tests at Erf 4747, Swakopmund in the Erongo Region Namibia (**Figure 1**). The aim of the investigation was to attempt to establish the rock depth below Natural Ground Level (NGL).

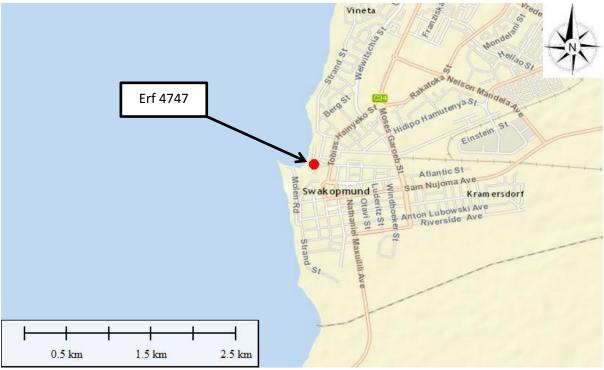


Figure 1: Location of Erf 4747, Swakopmund.

2. Site Description

The site is located where the old Municipal Swimming used to be in Swakopmund, on the beachfront known as the Mole. Buildings on site have mostly been demolished and there are currently shallow excavations.

3. Regional Geology

The majority of the central coastal region is covered by younger sediments forming either part of the visually impressive "Namib Sand Sea" (QGb), this only found south of Swakopmund, or surficial deposits (Qs) found blanketing the bedrock consisting of metamorphic rocks of the Swakop Group, Damara Sequence intruded by younger igneous intrusions (granites) as well as Karoo-aged dykes resulting in an intricate mixture of rock types as indicated in Figure 4.

The bedrock typically provides good bearing capacity, but excavations may prove to be difficult, requiring blasting in places, and the heterogeneity of the gneissic granite can cause uneven surfaces when excavated/blasted (Bulley, 1986).

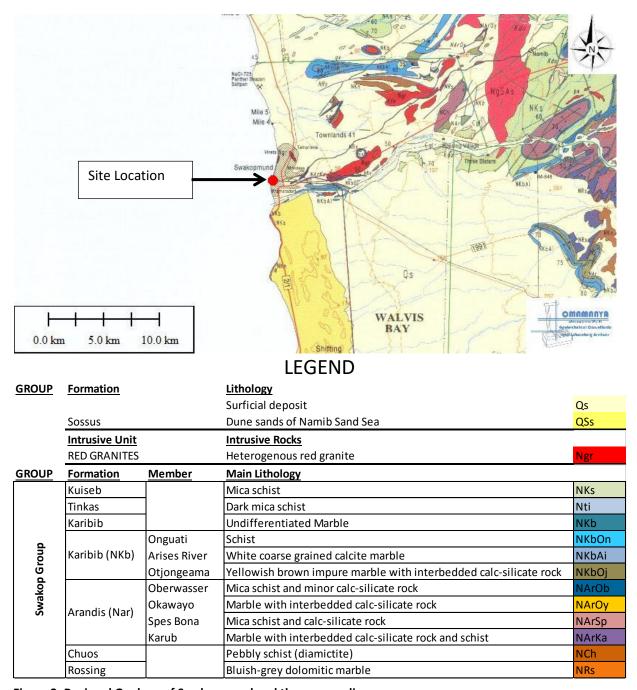


Figure 2: Regional Geology of Swakopmund and the surrounding area.

4. Results and Discussion

Four DPSH tests were conducted at the locations indicated on **Figure 3**. The tests were spaced to in order cover the site optimally. The DPSH Test Results are shown in **Table 1** below.

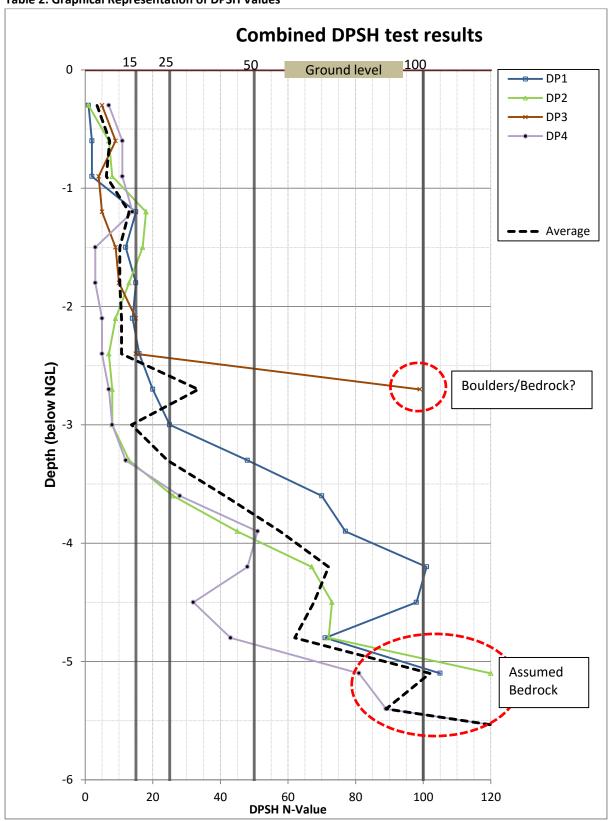


Figure 3: DPSH Test Locations on Site (Positions marked with handheld GPS)

Table 1: DPSH N-values with caclulated SPT N-Values, after (MacRobert et al.)

	Position no.									
Depth of Penetration (m)	DF	P1	DI	P2	Di	P3	DI	P4]	
	Number of blows							Minimum SPT	Empirical Soil	
	DPSH	*SPT N-	DPSH	*SPT N-	DPSH	*SPT N-	DPSH	*SPT N-	N-Value	Consistency
	N_{30}	Value	N_{30}	Value	N_{30}	Value	N_{30}	Value		
0.3	1	1	1	1	5	6	7	7	1	Very Loose
0.6	2	2	7	7	9	9	11	11	2	Very Loose
0.9	2	2	8	8	4	5	11	11	2	Very Loose
1.2	15	14	18	16	5	6	14	13	6	Loose
1.5	12	12	17	15	9	9	3	3	3	Very Loose
1.8	15	14	13	12	10	10	3	3	3	Very Loose
2.1	14	13	9	9	15	14	5	6	6	Loose
2.4	16	14	7	7	15	14	5	6	6	Loose
2.7	20	17	8	8	99	36	7	7	7	Loose
3.0	25	19	8	8	Refusal		8	8	8	Loose
3.3	48	27	13	12			12	12	12	Medium Dense
3.6	70	32	26	20			28	21	20	Medium Dense
3.9	77	33	45	26			51	28	26	Medium Dense
4.2	101	36	67	31			48	27	27	Medium Dense
4.5	98	36	73	32			32	22	22	Medium Dense
4.8	71	32	72	32			43	26	26	Medium Dense
5.1	105	36	120	38			81	33	33	Dense
5.4	Refusal		Refusal				89	34	34	Dense
5.7							158	40	40	Dense
6.0							Refusal		Refusal	·

Table 2: Graphical Representation of DPSH Values



The DPSH tests at DP1, DP2 and DP4 follow a similar trend with a consistency ranging from very loose to loose to a depth of 3m below NGL, from where the consistency increases to medium dense up to a depth of 4,8m, and dense to a maximum depth of 5,7m below NGL. The maximum depth of refusal was found at a depth of 5,7m at DP4. Considering the similarity in trends, it is assumed that refusal was encountered on bedrock.

At DP2, the consistency was similar to what was encountered at the other test positions to a depth of 2,4m below NGL, but from a depth of 2,4m to 2,7m the consistency drastically increased end refusal was encountered at 2.7m. Refusal is assumed to be on bedrock, as the NGL at DP2 is lower than at the other test locations which explain the shallow depth of refusal.

The water table depth could not be established during the DPSH tests.

5. Recommendations

Based on the knowledge that multi-storey basement excavation will take place, it is recommended that the upper 4,0m below the NGL be excavated to stockpile for re-use in all backfill operations. It is likely that an uneven residual bedrock surface will be exposed using a large tracked excavator (>22ton and possibly with a rock bucket). Should further refusal be encountered during the excavation process then the desired founding depth will have been achieved and further excavation is not necessary.

A level founding platform can be created by employing either of the following methods:

- Use a pecker (Montebehr) to reduce any hard rock protrusions to ≥ 1.0m below any structural foundations invert level.
- Backfill using the excavated/stockpiled sand in 150-300mm layers, saturate and compact to 100% of Mod AASHTO density providing a safe bearing capacity of 200kPa – cap with a 150mm subbase layer (PI<6) compacted to 95% of Mod. AASHTO density as a working surface; or
- Backfill with an imported G5 material compacted to 95% of Mod AASHTO density (with a Plasticity Index (PI) of <6) in layers of 150mm to final thickness below any foundation invert level, in which case a safe bearing capacity of 450kPa can be assumed.

Due to the high assumed permanent water table, basements should be designed as a watertight retaining structure. Water stops are recommended for all construction joints up to the proposed final ground level.

It is recommended that a penetron admix (xypex) be included in all concrete works below water level.

6. Conclusion

In the present state of the site, it is recommended that the alluvial sand on the site should be excavated in accordance with the recommendations above in order to assure a suitable founding platform. This investigation, although test position specific, has sought to highlight potential founding, excavation difficulties, and possible rock depth and does not obviate the variable ground conditions and isolated zones of poor foundation / rock material not identified in this report.



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DPSH - DYNAMIC PENETROMETER SUPER HEAVY TEST PROJECT: THE LIGHTHOUSE - SWAKOPMUND **OPERATOR:** GEORGE BRITTNELL BH No: DATE: 25/06/2016 GEOLOGY: BEACH ALLUVIUM CLIENT: OMAMANYA GEOTECHNICAL CONSULTANTS COORDINATES: S 22 40 29.70 E 14 31 26.60 ELEVATION: 8 m AMSL EQUIPMENT: 63,5 KG HAMMER - 60 Deg 50 mm CONE PAGE: 1 OF 2 DEPTH: **BLOWS / PENETRATION** REMARKS ADJUSTED NO PENETRATION AT OF BLOWS ADJUSTED BLOWS No. Of Blows 0 0 O 1 300mm 1 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 0 1 0 2 300mm BEACH ALLUVIUM 75 Penetration (mm) 75 75 75 No. Of Blows 0 0 1 2 300mm BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 5 3 4 15 300mm 15 BEACH ALLUVIUM 75 Penetration (mm) 75 75 75 No. Of Blows 3 12 300mm 12 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 4 15 300mm 15 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 3 3 14 300mm 14 BEACH ALLUVIUM 75 Penetration (mm) 75 75 75 No. Of Blows 16 300mm 16 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 5 6 20 300mm 20 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 7 6 6 6 25 300mm 25 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 9 11 14 14 48 300mm 48 BEACH ALLUVIUM Penetration (mm) 75 75 75 75



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DPSH - DYNAMIC PENETROMETER SUPER HEAVY TEST PROJECT: THE LIGHTHOUSE - SWAKOPMUND OPERATOR: GEORGE BRITTNELL DP 2 BH No: DATE: 26/06/2016 BEACH ALLUVIUM GEOLOGY: CLIENT: OMAMANYA GEOTECHNICAL CONSULTANTS COORDINATES: S 22 40 28.10 E 14 31 26.50 **ELEVATION:** 8 m AMSL EQUIPMENT: 63,5 KG HAMMER - 60 Deg 50 mm CONE 1 OF 2 PAGE: DEPTH: **BLOWS / PENETRATION** REMARKS ADJUSTED NO PENETRATION AT No. Of Blows 0 0 0 1 1 300mm BEACH ALLUVIUM 75 75 Penetration (mm) 75 75 No. Of Blows 2 2 1 2 7 300mm BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 2 2 8 300mm BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 4 5 5 18 300mm 18 BEACH ALLUVIUM 75 Penetration (mm) 75 75 75 No. Of Blows 4 5 4 4 17 300mm 17 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 3 4 3 13 300mm 13 BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 2 2 9 300mm BEACH ALLUVIUM 75 75 Penetration (mm) 75 75 No. Of Blows 2 2 2 7 300mm BEACH ALLUVIUM Penetration (mm) 75 75 75 75 AT 2.40m No. Of Blows 8 300mm BEACH ALLUVIUM 75 Penetration (mm) 75 75 75 No. Of Blows 2 2 8 300mm BEACH ALLUVIUM Penetration (mm) 75 75 75 75 No. Of Blows 2 3 4 4 13 300mm 13 BEACH ALLUVIUM Penetration (mm) 75 75 75 75



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